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Item No.	Inspection and Test Point	Detail	Acceptance / Conformance Criteria	Standard / Specification	Verifying Document	Required (based from schedule qtys)	Activity	By	Downer Signature	Date	Comments / Closeout Details
SECTION	I 1 – PRE-CONSTRUCTION HOLD POINTS										
1.1	General										
1.1.1	Earthworks Methodology	The Contractor shall submit the construction and QA methodology, for each element of Earthworks, incl construction, Site Access, dewatering, wet weather, testing, inspection and hold points for review and approval prior to works.	Engineers Approval	Appendix V	Approved Methodology	Prior to Earthworks	НР	ENG			
1.1.2	Ecology Sign Off	Confirmation from Ecology that earthworks can proceed	Email	Internal Hold Point	Email from Ecology NZ	Prior to Earthworks	Н	QM			
1.1.3	Erosion & Sedimentation Controls	Confirm that Erosion and Sedimentation Controls are in place prior to earthworks proceeding	As-built Completed	Internal Hold Point	ESC As-built	Prior to Earthworks	н	QM			
1.2	Imported Material Testing - Source Prope	erty Tests for Type A1 Imported Structural Fill - ALL TESTS TO BE NO OLDI	ER THAN 3 MONTHS								
1.2.1	Weathering Resistance	NZS4407:2015, Test 3.11 - The Weathering Quality Index of Coarse Aggregate	AA, AB, AC, BA, BB, or CA	Appendix XVIII PS 16.5.5 Appendix V	IANZ Accredited Report	2 Tests Per Material, Per Source Results to Engineer 2 wks before importing	НР	ENG			
1.2.2	Los Angeles Abrasion Test	NZS4407:2015, Test 3.12 - The Abrasion Resistance of Aggregate by use of the Los Angeles Machine	50%	PS 16.5.5 Appendix V	IANZ Accredited Report	1 Test Required  Results to Engineer 2 wks before importing	НР	ENG			
1.2.3	Particle Size Distribution	NZS4407:2015, Test 3.8.1 - Particle Size Distribution	No Particles >200mm Maximum fines 10% passing 75micron sieve	Appendix XVIII PS 16.5.5 Appendix V	IANZ Accredited Report	additional 1,000m3  Results to Engineer 2 wks	НР	ENG			
1.2.4	Maximum Dry Density and Optimum Water Content	NZS4402:1986, Test 4.1.2 - Heavy Compaction Test	Report Value Only	Appendix XVIII Appendix V	IANZ Accredited Report	before importing  1 Test Per Material, Per Source  Results to Engineer 2 wks before importing	НР	ENG			
1.2.5	Soaked CBR	NZS4407:2015, Test 3.15 The California Bearing Ratio (CBR)	>25%	PS 16.5.5 Appendix V	IANZ Accredited Report	2 Tests Per Material, Per Source Results to Engineer 2 wks before importing	НР	ENG			
1.2.6	Solid Density	NZS4407:2015, Test 3.7.1 - The Solid Density of Aggregate Particles	Report Value Only	Appendix XVIII Appendix V	IANZ Accredited Report	2 Tests Per Material, Per Source Results to Engineer 2 wks before importing	НР	ENG			
1.2.7	Crushing Resistance	NZS4407:2015, Test 3.10 - The Crushing Resistance of Coarse Aggregate Under a Specified Load	>100KN	Appendix XVIII Appendix V	IANZ Accredited Report	2 Tests Per Material, Per Source Results to Engineer 2 wks before importing	НР	ENG			





		I	ı			Testing Quantity	Verificati	ion Activity	Downer Conformance of Compliance Signoff		
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1.3	Pumice Sand Fill										
1.3.1	Particle Size Distribution	NZS4407:2015, Test 3.8.1 - Particle Size Distribution	Report Value Only	PS 16.5.12	IANZ Accredited Report	Number of Tests Required?  Stockpile size: 0 to 400m3 – 2 tests 400 to 1500m3 – 3 tests 1500 to 4000m3 – 4 tests >4000m3 – 1 test for each additional 1,000m3  Results to Engineer 2wks before importing	НР	ENG			
1.3.2	Maximum Dry Density and Optimum Water Content	NZS4402:1986, Test 4.1.2 - Heavy Compaction Test	Report Value Only	PS 16.5.12	IANZ Accredited Report	Test Per Material, Per Source  Results to Engineer 2 wks before importing	НР	ENG			
1.4	Drainage Aggregate - Used in Subsoil Tre	ench Drains & Drainage Blankets									
1.4.1	Crushing Resistance	NZS4407:2015, Test 3.10 - The Crushing Resistance of Coarse Aggregate Under a Specified Load	>100KN	Appendix XVIII	IANZ Accredited Report	2 Tests Per Material, Per Source Results to Engineer 2 wks before importing	НР	ENG			
1.4.2	Particle Size Distribution	NZS4407:2015, Test 3.8.1 - Particle Size Distribution	Max Particle Size 53mm  Max 10% passing 13.2mm sieve	Appendix XVIII	IANZ Accredited Report	1 Test per 100m3  Results to Engineer 2wks before importing	НР	ENG			
1.4.3	Weathering Resistance	NZS4407:2015, Test 3.11 - The Weathering Quality Index of Coarse Aggregate	CA or better	Appendix XVIII PS 16.5.5 Appendix V	IANZ Accredited Report	2 Tests Per Material, Per Source Results to Engineer 2 wks before importing	НР	ENG			
1.5	Geotextile										
1.5.1	Geotextile	The Contractor shall submit the full details of the filter fabric they propose to use to the Engineer for approval prior to installation.	Engineers Approval	PS 16.5.7 Appendix V	Data Sheets	Results to Engineer for Approval	НР	ENG			
1.6	Topsoil Source Testing										
1.6.1	Laboratory Testing for Soil Suitability	Soil Test Results from Hill Laboratories to confirm that topsoil suitable to for reuse Soil Reaction Electrical Conductivity Mechanical Analysis (texture) Stone Content Nutrient Levels of nitrogen, available phosphorus, available potassium and available magnesium Organic Matter Content Check for Toxic or Hazardous substances	Engineers Approval	NZTA P39:2013 3.1	Hill Laboratories Test Report	10 equal samples taken from site, mixed together, then 1kg of soil sealed into a plastic bag and sent to lab for sampling	1	SE			
1.7	Rock Ballast Aggregate								_		



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Item No.	Inspection and Test Point	Detail	Conformance Criteria	Specification	Document	Required (based from schedule qtys)	Activity	Ву	Downer Signature	Date	Comments / Closeout Details		
1.7.1	Crushing Resistance	NZS4407:2015, Test 3.10 - The Crushing Resistance of Coarse Aggregate Under a Specified Load	>100KN	PS 16.5.15	IANZ Accredited Report	1 Test Per Material, Per Source Results to Engineer 2 wks before importing	НР	ENG					
1.7.2	Particle Size Distribution	NZS4407:2015, Test 3.8.1 - Particle Size Distribution	Max Particle Size 130mm Max 5% passing 19mm sieve  Grading Sieve Size % Passing 130mm 100 100mm 70-80 63mm 0-50 37.5mm 20-25 19mm 0-5	PS16.5.15	IANZ Accredited Report	1 Test Per Material, Per Source Results to Engineer 2 wks before importing	НР	ENG					
SECTION 2 – SITE WON LANDSCAPING FILL													
2.1	Type A3 Material - Site Won Landscaping	Fill - Field Test											
2.1.1	On Site track-able Testing	Fine grained Type A3 site won materials may be considered suitable for use as landscape fill providing that it is 'track-able' by the contractor's plant. This shall be determined before Sampling.	Trackable by Contractors Plant	PS 16.5.5	Visual Site Record	Engineers Approval	R	ENG					
2.1.2	Shear Vane	NZ Geotechncial Society 2001 - Guideline for Hand Held Shear Vane Test Fine grained Type A3 site won materials may be considered suitable for use as landscape fill providing that it meets minimum shear vane strength of 80kPa	Min >80kPa	PS 16.5.5	Shear Vane Report	Results to Engineer for Approval	R	ENG					
2.2	Type A3 Material - Site Won Landscaping	Fill - Placement											
2.2.1		To be visually free of deletorious material, litter etc, and track rolled for compaction, with no slumping once placed	Visual Inspection	PS 16.5.6	Visual Site Record	As Required	V	SE					
SECTION	I 3 – CUT SUBGRADE TESTING												
3.1	Cut Subgrade Testing - All Soils												
3.1.1	Proof Roll	Visual Inspection of Proof Roll	Displacement <15mm	Appendix XVIII	Visual Observation	Across stripped area	V	SE					
3.1.2	Cut Subgrade Inspection	Engineer to inspect all foundation surface inspections	Engineers Approval	Appendix V	Visual Observation	Across stripped area	НР	ENG					
3.2	Cut Subgrade Testing - Cohesive Soils												
3.2.1	Cut Subgrade Testing - Cohesive Soil Shear Vane	NZ Geotechncial Society 2001 - Guideline for Hand Held Shear Vane Test  This test shall be undertaken at the design subgrade level to a depth of 2-3m below the design subgrade level	Average Minimum SU:  > 60kPa  Single Test Min SU:  >50kPa	Appendix XVIII	Shear Vane Report	4 test per 400M2	I	SE					



Name No. Repeated on and Test Points	Relationsin	Number and Revision: DN1210 - 003 - Rev IFC:												
Conformation of Conformation Conformation   Conformation Conformation   Conformation Conformation   Conformation Conformation   Conformatio				A 1	Oten dend /	Varifiin n	Testing Quantity	Verificati	ion Activity	Downer Conf	ormance of C	ompliance Signoff		
Scale personness to compare Technique Soil   Scale personness or receiving to be understeam national of abservant tests   Minimum 3 blows period   Appendix XVIII   Scale personness or selected by the sele	Item No.	Inspection and Test Point		•			Required (based from	Activity	Ву		Date	Comments / Closeout Details		
Scale Percent Post   Scale Report	3.2.2		Scala penetrometer testing to be undertaken instead of shear vane tests		Appendix XVIII	Scala Report	4 test per 400M2	I	SE					
2.3.1 Scale Protection of the protection of the design subgrade level to a disprit of 2-3m (blue protection of policy the design subgrade level to a disprit of 2-3m (blue protection of policy the design subgrade level to a disprit of 2-3m (blue protection of policy the design subgrade level to a disprit of 2-3m (blue protection) of the protection of th	3.3	Cut Subgrade Testing - Granular Soils												
3.4.1 Rolesse of Hold Point	3.3.1	Scala Penetrometer	This test shall be undertaken at the design subgrade level to a depth of 2-3m		Appendix XVIII	Scala Report	4 test per 400M2	I	SE					
SECTION 4 — CUT SLOPE INSPECTIONS  4.1 Cut Stope Inspection  4.1.1 Cut Stope Inspection  Inspection records to include prolographs, logged soils (to NZ Geolechincal Guidelines) and shear varie testing  At 1.2 Cut Stope Inspection  Inspection records to include prolographs, logged soils (to NZ Geolechincal Guidelines) and shear varie testing  At 1.2 Cut Stope Inspection  Inspection records to include prolographs, logged soils (to NZ Geolechincal Guidelines)  At 2 Geolechincal Society 2001 - Guidelines of Adjustment of Stope Soil Log  NZ Geolechincal Society 2001 - Guideline of Pland Hold Shear varie Test  Average Minimum SU:  - 50kPa  Autrage Minimum SU: - 50kPa  Shear Varie Test  This test shall be undertaken at the design subgrade level to a depth of 2-3m below the design subgrade level to a depth of 2-3m below the design subgrade level to a depth of 2-3m below the design subgrade level or a depth of 2-3m below the design and Grade Chack  Confirm cut slopes have been correctly excavated to design and grade  4.1.5 Design and Grade Chack  Confirm cut slopes have been correctly excavated to design and grade  At 1.5 Design and Grade Chack  Confirm cut slopes have been correctly excavated to design and grade  At 2 Foundation Subgrade Hold Point Release  4.2 Figheers Approval  Engineers Approval  Engineers Approval  Engineers Approval  Appendix V  Engineers Approval  Figheers Ap	3.4	Foundation Subgrade Hold Point Release												
4.1. Cut Slope Inspection  4.1.1 Cut Slide Slope Inspection	3.4.1	Release of Hold Point	Contractor shall provide written certification that all insepcted and tested	Engineers Approval	PS 16.9.3		Engineers Approval	НР	ENG					
4.1.1 Cut Side Sitiop Inspection Guidelines and the Authorities (Cut Side Side Inspection Society 2001 - Guidelines) and the arvante testing (Cut Side Side Inspection Society 2001 - Guidelines (Cut Side Side Inspection Society 2001 - Guidelines (Cut Side Side Inspection Society 2001 - Guidelines (Cut Side Side Inspection Society 2001 - Guideline for Hand Held Shear Vane Test (Cut Side Side Inspection Society 2001 - Guideline for Hand Held Shear Vane Test (Cut Side Side Inspection Society 2001 - Guideline for Hand Held Shear Vane Test (Cut Side Side Inspection Society 2001 - Guideline for Hand Held Shear Vane Test (Cut Side Side Inspection Society 2001 - Guideline for Hand Held Shear Vane Test (Cut Side Side Inspection Society 2001 - Guideline for Hand Held Shear Vane Test (Cut Side Side Inspection Society 2001 - Guideline for Hand Held Shear Vane Test (Cut Side Side Inspection Society 2001 - Guideline for Hand Held Shear Vane Test (Cut Side Side Inspection Society 2001 - Guideline for Hand Held Shear Vane Test (Cut Side Side Inspection Society 2001 - Guideline for Hand Held Shear Vane Test (Cut Side Side Inspection Society 2001 - Guideline for Hand Held Shear Vane Test (Cut Side Side Inspection Society 2001 - Guideline for Hand Held Shear Vane Test (Cut Side Side Inspection Society 2001 - Guideline for Hand Held Shear Vane Test (Cut Side Side Inspection Society 2001 - Guideline for Hand Held Shear Vane Test (Cut Side Side Inspection Society 2001 - Guideline for Hand Held Shear Vane Test (Cut Side Side Inspection Society 2001 - Guideline for Hand Held Shear Vane Test (Cut Side Side Inspection Society 2001 - Guideline for Hand Held Shear Vane Test (Cut Side Side Inspection Society 2001 - Guideline Society 2001 - Guidel	SECTION 4 – CUT SLOPE INSPECTIONS													
4.1.2 Cut Slope - Soil Log Soils to be logged to NZ Geotechnical Guidelines National PS 16.9.4 Soil Log Soils to be logged to NZ Geotechnical Guidelines NZ Geotechnical Guideline for Hand Held Shear Vane Test This test shall be undertaken at the design subgrade level to a depth of 2-3m below the Report NA per Engineer Approval  4.1.4. Design and Grade Check  4.1.5. Design and Grade Check  4.1.6. Stabilisation of Cut Stops Surface  5.1.6.4. Site Records  5.1.1. Undercut to Waste & BackFill  5.1.1. Undercut to Waste & BackFill  5.1.2. Undercut of Waste & BackFill  5.1.3. Undercut for Waste & BackFill  5.1.4. Blessmet of Cottorile of Undercut Surface  6.1.5. Blessmet of Cottorile of Undercut Surface  6.1.5. Blessmet of Cottorile of Undercut Surface  6.	4.1 Cut Slope Inspection													
Autrage Minimum SU: 50kPa  Average Minimum SU: 50kPa  Average Minimum SU: 50kPa  PS 16.6.4  Shear Vane Report  I SE  Average Minimum SU: 50kPa  Average Minimum SU: 50kPa  Single Test Min SU: 50kPa  PS 16.6.4  PS 16.6.4  Shear Vane Report  I SE  I SE  Average Minimum SU: 50kPa  Average Minimum SU: 50kPa  Single Test Min SU: 50kPa  Average Minimum SU: 50kPa  PS 16.6.4  Shear Vane Report  I SE  I SE  Average Minimum SU: 50kPa  Single Test Min SU: 50kPa  Average Minimum SU: 50kPa  PS 16.6.4  Shear Vane Report  I SE  I SE  Average Minimum SU: 50kPa  Shear Vane Report  I SE  I See Vane Report  As per Engineer I Engineer Approval As per Engineer I ENG  SV  Average Minimum SU: 50kPa  Shear Vane Report  I SE  I See Vane Report  As per Engineer I ENG  SV  Average Minimum SU: 50kPa  Shear Vane Report  I SE  Shear Vane Report  I SE  Shear Vane Report  I SE  SHE  Shear Vane Report  I ENG  SN  SN  SN  SN  SN  SN  SN  SN  SN	4.1.1	Cut Side Slope Inspection	Guidelines) and shear vane testing	Engineers Approval		Site Records	As per Engineer	WP	ENG					
A.1.3 Cut Slope - Shear Vane Test  NZ Geotechnoial Society 2001 - Guideline for Hand Held Shear Vane Test This test shall be undertaken at the design subgrade level to a depth of 2-3m below the design subgrade level to a design and grade  PS 16.6.4 Size Records  PS 16.6.4 Siz	4.1.2	Cut Slope - Soil Log	Soils to be logged to NZ Geotechnical Guidelines		PS 16.6.4	Soil Log		I	SE					
Into test shall be undertaken at the design subgrade level below the design subgrade level be design and grade level below the design subgrade level below the design and grade level below the design subgrade level below the design and grade level level and	4.1.3	Cut Slope - Shear Vane Test	·	> 60kPa	PS 16.6.4			ı	SE					
4.1.4 Drainage weasures it Seepage Encountered Borenois Drains or balasting may be required, to be directed by Engineer Engineer Approval PS 16.6.4 approval As per Engineer I ENG 4.1.5 Design and Grade Check Confirm cut slopes have been correctly excavated to design and grade PS 16.7.1 Survey Records D SV 4.1.6 Stabilisation of Cut Slope Surface Cut slope surfaces to be topsoiled and grass seeded or planted as soon as possible PS 16.6.4 Site Records C SE 7.1.1 Survey Records D SV 8.1.1.1 Stabilisation of Cut Slope Surface PS 16.6.4 Site Records D SV 8.1.1.1 Stabilisation of Cut Slope Surface PS 16.6.4 Site Records D SV 8.1.1.1 Stabilisation of Cut Slope Surface Surface PS 16.6.4 Site Records D SV 8.1.1.1 Stabilisation of Cut Slope Surface Surface Surface Subgrade hold point and be able to start placing fill, the Contractor shall provide written certification that all insepcted and tested subgrade comiles with the requirements in PS 16.6 Subgrade comiles with the requirements in PS	0			•		Report		,	01					
4.1.6 Stabilisation of Cut Slope Surface	4.1.4	Drainage Measures if Seepage Encountered	Borehole Drains or ballasting may be required, to be directed by Engineer	Engineers Approval	PS 16.6.4		As per Engineer	I	ENG					
4.2 Foundation Subgrade Hold Point Release  4.2.1 Release of Hold Point Driving written certification that all insepcted and tested subgrade comilies with the requirements in PS 16.6 Beginners Approval Driving Written Certification PS 16.9 Beginners Approval Driving Written Driving Written Certification PS 16.6 Beginners Approval Driving Written Driving Written Certification PS 16.9 Beginners Approval Driving Written Driving W	4.1.5	Design and Grade Check	, , , , ,		PS 16.7.1	Survey Records		D	SV					
4.2.1 Release of Hold Point To release the subgrade hold point and be able to start placing fill, the Contractor shall provide written certification that all insepcted and tested subgrade comlies with the requirements in PS 16.6  SECTION 5 – UNDERCUT TO WASTE & BACKFILL  5.1 Undercut to Waste & Backfill  5.1.1 Undercut Surface Inspection Engineer to inspect undercut surface Engineers Approval Appendix V Engineers Approval Appendix V Engineers Approval PS 16.6.4  Geotextile to be placed across the undercut base and up the sides, with joints Engineers Approval PS 16.6.4  WE NOTE: The context of Contextile on Undercut and Engineers Approval PS 16.6.4	4.1.6	Stabilisation of Cut Slope Surface			PS 16.6.4	Site Records		С	SE					
4.2.1 Release of Hold Point Contractor shall provide written certification that all insepcted and tested subgrade comlies with the requirements in PS 16.6  SECTION 5 - UNDERCUT TO WASTE & BACKFILL  5.1 Undercut to Waste & Backfill  5.1.1 Undercut Surface Inspection Engineer to inspect undercut surface Engineers Approval Appendix V Engineers Approval Approval PS 16.6.4  6.1.2 Placement of Contraction shall provide written certification that all insepcted and tested subgrade comlies with the requirements in PS 16.6.4  Engineers Approval PS 16.9.3 Certification Engineers Approval PS 16.9.4 Certification Engineers Approval	4.2	Foundation Subgrade Hold Point Release												
5.1 Undercut to Waste & Backfill  5.1.1 Undercut Surface Inspection Engineer to inspect undercut surface Engineers Approval Appendix V Engineers Approval Across stripped area WP ENG  5.1.2 Placement of Contextile on Undercut  Geotextile to be placed across the undercut base and up the sides, with joints Engineers Approval PS 16.6.4	4.2.1	Release of Hold Point	Contractor shall provide written certification that all insepcted and tested	Engineers Approval	PS 16.9.3		Engineers Approval	НР	ENG					
5.1.1 Undercut Surface Inspection Engineer to inspect undercut surface Engineers Approval Appendix V Engineers Approval Across stripped area WP ENG  5.1.2 Placement of Contextile on Undercut  Geotextile to be placed across the undercut base and up the sides, with joints Engineers Approval PS 16.6.4  WP ENG	SECTION	5 – UNDERCUT TO WASTE & BACKFILL												
5.1.1 Ordercut surface hispection Engineer to hispect undercut surface Approval Approval Approval Approval Approval Approval Approval PS 16.6.4  Geotextile to be placed across the undercut base and up the sides, with joints Engineers Approval PS 16.6.4	5.1	Undercut to Waste & Backfill												
	5.1.1	Undercut Surface Inspection	Engineer to inspect undercut surface	Engineers Approval	Appendix V		Across stripped area	WP	ENG					
	5.1.2	Placement of Geotextile on Undercut		Engineers Approval				WP	ENG					
5.1.3 Subsoil Drain inspection Subsoils to be placed at base of cuts where directed by the Engineer Engineers Approval PS 16.6.4 No less than 24 hours prior to placement of backfill	5.1.3	Subsoil Drain inspection	Subsoils to be placed at base of cuts where directed by the Engineer	Engineers Approval	PS 16.6.4			WP	ENG					
5.2 Foundation Subgrade Hold Point Release	5.2	Foundation Subgrade Hold Point Release												
To release the subgrade hold point and be able to start placing fill, the Contractor shall provide written certification that all insepcted and tested subgrade comlies with the requirements in PS 16.6  To release the subgrade hold point and be able to start placing fill, the Engineers Approval PS 16.9.3  Written Certification Engineers Approval Engineers Approval PS 16.9.3	5.2.1	Release of Hold Point	Contractor shall provide written certification that all insepcted and tested	Engineers Approval	PS 16.9.3		Engineers Approval	НР	ENG					



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SECTION	6 - COMPACTION OF IMPORTED FILL											
6.1	Placement of Geotextile Under Structural	Fill										
6.1.1		A layer of Class B Geotextile Fabric to be placed between cut surface and imported A1 Structural Fill with 500mm overlaps	Visual Inspection	NTC084	Visual Observation	As Required	I	SE				
6.2	Compaction Requirements for Type A1 Im	ported Structural Fill (Granular Bulk Fill, GAP40, GAP65)										
6.2.1	Compaction - Clegg Hammer	ASTM D5874-16:2016 Clegg Hammer Testing	Structural Fill Min CIV 18 Ave CIV 20  Non-Structural Fill Min CIV 10 Max CIV 12	Appendix XVIII		1 set (5 tests per 5x5m grid) per 1000m2 of placed per area worked each day, per lift	ı	SE				
6.2.2	Compaction - NDM	NZS4407:2015, Test 4.3 Nuclear Moisture Density Gauge (Backscatter Mode)	Ave >95% of MDD Min >92% of MDD Non Granular Material Max Air Voids 8%	Appendix XVIII	IANZ Accredited Report	1 set (5 tests) per 400mm lift (every 2nd lift) of fill per 1000m2 of placed	I	SE				
6.2.3	Compaction Trials	The Contractor shall carry out compaction trials of the various soil types and combinations on site, and any imported fill materials, using different layer thicknesses, types of rollers and conditioning plant (ie construction discs and scarifiers). The trials shall be conducted over the duration of the earthworks operation with the objective of determining the most suitable and efficient techniques to meet the compaction requirements. The trial details, test data and conclusions shall be reported and copied to the Engineer within five days of completion of each trial.	Engineers Approval	PS 16.6.5	Trial Results		R	ENG				
6.3	Compaction Requirements for Pumice Sar	nd Fill										
6.3.1	Compaction - Structural Fill Scala Penetrometer	NZS4402:1988, Test 6.5.2 - Dynamic Cone Penetrometer  Scala penetrometer testing to be undertaken instead of shear vane tests where shear vane is unable to penetrate subgrade material	Minimum 5 blows per 100mm penetration	PS 16.5.12	Scala Report	Minimum 1 test per 500m3 of filling evenly distributed over the extent and depth of the filling. A minimum of 1 test per 500mm lift or part thereof to be completed through the full lift thickness.	I	SE				
6.3.2	Compaction - Structural Fill NDM	NZS4407:2015, Test 4.3 Nuclear Moisture Density Gauge (Backscatter Mode)	Ave >95% of MDD Min >92% of MDD	PS 16.5.12	IANZ Accredited Report	1 set (5 tests) per 400mm lift (every 2nd lift) of fill per 1000m2 of placed	I	SE				
6.3.3	Pumice Sand Fill	Pumice Sand Material shall be contained with an outer layer of fill with a minimum thickness of 600mm. Must be Hardfill and/or imported Type A1 material and/or Type A2 material and/or Type A3 material		PS 16.5.12	Site Records		E	SE				
SECTION	7 - TOPSOIL PLACEMENT											
7.1	Cut and fill embankments											
7.1.2	Topsoil Placement on cut and fill embankments	Confirmation of thickness and material suitability	Meets spec and thickness requirements as per drawings	PS 16.6.3 b)	Visual Observation	Once per lot after topsoil re spread	I	SE				
SECTION	8 - SETTLEMENT INSTALLATION & MONI	TORING										
8.1	Settlement Monitoring											
8.1.1	Instrument Reading Notice	The Contractor shall inform the Engineer two days in advance of planned instrument readings so that the Engineer may observe the work.	2 days notice to Eng	PS 16.8.2	Site Records	Every Time	I	SE				



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8.1.2	Results Submitted to Engineer	Results will be submitted to the Engineer within 1 day of reading the instruments	To Engineer within 1 day of Reading	PS 16.8.2	Site Records	Every Time	1	SE			
8.1.3	Baseline Readings	The Contractor shall take 3 sets of baselines readings from all instruments at 2 day intervals	Engineers Approval	PS 16.8.2	Site Records	Every 2 days	I	SE			
8.1.4	Readings During Active Fill Placement	During active fill placement, the Contractor shall read all monitoring plates twice a week or every 1 metre of fill whichever is more frequent	To Engineer within 1 day of Reading	PS 16.8.2	Site Records	Twice a week	R	SV			
8.1.5	Readings During No Active Earthworks in Area	The Contractor shall read all monitoring plates weekly if there is no active earthworks taking place in the area	To Engineer within 1 day of Reading	PS 16.8.2	Site Records	Weekly	R	SV			
8.1.6		evaluated by the Engineer to assess the need to extend the monitoring.	Either: 3 Consecutive Measurements showing T90 settlement value achieved Or Rate of settlement less than 10% of the maximum rate measured immediately after the fill was completed	PS 16.8.5	Site Records	Engineers Approval	НР	ENG			
SECTION	9 – AS-BUILTS										
9.1.1	As-Builts	As-Built records and drawings shall accurately record the layout and extents of all the work including all of the cuts and fills, monitoring instrumentation, ground improvements, and locations of foundation / subsurface drainage systems and their outlets.		PS 16.7.6	Survey Records	During Earthworks	D	SV			