J. II. CONCRETE WORKS:

I. FOOTING

F-1 @ 13 sets

L: 3800mm

W: 3800 mm

T: 500mm

D: 1500mm

$$V = L \times W \times T \times QTY$$

$$V = (3800) \times (3800) \times 500 \times 13 = (9.386 \times 10^{10} \text{ mm} \cdot 3 / 10^9) = 93.86 \text{ m} \cdot 3$$

To Convert mm3 to m3, divide the answer with 10^9

🖳 Add Structural M	/lember	×
Footing (Column)	∨ Name: F-1	Unit: Millimeter
Footing Type:	Isolated Footing	~
Dimensions		
Length:	3800	
Width:	3800	
Thickness:	500	
Quantity:	13	
Depth:	1500	
Longitudi	inal Reinforcement	Transverse Reinforcement
Diameter:	0	Diameter: 0
Quantity:	0	Quantity: 0
Hook Type:	90 ~	Hook Type: 90 ∨
		Save

CF-1 @ 13 sets or qty

L: 5500mm

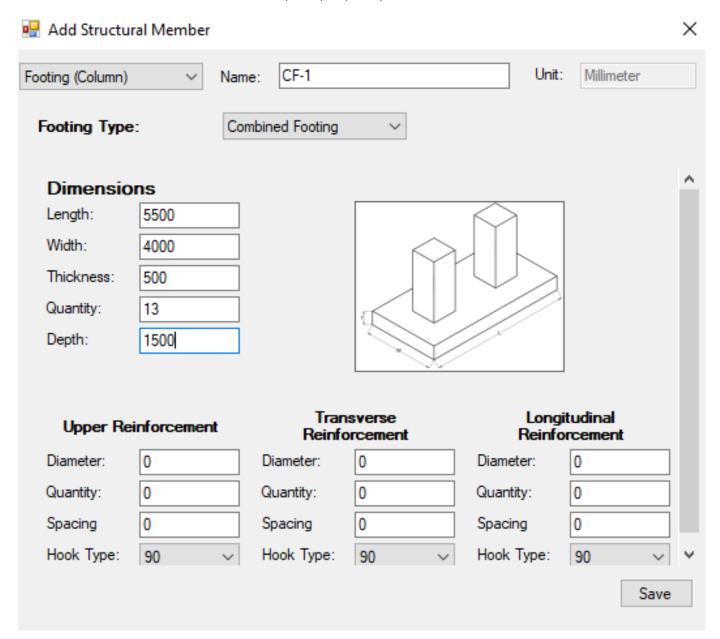
W: 4000mm

T: 500mm

D:1500mm

$V = L \times W \times T \times QTY$

 $V = (5500) \times (4000) \times 500 \times 13 = 143 \text{ m}$



WF-1

L: 30.44m

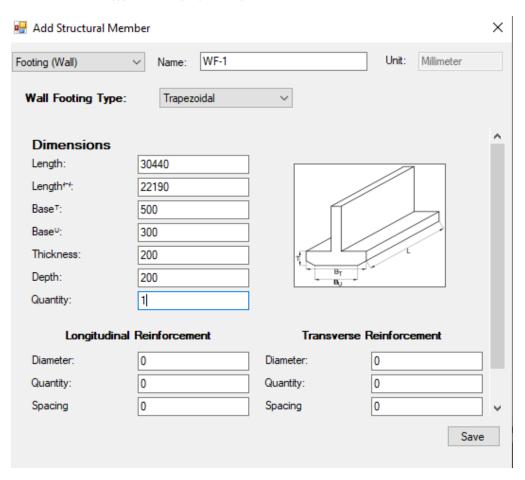
L f-f: 22.19

BT: 500mm

BU: 300mm

T: 200mm

D: 200mm



VOLUME TOTAL (FOOTINGS) = 93.86 + 143 + 2.4352 = 239.2952 or **239.3 m3** 239.3 m @ 4000psi (P 4910.00 / m3) = 239.3 m3 x P 4910.00 / m3 = **P 1,174,963.00**

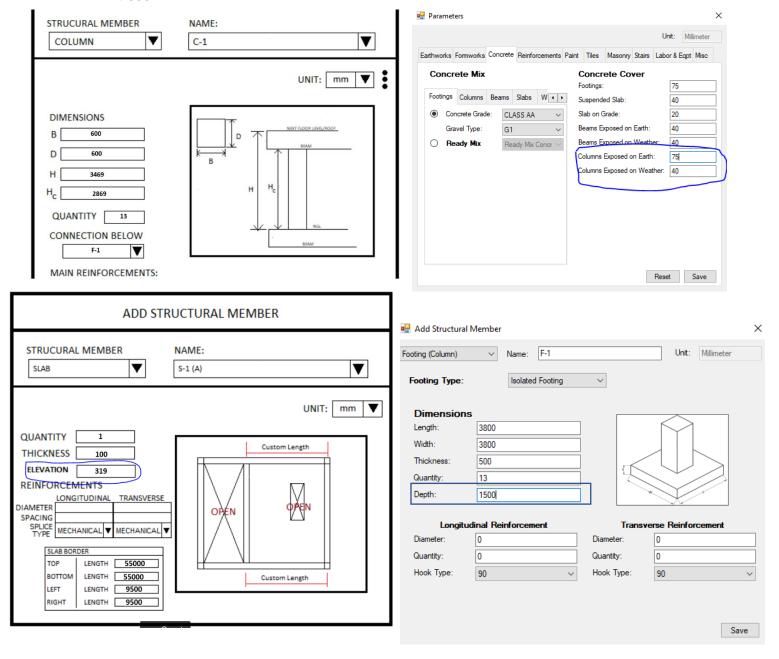
F.S. $5\% = 239.3 \times 5\% = 11.965 \text{ or } 11.97 \text{ m} 3 \times 4910 = \text{Php } 58,772.70$

Labor cost = $P 400.00 / m3 = 239.3 \times 400 = P 95,720.00$

II. COLUMN

C-1 @ 13 sets (GROUND FLR AND FIRST FLOOR)

H1: 3469 mm B: 600 mm D: 600 mm



$$V_T = V_1 + V_2$$

 $V_1 = (DEPTH\ OF\ FOOTING + AVERAGE\ SLAB\ ELEVATION - THICKNESS\ OF\ FOOTING)$ (B+2(CCcolumn exposed to Earth – CCcolumn exposed to Weather) (D+2(CCcolumn\ Exposed\ to\ Earth – CCcolumn\ Exposed\ to\ Weather)

$$V_2 = B*D*(H-AVERAGE\ SLAB\ ELEVATION)$$

$$V_1 = (1500+319-500)(600+2(75-40)(600+2(75-40)=0.5920991\ m3)$$

$$V_2 = 600*600*(3469-319)=1.134\ m3$$

$$V_T = 0.592+1.134=1.726\ m3*13\ sets = 22.438\ m3$$

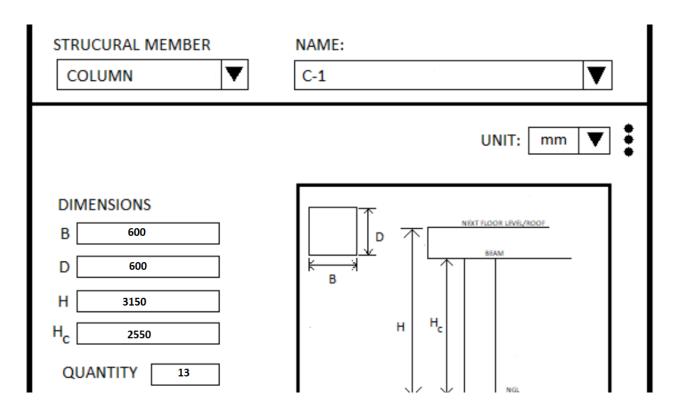
C-1 @ **13** sets (**2**ND **FLOOR** – **4**TH **FLOOR**)

H: 3150 mm

B:600

D:600

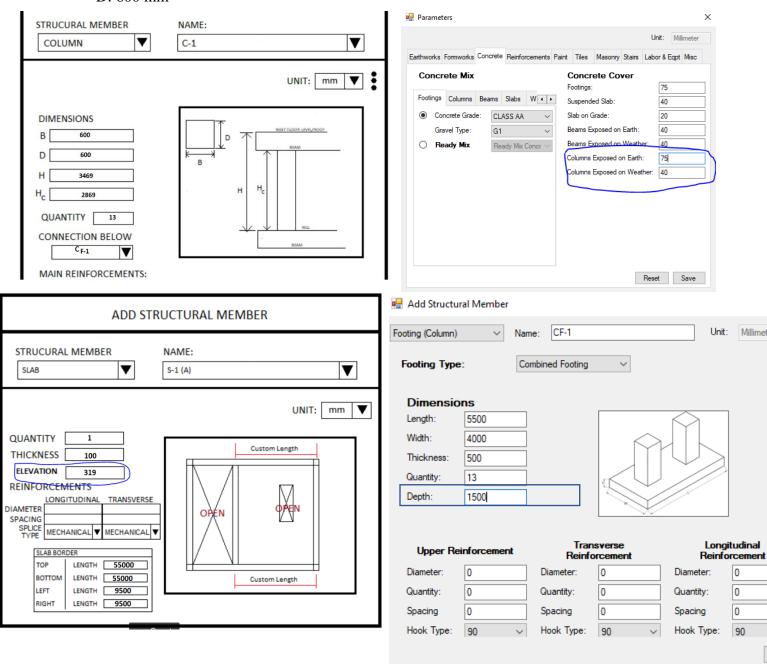
 $v = 3150x 600 \times 600 \times 13 = 14.742 \text{ m}$



C-2 @ 13 sets (GROUND FLR AND FIRST FLOOR)

H1: 3469 mm

B: 600 mm D: 600 mm



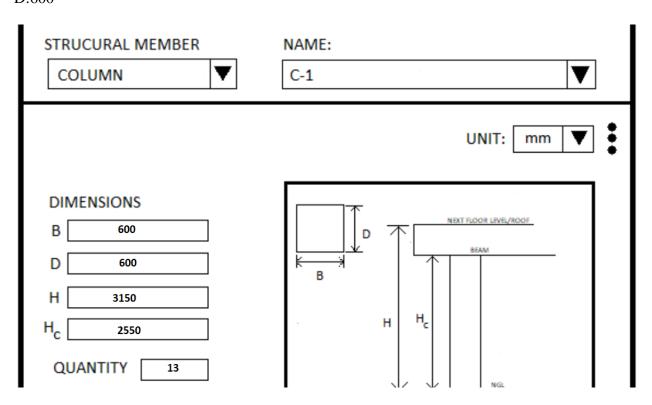
 $V_1 = (DEPTH\ OF\ FOOTING + AVERAGE\ SLAB\ ELEVATION - THICKNESS\ OF\ FOOTING) (B+2(CCCOLUMN\ EXPOSED\ TO\ EARTH-CCCOLUMN\ EXPOSED\ TO\ WEATHER)$ (D+2(CCCOLUMN\ EXPOSED\ TO\ EARTH-CCCOLUMN\ EXPOSED\ TO\ WEATHER)

$$V_2 = B * D * (H - AVERAGE SLAB ELEVATION)$$

 $V_1 = (1500 + 319 - 500) * [600 + 2 * (75 - 40)] * [600 + 2 * (75 - 40)]$
 $= 0.5920991 \ m3$
 $V_2 = 600 * 600 * (3469 - 319) = 1.134 \ m3$
 $V_T = 0.592 + 1.134 = 1.726 \ m3 * 13 \ sets = 22.438 \ m3$

C-2 @ 13 sets (2ND FLOOR – 4TH FLOOR)

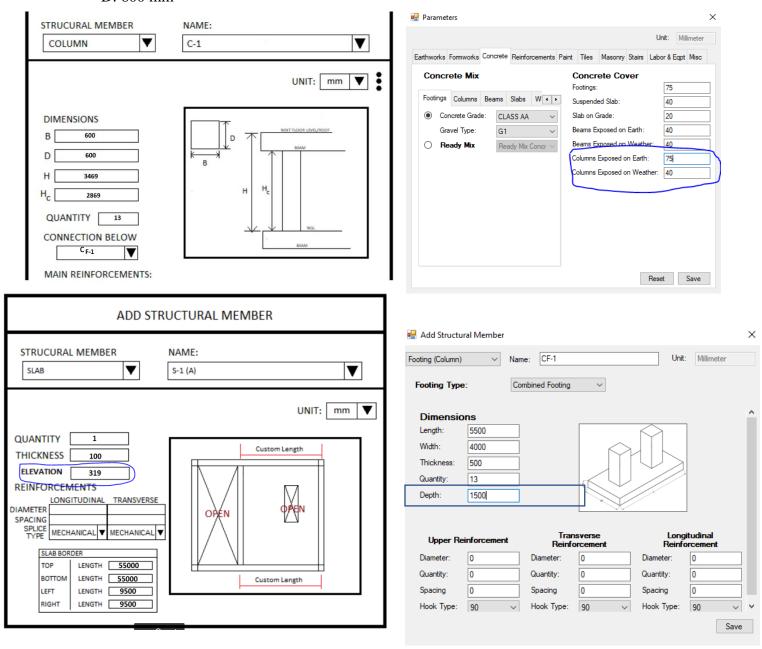
H: 3150 mm B:600s D:600



 $v = 3150x 600 \times 600 \times 13 \text{ sets} = 14.742 \text{ m}$

C-3 @ 13 sets (GROUND FLR AND FIRST FLOOR)

H1: 3469 mm B: 600 mm D: 600 mm



$$V_T = V_1 + V_2$$

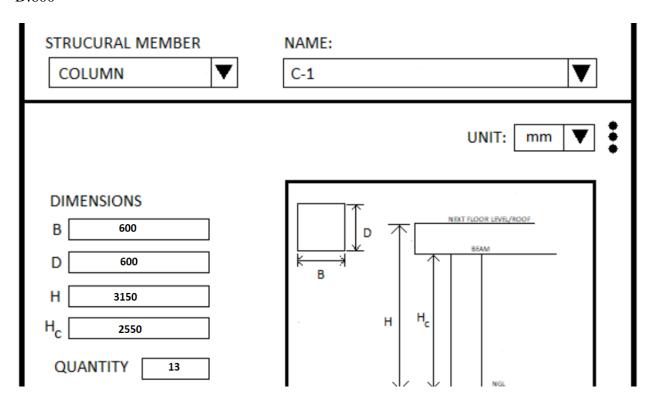
 $V_1 = (DEPTH\ OF\ FOOTING + AVERAGE\ SLAB\ ELEVATION - THICKNESS\ OF\ FOOTING)$ (B+2(CCcolumn exposed to Earth – CCcolumn exposed to Weather) (D+2(CCcolumn\ Exposed\ to\ Earth – CCcolumn\ Exposed\ to\ Weather)

$$V_2 = B * D * (H - AVERAGE SLAB ELEVATION)$$

 $V_1 = (1500 + 319 - 500)(600 + 2(75 - 40)(600 + 2(75 - 40)) = 0.5920991 m3$
 $V_2 = 600 * 600 * (3469 - 319) = 1.134 m3$
 $V_T = 0.592 + 1.134 = 1.726 m3 * 13 sets = 22.438 m3$

C-3 @ 13 sets (2ND FLOOR – 4TH FLOOR)

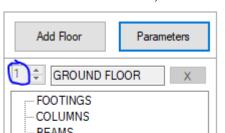
H: 3150 mm B:600s D:600



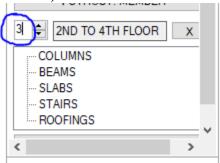
 $v = 3150x 600 \times 600 \times 13 \text{ sets} = 14.742 \text{ m}$

COLUMN CONCRETE VOLUME TOTAL

C-1 @ 13 sets (GROUND FLR AND FIRST FLOOR) = 22.438 m3 * 1 FLOOR = 22.438 m3 C-2 @ 13 sets (GROUND FLR AND FIRST FLOOR) = 22.438 m3 * 1 FLOOR = 22.438 m3 C-3 @ 13 sets (GROUND FLR AND FIRST FLOOR) = 22.438 m3 * 1 FLOOR = 22.438 m3



C-1 @ 13 sets (2^{ND} FLOOR -4^{TH} FLOOR) =14.742 m3 * 3 *FLOOR* = 44.226 *m*3 C-2 @ 13 sets (2^{ND} FLOOR -4^{TH} FLOOR) =14.742 m3 * 3 *FLOOR* = 44.226 *m*3 C-3 @ 13 sets (2^{ND} FLOOR -4^{TH} FLOOR) =14.742 m3 * 3 *FLOOR* = 44.226 *m*3



TOTAL = 22.438 + 22.438 + 22.438 + 44.226 + 44.226 + 44.226 = 199.992 m

199.992 m3 @ 4000psi (P 4910.00 / m3) = **P 981,960.72**

F.S. $5\% = 198.89 \times 5\% = 9.996 \text{ or } 10 \text{ m} 3 \times 4910 = \mathbf{P} 49,100.00$

Labor cost = P450.00 / m3 = 199.992 m3 x 450.00 =**P 89,996.40**

III. BEAMS

RED FONTS ARE INPUTS AND THIS IS HOW THE SYSTEM WILL COMPUTE: ALL THE DIMENSIONS FOR B AND D IN THIS EXAMPLE FOR ALL FOOTING TIE BEAMS (FTB) ARE THE SAME. B = 400 mm or 0.4 m; D = 600 mm or 0.6 m

VOLUME = (B X D X CLEAR LENGTH) X QUANTITY OF BEAM FTB -1A

 $V = (0.4 \times 0.6 \times 4.23) \times 1 \text{ QTY} = 1.0152 \text{ m}$

FTB-1

 $V = (0.4 \times 0.6 \times 3.83) \times 10 \text{ QTY} = 9.192 \text{ m}$

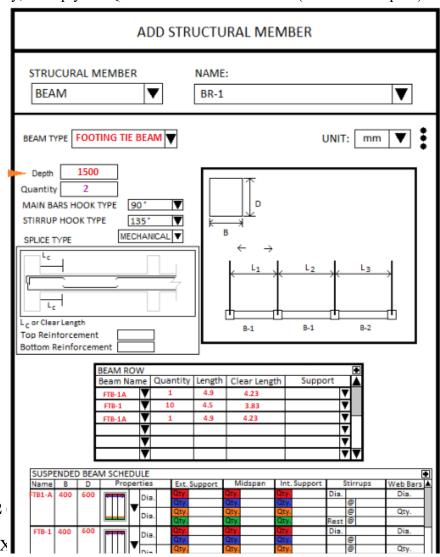
FTB-1A

 $V = (0.4 \times 0.6 \times 4.23) \times 1 \text{ QTY} = 1.0152 \text{ m}$

TOTAL VOLUME FOR BEAM ROW 1

= 1.0152 m3+9.192 m3 +1.0152 m3 = 11.224 m3 x 2 QTY = **22.4448 m3**

Finally, multiply the QTY of the whole beam row (UI under "Depth")



BEAM ROW 2 FTB -1

V = (0.4 A)

FTB-1 V = (0.4 X 0.6 X 3.83) X 10 QTY = 9.192 m3

FTB-1

V = (0.4 X 0.6 X 4.23) X 1 QTY = 1.0152 m3

TOTAL VOLUME FOR BEAM ROW 2

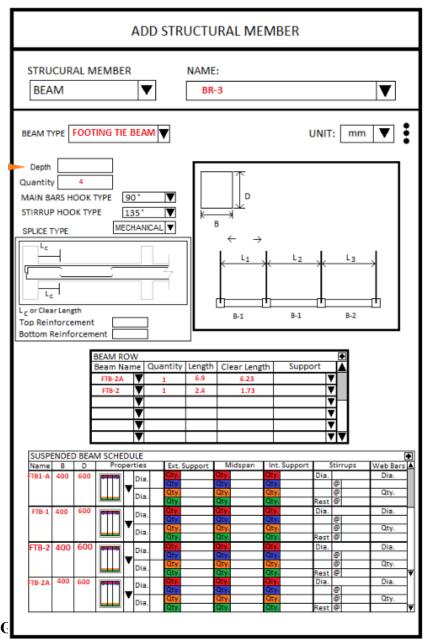
= 1.0152 m3+9.192 m3 +1.0152 m3 = **11.224 m3**

	ADD STRUCTURAL MEMBER
	STRUCURAL MEMBER NAME: BEAM ▼ BR-2 ▼
	BEAM TYPE FOOTING TIE BEAM ▼ UNIT: mm ▼
	Depth Quantity 1 MAIN BARS HOOK TYPE 90° STIRRUP HOOK TYPE 135° SPLICE TYPE MECHANICAL BEAM ROW Beam Name Quantity Length Clear Length Support
	FTB-1 ▼ 1 4.9 4.23 ▼ FTB-1 ▼ 10 4.5 3.83 ▼ FTB-1 ▼ 1 4.9 4.23 ▼ ▼ ▼ ▼ ▼
	SUSPENDED BEAM SCHEDULE SUSPENDED BEAM SCHEDULE SUSPENDED BEAM SCHEDUL
BEAM ROY	

FTB -2A V = (0.4 X 0.6 X 6.23) X 1 QTY = 1.4952 m

FTB -2 $V = (0.4 \times 0.6 \times 1.73) \times 1 \text{ QTY} = 0.4152 \text{ m}$

VOLUME TOTAL FOR BEAM ROW 3 = 1.4952 m 3 + 0.4152 m 3 = 1.9104 m 3 x 4 QTY

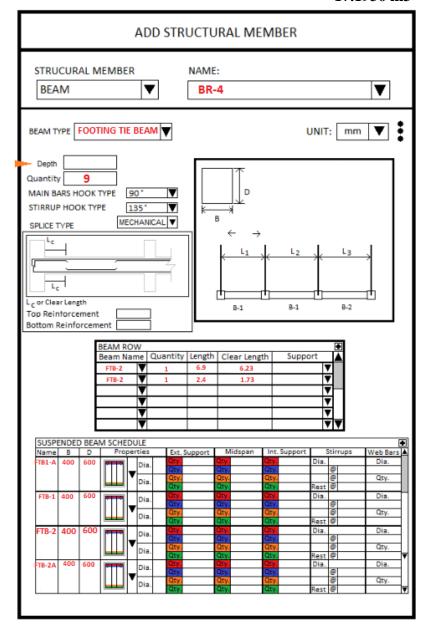


BEAM ROW 4 (C

FTB -2A V= (0.4 X 0.6 X 6.23) X 1 QTY = 1.4952 m3

FTB -2 V = (0.4 X 0.6 X 1.73) X 1 QTY = 0.4152 m3

VOLUME TOTAL FOR BEAM ROW 4 = 1.4952 m 3 + 0.4152 m 3 = 1.9104 m 3 x 9 QTY

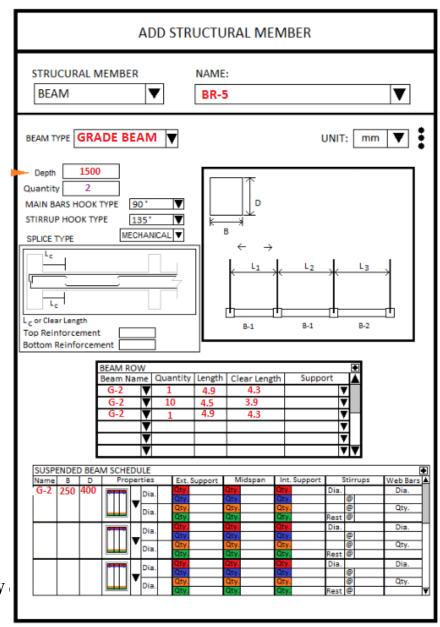


Total volume of concrete for ground floor = 22.4448 + 11.224 + 7.6416 + 17.936 = 58.504 m3 **BEAM ROW 5 (2ND TO 4TH FLOOR)**

G-2

V = (0.25 X 0.4 X 4.3) X 1 QTY = 0.43 m3

VOLUME TOTAL FOR BEAM ROW $5 = 0.43 \text{ m} + 3.9 \text{ m} + 0.43 \text{ m} = 4.76 \text{m} \times 2 \text{ QTY}$ = **9.52 m3**



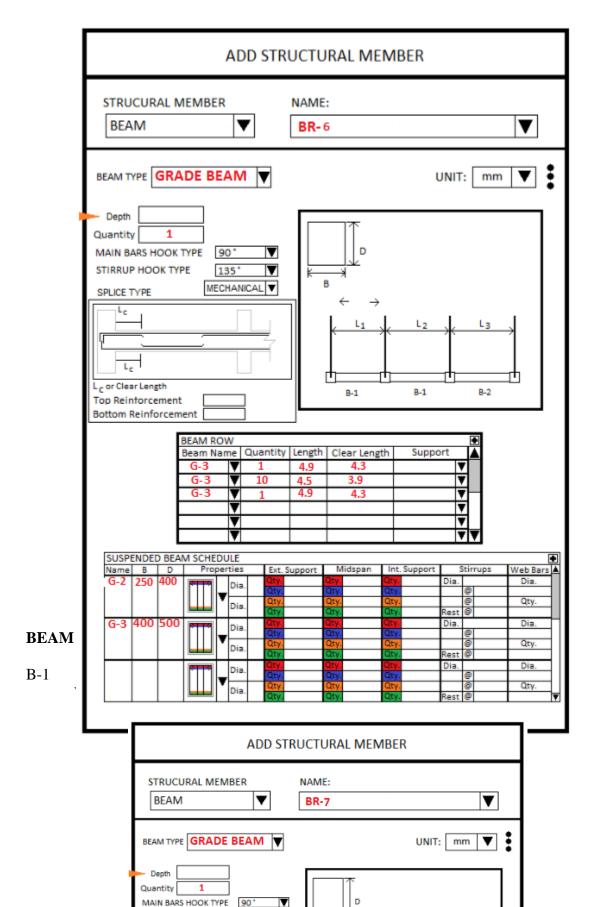
BEAM ROW

G-3 V= (0.4 X 0.5 X 4.3) X 1 QTY = 0.86m3

G-3 V = (0.4 X 0.5 X 3.9) X 10 QTY = 7.8 m3

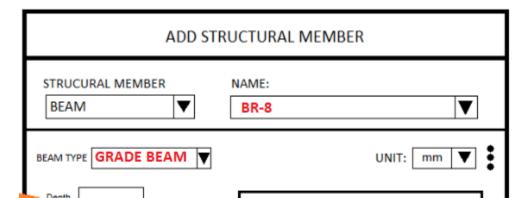
G-3

VOLUME TOTAL FOR BEAM ROW 6 = 0.86 m + 7.8 m + 0.86 m = 9.52 m



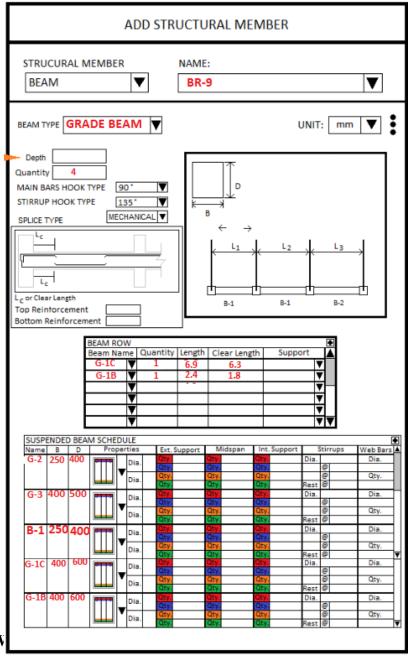
BEAM ROW 8 (2ND TO 4TH FLOOR)

B-1 V = (0.25 X 0.4 X 3.47) X 2 QTY = 0.694 m3



BEAM ROW 9 (2ND TO 4TH FLOOR)

VOLUME TOTAL FOR BR-9 = 1.512 m3 + 0.432 m3 = 1.944 m3 x 4sets =**7.776 m}3**



BEAM ROV

G-1 V = (0.4 X 0.6 X 6.3) X 1 QTY = 1.512 m3

G-1A $V = (0.4X \ 0.6 \ X \ 1.8) \ X \ 1 \ QTY = 0.432 \ m3$

VOLUME TOTAL FOR BR-9 = 1.512 m3 + 0.432 m3 = 1.944 m3 x 9 sets = 17.496 m3

	ADD ST	RUCTURAL MEMBER
	STRUCURAL MEMBER BEAM	NAME: BR- 10
	BEAM TYPE GRADE BEAM ▼	UNIT: mm V
	Depth Quantity MAIN BARS HOOK TYPE STIRRUP HOOK TYPE SPLICE TYPE MECHANICAL Lc Lc Cor Clear Length Top Reinforcement Bottom Reinforcement	
	BEAM ROW Beam Name Quanti G-1 V 1 G-1A V 1	ity Length Clear Length Support 6.9 6.3 2.4 1.8
	G-2 250 400 Dia. Ob Di	Oty. Oty. © Oty.
	B-1 250 400 Dia. Other	
	G-1C 400 600 Dia. Gas	
BEAM ROW 11 (.	G-1 400 600 Dia. Gas	
B-1	Dia. Ott	yy Qty Qty. Rest © Ty.

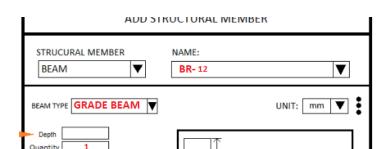
 $V = (0.25 \times 0.4 \times 4.1) \times 10 \text{ QTY} = 4.1 \text{ m}3$

ADD ST	RUCTURAL MEMBER
STRUCURAL MEMBER BEAM	NAME: BR-11
BEAM TYPE GRADE BEAM	UNIT: mm 🔻
Quantity 1 MAIN BARS HOOK TYPE 90*	

BEAM ROW 12 (2ND TO 4TH FLOOR)

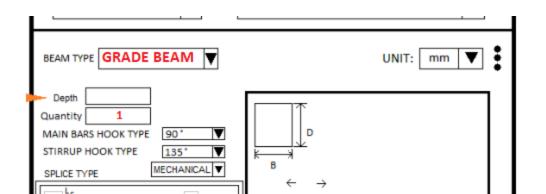
CB-1

V = (0.25 X 0.4 X1) X 1 QTY = 0.1 m3



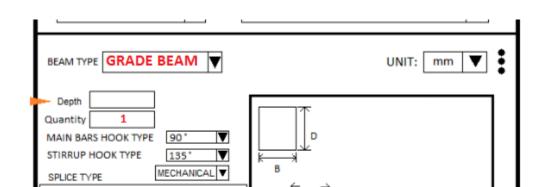
BEAM ROW 13 (2ND TO 4TH FLOOR)

CB-1 V = (0.25 X 0.4 X1) X 1 QTY = 0.1 m3



BEAM ROW 14 (2ND TO 4TH FLOOR)

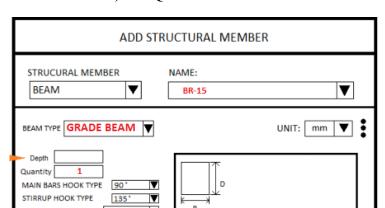
CB-1 V = (0.25 X 0.4 X1) X 1 QTY = 0.1 m3



IV. STAIR BEAM ROW 15 (2ND TO 4TH FLOOR)

B-2

V = (0.25 X 0.4 X 4.425) X 2 QTY = 0.885 m3



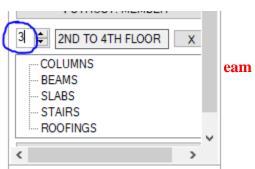
Total volume of concrete for a single floor: =9.52+9.52+0.88+0.694+7.776+17.496+4.1+0.1+0.1+0.1+0.885 = 51.171 m3

Total Volume of Concrete for 2nd floor to 4th floor : 51.171 m3 x 3 floors = 153.513 m3

ROOF BEAM (note: same illustrations and inputs ar 2nd to 4th flr)

BEAM ROW 16 (ROOF DECK)

RG-2 V= (0.25 X 0.4 X 4.3) X 1 QTY = 0.43 m3

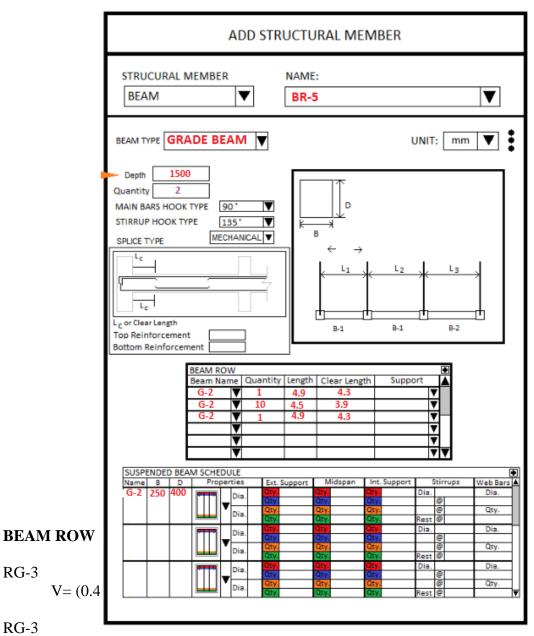


RG-2

V = (0.25 X 0.4 X 3.9) X 10 QTY = 3.9 m3

RG-2 $V = (0.25 \times 0.4 \times 4.3) \times 1 \text{ QTY} = 0.43 \text{ m}$

VOLUME TOTAL FOR BEAM ROW $5 = 0.43 \text{ m} + 3.9 \text{ m} + 0.43 \text{ m} = 4.76 \text{m} \times 2 \text{ QTY}$ = 9.52 m3



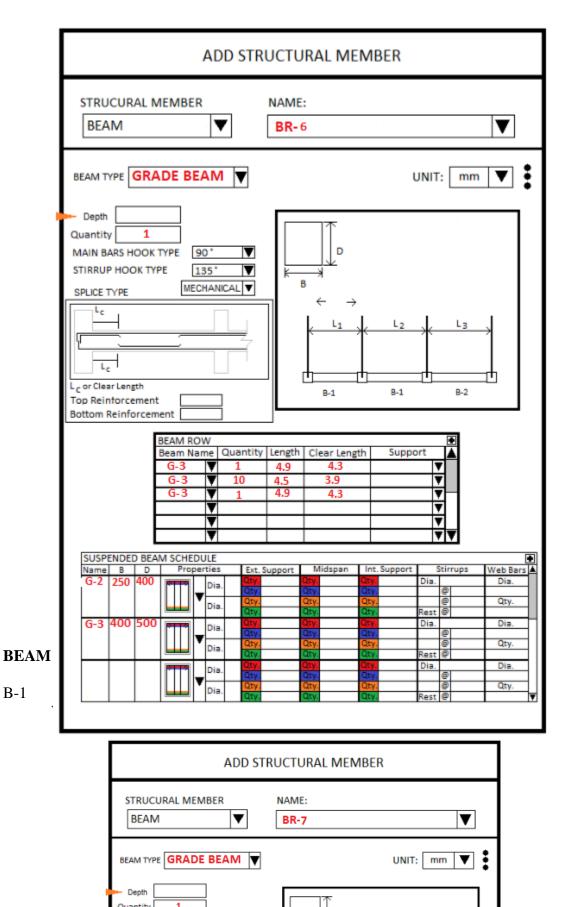
RG-3

RG-3

 $V = (0.4 \times 0.5 \times 3.9) \times 10 \text{ QTY} = 7.8 \text{ m}$

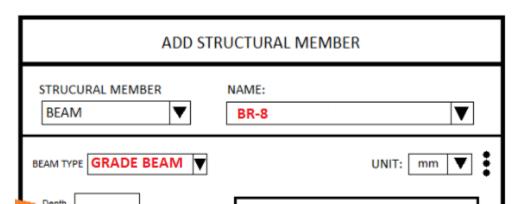
RG-3

VOLUME TOTAL FOR BEAM ROW 6 = 0.86 m + 7.8 m + 0.86 m = 9.52 m



BEAM ROW 19 (ROOF DECK)

B-1 V = (0.25 X 0.4 X 3.47) X 2 QTY = 0.694 m3



BEAM ROW 20 (ROOF DECK)

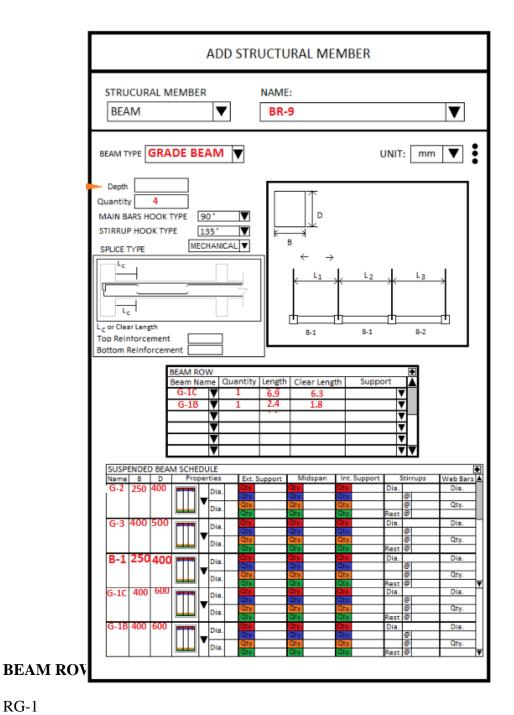
RG-1C

V = (0.4 X 0.6 X 6.3) X 1 QTY = 1.512 m3

RG-1B

 $V = (0.4X\ 0.6\ X\ 1.8)\ X\ 1\ QTY = 0.432\ m3$

VOLUME TOTAL FOR BR-20 = 1.512 m3 + 0.432 m3 = 1.944 m3 x 4sets =**7.776 m}3**



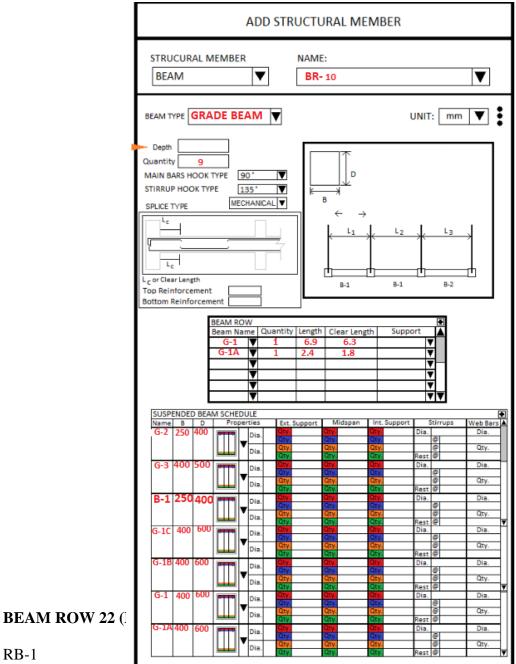
RG-1

 $V = (0.4 \times 0.6 \times 6.3) \times 1 \text{ QTY} = 1.512 \text{ m}$

RG-1A

 $V = (0.4 \times 0.6 \times 1.8) \times 1 \text{ QTY} = 0.432 \text{ m}$

VOLUME TOTAL FOR BR-21 = 1.512 m3 + 0.432 m3 = 1.944 m3 x 9 sets = 17.496 m3

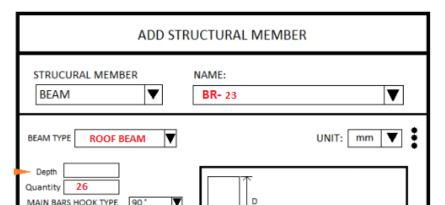


RB-1 V = (0.25 X 0.4 X 4.1) X 10 QTY = 4.1 m3

ADD STR	RUCTURAL MEMBER
STRUCURAL MEMBER BEAM	NAME: BR-11
BEAM TYPE GRADE BEAM	UNIT: mm 🔻
Quantity 1 MAIN BARS HOOK TYPE 90° ▼	

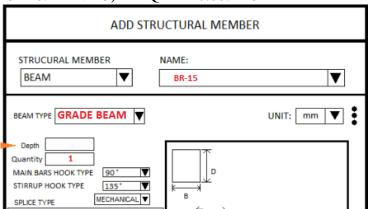
BEAM ROW 23 (ROOF DECK)

RCB-1 V = (0.25 X 0.4 X 1) X 1 QTY = 0.1 m3 x 26 QTY = 2.6 m3



BEAM ROW 24 (ROOF DECK)

B-2 V = (0.25 X 0.4 X 4.425) X 2 QTY = 0.885 m3



Total volume of concrete for a single roof deck floor: =9.52+9.52+0.88+0.694+7.776+17.496+4.1+2.6+0.885 = **53.471** m3

Total volume for all beams = 58.504 + 153.513 + 53.471 = 265.488 m3

Total cost: $265.488 \times 4910 = \mathbf{P} \, \mathbf{1,303,546.08}$

V. STAIRS CONCRETE

STAIRS (U-STAIRS) @ 6 SETS (2 PER FLOOR, 1st to 3rd Flr.)

TREAD WIDTH: 300mm

Riser: 175mm

STEPS FLIGHT 1: 9 STEPS FLIGHT 2: 9

Waist slab thickness: 150 mm Landing slab thickness: 150 mm Stair Width: 1.6m Gap: 100mm

Landing width: 1.2m

@ 4000 psi (27.6 mpa)

WAIST SLAB:

Length = $(0.3*9)^2 + (0.175*9)^2 = 3125.799$ mm or **3150mm**

$$= 0.150 \times 1.6 \times 3.15$$

 $= 0.756 \text{ m} 3 \times 2 \text{ (1st and 2nd flight)} = 1.52 \text{ m} 3$

STEPS:

= [(0.3*0.1752)/2] (1.6) (18) = **0.756 m3**

LANDING:

= $0.15 \times 1.2 \times (1.6 + 0.1 + 1.6) = 0.594 \text{ m}$

Total volume for a U-Stair = 1.52 + 0.756 + 0.594 = 2.87 m

Total volume for all U Stairs = $2.87 \times 6 = 17.22 \text{ m}$

STAIRS (L-STAIRS) @ 2 SETS (FOURTH FLOOR,)

TREAD WIDTH: 250 mm

Riser: 175 mm

STEPS FLIGHT 1: 9 STEPS FLIGHT 2: 9

Waist slab thickness: 150 mm Landing slab thickness: 150 mm

Stair Width: 1m Landing width: 1m

Concrete:

@ 4000 psi (27.6 mpa)

WAIST SLAB:

Length = (0.25*9)2+(0.175*9)2 = 2.75 m or 2750mm

$$= 0.150 \times 1 \times 2.75$$

= $0.4125 \text{ m}3 \times 2 \text{ (1st and 2nd flight)} = \mathbf{0.825 m}3$

STEPS:

= (0.25*0.1752)(1)(18) = 0.39375 m

LANDING:

 $= 0.15 \times 1 \times 1 = 0.15 \text{ m}$

Total Volume for an L-stair = 0.825 + 0.39375 + 0.15 = 1.37 m3 Total Volume for all L-stair = $1.37 \times 2 = 2.74$ m3

Total Volume of Concrete for Stairs = 17.22 +2.74 = 19.96 or **20 m3** CONCRETE: 20 m3 @ 4000psi (P 4910.00 / m3) = **P 98,200.00**

F.S. $5\% = 20 \text{m} 3 \times 5\% = 1 \text{ m} 3 \times 4910 = P 4,910.00$

Labor cost = P450.00 / m3 = 20 m3 x 450.00 =**P 9,000.00**

V. SLAB

Slab on grade

L TOP: 55000mm W LEFT: 9500mm

T: 100mm

ADD STR	UCTURAL MEMBER
STRUCURAL MEMBER	NAME: S-1 (A)
QUANTITY 1 THICKNESS 100 ELEVATION 319 REINFORCEMENTS LONGITUDINAL TRANSVERSE DIAMETER SPACING SPLICE TYPE MECHANICAL WECHANICAL	Custom Length OPEN OPEN
SLAB BORDER TOP LENGTH 55000 BOTTOM LENGTH 55000 LEFT LENGTH 9500 RIGHT LENGTH 9500	Custom Length

CONCRETE: 52.25 m3 @ 4000psi (P 4910.00 / m3) = **P 256,547.50**

F.S. $5\% = 52.25 \text{ m3} \times 5\% = 2.6125 \text{ m3} \times 4910 = P 12,827.38$

Labor cost = $P450.00 / m3 = 52.25 m3 \times 450.00 = P 23,512.50$

Hanging or Suspended Slab (2^{nd} floor to 4^{th} floor)

S-1 (A) @ 2 sets L TOP: 4400mm W LEFT: 2675mm

T: 100mm

ADD STRU	JCTURAL MEMBER
STRUCURAL MEMBER SLAB	NAME: S-1(A) ▼
	UNIT: mm ▼ 🕻
SLAB MARK S-1 QUANTITY SLAB POSITION SLAB DETAIL La Bent Up/Continuous Lb Bent Up/Continuous	Custom Length Custom Length
LONGITUDINAL TRANSVERSE TOP MECHANICAL ▼ MECHANICAL ▼ BOTTOM MECHANICAL ▼ MECHANICAL ▼ SLAB SCHEDULE SLAB HICKNESS SIZE EXT. SUPP. MIDSPAN (DAIL) TOP BOTT. TOP BOTT.	SLAB BORDER TOP LENGTH CLEAR LENGTH 4400 BOTTOM LENGTH CLEAR LENGTH 4400 LEFT LENGTH CLEAR LENGTH 2675 RIGHT LENGTH CLEAR LENGTH 2575 DIRECTION REBAR SPACING ALONG LONG DIRECTION INT. SUPP. SIZE EXT. SUPP. MIDSPAN INT. SUPP. REMARK TOP BOTT. GMAI TOP BOTT. TOP BOTT. TOP BOTT.

S-1 (B) @ 10 sets L TOP: 4100mm W LEFT: 2800mm

T: 100mm

V = 4.1 x 2.800 x .100 = 1.148 m3 x 10 = 11.48 m3

ADD S	STRUCTURAL MEMBER	
STRUCURAL MEMBER	NAME:	

S-1(B)

10

4100

4100 2800

2800

S-1 100

S-1 (C) @ 10 sets L TOP: 4100mm W LEFT: 3350mm

T: 100mm

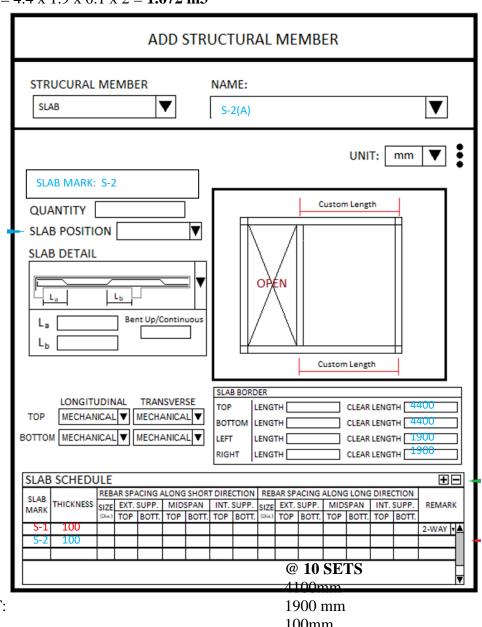
 $V = 4.1 \times 3.35 \times .100 = 1.3735 \text{ m} 3 \times 10 = 13.735 \text{ m} 3$

S-2 (A) @ 2 SETS

L TOP: 4400mm W LEFT: 1900 mm

T: 100mm

Volume = $4.4 \times 1.9 \times 0.1 \times 2 = 1.672 \text{ m}$



S-2(A)L TOP:

W LEFT:

T:

V = 4.1 x

100mm

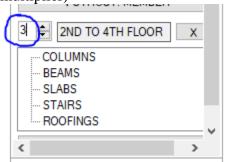
 $1.9 \times .1 \times 10 = 7.79 \text{ m}$

L TOP: 1250mm W LEFT: 3475mm

T: 100mm

 $V = 1.25 \times 3.475 \times .100 = 0.434375 \text{ m3} \times 2 = 0.86875 \text{ m3}$

V= 37.899 m3 (single floor, 2nd floor. Since 2nd floor to 4th floor is the same, use multiplier)

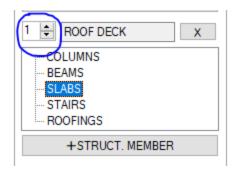


VOLUME SUSPENDED SLAB (FLOOR 2-4) = $37.899 \times 3 = 113.7 \text{ m}$

ROOF DECK S-1 (A) @ 2 SETS

L: 4400 mm W: 2675 mm T: 100mm

 $V = 4.4 \times 2.675 \times 0.1 \times 2 = 2.354 \text{ m}$



VOLUME TOTAL = 116.054 m3

CONCRETE: 116.054 m3 @ 4000psi (P 4910.00 / m3) = **P 569,825.14**

F.S. 5%=116.054 m3 x 5% = 5.8027 m3 x 4910 = **P28,491.257**

Labor cost = $P450.00 / m3 = 116.054 m3 \times 450.00 = P52,224.30$