

## Design of Beams

### 1 MODULE No. 1: ULTIMATE FLEXURE STRENGTH (R-SECTION)

#### 1.1 Inputs

##### 1.1.1 Material Properties

- ✓ Concrete compressive strength ( $f_{cu}$ )
- ✓ Reduction factor of concrete compressive strength ( $\gamma_c=1.50$ )
- ✓ Yield strength of longitudinal reinforcing steel bars ( $f_y$ )
- ✓ Reduction factor of reinforcing steel yield strength ( $\gamma_s=1.15$ )
- ✓ Modulus of elasticity of reinforcing steel bars ( $E_s = 200,000 \text{ N/mm}^2$ )

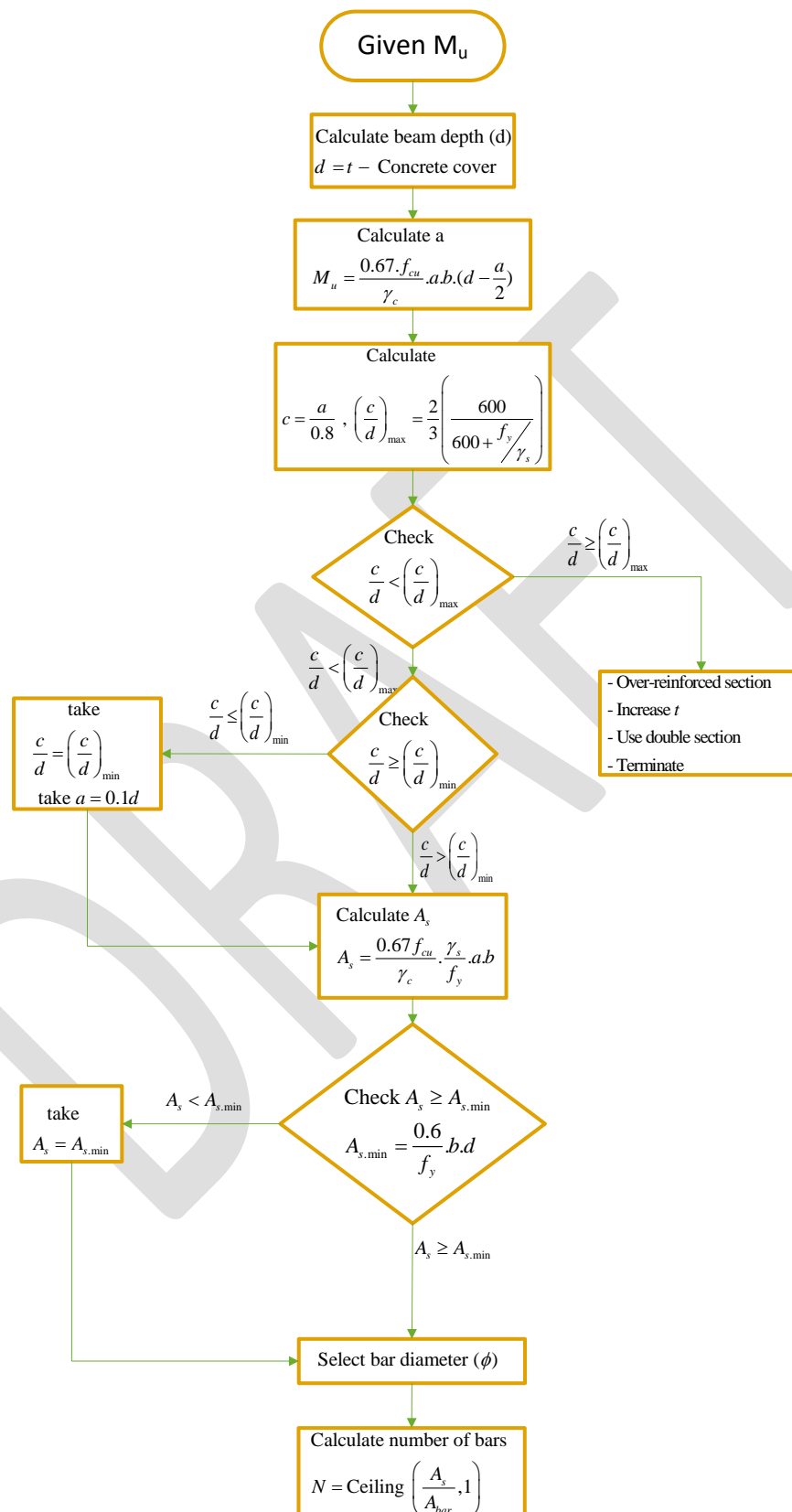
##### 1.1.2 Section Definition

- ✓ Beam width ( $b$ )
- ✓ Beam total depth ( $t$ )
- ✓ Concrete cover

##### 1.1.3 Internal Forces

- ✓ Ultimate Factored Moment from analysis ( $M_u$ )

## 1.2 Calculation Procedure



### 1.3 Design Outputs

- ✓ Beam total depth (t).
- ✓ Diameter of bars.
- ✓ Number of bars.

DRAFT

## 1.4 Example of Calculations using Mathcad

### 1.1 Input

#### 1.1.1 Material Properties

$$f_{cu} := 30 \frac{N}{mm^2}$$

Concrete compressive strength

$$f_y := 360 \frac{N}{mm^2}$$

Yield strength of reinforcing steel bar

$$E_s := 200000 \frac{N}{mm^2}$$

Modulus of elasticity of steel

$$\gamma_c := 1.5$$

Concrete strength reduction factor

$$\gamma_s := 1.15$$

Steel strength reduction factor

#### 1.1.2 Section Definition

$$b := 250 \text{ mm}$$

Beam width

$$t := 700 \text{ mm}$$

Total beam depth

$$\text{conc.cover} := 50 \text{ mm}$$

Concrete Cover

#### 1.1.3 Internal Forces

$$M_u := 300 \text{ kN}\cdot\text{m}$$

Ultimate Factored Moment from analysis

### 1.2 Calculation Procedure

$$d := t - \text{conc.cover} = 650 \text{ mm}$$

$$a := 1 \text{ mm}$$

$$a := \text{root} \left( M_u - 0.67 \frac{f_{cu}}{\gamma_c} \cdot a \cdot b \cdot \left( d - \frac{a}{2} \right), a \right)$$

$$a = 156.649 \text{ mm}$$

$$c := \frac{a}{0.8} = 195.8 \text{ mm}$$

$$c/d = f_{c1} := \text{if} \left( \frac{c}{d} \geq \frac{2}{3} \cdot \frac{0.003}{0.003 + \frac{f_y}{\gamma_s \cdot E_s}}, \text{"Over reinforced section, increase ts"}, \frac{c}{d} \right)$$

$$f_{c1} = 0.301$$

$$c := \text{if} (c < 0.125 \cdot d, 0.125 \cdot d, c)$$

$$c = 195.811 \text{ mm}$$

$$a := 0.8 \cdot c = 156.649 \text{ mm}$$

$$A_s := 0.67 \cdot \frac{f_{cu}}{\gamma_c} \cdot \frac{\gamma_s}{f_y} \cdot a \cdot b = (2 \cdot 10^3) \text{ mm}^2$$

Calculate As required

$$A_{smin} := \frac{0.6 \frac{N}{\text{mm}^2} \cdot b \cdot d}{f_y} = 271 \text{ mm}^2$$

Check As &gt;= Asmin

$$A_s := \text{if}(A_s < A_{smin}, A_{smin}, A_s)$$

$$A_s = (2 \cdot 10^3) \text{ mm}^2$$

$$\phi 6 := 28.3 \text{ mm}^2$$

$$\phi 8 := 50.3 \text{ mm}^2$$

$$\phi 10 := 78.5 \text{ mm}^2$$

$$\phi 12 := 113 \text{ mm}^2$$

$$\phi 14 := 154 \text{ mm}^2$$

$$\phi 16 := 201 \text{ mm}^2$$

$$\phi 18 := 254 \text{ mm}^2$$

$$\phi 20 := 314 \text{ mm}^2$$

$$\phi 22 := 380 \text{ mm}^2$$

$$\phi 25 := 491 \text{ mm}^2$$

$$\phi 28 := 616 \text{ mm}^2$$

$$\phi 32 := 804 \text{ mm}^2$$

$$\phi 38 := 1134 \text{ mm}^2$$

$$N := \frac{A_s}{\phi 16} = 8.3$$

$$N := \text{Ceil}(N, 1) = 9$$

### 1.3 Output

$$t = 700 \text{ mm}$$

$$b = 250 \text{ mm}$$

$$\text{Use } N = 9 \quad \phi 16$$