

$$f(J_2) = \sqrt{3J_2} - Y = 0$$

Other yield surfaces

Maximum shear stress (Tresca criterion)

$$f = \frac{1}{2} \max \left\{ |\sigma_I - \sigma_{II}|, |\sigma_{II} - \sigma_{III}|, |\underbrace{\sigma_I - \sigma_{III}}| \right\}$$

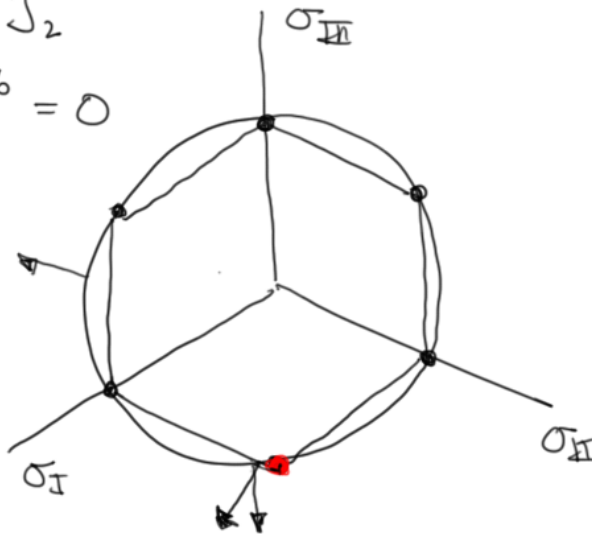
$$= 4J_2^3 - 27J_3^2 - 9J_2^2 + Y^3 - 6J_2Y^2 - Y^6 = 0$$

$$J_3 = \det(\sigma) = \sigma_I \sigma_{II} \sigma_{III}$$

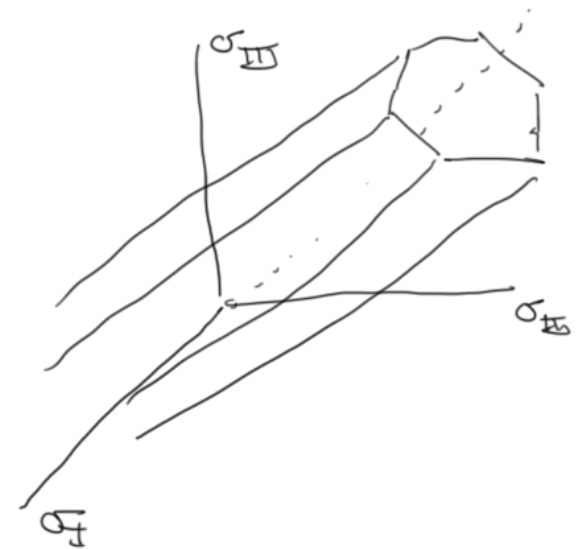
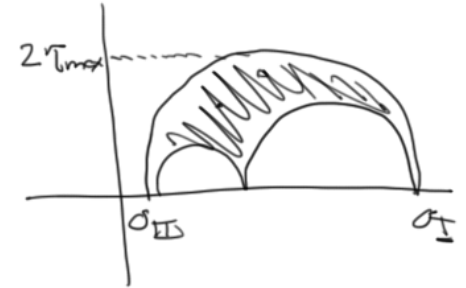
$$S_{ij} = \begin{bmatrix} -\frac{2}{3}Y & 0 & 0 \\ 0 & +\frac{1}{3}Y & 0 \\ 0 & 0 & +\frac{1}{3}Y \end{bmatrix}$$

$$J_3 = \frac{2}{27} Y^3 \quad \text{tension}$$

$$J_3 = -\frac{2}{27} Y^3 \quad \text{compression}$$



$$\dot{\epsilon}^P = \dot{\lambda} \frac{\partial f}{\partial \sigma_{ij}} = \dot{\lambda} S_{ij}$$



Drucker - Prager

$$f(p, \bar{\sigma}_2) = \sqrt{3\bar{\sigma}_2} - (\beta p) - \gamma = 0$$

$$p = -\frac{1}{3} \sigma_{kk}$$

$$\frac{\partial f}{\partial \sigma_{ij}}$$

$$\beta \frac{\partial f}{\partial \sigma_{ij}} = \frac{2}{2\sigma_{ij}} \left\{ \frac{1}{3} \sigma_{kk} \right\} = -\frac{1}{3} \delta_{ik} \delta_{jk} = \left(-\frac{1}{3} \delta_{ij} \right)$$

pressure dep.
in plastic flow



Mohr - Coulomb

$$\sigma_I - \sigma_{III} + (\sigma_I + \sigma_{III}) \sin(\phi) = \gamma \cos(\phi)$$

In terms of invariants

$$\frac{1}{3} I_1 \sin \phi + \sqrt{\bar{\sigma}_2} \left\{ \cos \theta - \frac{1}{\sqrt{3}} \sin \theta \sin \phi \right\} = \frac{\gamma}{2} \cos \phi$$

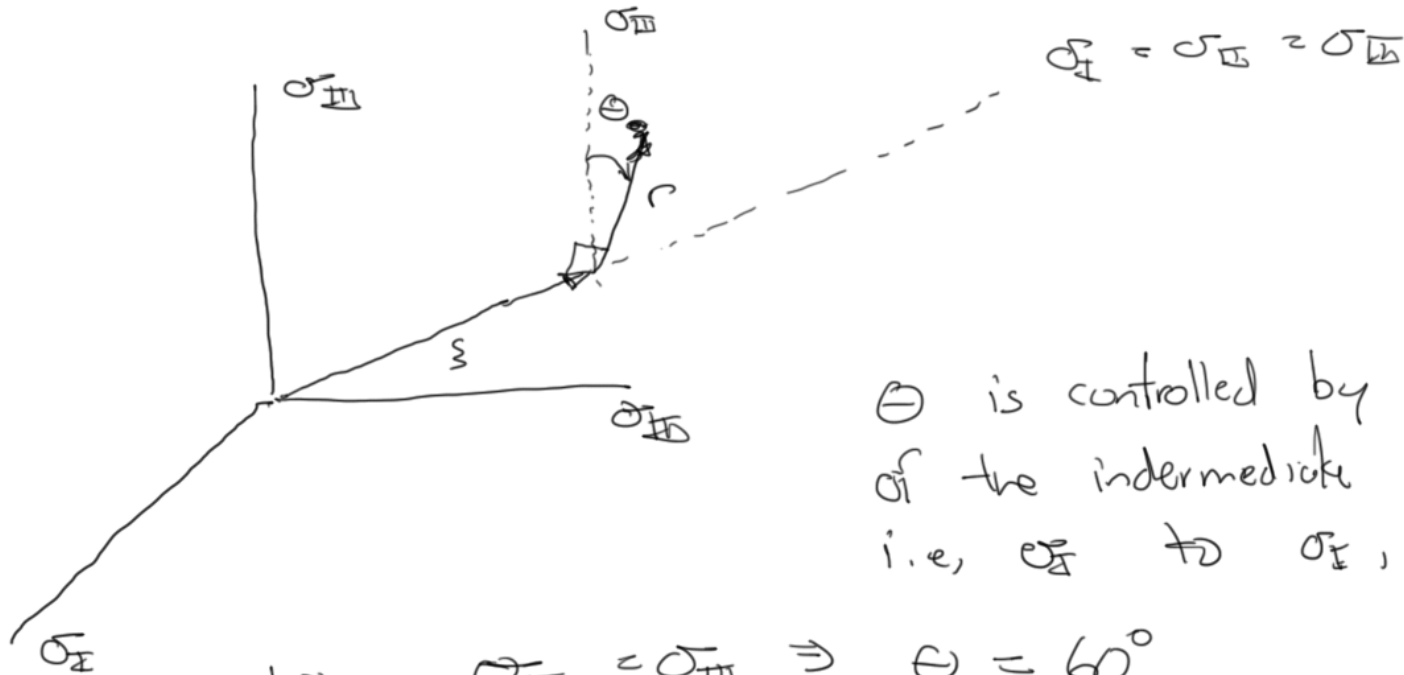
angle of repose
angle of internal friction



Lode angle

$$\Theta = \frac{1}{3} \sin^{-1} \left(\frac{3\sqrt{3}}{2} \frac{J_3}{J_2^{3/2}} \right)$$

$$-\frac{\pi}{6} \leq \Theta \leq \frac{\pi}{6}$$



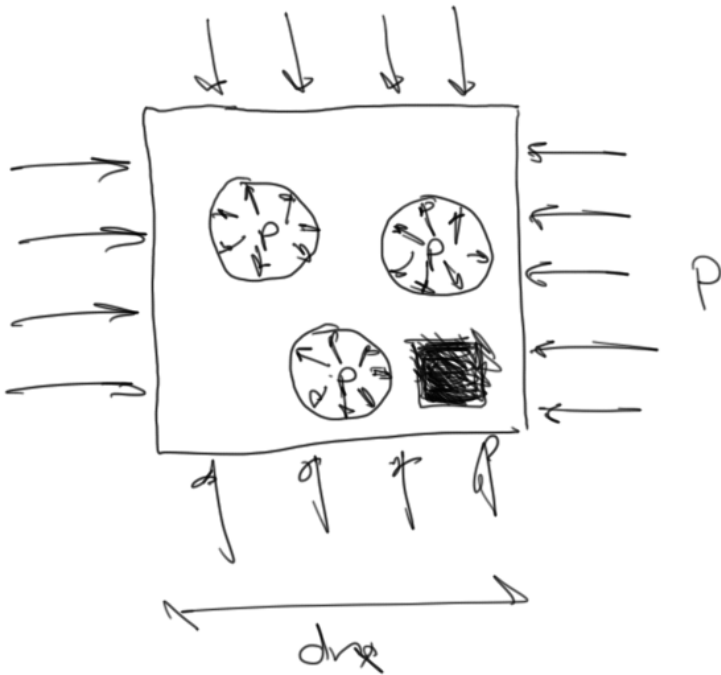
Θ is controlled by the relationship of the intermediate principle stress, i.e, σ_{II} to σ_I, σ_{III}

$$\text{When } \sigma_{II} = \sigma_{III} \Rightarrow \Theta = 60^\circ$$

$$\sigma_{II} = \sigma_I \Rightarrow \Theta = 0^\circ$$

$$\sigma_{ij} = S_{ij} + \frac{1}{3} \sigma_{kk} \delta_{ij} = S_{ij} - p \delta_{ij}$$

$$p = -\frac{1}{3} \sigma_{kk}$$



$$\underline{\sigma_{ij} = -p \delta_{ij}}$$

$$\sigma_{ij} = \lambda \epsilon_{kk} \delta_{ij} + 2\mu \epsilon_{ij}$$

$$\lambda = \left(K - \frac{2}{3} \mu \right)$$

$$\sigma_{ij} = \left(K - \frac{2}{3} \mu \right) \epsilon_{kk} \delta_{ij} + 2\mu \epsilon_{ij}$$

$$= K \epsilon_{kk} \delta_{ij} - 2\mu \left(\frac{1}{3} \epsilon_{kk} \delta_{ij} \right) + 2\mu \epsilon_{ij}$$

$$= K \epsilon_{kk} \delta_{ij} + 2\mu \left(\underbrace{\epsilon_{ij} - \frac{1}{3} \epsilon_{kk} \delta_{ij}}_{\epsilon'_{ij}} \right)$$

$$\sigma_{ij} = K \epsilon_{kk} \delta_{ij}$$

$$-p \delta_{ij} = K \epsilon_{kk} \delta_{ij} \Rightarrow \epsilon_{kk} = -\frac{p}{K_s}$$

$$\begin{aligned}\varepsilon_{ij} &= \cancel{\sigma_{ij}}^0 + \frac{1}{3} \varepsilon_{kk} \delta_{ij} \\ &= \frac{-p}{3K_s} \delta_{ij} \quad \leftarrow\end{aligned}$$

$$\sigma_{ij} = C_{ijkl} \varepsilon_{kl} \Rightarrow \varepsilon_{kl} = D_{kl ij} \sigma_{ij}$$

$$\text{where } D = C^{-1} \quad \text{or} \quad C_{ijkl} D_{mnop} = \underbrace{\delta_{im} \delta_{jn} \delta_{ko} \delta_{lp}}_{4^{\text{th}} \text{ identity tensor}}$$

so for arbitrary σ + p

$$\varepsilon_{kl} = \underbrace{D_{kl ij}}_{\text{effective stress}} (\sigma_{ij} + p \delta_{ij}) - \frac{1}{3K_s} p \delta_{kl}$$

effective stress

$$\begin{aligned}\sigma' &= \sigma_{ij} + \alpha p \delta_{ij} = \underbrace{C_{ijkl}}_{\delta_{ik} \delta_{jl} \delta_{mi} \delta_{nj} \sigma_{ij}} \varepsilon_{kl} = \underbrace{C_{ijkl}}_{\delta_{ik} \delta_{jl} \delta_{mi} \delta_{nj} \sigma_{ij}} \left[\underbrace{D_{kl ij}}_{\sigma_{ij}} (\sigma_{ij} + p \delta_{ij}) - \frac{1}{3K_s} p \delta_{kl} \right] \\ \cancel{\sigma_{ij}} + \alpha p \delta_{ij} &= \cancel{\sigma_{ij}} + p \delta_{ij} - \frac{C_{ijkl}}{3K_s} p \delta_{kl} \delta_{ij} \\ \cancel{\frac{\sigma}{3}} + \frac{\alpha}{3} p &= \frac{\sigma}{3} + \frac{\delta_{ij} C_{ijkl} \delta_{kl}}{3 \cdot 3 K_s} p\end{aligned}$$

$$\alpha = 1 - \frac{\delta_{ij} C_{ijhe} \delta_{he}}{9 K_S}$$

$$\frac{\delta_{ij} C_{ijhe} \delta_{he}}{9} = \frac{9\lambda + 6\mu}{9} = K_T$$

$$\alpha = 1 - \frac{K_T}{K_S}$$



Biot's coef.

$$K_S \gg K_T \quad \alpha \rightarrow 1$$

$$\text{rocks + concrete} \quad \alpha \approx \frac{2}{3}$$

$$\sigma' = \sigma_{ij} + \alpha P \delta_{ij}$$