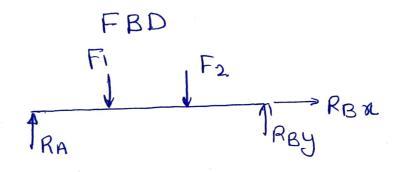
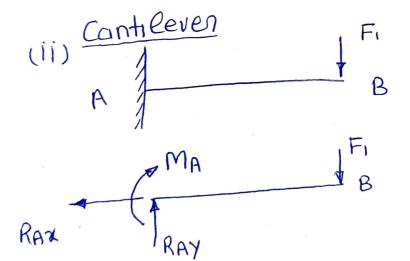
Shear Force and Bending Moment

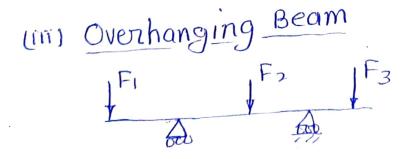
Types of Beams

(i) Simply Supported Beam.





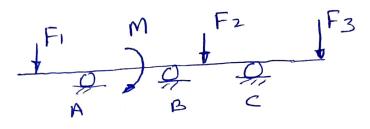








(V) Continuous Beam

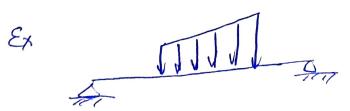


Types of Loading

- (1) Concentrated Force
- (2) Concentrated moment
- (3) Uniformly Distoibuted Load

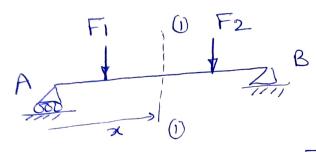
Ex 1 mm

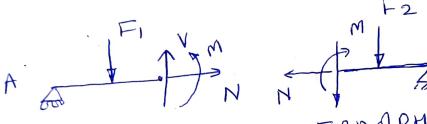
(4) Variable Intensity Loading (UVL)



Determining of Support Reaction For statically determinate beam. EFx=0, EFy=0, EM2=0

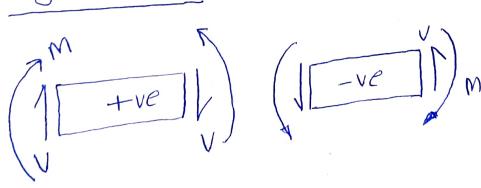
Shew Force & Bending Moment





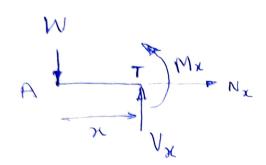
FBD & L.H.S (i) FBD & R.M.P. (ii)
According to Newtons third Law

Sign Convention



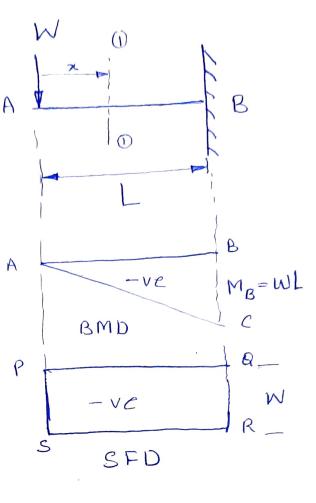
OSXSL

FBDJ1H.S



$$0 \quad \text{EF}_{x} = 0$$

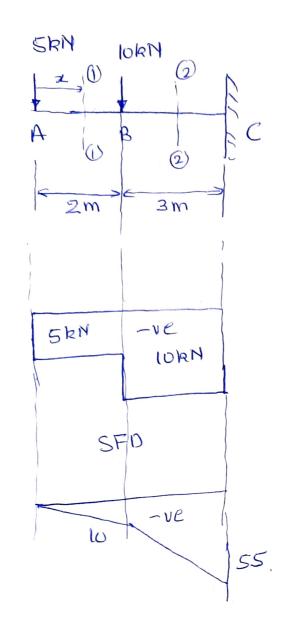
$$N_{x} = 0 - 0$$



Wx + Mx = 0

$$M_{x} = -Wx - (6)$$

$$\frac{dMx}{dx} = -W$$

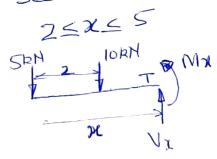


Taking Section (1) - (1) at a distance X from A. Draw FBD

$$3 \Rightarrow \text{EM}_{T} = 0$$

$$5 \times + 15 \times = 0$$

$$M_{N} = -5 \times -5$$
Section 6-0



$$5x + 10(x-2) + Mx = 0$$

 $M_x = -5x - 10(x-2) - (7)$

6

Example: Simply supported Beam counting A concentrated Load.

O Reaction

$$R_{B}XL - \frac{WL}{2} = 0$$

$$R_{B} = \frac{W}{2} - (4)$$

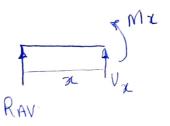
(22)

$$RAV = \frac{W}{2} - (5)$$

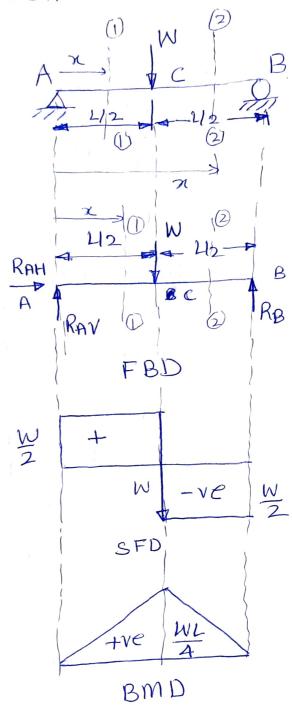
$$0 \Rightarrow EF_{2}=0$$

$$RAH=0 \qquad (6)$$

Section (1-0)



$$M_{\chi} = \frac{W}{2}\chi - (7)$$



$$V_X = \frac{W}{2}$$

$$\frac{N}{2}x - N(2L - \frac{L}{2}) - Mx = 0$$

$$M_{x} = \frac{W}{2} (2-x)$$

at
$$x = 42$$

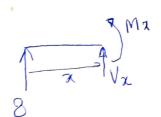
$$M_{x} = \frac{WL}{4}$$

Examplo

Sol.

8

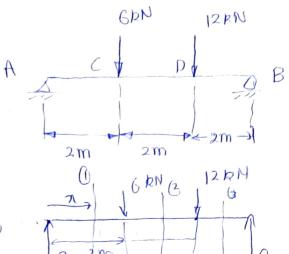
Section (1)

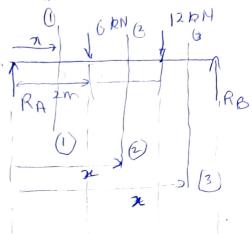


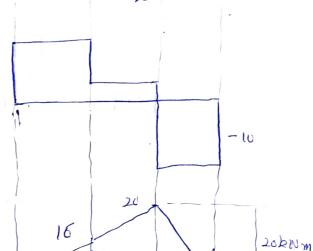
$$g + V_x = 0$$

$$8 + V_{x} = 0$$

$$V_{x} = -8$$

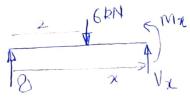






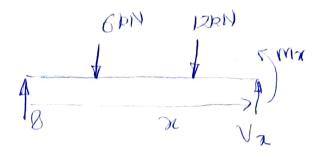
BMD

Section 2-2



(3)
$$EM6(-2) = 0$$

 $8x - 6(x-2) - Mx = 0$
 $Mx = 2x + 12$



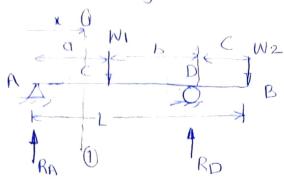
$$8 + V_2 - 6 - 12 = 0$$

$$V_{\chi} = 10$$
 (eve)

$$8x - 6(x-2) - 12(x-4) - Mx = 0$$



SFD&BMD& Overhanging Beam



$$W_1a + W_2L = R_D(a+b)$$

$$RD = \frac{W_1 a + W_2 L}{a + b} - 0$$

$$R_A = W_1 + W_2 - R_D - G$$

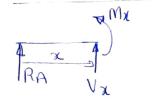
Section O-O

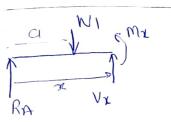
$$V_{x} = -RA$$

For Sec (2)-(3

$$V_{\chi} = -R_A + W_I$$

$$M_X - R_{AX} + W_1(x-a) = 0$$

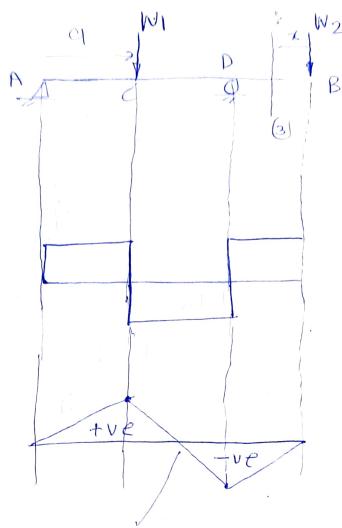




RMS. Section 18

W2

 $V_{x} = W_{2}$ $M_{x} = W_{2}C$

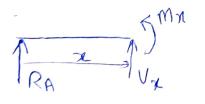


Point of Contra flexural.

$$10 \times 4 + 4 \times 10$$
$$= 8R_{D}$$

Section

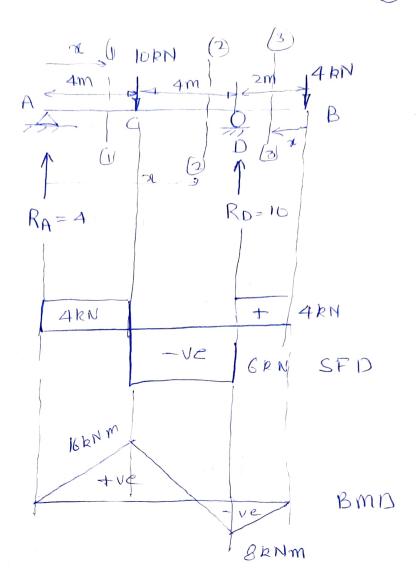
$$(-()$$

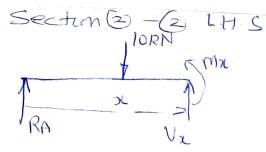


$$V_X = -4kN$$
 (tuc)

$$R_A \mathcal{H} - M_{x} = 0$$

$$M_{x} = 4x$$





$$R_A + V_{R} = 10$$

$$V_{R} = 6 - (A)$$



$$V_{\chi} - 4 = 0$$

$$V_{\chi} = 4 l 2 N (+ ve)$$

$$M_{\chi} = 4 L (- ve)$$

$$M_{\chi} = 16$$

Point of contrateoxure intortion

From
$$Eq(B)$$
 but $Mn = 0$

$$-6n + 40 = 0$$

$$x = 6.667 \text{ m}$$

Beam Subjected to Couplo

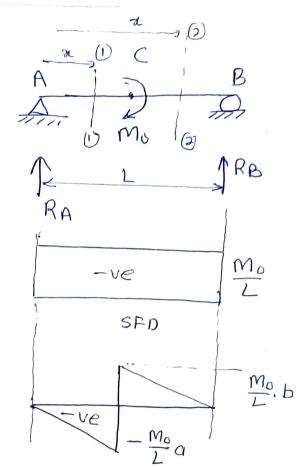
$$R_A = -\frac{M_o}{L}$$

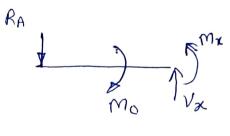
$$R_A + R_B = 0$$

$$R_B = \frac{M_0}{L}$$

Section (D-(1) L.H.S.,FBD

$$V_x - R_A = 0$$





$$V_{x} = RA$$

$$\leq M_{\mathcal{H}} = 0$$

$$R_A \times + M_X - M_0 = 0$$

at
$$x = 1$$
 $M_{x} = 0$

$$at x = 0$$

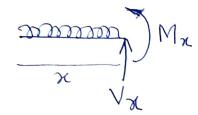
Unitormly distributed Load

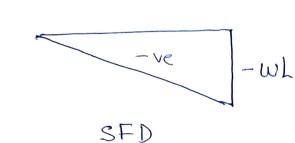
Section (1-1)

L. H.S. Sbeam

FBD

0 sa < L



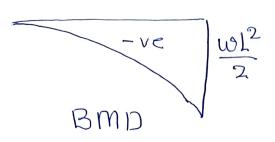


W/Unit Penson

$$V_X - WX = U$$

$$\forall x = wx$$

(-ve)



$$EM_{(1)-(1)}=0$$

$$Wx.\frac{x}{2} + Mx = 0$$

$$Mx = -\frac{Wx^2}{2}$$

$$M_{x} = -\frac{wx^2}{2}$$

Section O-D L.H.S. 6 bN/m W. W. W.

$$0 \le x \le 3$$

 $EFy=0$

$$6x - v_x = 0$$

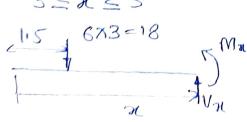
$$V_X = 6 \times - 4$$

$$EM_{(1)}=0$$

$$6x. \frac{\chi}{2} + M_{\chi} = 0$$

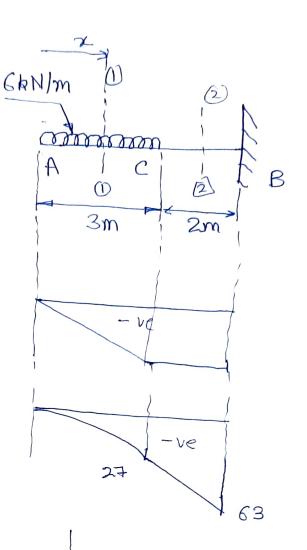
$$M_{\chi} = 3 \chi^2 - (5)$$

Section (2)-(2) LHS



$$18 - V_{x} = 0$$

$$V_{x} = 18(-ve)$$



$$M^{x} = -18(x-1.2)$$

$$\chi = 5$$

$$M_{\pi} = -63$$

Simply Supported Beam Carrying UDL

Reaction

$$R_A = \frac{WL}{2}$$

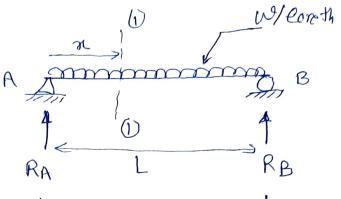
$$R_{B} = \frac{WL}{2}$$

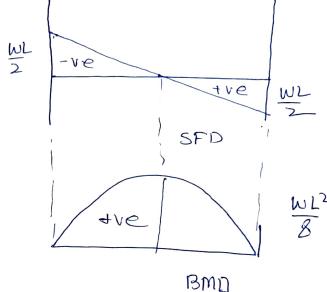
Section (1) -0

FBD Q L.H.S.

$$V_{x} = w_{x} - \frac{wL}{2}$$

$$V_{X} = -\frac{\omega L}{2}$$
 (1 ve)





$$\frac{\omega L}{2}$$
, $\chi - \frac{\omega \chi}{2} - M_{\chi = 0}$

$$M_{\chi} = \frac{\omega L}{2} \chi - \omega \frac{\chi^2}{2}$$

$$at x = \frac{L}{2}$$

$$M_{\chi} = \frac{\omega L^2}{8}$$

Linearly Varying Load

Reaction.

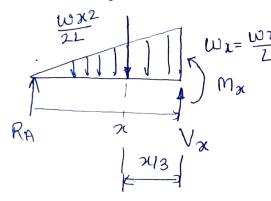
$$EM_B = 0$$

$$R_{AL} - \frac{\omega L}{2} \times \frac{L}{3} = 0$$

$$R_A = \frac{\omega L}{6} - (4)$$

$$R_{B} = \frac{\omega L}{3} - (5)$$

Section 0-0 LHS, FBD

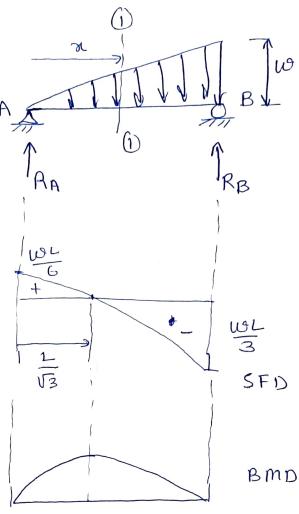


$$R_A + V_X - \frac{\omega x^2}{2L} = 0$$

$$V_X = \frac{\omega x^2}{2L} - \frac{\omega L}{6}$$

at
$$x=0$$
, $V_{x}=-\frac{\omega L}{6}$ (eve)
at $x=L$ $V_{x}=\frac{\omega L}{3}$

$$V_{K}=0 \Rightarrow 2 = 2153$$

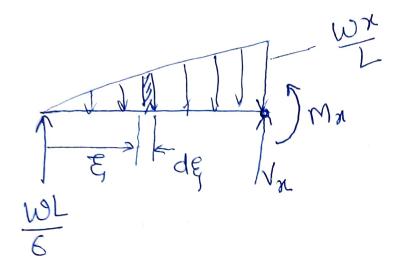


$$M_{x} + \frac{\omega x^{2}}{2L} \cdot \frac{x}{3} - \frac{\omega L}{6} x = 0$$

$$M_{x} = \frac{\omega L}{6} x - \frac{\omega x^{2}}{6L}$$
at $x = 0$, L

$$M_{\pi} = 0$$
at $\chi = \frac{113}{3}$

$$M_{\pi} = \frac{\omega L^2}{953}$$



$$R_{A}x - \int_{0}^{x} \frac{w\xi}{2L} d\xi (x-\xi) - M_{n=0}$$

$$M_{\chi} = \frac{\omega L}{6} \chi - \frac{\omega \chi^3}{6L}$$

Relation, between Rate of loading 255 2BM

Considering Equillibrium CD D Son

Elements

EFy=0

mx [] matdmx Vat dva

Vx-(Vx+dux)-wdx=0 Vx

dux = - wodr

$$\left[\frac{dVn}{dx} = -w\right] - (1)$$

EMD=0

Mx+dmx-Mn-Vn.dx+wenydx.dx=0

$$\left[\frac{dMx}{dx} = Vx\right] - 2$$

From (1) is w=0

F = Comptaint

$$\int dM_x = \int F dx$$

$$M = Fx + C$$

F = - wx 1 6.7 M = - wx 1 6.7

Ex.

