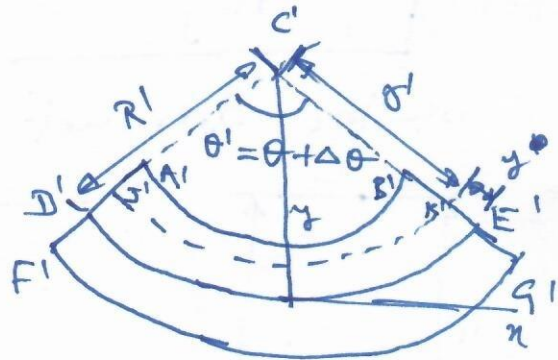
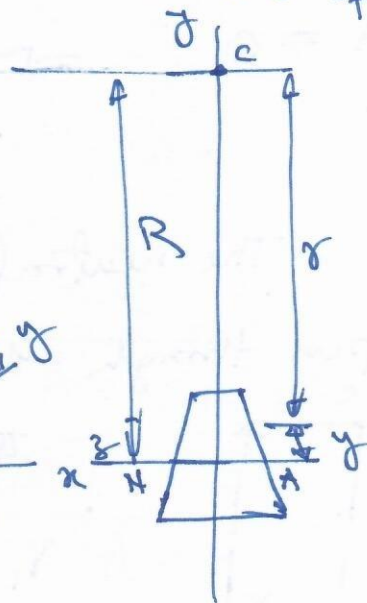
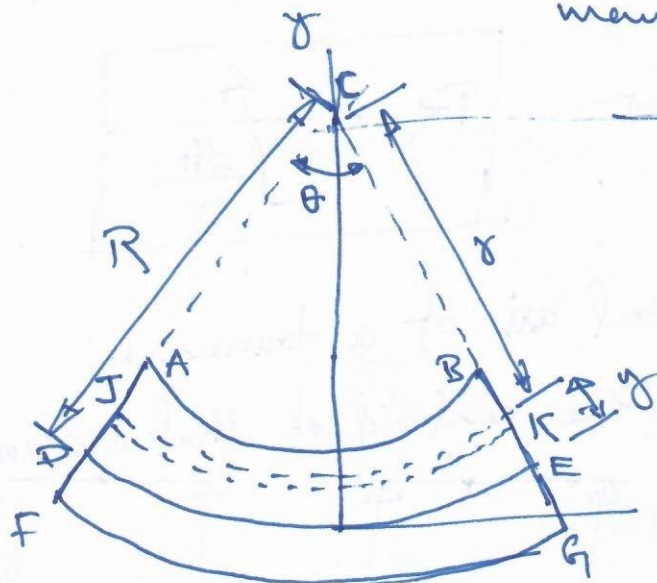


Bending of Curved beams:

- Assumptions:
- (a) Uniform cross section
 - (b) Section possessing a plane of symmetry
 - (c) Stresses are below proportional limit

Curved member: When the radius of curvature & the dimensions of the cross section of the member are of the same order.



$$R\theta = R'\theta'$$

Deformation of JK $\rightarrow S = r'\theta' - r\theta$

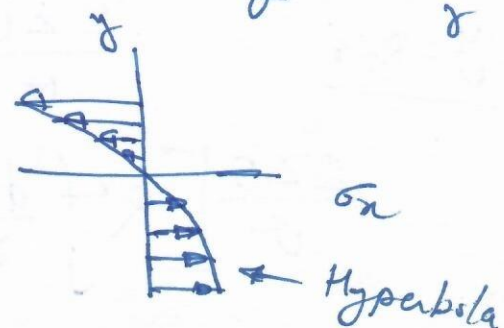
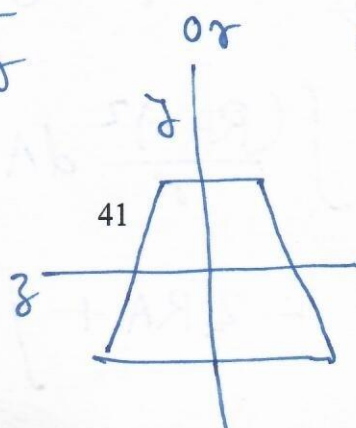
$$r = R - y, \quad r' = R' - y \quad \Rightarrow \quad S = (R' - y)\theta' - (R - y)\theta$$

$$S = -y\Delta\theta$$

$$\epsilon_x = \frac{S}{r\theta} = -\frac{y\Delta\theta}{r\theta} \quad \Rightarrow \quad \epsilon_x = -\frac{\Delta\theta}{\theta} \frac{y}{(R-y)}$$

$$\sigma_x = -\frac{E\Delta\theta}{\theta} \frac{y}{R-y}$$

$$\sigma_x = -\frac{E\Delta\theta}{\theta} \frac{(R-r)}{r}$$



Location of neutral surface:

$$\int \sigma_x dA = 0 \quad \text{--- (1)} \quad \text{and} \quad \int (-y \sigma_x dA) = M \quad \text{--- (2)}$$

from (1)
$$-\int \frac{E \Delta \theta}{\rho} \frac{R-r}{r} dA = 0$$

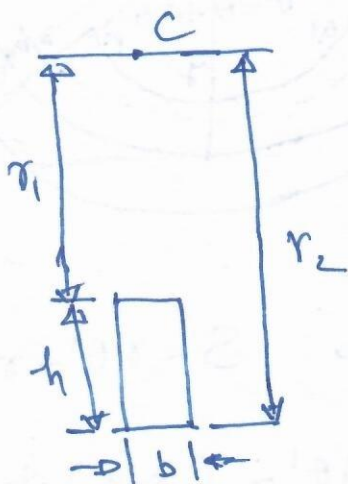
$$\int \frac{R-r}{r} dA = 0$$

$$R \int \frac{dA}{r} - \int dA = 0 \quad \Rightarrow$$

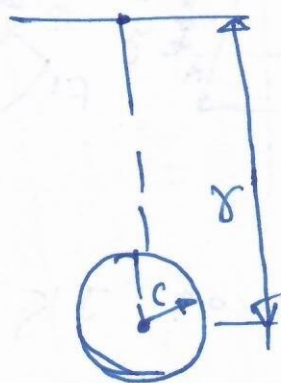
$$R = \frac{A}{\int \frac{dA}{r}}$$

$$\bar{r} = \frac{1}{A} \int r dA$$

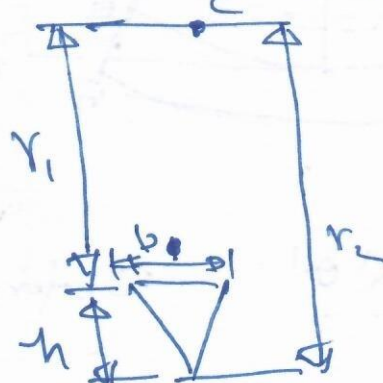
The neutral axis of a transverse section does not pass through the centroid of that section



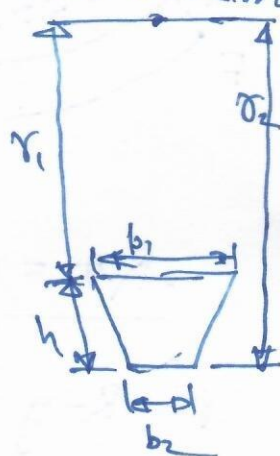
$$R = \frac{h}{\ln \frac{r_2}{r_1}}$$



$$R = \frac{1}{2} (\bar{r} + \sqrt{\bar{r}^2 - c^2})$$



$$R = \frac{\frac{1}{2} h}{\frac{r_2}{h} \ln \frac{r_2}{r_1} - 1}$$



from (2)

$$\int \frac{E \Delta \theta}{\rho} \frac{R-r}{r} y dA = M$$

$$y = R - r$$

$$\frac{E \Delta \theta}{\rho} \int \frac{(R-r)^2}{r} dA = M$$

$$\frac{E \Delta \theta}{\rho} \left[R^2 \int \frac{dA}{r} - 2RA + \int r dA \right] = M$$

$$R \int \frac{dA}{r} = A \quad \Leftarrow \quad \int r dA = \bar{r} A$$

$$\frac{E \Delta \theta}{\theta} = \frac{M}{A(\bar{r} - R)}$$

$$\Delta \theta > 0 \quad \text{for } M > 0$$

$$\bar{r} - R > 0 \quad \text{for } R < \bar{r}$$

$$\bar{r} - R = e$$

$$\frac{E \Delta \theta}{\theta} = \frac{M}{Ae}$$

$$\sigma_x = - \frac{M y}{Ae(R - y)}$$

$$\sigma_r = \frac{M(r - R)}{Aer}$$

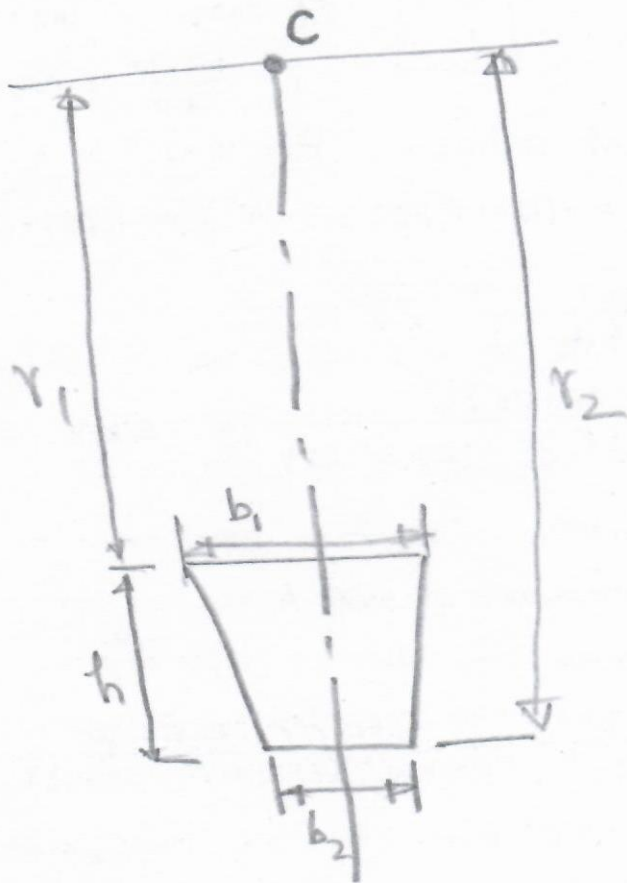
Change in curvature of neutral surface.

$$\frac{1}{R'} = \frac{1}{R} \frac{\theta'}{\theta}$$

$$\theta' = \theta + \Delta \theta$$

$$\frac{1}{R'} = \frac{1}{R} \left(1 + \frac{\Delta \theta}{\theta} \right) = \frac{1}{R} \left(1 + \frac{M}{EAe} \right)$$

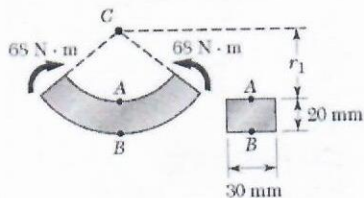
$$\frac{1}{R'} - \frac{1}{R} = \frac{M}{EAeR}$$



$$R = \frac{\frac{1}{2} h^2 (b_1 + b_2)}{(b_1 r_2 - b_2 r_1) \ln \frac{r_2}{r_1} - h (b_1 - b_2)}$$

Problem 4.156

4.156 For the curved bar and loading shown, determine the stress point A when (a) $r_1 = 30$ mm, (b) $r_1 = 50$ mm.



$$h = 20 \text{ mm} \quad A = (30)(20) = 600 \text{ mm}^2$$

$$(a) \quad r_1 = 30 \text{ mm} \quad r_2 = 50 \text{ mm}$$

$$R = \frac{h}{\ln \frac{r_2}{r_1}} = \frac{20}{\ln \frac{5}{3}} = 39.15 \text{ mm}$$

$$\bar{r} = \frac{1}{2}(r_1 + r_2) = 40 \text{ mm}$$

$$e = \bar{r} - R = 0.85 \text{ mm}$$

$$y_A = 39.15 - 30 = 9.15 \text{ mm}$$

$$r_A = r_1 = 30 \text{ mm}$$

$$\sigma_A = -\frac{M y_A}{A e r_A} = -\frac{(68)(0.00915)}{(600 \times 10^{-6})(0.0085)(0.03)} = -4.07 \text{ MPa}$$

$$-4.07 \text{ MPa}$$

$$(b) \quad r_1 = 50 \text{ mm} \quad r_2 = 70 \text{ mm}$$

$$R = \frac{h}{\ln \frac{r_2}{r_1}} = \frac{20}{\ln \frac{7}{5}} = 59.44 \text{ mm}$$

$$\bar{r} = \frac{1}{2}(r_1 + r_2) = 60 \text{ mm}$$

$$e = \bar{r} - R = 0.56 \text{ mm}$$

$$y_A = 59.44 - 50 = 9.44 \text{ mm}$$

$$r_A = r_1 = 50 \text{ mm}$$

$$\sigma_A = -\frac{M y_A}{A e r_A} = -\frac{(68)(0.00944)}{(600 \times 10^{-6})(0.0056)(0.05)} = -38.2 \text{ MPa}$$

$$-38.2 \text{ MPa}$$