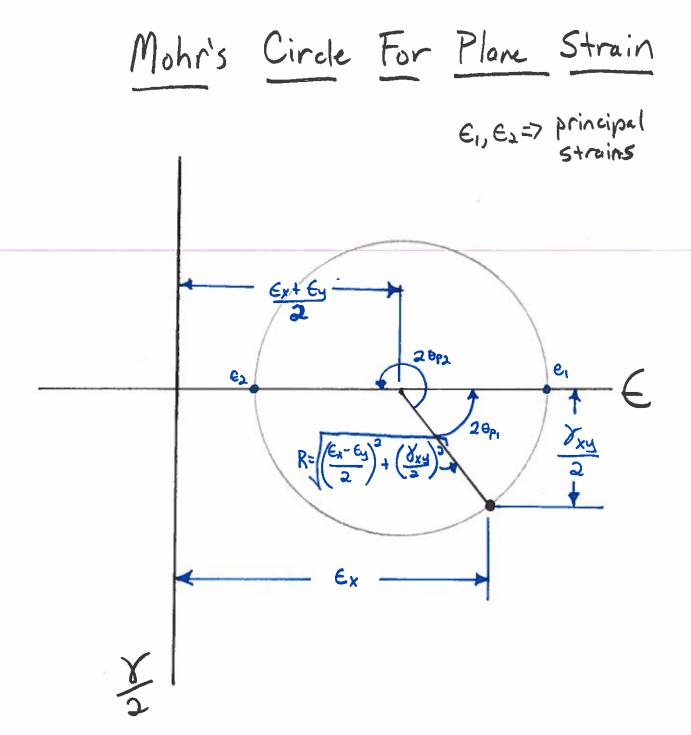
## EME 150A FALL 2015 LECTURE 10



## Curved Beams in Bending

Can't use same analysis as Straight beams!

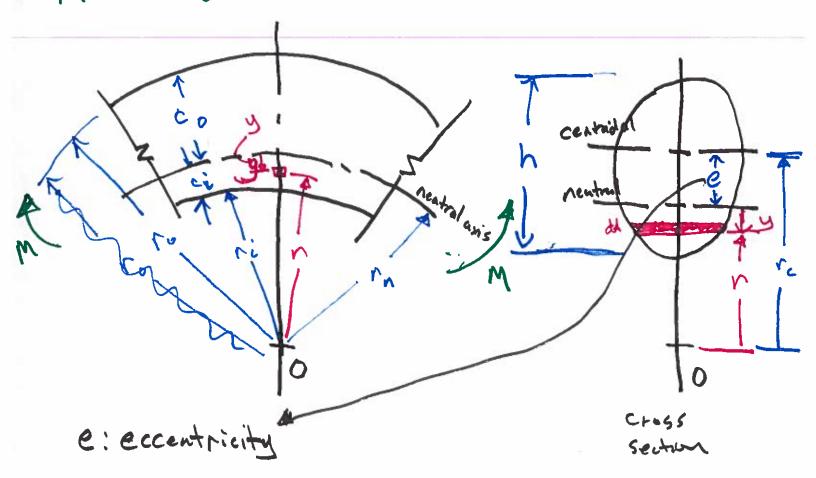
Centroidal axis 7 neutral axis

## Assumptions

- in the plane of bending
- plane cross sections remain plane
- modulus of elasticity is same tension and compression



## M: decrease the curvature



neutral axis

 $r_n = \frac{A}{\int_{C_n}^{R_n} \frac{dA}{R}}$ 

Centroidal axis

Fc = SradA

Normal Stress Distributions

Ty = My hyperbolic function

Mai Ae(rn-y) in y

+: tension

Nei Aeri

Aeri

Fig 3.35 びいろで。 ſį 1,0?

Example F=50001b Book ex Ji=16.9 Kpsi J=-5.63 Kpsi Table 3-4 bo-bi+[(biro-bori)/h] In (ro)

6

$$A = b_i h - \frac{(bi - bo)}{2} h$$

$$A \approx 3 in^2$$

1:0"
h=4"
bi=4"
bi=6"
To=6"

rn= 3.44" Ci=1.44" e= 6.34"

Ci=rn-ri

5; = (20000 in 16) (1.44") (3in') (0.34") (5")

(3in') (0.34") (5")

00 = -7.3 Kpsi

Jores => -5,63 Kps;

compression sympsess

Stress in Pressure Vessels "Thick walked" Wall Hickness Or = radial stress Ut = tangential

Jt = Piri - Po (po - Pi)/2

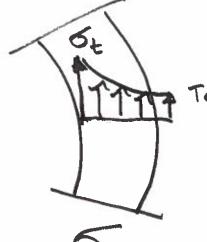
Or 2 Piri - Poro + ri2ro (Po-Pi)

Log - L. 5

Special Case

$$QF = \frac{C_3 - C_7}{C_5 b_5} \left( 1 + \frac{\lambda_3}{C_5} \right)$$

$$Q^{L} = \frac{U_{3}^{2} - U_{i}}{U_{5}^{2} + U_{5}} \left(1 - \frac{L_{3}}{V_{3}}\right)$$



4164

stress The

compression

) t

Po=0

7

Longitudial stress