SLOPE STABILITY ANALYSIS BY FINITE ELEMENTS

A guide to the use of Program slope1

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Downloading the slope1 program

The source code for **slope1** is written in Fortran 95, but users are provided with the executable file **slope1.exe** and the example data files **ex1.dat** thru **ex7.dat** as described in this report. The slope stability software is in folder

4th ed/executable/slope stab/slope1

The files needed to run the slope program with one of the examples are as follows:

slope1.exe An executable file of the slope1 program **ex?.dat** A typical example data file as described in this report

Running an example problem

In order to run slope1 with an example data file, e.g. ex4.dat, use Windows Explorer to navigate to the folder 4th_ed/executable/slope_stab/slope1, double click on the executable icon slope1.exe, and when prompted type the base-name of the data file, namely ex4

If the program runs properly, you should see evidence of iterations being reported back to the screen. When the job is complete you should see the following additional files in the **slope1** folder:

ex4.res Output file giving the estimated Factor of Safety.
 ex4.msh PostScript file showing the finite element mesh.
 ex4.vec PostScript file showing nodal displacement vectors at failure.
 ex4.dis PostScript file showing the deformed mesh at failure.

Running your own problem

Create your own data file. This report explains the layout of data for use with **slope1**. It may help to make a copy of one of the example data files closest to your own problem and edit that as needed. Let us assume your data file is called **fred.dat**. Note that the extension <u>must</u> be of type .dat.

In order to run your own data file **fred.dat**, once more double- click on the executable icon **slope1.exe**, and when prompted type the basename of the data file, namely **fred**

If all goes well, the following additional files will appear in your folder: fred.res, fred.msh, fred.vec and fred.dat

Note: Program slope1.f95 is based closely on p63.f95 in the textbook, "Programming the Finite Element Method" by I.M. Smith and D.V. Griffiths. 4th ed., 2004. The main difference lies in the mesh generation and the automatic search for the critical strength reduction factor. Users of slope1 are encouraged to refer to this text and the companion paper, "Slope stability analysis by finite elements", by D.V. Griffiths and P.A. Lane, *Géotechnique* 49, no.3, pp.387-403, (1999).

Explanation of data for Program slope1.exe

A typical configuration is shown in Figure 1.

Slope geometry data:

w1 = Width of top of embankment

s1 = Width of sloping portion of embankment

w2 = Distance foundation extends to right of embankment toe

h1 = Height of embankment

h2 = Thickness of foundation layer

Element discretization data:

nx1 = Number of x-elements in embankment

nx2 = Number of x-elements to right of embankment toe

ny1 = Number of y-elements in embankment

ny2 = Number of y-elements in foundation

Soil property data: 1

np_types = Number of different property groups

phi,c,psi,gamma,e,v = Material properties $\phi',c',\psi,\gamma,E,\nu$ (np_types times)

etype = Property group assigned to each element (nels times)

(data not needed if np_types=1)

Pseudo-static analysis: 2

kh = Horizontal acceleration factor

Free surface data: ³

nosurf = Number of free surface coordinates

x,y = x- and y- coordinates of free-surface (nosurf times), or the r_u value

 $gam_w = Unit weight of water, \gamma_w$

Iteration ceiling:

limit = Iteration ceiling (suggested value, 500)

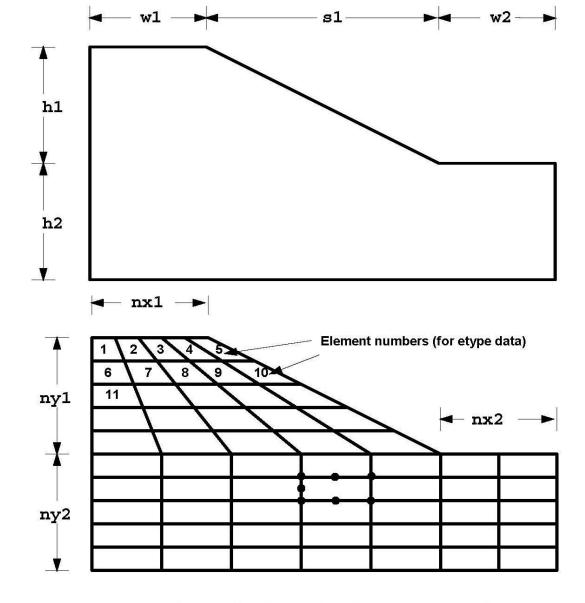
Factor of Safety Tolerance:

fos_tol = Factor of safety tolerance (suggested value, 0.05)

 $^{1}\phi^{'}$ is the effective friction angle; $c^{'}$ is the effective cohesion; ψ is the dilation angle and can usually be set to zero; γ is the total unit weight; E is Young's modulus and is often set to a nominal value (e.g. 10^{5}); ν is Poisson's ratio and is often set to a nominal value (e.g. 0.3), nels is the total number of elements in the mesh and is computed internally by the program

 $^{2}k_{h}$ is the horizontal pseudo-static acceleration factor, e.g. for a horizontal acceleration of 0.2g, set $k_{h}=0.2$

³If nosurf=1, then instead of x,y data, read a single value of r_u .



Number of elements in each section

Fig. 1 Layout and dimensions of embankment geometry

Example 1: A homogeneous slope

The stability analysis is of the homogeneous c'- ϕ' slope shown in Figure 1.1

Data for Example 1 (ex1.dat)

10

```
"Example 1: A homogeneous slope"
"Width of top of embankment (w1)"
1.2
"Width of sloping portion of embankment (s1)"
2.0
"Distance foundation extends to right of
embankment toe (w2)"
1.2
"Height of embankment (h1)"
1.0
"Thickness of foundation layer (h2)"
1.0
"Number of x-elements in embankment (nx1)"
32
"Number of x-elements to right of embankment toe (nx2)"
12
"Number of y-elements in embankment (ny1)"
10
"Number of y-elements in foundation (ny2)"
```

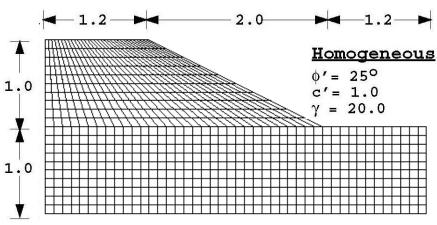


Fig 1.1 Finite element mesh for Example 1

```
"Number of different property groups (np types)"
1
"Material properties (phi,c,psi,gamma,e,v) for each group"
25.0 1.0 0.0 20.0 1.e5 0.3
"Property group assigned to each element (etype, data not needed if np types=1)"
"Pseudo-static analysis: Horizontal acceleration factor (k h)"
0.0
"Number of free surface points and their coordinates (nosurf, surf(2,nosurf))"
0
"Unit weight of water (gam w)"
0.0
"Iteration ceiling (limit)"
1000
"Factor of Safety accuracy tolerance (fos tol)"
0.05
```

Output for Example 1 (ex1.res)

trial factor	max displ	iterations
0.5000	0.3050E-03	2
1.0000	0.3074E-03	8
1.5000	0.3890E-03	42
1.5625	0.4039E-03	250
1.5781	0.4076E-03	428
1.5938	0.4511E-03	1000

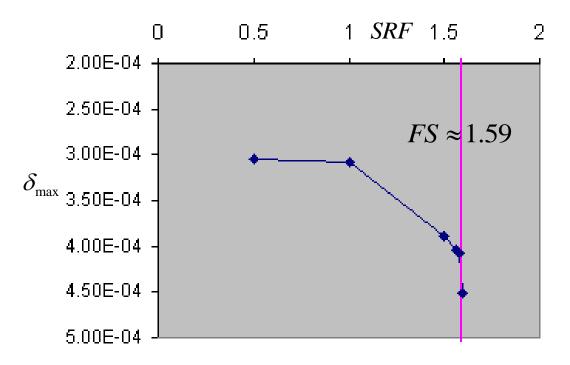
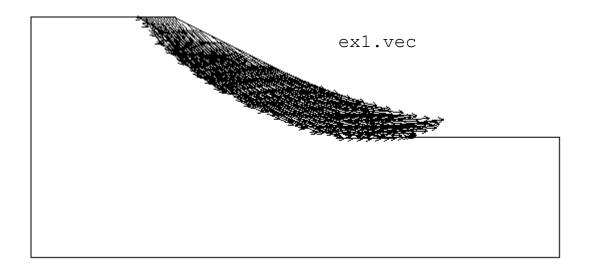


Fig 1.2 SRF vs. d_{max} for Example 1



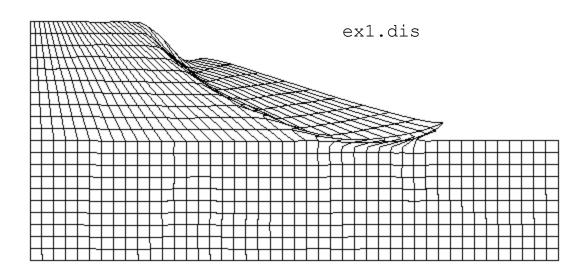


Fig 1.2 Displacement vectors and deformed mesh at failure for Example 1

Example 2: A two-layer slope

The stability analysis is of a two-layer $c'-\phi'$ slope consisting of a stronger soil in the embankment overlying a weaker soil in the foundation as shown in Figure 2.1.

Data for Example 2 (ex2.dat)

```
"Example 2: A two-layer slope"

"Width of top of embankment (w1)"

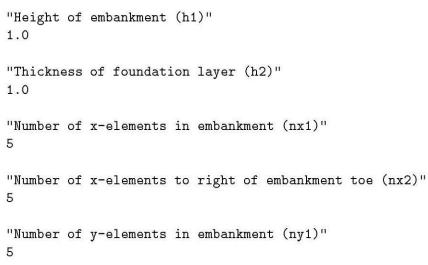
1.2

"Width of sloping portion of embankment (s1)"

2.0

"Distance foundation extends to right of embankment toe (w2)"

1.2
```



"Number of y-elements in foundation (ny2)"

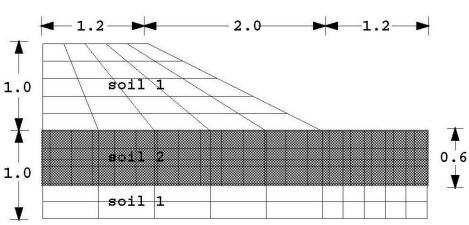


Fig 2.1 Finite element mesh for Example 2

Two-layer

Soil 1 φ'= 25° c'= 1.0 γ = 20.0 Soil 2 φ'= 15° c'= 0.5 γ = 20.0

```
"Number of different property groups (np_types)"
2
"Material properties (phi,c,psi,gamma,e,v) for each group"
25.0 1.0 0.0 20.0 1.e5 0.3
15.0 0.5 0.0 20.0 1.e5 0.3
"Property group assigned to each element (etype, data not needed if np_types=1)"
1 1 1 1 1
1 1 1 1 1
1 1 1 1 1
1 1 1 1 1
1 1 1 1 1
2 2 2 2 2 2 2 2 2 2 2
2 2 2 2 2 2 2 2 2 2 2
2 2 2 2 2 2 2 2 2 2 2
1111111111
1 1 1 1 1 1 1 1 1 1
"Pseudo-static analysis: Horizontal acceleration factor (k_h)"
0.0
"Number of free surface points and their coordinates (nosurf, surf(2,nosurf))"
0
"Unit weight of water (gam_w)"
9.81
"Iteration ceiling (limit)"
500
"Factor of safety tolerance (fos_tol)"
0.05
```

Output for Example 2 (ex2.res)

ations
2
3
1
9
3
)
1

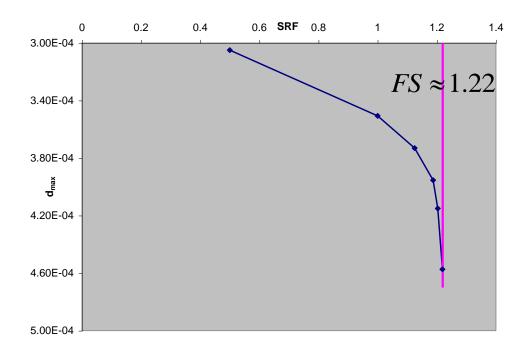
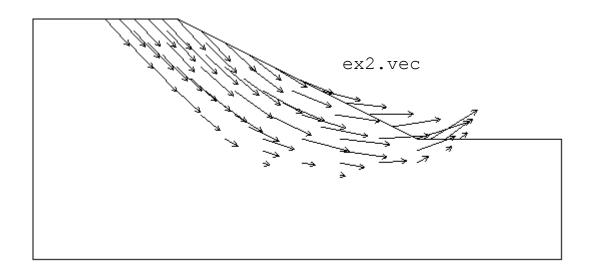


Fig 2.2 SRF vs. d_{max} for Example 2



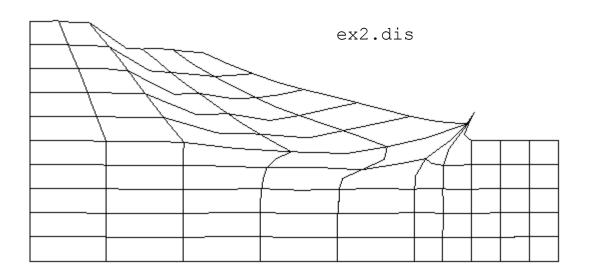


Fig 2.3 Displacement vectors and deformed mesh at failure for Example 2

Example 3: A two-layer undrained clay slope

The stability analysis is of a two-layer undrained clay slope consisting of embankment overlying a stronger soil in the foundation as shown in Figur

Data for Example 3 (ex3.dat)

```
"Example 3: A two-layer undrained clay slope"
"Width of top of embankment (w1)"
20.0
"Width of sloping portion of embankment (s1)"
20.0
"Distance foundation extends to right of embankment toe (w2)"
20.0
"Height of embankment (h1)"
10.0
"Thickness of foundation layer (h2)"
10.0
"Number of x-elements in embankment (nx1)"
20
"Number of x-elements to right of embankment toe (nx2)"
10
"Number of y-elements in embankment (ny1)"
5
"Number of y-elements in foundation (ny2)"
5
```

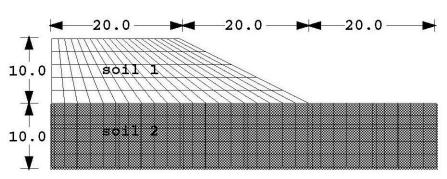


Fig 3.1 Finite element mesh for Example 3

Two-layer Undrained Clay

Soil 1 $\phi_{u} = 0$ $c_{u} = 50.0$ $\gamma = 20.0$ Soil 2 $\phi_{u} = 0$ $c_{u} = 73.1$ $\gamma = 20.0$

```
"Number of different property groups (np_types)"
"Material properties (phi,c,psi,gamma,e,v) for each group"
0.0 50.0 0.0 20.0 1.e5 0.3
0.0 73.1 0.0 20.0 1.e5 0.3
"Property group assigned to each element (etype, data not needed if np_types=1)"
111111111111111111111111
1111111111111111111111
11111111111111111111111
111111111111111111111111
"Pseudo-static analysis: Horizontal acceleration factor (k_h)"
0.0
"Number of free surface points and their coordinates (nosurf, surf(2,nosurf))"
0
"Unit weight of water (gam_w)"
9.81
"Iteration ceiling (limit)"
500
"Factor of safety tolerance (fos_tol)"
0.05
```

Output for Example 3 (ex3.res)

trial factor	max displ	iterations
0.5000	0.3044E-01	2
1.0000	0.3535E-01	6
1.5000	0.4629E-01	35
1.7500	0.5365E-01	67
1.8750	0.5784E-01	82
1.9375	0.6063E-01	107
1.9688	0.6244E-01	143
1.9844	0.6389E-01	195
2.0000	0.7257E-01	500

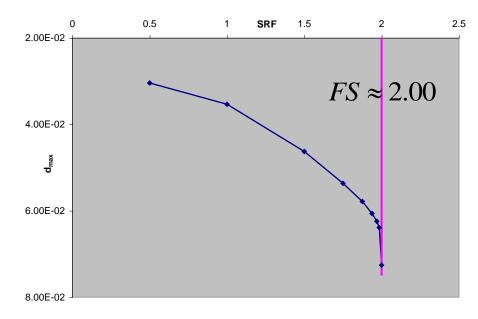
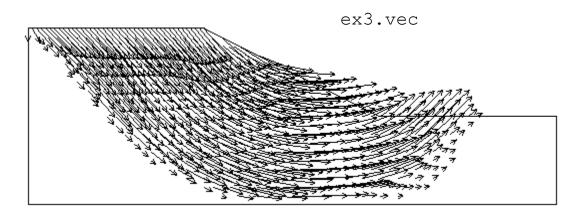


Fig 3.2 SRF vs. d_{max} for Example 3



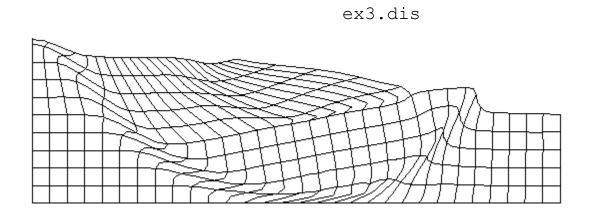


Fig 3.3 Displacement vectors and deformed mesh at failure for Example 3

Example 4: A homogeneous slope including a free-surface

The stability analysis is of the homogeneous $c'-\phi'$ slope including a free surface as shown in Figure 4.1. It is assumed in this case that the saturated unit weight is applicable both below and above the free-surface, so only one property type is needed.

Data for Example 4 (ex4.dat)

```
"Example 4: A homogeneous slope including a
free-surface"
"Width of top of embankment (w1)"
25.0
"Width of sloping portion of embankment (s1)"
17.0
"Distance foundation extends to right of
embankment toe (w2)"
24.0
"Height of embankment (h1)"
10.0
"Thickness of foundation layer (h2)"
10.0
"Number of x-elements in embankment (nx1)"
21
"Number of x-elements to right of embankment
toe (nx2)"
12
"Number of y-elements in embankment (ny1)"
```

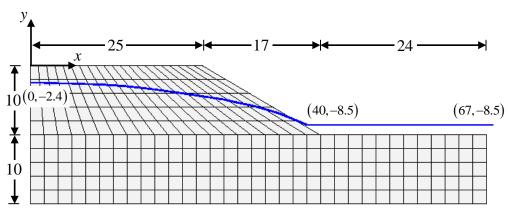


Fig 4.1 Finite element mesh for Example 4

Homogeneous with free surface φ'= 5° c'= 200 γ = 120

```
"Number of y-elements in foundation (ny2)"
5
"Number of different property groups (np types)"
1
"Material properties (phi,c,psi,gamma,e,v) for each group"
5.0 200.0 0.0 120.0 1.e5 0.3
"Property group assigned to each element (etype, data not needed if np types=1)"
"Pseudo-static analysis: Horizontal acceleration factor (k h)"
0.0
"Number of free surface points and their coordinates (nosurf, surf(2, nosurf))"
22
0.0 - 2.40 \quad 2.0 - 2.45 \quad 4.0 \quad -2.50 \quad 6.0 \quad -2.55 \quad 8.0 \quad -2.60
10.0 -2.67 12.0 -2.80 14.0 -2.95 16.0 -3.15 18.0 -3.40
20.0 -3.65 22.0 -3.95 24.0 -4.25 26.0 -4.55 28.0 -4.85
30.0 -5.30 \ 32.0 -5.70 \ 34.0 -6.25 \ 36.0 -6.85 \ 38.0 -7.65
40.0 -8.50 67.0 -8.50
"Unit weight of water (gam w)"
62.4
"Iteration ceiling (limit)"
1000
"Factor of Safety accuracy tolerance (fos tol)"
0.05
```

Output for Example 4 (ex4.res)

0.5000	0.1950E+00	5
1.0000	0.2803E+00	33
1.2500	0.3630E+00	184
1.2656	0.4893E+00	1000

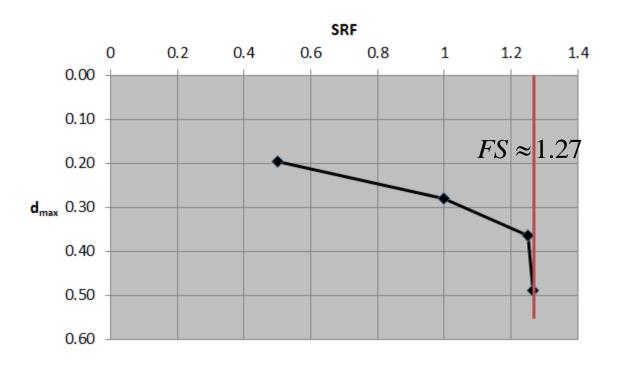
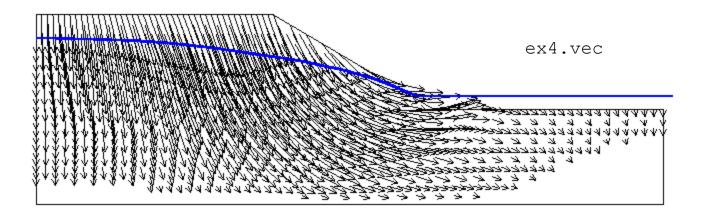


Fig 4.2 SRF vs. d_{max} for Example 4



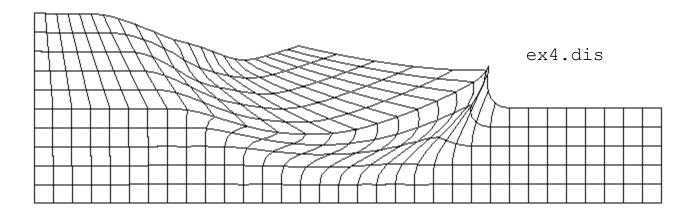


Fig 4.3 Displacement vectors and deformed mesh at failure for Example 4

Example 5: A completely submerged homogeneous slope

The stability analysis is of a homogeneous $c'-\phi'$ slope completely submerged beneath 2m of water as shown in Figure 5.1.

Data for Example 5 (ex5.dat)

```
"Example 5: A completely submerged homogeneous slope"

"Width of top of embankment (w1)"

30.0

"Width of sloping portion of embankment (s1)"

20.0
```

```
"Distance foundation extends to right of embankment toe (w2)" \ensuremath{\text{0.0}}
```

```
"Height of embankment (h1)" 10.0
```

```
"Thickness of foundation layer (h2)" 0.0
```

```
"Number of x-elements in embankment (nx1)" 20
```

```
"Number of x-elements to right of embankment toe (nx2)" 0
```

```
"Number of y-elements in embankment (ny1)" 10
```

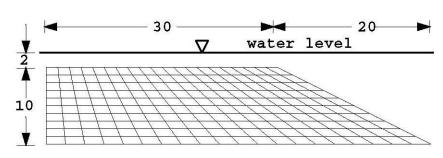


Fig 5.1 Finite element mesh for Example 5

Fully submerged and homogeneous

$$\phi' = 20^{\circ}$$
 $c' = 10$
 $\gamma = 20$

```
"Number of different property groups (np_types)"
1
"Material properties (phi,c,psi,gamma,e,v) for each group"
20.0 10.0 0.0 20.0 1.e5 0.3
"Property group assigned to each element (etype, data not needed if np_types=1)"
"Pseudo-static analysis: Horizontal acceleration factor (k_h)"
0.0
"Number of free surface points and their coordinates (nosurf, surf(2,nosurf))"
0.0 2.0
50.0 2.0
"Unit weight of water (gam_w)"
9.81
"Iteration ceiling (limit)"
500
"Factor of safety tolerance (fos_tol)"
0.05
```

Output for Example 5 (ex5.res)

0.5000	0.9791E-02	9
1.0000	0.1162E-01	10
1.5000	0.1285E-01	20
1.7500	0.1344E-01	36
1.8125	0.1380E-01	94
1.8281	0.1496E-01	500

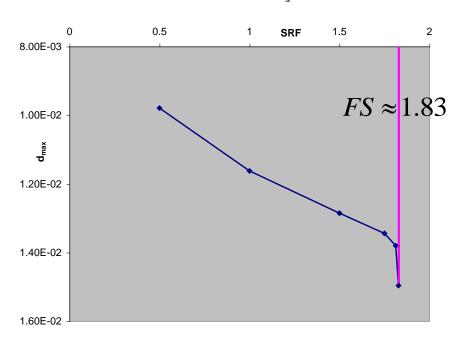


Fig 5.2 SRF vs. d_{max} for Example 5

Example 6: Rapid drawdown of a homogeneous slope from full submergence

The stability analysis is of a homogeneous $c'-\phi'$ slope (same slope as in Example 5) following 5m of rapid drawdown from full submergence as shown in Figure 6.1.

Data for Example 6 (ex6.dat)

```
"Example 6: Rapid drawdown of a homogeneous slope from full submergence"
```

```
"Width of top of embankment (w1)"
30.0
"Width of sloping portion of embankment (s1)"
20.0
"Distance foundation extends to right of embankment toe (w2)"
0.0
"Height of embankment (h1)"
10.0
"Thickness of foundation layer (h2)"
0.0
"Number of x-elements in embankment (nx1)"
20
"Number of x-elements to right of embankment toe (nx2)"
0
"Number of y-elements in embankment (ny1)"
10
"Number of y-elements in foundation (ny2)"
```

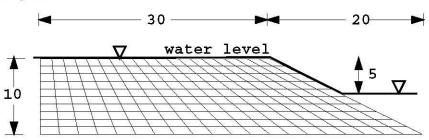


Fig 6.1 Finite element mesh for Example 6

Rapid drawdown from full submergence, homogeneous

 $\phi' = 20^{\circ}$ c' = 10 $\gamma = 20$

```
"Number of different property groups (np_types)"
1
"Material properties (phi,c,psi,gamma,e,v) for each group"
20.0 10.0 0.0 20.0 1.e5 0.3
"Property group assigned to each element (etype, data not needed if np_types=1)"
"Pseudo-static analysis: Horizontal acceleration factor (k_h)"
0.0
"Number of free surface points and their coordinates (nosurf, surf(2,nosurf))"
4
  0.0
        0.0
 30.0
       0.0
 40.0
       -5.0
       -5.0
 50.0
"Unit weight of water (gam_w)"
9.81
"Iteration ceiling (limit)"
500
"Factor of safety tolerance (fos_tol)"
0.05
```

Output for Example 6 (ex6.res)

0.5000	0.8548E-02	24
1.0000	0.1272E-01	111
1.0156	0.1327E-01	179
1.0312	0.1607E-01	500

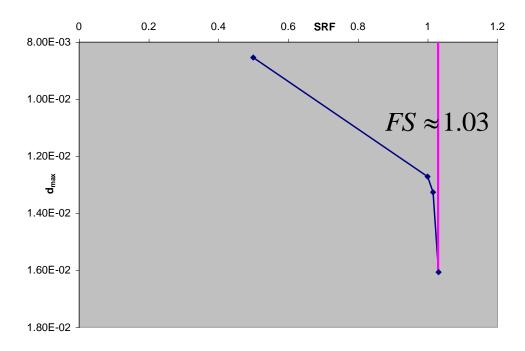
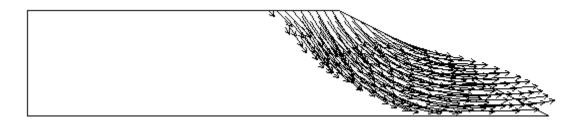


Fig 6.2 SRF vs. d_{max} for Example 6





ex6.dis

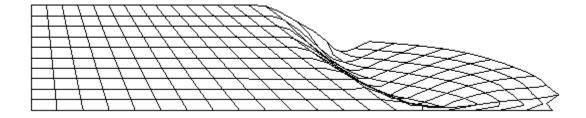


Fig 6.3 Displacement vectors and deformed mesh at failure for Example 6

Example 7: A homogeneous slope subjected to a horizontal pseudo-acceleration

The stability analysis is of a homogeneous slope subjected to a horizontal acceleration factor of 0.25g as shown in Figure 7.1

Data for Example 7 (ex7.dat)

"Width of top of embankment (w1)"

```
'Example 7: A homogeneous slope subjected to a horizontal pseudo-acceleration'
```

```
30.0
"Width of sloping portion of embankment (s1)"
20.0
"Distance foundation extends to right of embankment toe (w2)"
30.0
"Height of embankment (h1)"
10.0
"Thickness of foundation layer (h2)"
10.0
"Number of x-elements in embankment (nx1)"
50
"Number of x-elements to right of embankment toe (nx2)"
30
"Number of y-elements in embankment (ny1)"
10
"Number of y-elements in foundation (ny2)"
10
```

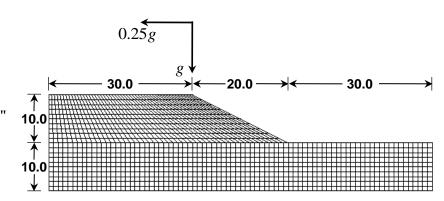


Fig 7.1 Finite element mesh for Example 7

Homogeneous slope with a horizontal acceleration of 0.25g

$$\phi' = 30^{\circ}$$

$$c' = 20.0$$

$$\gamma = 20.0$$

```
"Number of different property groups (np_types)"

"Material properties (phi,c,psi,gamma,e,v) for each group"
30.0 20.0 0.0 20.0 1.e5 0.3

"Property group assigned to each element (etype, data not needed if np_types=1)"

"Pseudo-static analysis: Horizontal acceleration factor (k_h)"
0.25

"Number of free surface points and their coordinates (nosurf, surf(2,nosurf))"
0

"Unit weight of water (gam_w)"
9.81

"Iteration ceiling (limit)"
2000

"Factor of Safety accuracy tolerance (fos_tol)"
0.05
```

Output for Example 7 (ex7.res)

0.5000	0.4984E-01	227
1.0000	0.5313E-01	31
1.2500	0.6154E-01	79
1.3750	0.6896E-01	350
1.3906	0.6978E-01	599
1.4062	0.8632E-01	2000

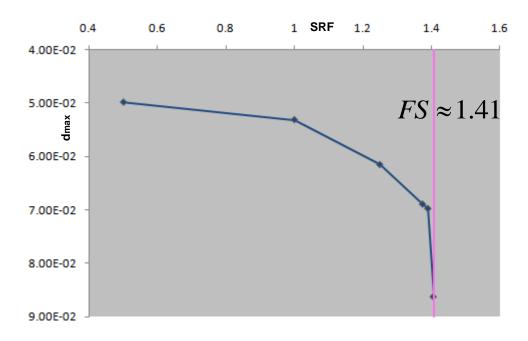
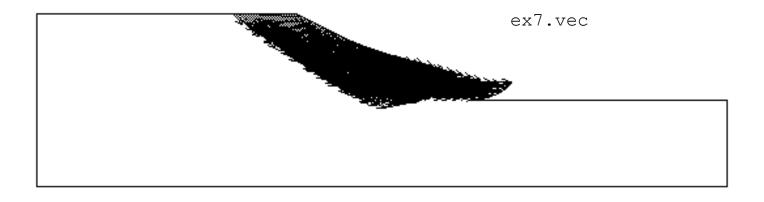


Fig 7.2 SRF vs. d_{max} for Example 7



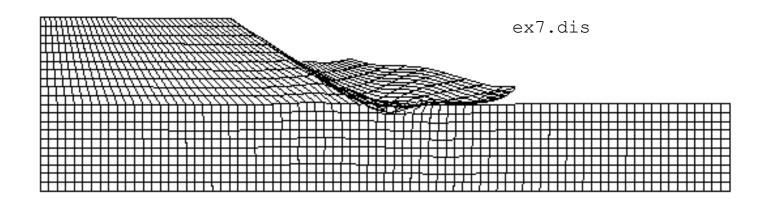


Fig 7.3 Displacement vectors and deformed mesh at failure for Example 7