

Design of Beams

3 MODULE NO. 2: ULTIMATE FLEXURE STRENGTH (R_DOUBLE-SECTION)

3.1 Inputs

3.1.1 Material Properties

- ✓ Concrete compressive strength (f_{cu})
- ✓ Reduction factor of concrete compressive strength ($\gamma_c=1.50$)
- ✓ Yield strength of longitudinal reinforcing steel bars (f_y)
- ✓ Reduction factor of reinforcing steel yield strength ($\gamma_s=1.15$)
- ✓ Modulus of elasticity of reinforcing steel bars ($E_s = 200,000 \text{ N/mm}^2$)

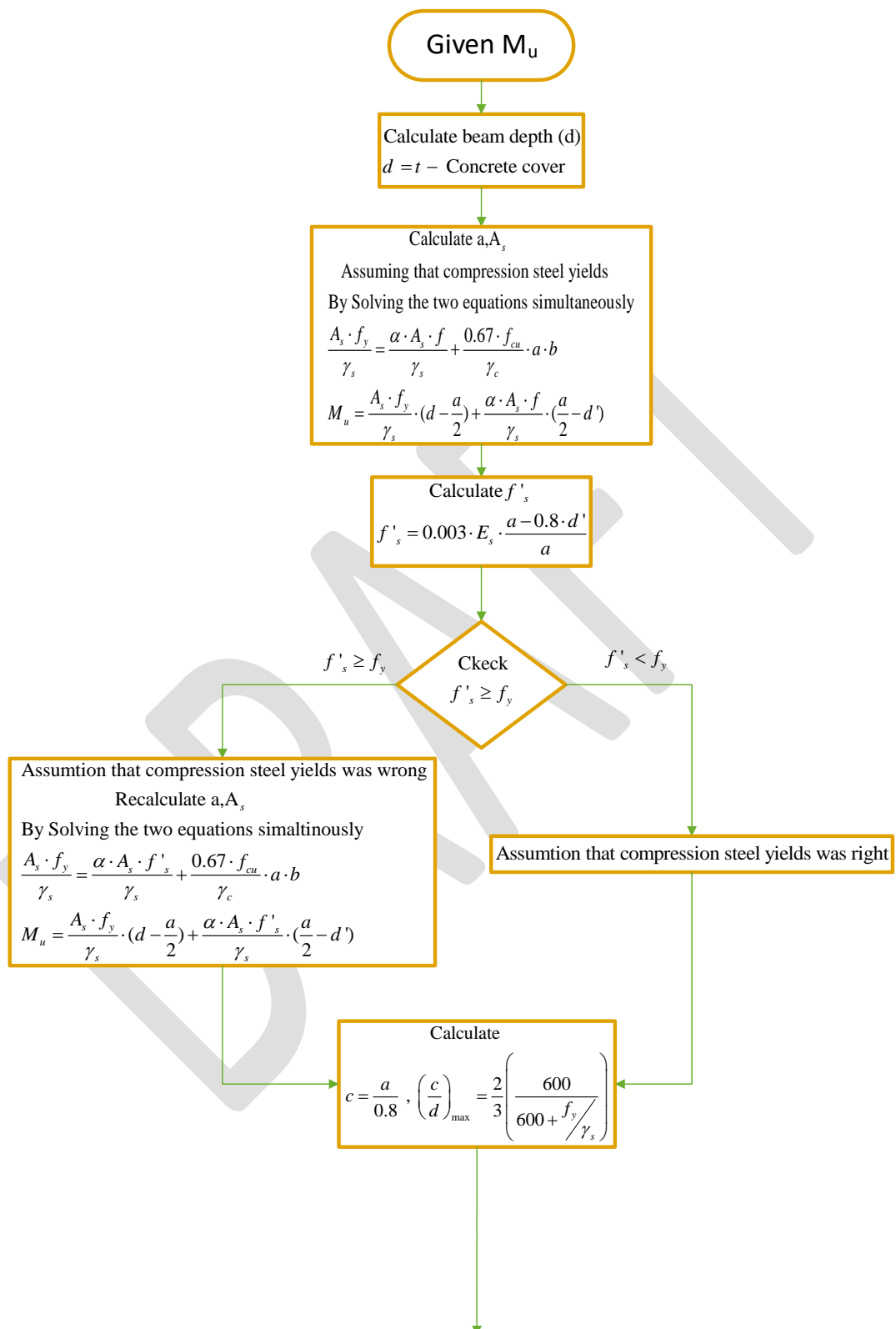
3.1.2 Section Definition

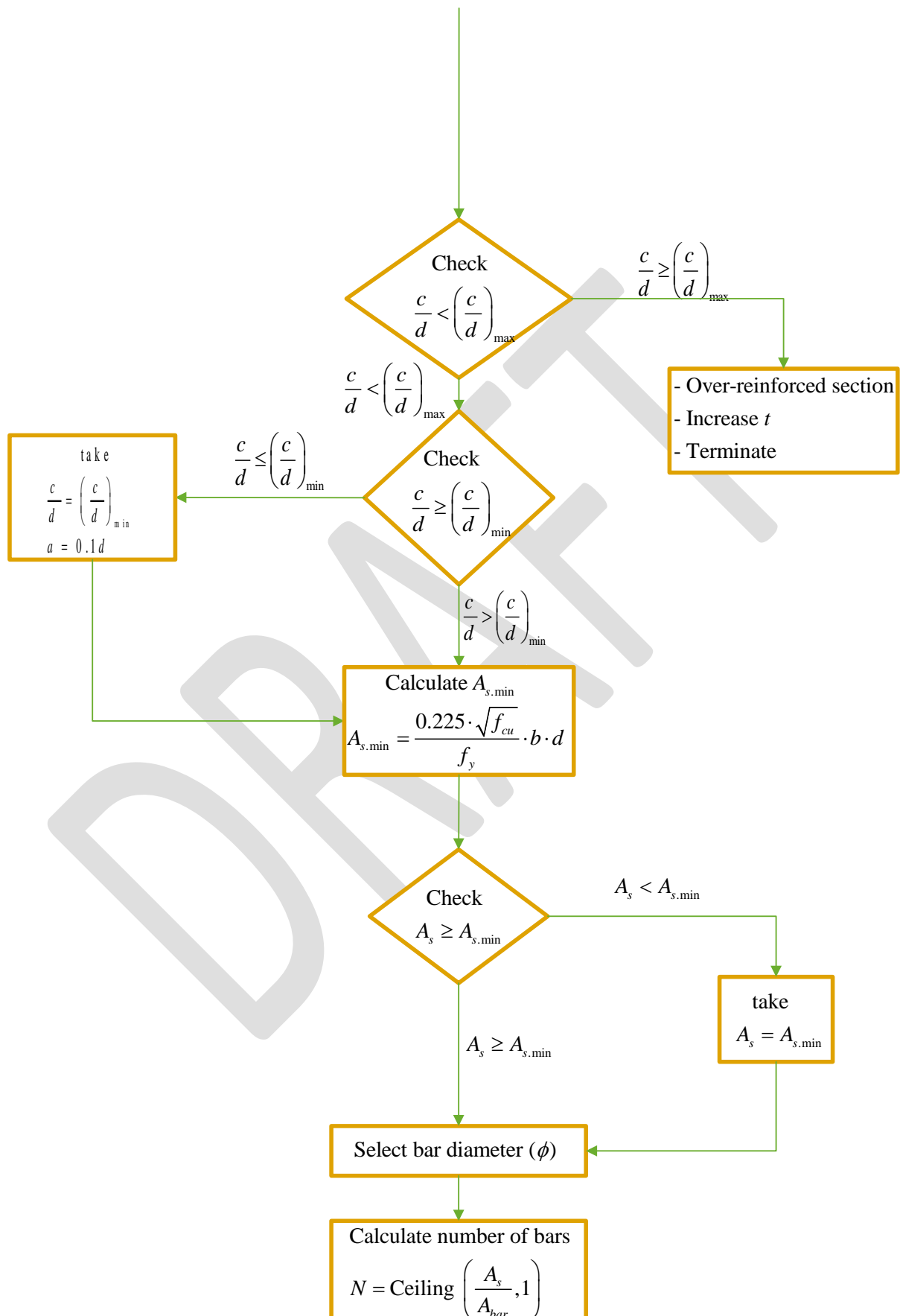
- ✓ Beam width (b)
- ✓ Total beam depth (t)
- ✓ Concrete cover
- ✓ Beam span (L)

3.1.3 Internal Forces

- ✓ Ultimate Factored Moment from analysis (M_u)

3.2 Calculation Procedure





3.3 Design Outputs

- ✓ Beam total depth (t).
- ✓ Diameter of bars.
- ✓ Number of bars.

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3.4 Example of Calculations using Mathcad

3.1 Input

3.1.1 Material Properties

$$f_{cu} := 30 \text{ MPa}$$

Concrete compressive strength

$$f_y := 360 \text{ MPa}$$

Yield strength of reinforcing steel bar

$$E_s := 200000 \text{ MPa}$$

Modulus of elasticity of steel

$$\gamma_c := 1.5$$

Concrete strength reduction factor

$$\gamma_s := 1.15$$

Steel strength reduction factor

3.1.2 Section Definition

$$b := 250 \text{ mm}$$

Beam width

$$t := 700 \text{ mm}$$

Total beam depth

$$\text{conc.cover} := 50 \text{ mm}$$

Concrete Cover

$$d' := 100 \text{ mm}$$

Cover for compression steel

$$\alpha := 0.3$$

Percentage of compression steel
to tension steel

3.1.3 Internal Forces

$$M_u := 400 \text{ kN}\cdot\text{m}$$

Ultimate Factored Moment from analysis



3.2 Calculation Procedure

$$d := t - \text{conc.cover} = 650 \text{ mm}$$

$$a := 1 \text{ mm}$$

$$A_s := 1 \text{ mm}^2$$

$$\frac{A_s \cdot f_y}{\gamma_s} = \frac{\alpha \cdot A_s \cdot f_y}{\gamma_s} + \frac{0.67 \cdot a \cdot b \cdot f_{cu}}{\gamma_c}$$

$$\frac{A_s \cdot f_y}{\gamma_s} \cdot \left(d - \frac{a}{2}\right) + \frac{\alpha \cdot A_s \cdot f_y}{\gamma_s} \cdot \left(\frac{a}{2} - d'\right) = M_u$$

$$\begin{bmatrix} a_1 \\ A_{s1} \end{bmatrix} := \text{find}(a, A_s) = \begin{bmatrix} 0.147 \text{ m} \\ 0.002 \text{ m}^2 \end{bmatrix}$$

$$a_1 = 147.01 \text{ mm}$$

$$A_{s1} = 2247.447 \text{ mm}^2$$

$$a := 1 \text{ mm}$$

$$A_s := 1 \text{ mm}^2$$

$$\frac{A_s \cdot f_y}{\gamma_s} = \frac{\alpha \cdot A_s \cdot 0.003 \cdot E_s \cdot \frac{a - 0.8 \cdot d'}{a}}{\gamma_s} + \frac{0.67 \cdot a \cdot b \cdot f_{cu}}{\gamma_c}$$

$$\frac{A_s \cdot f_y}{\gamma_s} \cdot \left(d - \frac{a}{2}\right) + \frac{\alpha \cdot A_s \cdot 0.003 \cdot E_s \cdot \frac{a - 0.8 \cdot d'}{a}}{\gamma_s} \cdot \left(\frac{a}{2} - d'\right) = M_u$$

$$\begin{bmatrix} a_2 \\ A_{s2} \end{bmatrix} := \text{find}(a, A_s) = \begin{bmatrix} 0.159 \text{ m} \\ 0.002 \text{ m}^2 \end{bmatrix}$$

$$a_2 = 158.768 \text{ mm}$$

$$A_{s2} = 2259.547 \text{ mm}^2$$

$$\begin{bmatrix} a \\ A_s \end{bmatrix} := \text{if } 0.003 \cdot E_s \cdot \frac{a_1 - 0.8 \cdot d'}{a_1} \geq f_y \left| \begin{bmatrix} a_1 \\ A_{s1} \end{bmatrix} \right. \\ \text{else} \\ \left| \begin{bmatrix} a_2 \\ A_{s2} \end{bmatrix} \right|$$

$$A_s = 2259.547 \text{ mm}^2$$

$$A'_s := \alpha \cdot A_s = 677.864 \text{ mm}^2$$

$$a = 158.768 \text{ mm}$$

$$c := \frac{a}{0.8} = 198.46 \text{ mm}$$

$$Check_1 := \text{if } \frac{c}{d} \geq \frac{2}{3} \cdot \frac{0.003}{0.003 + \frac{f_y}{\gamma_s \cdot E_s}} \left\{ \begin{array}{l} \text{"Over reinforced section, increase t"} \\ \text{else} \\ \text{"No need to increase t"} \end{array} \right.$$

$$Check_1 = \text{"No need to increase t"}$$

$$c := \text{if}(c < 0.125 \cdot d, 0.125 \cdot d, c)$$

$$c = 198.46 \text{ mm}$$

$$a := 0.8 \cdot c = 158.768 \text{ mm}$$

$$A_{smin} := \frac{0.225 \sqrt{MPa} \cdot \sqrt{f_{cu}} \cdot b \cdot d}{f_y} = 556.281 \text{ mm}^2$$

$$A_s := \text{if}(A_s < A_{smin}, A_{smin}, A_s)$$

$$A_s = 2260 \text{ mm}^2$$

$$\phi 6 := 28.3 \text{ mm}^2$$

$$\phi 8 := 50.3 \text{ mm}^2$$

$$\phi 10 := 78.5 \text{ mm}^2$$

$$\phi 12 := 113 \text{ mm}^2$$

$$\phi 14 := 154 \text{ mm}^2$$

$$\phi 16 := 201 \text{ mm}^2$$

$$\phi 18 := 254 \text{ mm}^2$$

$$\phi 20 := 314 \text{ mm}^2$$

$$\phi 22 := 380 \text{ mm}^2$$

$$\phi 25 := 491 \text{ mm}^2$$

$$\phi 28 := 616 \text{ mm}^2$$

$$\phi 32 := 804 \text{ mm}^2$$

$$\phi 38 := 1134 \text{ mm}^2$$

$$N_1 := \text{Ceil}\left(\frac{A_s}{\phi 16}, 1\right) = 12$$

$$N_2 := \text{Ceil}\left(\frac{A'_s}{\phi 16}, 1\right) = 4$$

3.3 Output

$t = 700$ mm

$b = 250$ mm

Use $N_1 = 12$ $\phi 16$

Bottom Reinforcement

Use $N_2 = 4$ $\phi 16$

Top Reinforcement

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