

Cranes Loads

Middle span Crane loads :

	10	320	2.5					0.8	4.0	5.0			17.0	24.0	
	12.5	320	2.5					1.0	4.2	5.2			20.0	25.0	
32/5	16	320	2.5	1.4	1.9	16	0.5	1.1	4.3	5.3	1.0	14	24.0	27.0	2
	20	330	2.6					1.3	4.4	5.5			28.5	28.5	
	25	330	2.6					1.4	4.6	5.8			35.0	30.5	
→	32	330	2.6					1.6	5.1	6.4			43.0	33.0	

crane own weight : $(43 / 32) * 38 = 51 \text{ t}$

own weight per wheel = $51 / 4 = 12.75 \text{ t}$

crane max load = 32 t

load per wheel = $32 / 2 = 16 \text{ t}$

Total wheel load = $16 + 12.75 = 28.75 \text{ t}$

Impact factor = 1.25

Total wheel load with impact = 36 t

Left span crane loads :

	10	250	1.8					0.8	3.1	4.1			7.5	6.8	
	12.5	250	1.8					1.0	3.7	4.7			10.0	7.7	
10	16	270	1.8	1.0	0.8	16	0.3	1.1	3.9	4.9	2.8		12.9	8.7	2
	20	270	1.9					1.3	4.1	5.2			17.0	9.8	
→	25	280	1.9					1.4	4.6	5.6			21.7	11.5	
	32	280	1.9					1.5	5.1	6.1			27.5	12.7	

wheel total load : 11.5 t

Crane Loads Summary :

Middle Span Crane Wheel Load = 36 t

Left Span Crane Wheel Load = 11.5 t

Corrugated Sheets

roof Corrugated sheets :

Middle span Live load = $60 - 200/3 * \tan(\alpha) = 60 - 200/3 * 0.06 = 56 \text{ kg / m}^2$

- use corrugated sheet of allowable live load = 100 kg / m^2 , span **2 m**
use **continuous** corrugated sheet of thickness **0.55 mm**
- $O_w = 5.25 \text{ kg/m}^2$
- **Total load** = $5.25 + 56 = 61.25 \text{ kg/m}^2$

side spans live load = $60 - 200/3 * \tan(\alpha) = 60 - 200/3 * 0.1 = 53 \text{ kg / m}^2$

- use corrugated sheet of allowable live load = 100 kg / m^2 , span **2.5 m**
- use **continuous** corrugated sheet of thickness **0.7 mm**
- $ow = 6.66 \text{ kg/m}^2$
- **Total load** = $6.66 + 53 = 60 \text{ kg/m}^2$

side Corrugated sheets :

level > 10 m , Wind load = $C_e * K * q = 0.8 * 1.15 * 50 = 46 \text{ kg / m}^2$

- Use wind load = 50 kg / m^2 (lateral load) , Span = 2 m
- $O_w = 4.75 \text{ kg/m}^2$ (vertical load)
- Use **continuous** corrugated sheets for all side of thickness **0.5 mm**

level < 10 m , wind load = $C_e * K * q = 0.8 * 1.0 * 50 = 40 \text{ kg/m}^2$

- Use wind load = 50 kg / m^2 (lateral load) , Span = 2.5 m
- $O_w = 4.75 \text{ kg/m}^2$ (vertical load)
- Use **continuous** corrugated sheets for all side of thickness **0.5 mm**

Corrugated Sheets Summary :

- Use continuous in **middle span** roof corrugated sheets of **0.7 mm**
- Use continuous in **side spans** roof corrugated sheets of **0.55 mm**
- Use continuous in **side corrugated** sheets of **0.50 mm**

Mezanin

Flooring = 200 kg/m^2

Storage Floor Live Load = 500 Kg/m^2

Management Floors Live Load = 400 Kg/m^2

Walls distributed load = 200 kg/m^2

Deck span = 2.50 m

Use Metal Deck thickness = 1.2 mm

For **Storage Floor** Use concrete thickness = 8 cm

- concrete load = $2500 \text{ kg/m}^3 * 0.08 = 200 \text{ kg/m}^2$
- total dead load = 600 kg/m^2
- total live load = 500 kg/m^2
- total working load = 1100 kg/m^2
- Total ultimate load for storage floor = $1.4 * (200 + 200 + 200) + 1.6 * 500 = 1640 \text{ kg/m}^2$
- Allowable load for storage floor = **1758 kg/m^2**

For **management Floor** Use concrete thickness = 7 cm

- concrete load = $2500 \text{ kg/m}^3 * 0.07 = 175 \text{ kg/m}^2$
- total dead load = 575 kg / m^2
- total live load = 400 kg/m^2
- total working load = 975 kg/m^2

- Total ultimate load for mang. floor = $1.4 * (200 + 175 + 200) + 1.6 * 400 = 1445 \text{ kg/m}^2$
- Allowable load for management floor = **1542 kg/m²**

Mezanin Summery :

Use Metal Deck of **1.2 mm**

For Management Floors , Concrete Thickness = **7 cm**

For storage Floor , Concrete Thickness = **8 cm**

Thickness	0.8 mm.				1.0 mm.				1.2 mm.				1.6 mm.			
Concrete thickness	5 cm.	6 cm.	7 cm.	8 cm.	5 cm.	6 cm.	7 cm.	8 cm.	5 cm.	6 cm.	7 cm.	8 cm.	5 cm.	6 cm.	7 cm.	8 cm.
Span																
2.0 m	1500	1760	2070	2360	1630	1900	2210	2520	1850	2130	2440	2770	1940	2230	2540	2890
2.4	1000	1180	1400	1630	1100	1280	1500	1730	1290	1480	1700	1930	1350	1550	1770	2000
2.8	610	750	880	1050	720	860	990	1160	920	1060	1210	1380	990	1140	1300	1470

Beams load

Mezanin storage floor main beams (using metal deck allowable load) :

- Dead load = $2.2\text{t/m} * 6\text{m} = 13.2 \text{ t}$, secondary beams from one side , @2.5m spacing
- Live load = $1.25\text{t/m} * 6\text{m} = 7.5 \text{ t}$, secondary beams from one side , @2.5m spacing

Mezanin management floor main beams (using metal deck allowable load) :

- Dead load = $2.2\text{t/m} * 6\text{m} = 13.2 \text{ t}$, secondary beams from one side , @2.5m spacing
- Live load = $1\text{t/m} * 6\text{m} = 6 \text{ t}$, secondary beams from one side , @2.5m spacing

Mezanin Storage Floor Secondary Beams :

- Dead load = $0.6\text{t/m}^2 * 2.5\text{m} * 1.4(\text{f.o.s}) = 2.1 \text{ t/m}$
- Total Live load = $0.5\text{t/m}^2 * 2.5\text{m} = 1.25 \text{ t/m}$

- $ow = 100 \text{ kg / m}$
- Total Dead load = $0.1\text{t/m} + 2.1\text{t/m} = 2.2 \text{ t/m}$

Mezanin Management Floor Secondary Beams :

- Dead load = $0.58\text{t/m}^2 * 2.5\text{m} * 1.4(\text{f.o.s}) = 2.1 \text{ t/m}$
- $Ow = 0.1 \text{ t/m}$
- Total Dead load = 2.2 t/m
- Live load = $0.4\text{t/m}^2 * 2.5\text{m} = 1 \text{ t/m}$

Purlins Middle span :

- Live load = $56 \text{ kg/m}^2 * 2\text{m} = \mathbf{112 \text{ kg/m}}$
- $Ow = 25 \text{ kg/m}$
- Dead load = $5.25 \text{ kg/m}^2 * 2 + 25 = \mathbf{30.5 \text{ kg/m}}$
- Dead Load Reactions = $30.5\text{kg/m} * 6\text{m} / 2 = 91.5 \text{ kg} = 0.092\text{t}$
- Live load Reactions = $112\text{kg/m} * 6\text{m} / 2 = 336 \text{ kg} = 0.34\text{t}$
- Total load Reactions = $427.5 \text{ kg} = 0.43 \text{ t}$

Purlins Side Spans :

- Live Load = $53 \text{ kg/m}^2 * 2.5\text{m} = \mathbf{132.5 \text{ kg/m}}$
- $Ow = 25 \text{ kg/m}$
- Dead Load = $25\text{kg/m} + 6.66\text{kg/m}^2 * 2.5\text{m} = \mathbf{41.65 \text{ kg/m}}$
- Dead load Reactions = $41.65\text{kg/m} * 6\text{m} / 2 = 125 \text{ kg} = 0.125 \text{ t @ } 2.5\text{m span}$
- Live load Reactions = $132.5\text{kg/m} * 6\text{m} / 2 = 397.5 \text{ kg} = 0.4\text{t @ } 2.5\text{m span}$
- Total load = 0.525t

Frame girder load :

- Purlins concentrated Dead loads from each sides = $2 * 0.125\text{t} = \mathbf{0.25\text{t}}$
,9purlins@2.5m
- Purlins concentrated Live loads from each sides = $2 * 0.4\text{t} = \mathbf{0.8\text{t}}$
,9purlins@2.5m
- Utilities Dead loads = $\mathbf{0.1 \text{ t /m}}$

Truss loads :

- Purlins concentrated **Dead** loads at nodes from each sides = $2 * 0.092t =$ **0.185t** ,purlins@2.5m at each node
- Purlins concentrated **Live** loads at nodes from each sides = $2 * 0.34t =$ **0.68t** ,purlins@2.5m at each node
- Utilities **Dead** Load = 100 kg / node = **0.1t** /node