

Project #4: Stresses around wellbores and fractures

For this project you will have to download and install the software FreeFEM++ (<http://www.freefem.org/>). FreeFEM++ help will be given in class and by the TA. You will also need to run Matlab, Python or equivalent software.

1) Wellbore problem

Consider a 2D problem of a circular cavity subjected to far field effective stresses $\sigma_{xx} = 12$ MPa and $\sigma_{yy} = 3$ MPa. The diameter of the cavity is 0.2 m. Rock properties: $E = 10$ GPa, $\nu = 0.20$, unconfined compression strength UCS = 30 MPa, tensile strength $T_s = 2$ MPa.

- Using Kirsch equations compute (and plot) σ_{rr} , $\sigma_{\theta\theta}$ and $\sigma_{r\theta}$ for a domain $x = [-1\text{m}, +1\text{m}]$, and $y = [-1\text{m}, +1\text{m}]$. You may define a polar grid for (r, θ) . How far does the presence of the wellbore influence stresses?
- Using Kirsch equations compute (and plot) stresses in a line $\{x = [0.1\text{m}, 1\text{m}], y = 0\text{m}\}$ and $\{x = 0\text{m}, y = [0.1\text{m}, 1\text{m}]\}$.
- Using Kirsch equations compute (and plot) σ_{rr} and $\sigma_{\theta\theta}$ for $r = 0.1\text{m}$. Is there any section of the rock in shear or tensile failure? Where?
- Use FreeFEM++ to solve the same problem (σ_{xx} , σ_{yy} and σ_{xy}) assuming a domain size 2m by 2m. Compute σ_{xx} and σ_{yy} for the same lines as in point “b”, and compare with Kirsch analytical solution. Repeat the process for a domain size 0.5m by 0.5m. Are there any differences? Why?
- Plot the displacement field.
- EXTRA: compute principal stresses within FreeFEM++ and plot σ_{rr} and $\sigma_{\theta\theta}$.

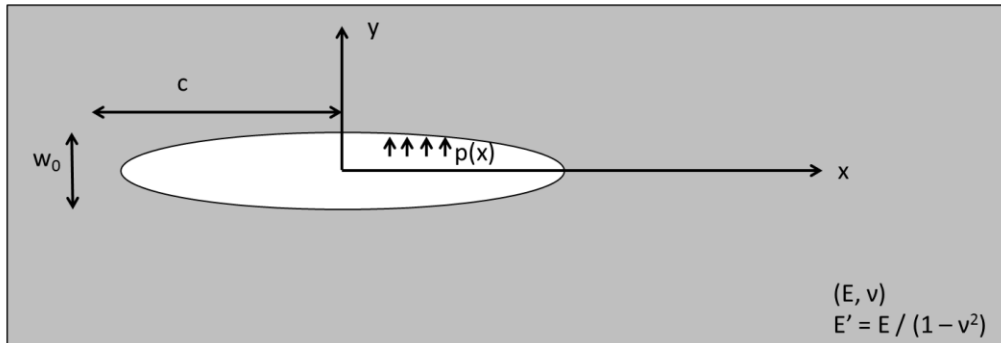
2) Fracture problem

Consider a 2D problem of an elliptical fracture (half-length $c = 10\text{m}$, $w_0 = 0.1\text{m}$). Set the fracture through an elliptical equation (major axis in direction y) at the center of a domain: $x = [-50\text{m}, 50\text{m}]$ and $y = [-50\text{m}, 50\text{m}]$. All other boundaries will have zero displacement. Rock properties: $E = 30$ GPa, $\nu = 0.20$.

- Use FreeFEM++ to solve for σ_{xx} , σ_{yy} and σ_{xy} imposing a fracture pressure $p = 10$ MPa. Plot results.
- Plot stress perpendicular to the fracture direction σ_{xx} at the middle of the fracture $\{x = 0, y = [w_0/2, 50\text{m}]\}$. How far does the influence of the fracture go?
- Plot x -displacements at the face of the fracture. Compare with analytical equation.
- Plot σ_{xx} along line $\{x = 0, y = [w_0/2, 50\text{m}]\}$ and compare with analytical solution.
- EXTRA: compare FreeFEM++ solution to analytical solution by Sneddon and Elliot, 1946.

(1) Width of an elliptical crack

Griffith crack problem

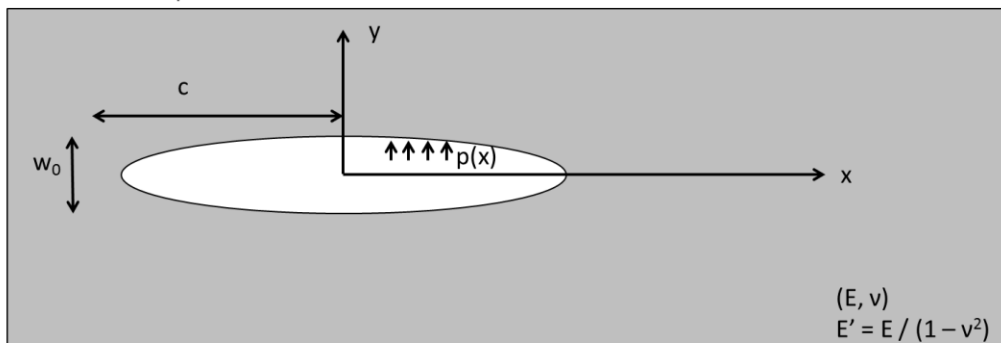


Boundary conditions

$\left\{ \begin{array}{ll} \sigma_{yy}(x,0) = -p(x) & \text{for } 0 \leq x \leq c \\ u_y(x,0) = 0 & \text{for } x > c \end{array} \right.$	<ul style="list-style-type: none"> Linear elastic and homogeneous solid Constant pressure p_0 No failure 	$\left\{ \begin{array}{l} \sigma_{yy}(x,0) = p_0[x/(x^2-c^2)^{1/2} - 1] \\ u_y(x,0) = 2p_0/E' * (c^2-x^2)^{1/2} \end{array} \right.$

(2) Stress Intensity Factor at Fracture Tip

Griffith crack problem



$\sigma_{yy}(x,0) = p_0[x/(x^2-c^2)^{1/2} - 1]$

- $\sigma_{yy} \rightarrow \infty$ at $x=c$!

A more convenient amount to compare is the stress intensity factor

$$K_I = \lim_{r \rightarrow 0^+} \left[2\pi r^{1/2} \sigma_{yy}(x=c+r, y=0) \right]$$

Constant pressure crack:

$$K_I = p_0(\pi c)^{1/2}$$

If $K_I > K_{IC}$ (Fracture Toughness)
 → Fracture propagates