

4.

```
using LinearAlgebra
using SparseArrays
using Printf
using DelimitedFiles

W = 1000.0
H = 400.0
r = 75.0
cx, cy = W/2, H/2

mesh_size = 8.0 # change to 4.0 or 2.0 for refinement
E = 210000.0
v = 0.3
thickness = 2.0
traction = 100.0 # N/mm on right edge
nx = Int(round(W/mesh_size)); ny = Int(round(H/mesh_size))
nx = max(nx,4); ny = max(ny,4)
dx = W/nx; dy = H/ny

nodeId(i,j) = 1 + i + (nx+1)*j
num_nodes = (nx+1)*(ny+1)
num_elems = nx*ny

coords = Array{Float64,2}(undef, num_nodes, 2)
for j in 0:ny
    for i in 0:nx
        nid = nodeId(i,j)
        coords[nid,1] = i * dx
        coords[nid,2] = j * dy
    end
end

elems = Array{Int,2}(undef, num_elems, 4)
e = 1
for j in 0:ny-1
    for i in 0:nx-1
        n1 = nodeId(i,j); n2 = nodeId(i+1,j)
        n3 = nodeId(i+1,j+1); n4 = nodeId(i,j+1)
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        elems[e,1] = n1

        elems[e,2] = n2

        elems[e,3] = n3

        elems[e,4] = n4

        e += 1

    end

end

cell_centers = Array{Float64,2}(undef, num_elems, 2)

mask = trues(num_elems)

for k in 1:num_elems

    n = elems[k,:]

    xc = (coords[n[1],1] + coords[n[2],1] + coords[n[3],1] + coords[n[4],1]) / 4.0

    yc = (coords[n[1],2] + coords[n[2],2] + coords[n[3],2] + coords[n[4],2]) / 4.0

    cell_centers[k,1] = xc; cell_centers[k,2] = yc

    if (xc - cx)^2 + (yc - cy)^2 < r^2

        mask[k] = false

    end

end

end

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@printf("Mesh: nx=%d ny=%d nodes=%d elems=%d masked=%d\n",

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        nx, ny, num_nodes, num_elems, count(x->!x, mask))

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C = E / (1 - v^2)

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D = C * [1.0 v 0.0;

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        v 1.0 0.0;

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        0.0 0.0 (1-v)/2.0]

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g = (-1/sqrt(3), 1/sqrt(3))

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gw = (1.0, 1.0)

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dN_dxi(ξ,η) = [-(1-η)/4, (1-η)/4, (1+η)/4, -(1+η)/4]

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dN_deta(ξ,η) = [-(1-ξ)/4, -(1+ξ)/4, (1+ξ)/4, (1-ξ)/4]

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function elem_stiffness(coords4)

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    Ke = zeros(8,8)

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    for iξ in 1:2, iη in 1:2

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        ξ = g[iξ]; η = g[iη]

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        dNξ = dN_dxi(ξ,η)

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        dNη = dN_deta(ξ,η)

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        J = zeros(2,2)

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        for a in 1:4

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J[1,1] += dNξ[a] * coords4[a,1]
J[1,2] += dNη[a] * coords4[a,1]
J[2,1] += dNξ[a] * coords4[a,2]
J[2,2] += dNη[a] * coords4[a,2]
end

detJ = det(J)
invJ = inv(J)
B = zeros(3,8)
for a in 1:4
    d = invJ * [dNξ[a], dNη[a]]
    dNx = d[1]; dNy = d[2]
    B[1,2*a-1] = dNx
    B[2,2*a] = dNy
    B[3,2*a-1] = dNy
    B[3,2*a] = dNx
end
weight = gw[iξ] * gw[iη]
Ke .+= thickness * (B' * D * B) * detJ * weight
end
return Ke
end

ndof = 2 * num_nodes
K = spzeros(ndof, ndof)
f = zeros(ndof)

for elem in 1:num_elems
    if !mask[elem]; continue; end
    n = elems[elem, :]
    coords4 = zeros(4,2)
    for a in 1:4
        coords4[a,1] = coords[n[a],1]; coords4[a,2] = coords[n[a],2]
    end
    Ke = elem_stiffness(coords4)
    dofs = Vector{Int}{undef,8}
    for a in 1:4
        dofs[2*a-1] = 2*(n[a]-1) + 1
        dofs[2*a] = 2*(n[a]-1) + 2
    end
end

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    for i in 1:8, j in 1:8

        K[dofs[i], dofs[j]] += Ke[i,j]

    end

end

for j in 0:ny-1

    n_bottom = nodeId(nx, j)

    n_top = nodeId(nx, j+1)

    Lseg = abs(coords[n_top,2] - coords[n_bottom,2])

    fx = traction * Lseg * thickness / 2.0

    f[2*(n_bottom-1)+1] += fx

    f[2*(n_top-1)+1] += fx

end

active = falses(num_nodes)

for elem in 1:num_elems

    if mask[elem]

        for a in 1:4

            active[elems[elem,a]] = true

        end

    end

end

end

fixed = Dict{Int,Float64}{}

for nid in 1:num_nodes

    if !active[nid]

        fixed[2*(nid-1)+1] = 0.0

        fixed[2*(nid-1)+2] = 0.0

    end

end

for j in 0:ny

    nid = nodeId(0,j)

    fixed[2*(nid-1)+1] = 0.0

end

fixed[2*(nodeId(0,0)-1)+2] = 0.0

for (d,val) in fixed

    K[d, :] .= 0.0

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K[:, d] = 0.0

K[d, d] = 1.0

f[d] = val

end

for i in 1:ndof
    if abs(K[i,i]) < 1e-16
        K[i,i] = K[i,i] + 1e-12
    end
end

println("Solving linear system: ndof = $ndof nnz(K) = $(nnz(K))")

u = K \ f

vm_elem = fill{NaN, num_elems}

for elem in 1:num_elems
    if !mask[elem]; continue; end
    n = elems[elem,:]

    coords4 = zeros(4,2)

    ue = zeros(8)

    for a in 1:4
        coords4[a,1] = coords[n[a],1]; coords4[a,2] = coords[n[a],2]

        ue[2*a-1] = u[2*(n[a]-1)+1]; ue[2*a] = u[2*(n[a]-1)+2]
    end

    ξ=0.0; η=0.0

    dNξ = dN_dxi(ξ,η); dNη = dN_deta(ξ,η)

    J = zeros(2,2)

    for a in 1:4
        J[1,1] += dNξ[a]*coords4[a,1]; J[1,2] += dNη[a]*coords4[a,1]

        J[2,1] += dNξ[a]*coords4[a,2]; J[2,2] += dNη[a]*coords4[a,2]
    end

    invJ = inv(J)

    B = zeros(3,8)

    for a in 1:4
        dxy = invJ * [dNξ[a], dNη[a]]

        dNx, dNy = dxy

        B[1,2*a-1] = dNx; B[2,2*a] = dNy; B[3,2*a-1] = dNy; B[3,2*a] = dNx
    end

    ve = B * ue

    σv = D * ve

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sxx, syy, sxy = ov[1], ov[2], ov[3]

vm_elem[elem] = sqrt(sxx^2 + syy^2 - sxx*syy + 3*sxy^2)

end

valid_idx = [i for i in 1:num_elems if mask[i]]

valid_vm = vm_elem[valid_idx]

vm_max = maximum(valid_vm)

locs = [ valid_idx[i] for i in findall(x->isapprox(x, vm_max; atol=1e-8), valid_vm) ]

println("\n=== RESULTS ===")

println("Max von Mises (cell-centered) = ", vm_max, " N/mm^2")

println("Element index(es): ", locs)

for ei in locs

    println(" center = (", cell_centers[ei,1], ", ", cell_centers[ei,2], ")")

end

println("Estimated Kt ( $\sigma_{\max}$  /  $\sigma_{\text{nom}}$  where  $\sigma_{\text{nom}}=50$ ) = ", vm_max / 50.0)

center_y = H/2

tol = dy/2 + 1e-9

xs = Float64[]; sigx = Float64[]

for elem in 1:num_elems

    if !mask[elem]; continue; end

    xc = cell_centers[elem,1]; yc = cell_centers[elem,2]

    if abs(yc - center_y) <= tol

        n = elems[elem,:]

        coords4 = zeros(4,2); ue = zeros(8)

        for a in 1:4

            coords4[a,1] = coords[n[a],1]; coords4[a,2] = coords[n[a],2]

            ue[2*a-1] = u[2*(n[a]-1)+1]; ue[2*a] = u[2*(n[a]-1)+2]

        end

         $\xi=0.0$ ;  $\eta=0.0$ 

        dN $\xi$  = dN_dxi( $\xi,\eta$ ); dN $\eta$  = dN_deta( $\xi,\eta$ )

        J=zeros(2,2)

        for a in 1:4

            J[1,1]+=dN $\xi$ [a]*coords4[a,1]; J[1,2]+=dN $\eta$ [a]*coords4[a,1]

            J[2,1]+=dN $\xi$ [a]*coords4[a,2]; J[2,2]+=dN $\eta$ [a]*coords4[a,2]

        end

        invJ = inv(J)

        B=zeros(3,8)

        for a in 1:4

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    dxy = invJ * [dNξ[a], dNη[a]]

    B[1,2*a-1] = dxy[1]; B[2,2*a] = dxy[2]; B[3,2*a-1] = dxy[2]; B[3,2*a] = dxy[1]

end

ve = B * ue; σv = D * ve

push!(xs, xc); push!(sigx, σv[1])

end

end

if isempty(xs)

    idx = sortperm(xs)

    data = hcat([xs[i] for i in idx], [sigx[i] for i in idx])

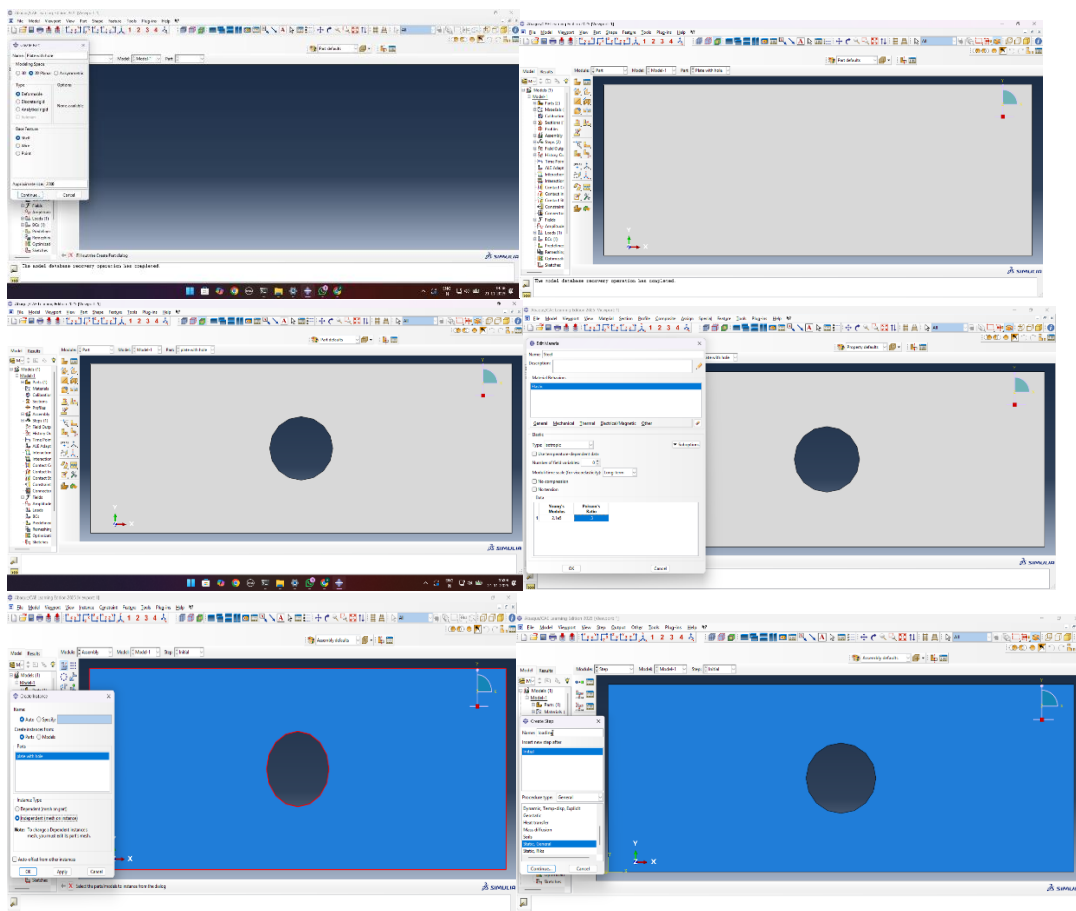
    writedlm("centerline_sigmax.csv", data, ',')

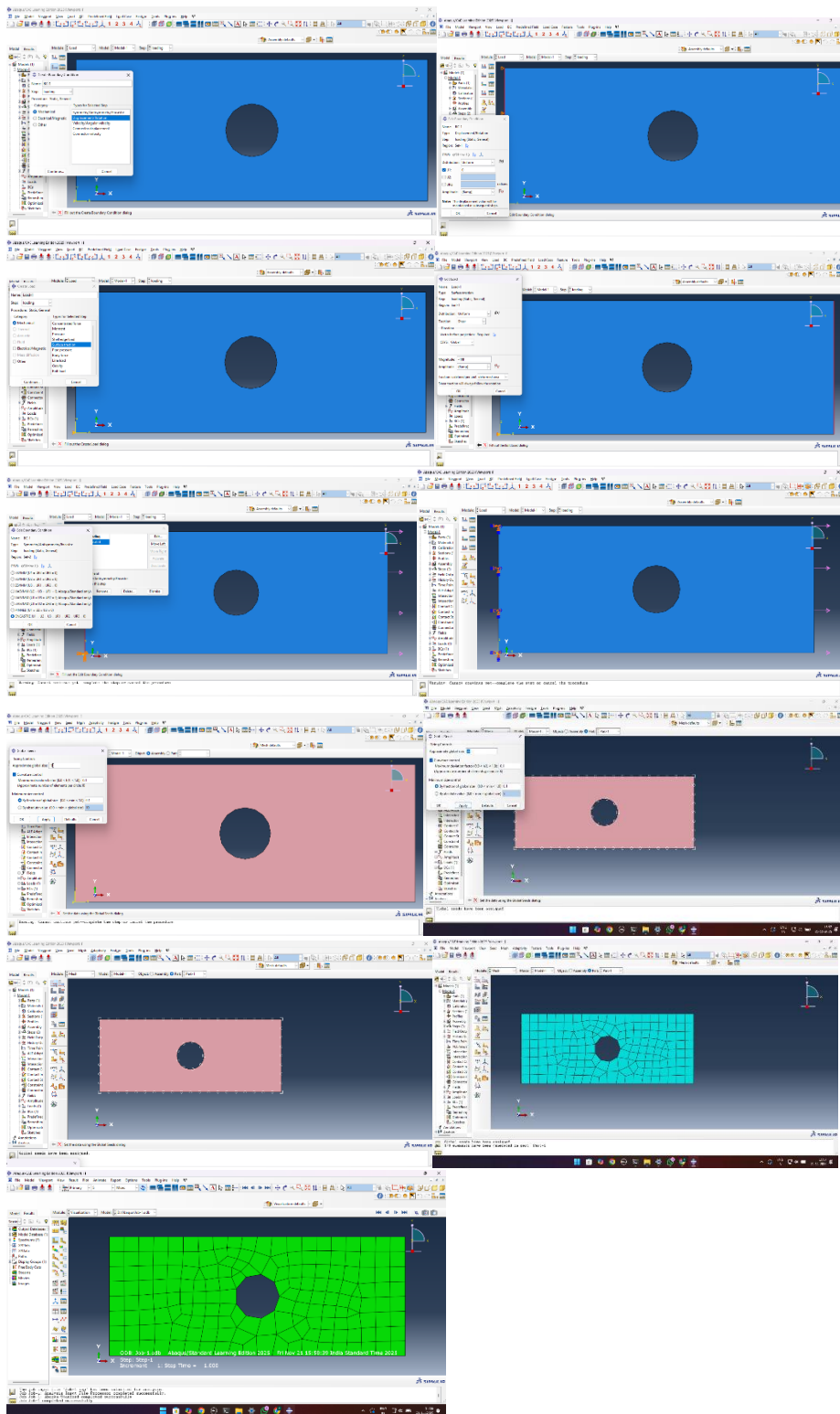
    println("Wrote centerline_sigmax.csv with ", size(data,1), " samples")

end

println("Done.")

```





5.

L = 1000.0

H = 200.0

thickness = 250.0

E = 25000.0

$\nu = 0.2$

P = 1000.0

$q = -P / H$

$n_x = 120$

$n_y = 24$

$n_{node_x} = n_x + 1$

$n_{node_y} = n_y + 1$

$n_{nodes} = n_{node_x} * n_{node_y}$

$n_{elem} = n_x * n_y$

$ndof = 2 * n_{nodes}$

println("Mesh: \$n_{nodes} nodes, \$n_{elem} elements")

$node_coords = zeros(n_{nodes}, 2)$

$xs = range(0.0, L, length=n_{node_x})$

$ys = range(0.0, H, length=n_{node_y})$

for j in 1:n_{node_y}

 for i in 1:n_{node_x}

$nid = (j-1)*n_{node_x} + i$

$node_coords[nid,1] = xs[i]$

$node_coords[nid,2] = ys[j]$

 end

end

$elems = Array{Int}\{undef, n_{elem}, 4\}$

e = 1

for j in 1:n_y

 for i in 1:n_x

$n1 = (j-1)*n_{node_x} + i$

$n2 = n1 + 1$

$n3 = n2 + n_{node_x}$

$n4 = n1 + n_{node_x}$

$elems[e,:] = [n1,n2,n3,n4]$

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        e += 1
    end
end

D = (E/(1-v^2))*[
    1.0  v   0.0
    v   1.0  0.0
    0.0  0.0 (1.0-v)/2
]

g = 1/sqrt(3)
gps = [(-g,-g),(g,-g),(g,g),(-g,g)]

function dshape_Q4(xi,eta)
    dN = zeros(4,2)
    dN[1,:] = [ -0.25*(1-eta), -0.25*(1-xi) ]
    dN[2,:] = [  0.25*(1-eta), -0.25*(1+xi) ]
    dN[3,:] = [  0.25*(1+eta),  0.25*(1+xi) ]
    dN[4,:] = [ -0.25*(1+eta),  0.25*(1-xi) ]
    return dN
end

K = zeros(ndof, ndof)
F = zeros(ndof)

println("Assembling stiffness matrix...")
for el in 1:n_elem
    conn = elems[el,:]
    xe = node_coords[conn,1]
    ye = node_coords[conn,2]

    ke = zeros(8,8)

    for (xi,eta) in gps
        dN = dshape_Q4(xi,eta)

        # Jacobian
        J = zeros(2,2)
        for a in 1:4
            J[1,1] += dN[a,1]*xe[a]
            J[1,2] += dN[a,2]*xe[a]

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J[2,1] += dN[a,1]*ye[a]
J[2,2] += dN[a,2]*ye[a]
end

detJ = det(J)
invJ = inv(J)

B = zeros(3,8)
for a in 1:4
    dNd = invJ * dN[a,:]
    B[1,2a-1] = dNd[1]
    B[2,2a] = dNd[2]
    B[3,2a-1] = dNd[2]
    B[3,2a] = dNd[1]
end

ke += B' * D * B * detJ * thickness
end

dofmap = [
    2*(conn[1]-1)+1, 2*(conn[1]-1)+2,
    2*(conn[2]-1)+1, 2*(conn[2]-1)+2,
    2*(conn[3]-1)+1, 2*(conn[3]-1)+2,
    2*(conn[4]-1)+1, 2*(conn[4]-1)+2
]

for i in 1:8, j in 1:8
    K[dofmap[i], dofmap[j]] += ke[i,j]
end
end

println("Stiffness assembly complete.")
println("Applying distributed traction...")

for j in 1:ny
    n_bottom = (j-1)*nnode_x + nnode_x
    n_top = j*nnode_x

    y1 = node_coords[n_bottom,2]

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y2 = node_coords[n_top,2]

Ledge = abs(y2 - y1)

f1 = q * Ledge * thickness / 2
f2 = q * Ledge * thickness / 2

F[2*(n_bottom-1)+2] += f1
F[2*(n_top-1)+2] += f2
end

top_right = (nnode_y-1)*nnode_x + nnode_x
F[2*(top_right-1)+2] += q * thickness * (H/ny) / 2

println("Traction applied.")
fixed = falses(ndof)

for nid in 1:n_nodes
    if abs(node_coords[nid,1]) < 1e-8
        fixed[2*(nid-1)+1] = true
        fixed[2*(nid-1)+2] = true
    end
end

free = findall(!, fixed)
println("Free DOFs: ", length(free))
println("Solving system...")
u_free = K[free,free] \ F[free]
U = zeros(ndof)
U[free] = u_free

println("Solve complete.")
uy_tip = U[2*(top_right-1)+2]
println("Vertical displacement at free top corner = $uy_tip mm")
println("Writing VTK file...")

function writeVTK(filename)
    open(filename,"w") do io
        println(io,"# vtk DataFile Version 2.0")
        println(io,"Cantilever Q4 FEM result")
    end
end

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```

println(io,"ASCII")

println(io,"DATASET UNSTRUCTURED_GRID")

println(io,"POINTS $n_nodes float")

for i in 1:n_nodes
    x = node_coords[i,1]
    y = node_coords[i,2]
    println(io,"$x $y 0.0")
end

total = n_elem * 5

println(io,"CELLS $n_elem $total")

for el in 1:n_elem
    conn = elems[el,:].- 1
    println(io,"4 $(conn[1]) $(conn[2]) $(conn[3]) $(conn[4])")
end

println(io,"CELL_TYPES $n_elem")

for i in 1:n_elem
    println(io,"9") # quad
end

println(io,"POINT_DATA $n_nodes")

println(io,"VECTORS displacement float")

for i in 1:n_nodes
    ux = U[2*(i-1)+1]
    uy = U[2*(i-1)+2]
    println(io,"$ux $uy 0.0")
end

end

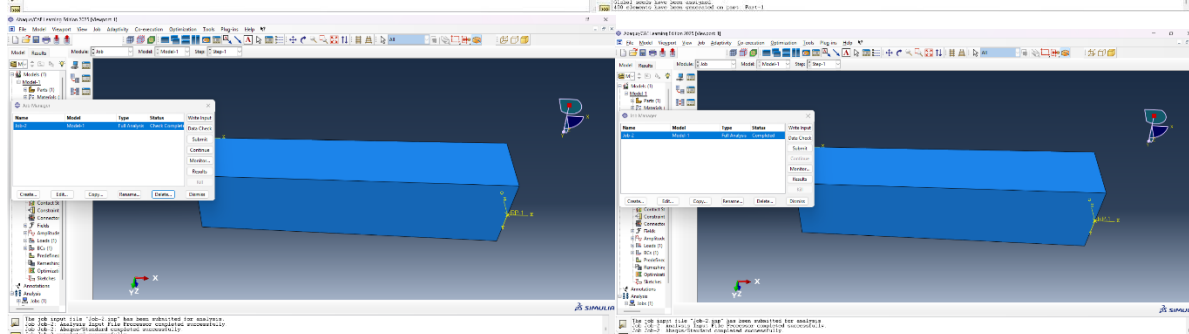
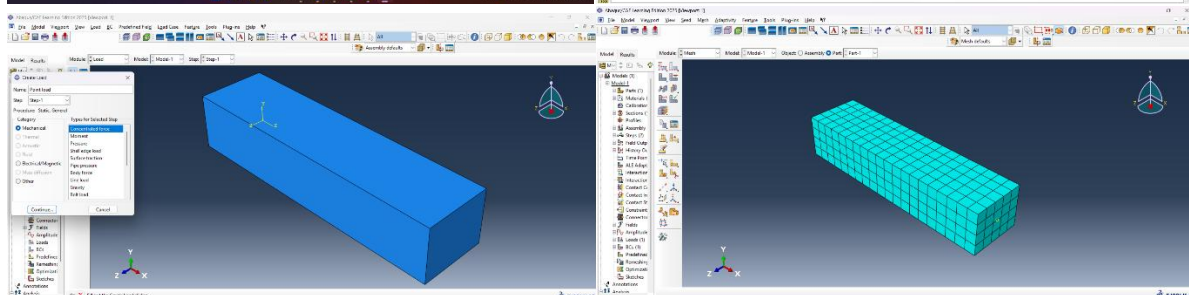
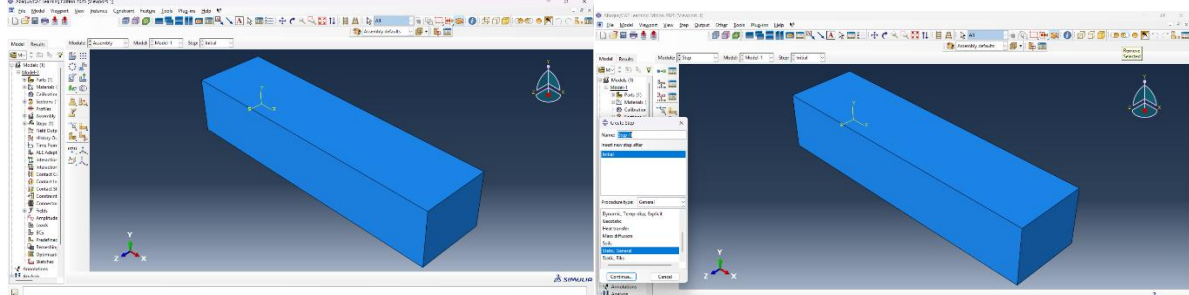
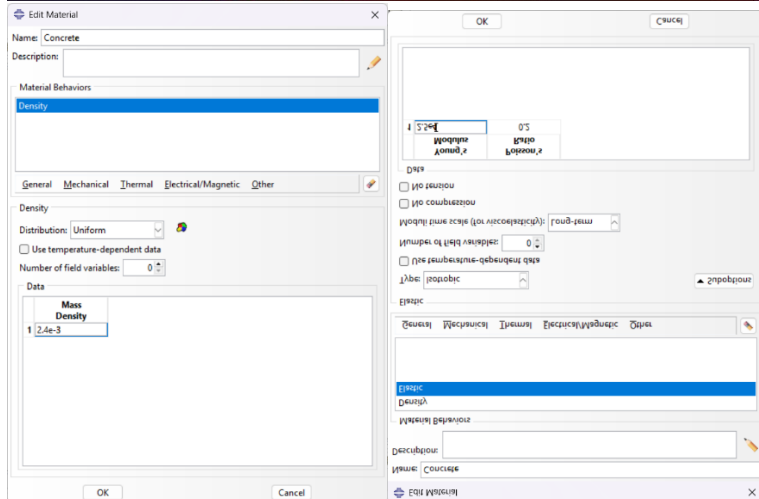
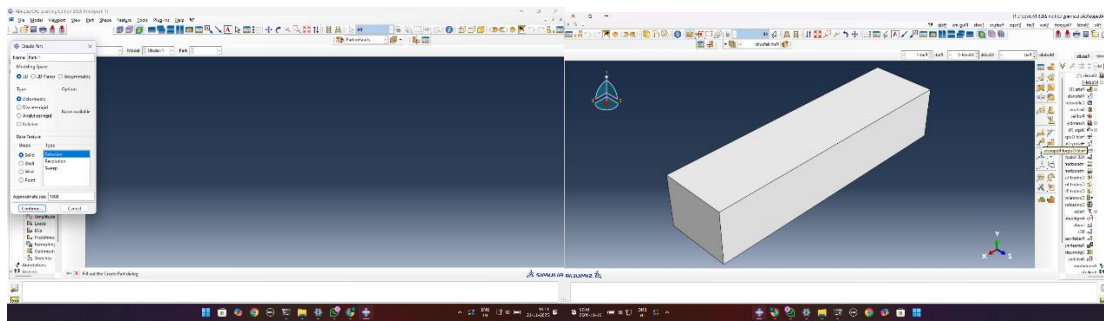
end

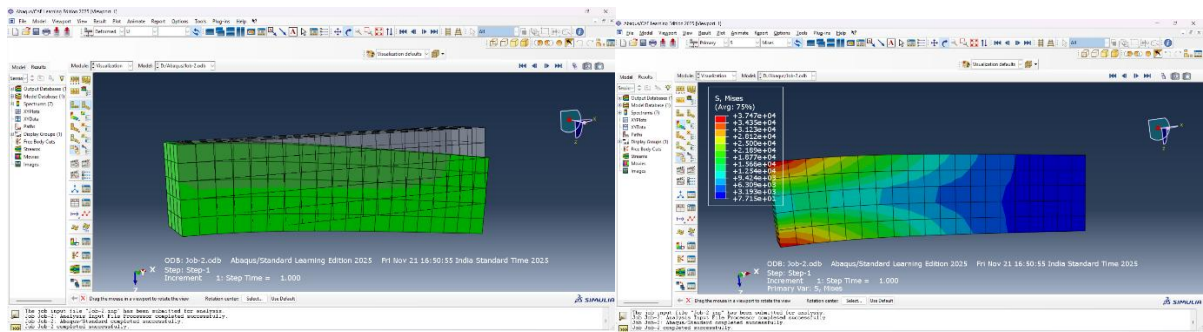
writeVTK("cantilever.vtk")

println("VTK written to cantilever.vtk")

println("DONE.")

```





6.

using Plots

L = 1000.0

b = 250.0

h = 200.0

p = 1000.0

w = p * b

I = b * h^3 / 12.0

c = h/2.0

nx = 201

ny = 81

x = range(0.0, stop=L, length=nx)

y = range(-c, stop=c, length=ny)

M = @. -w * (L - x)^2 / 2.0

σ = [M[j] * y_i / I for y_i in y, j in 1:length(x)] # N/mm²

M_fixed = M[1]

$\sigma_{\text{top_fixed}}$ = $\sigma[\text{end},1]$ # top fiber (y = +c) at fixed end

$\sigma_{\text{bottom_fixed}}$ = $\sigma[1,1]$ # bottom fiber (y = -c) at fixed end

println("w (N/mm) = ", w)

println("I (mm⁴) = ", I)

println("M at fixed end (N·mm) = ", M_fixed)

println("Max fiber stress at fixed end (top) σ = \$(round($\sigma_{\text{top_fixed}}$, sigdigits=6)) N/mm²")

println("Max fiber stress at fixed end (bottom) σ = \$(round($\sigma_{\text{bottom_fixed}}$, sigdigits=6)) N/mm²")

gr()

p1 = plot(x, M, xlabel="x (mm)", ylabel="M(x) (N·mm)",

title="Bending Moment along Cantilever", legend=false, lw=2)

p2 = contourf(x, y, σ ; xlabel="x (mm)", ylabel="y (mm)",

title="Bending stress $\sigma(x,y)$ (N/mm²)", colorbar_title=" σ ",

levels=20)

plot(p1, p2, layout = @layout([a; b]), size={800,900})

