

Company Name		Project Title	
Group/Team Name		Subtitle	
Designer		Job Number	
Date	30 /05 /2020	Client	

# 1 Input Parameters

Module	Tension Members Bolted Design
Axial (kN) *	200.0
Length(mm) *	5000.0
Section Size*	Ref List of Input Section
<b>Bolt Details</b>	
Diameter (mm)*	[8.0, 10.0, 12.0, 14.0, 16.0, 18.0, 20.0, 22.0, 24.0, 27.0, 30.0, 33.0, 36.0, 39.0]
Grade *	[3.6, 4.6, 4.8, 5.6, 5.8, 6.8, 8.8, 9.8, 10.9, 12.9]
Type *	Bearing Bolt
Bolt hole type	Standard
Slip factor ( $\mu_f$ )	0.3
Type of edges	a - Sheared or hand flame cut
Gap between beam and  support (mm)	0.0
Are the members exposed to  corrosive influences	False
<b>Safety Factors - IS 800:2007 Table 5 (Clause 5.4.1)</b>	
Governed by Yielding	$\gamma_{m0} = 1.1$
Governed by Ultimate Stress	$\gamma_{m1} = 1.25$
Connection Bolts - Bearing Type	$\gamma_{mb} = 1.25$

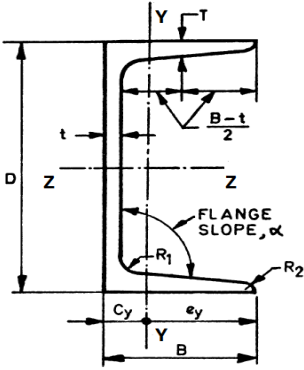
## 1.1 List of Input Section

Section Size*	['MC 75', 'MC 100', 'MC 125', 'MC 125*', 'MC 150', 'MC 150*', 'MC 175', 'MC 175*', 'MC 200', 'MC 200*', 'MC 225', 'MC 225*', 'MC 250', 'MC 250*', 'MC 250*', 'MC 300', 'MC 300*', 'MC 300*', 'MC 350', 'MC 400', 'JC 100', 'JC 125', 'JC 150', 'JC 175', 'JC 200', 'LC 75', 'LC 100', 'LC 125', 'LC (P) 125', 'LC 150', 'LC (P) 150', 'LC 175', 'LC 200', 'LC (P) 200', 'LC 225', 'LC 250', 'LC 300', 'LC (P) 300', 'LC 350', 'LC 400', 'MPC 75', 'MPC 100', 'MPC 125', 'MPC 125*', 'MPC 150', 'MPC 150*', 'MPC 175', 'MPC 175*', 'MPC 200', 'MPC 200*', 'MPC 225', 'MPC 225*', 'MPC 250', 'MPC 250*', 'MPC 250*', 'MPC 300', 'MPC 300*', 'MPC 300*', 'MPC 300*', 'MPC 350', 'MPC 400']
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## 2 Design Checks

### 2.1 Selected Member Data

	Section Size*		('JC 125', 'Channels')	
	Material *		E 250 (Fe 410 W)A	
	Ultimate strength, fu (MPa)		410	
	Yield Strength , fy (MPa)		250	
	Mass	7.9	Iz(mm4)	2690000.0
	Area(mm2) - A	1000.0	Iy(mm4)	251000.0
	D(mm)	125	rz(mm)	51.7
	B(mm)	50	ry(mm)	15.8
	t(mm)	3.0	Zz(mm3)	43100.0
	T(mm)	6.6	Zy(mm3)	7500.0
	FlangeSlope	91.5	Zpz(mm3)	49100.0
	R1(mm)	6.0	Zpy(mm3)	7500.0
	R2(mm)	2.4	r(mm)	15.8
	Cy(mm)	16.4		

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## 2.2 Spacing Checks

Check	Required	Provided	Remarks
Min.Diameter (mm)		$d = 14$	
Hole Diameter (mm)		$d_0 = 15$	
Min. Gauge (mm)	$p/g_{min} = 2.5 d$ $= 2.5 * 14.0 = 35.0$	35	Row Limit (rl) = 2
Min. Edge Distance (mm)	$e/e'_{min} = [1.5 \text{ or } 1.7] * d_0$ $= 1.7 * 15.0 = 25.5$	30	
Spacing Check	$depth = 2 * e + (rl - 1) * g$ $= 2 * 30 + (2 - 1) * 35$ $= 95$	99.8	Pass

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## 2.3 Member Checks

Check	Required	Provided	Remarks
Tension Yielding Capacity (kN)		$T_{dg} \text{ or } A_c = \frac{1 * A_g f_y}{\gamma_{m0}}$ $= \frac{1 * 1000.0 * 250}{1.1}$ $= 227.27$	
Tension Rupture Capacity (kN)		$\beta = 1.4 - 0.076 * \frac{w}{t} * \frac{f_y}{0.9 * f_u} * \frac{b_s}{L_c}$ $\leq \frac{0.9 * f_u * \gamma_{m0}}{f_y * \gamma_{m1}} \geq 0.7$ $= 1.4 - 0.076 * \frac{50}{3.0} * \frac{250}{0.9 * 410} * \frac{92.0}{225}$ $\leq \frac{0.9 * 410 * 1.1}{250 * 1.25} \geq 0.7$ $= 1.05$ $T_{dn} = 1 * \left( \frac{0.9 * A_{nc} * f_u}{\gamma_{m1}} + \frac{\beta * A_{go} * f_y}{\gamma_{m0}} \right)$ $= 1 * \left( \frac{0.9 * 245.4 * 410}{1.25} + \frac{1.05 * 660.0 * 250}{1.1} \right)$ $= 229.94$	
Block Shear Capacity (kN)		$T_{db1} = \frac{A_{vg} f_y}{\sqrt{3} \gamma_{m0}} + \frac{0.9 A_{tn} f_u}{\gamma_{m1}}$ $T_{db2} = \frac{0.9 * A_{vn} f_u}{\sqrt{3} \gamma_{m1}} + \frac{A_{tg} f_y}{\gamma_{m0}}$ $T_{db} = \min(T_{db1}, T_{db2}) = 200.26$	
Tension Capacity (kN)	200.0	$T_d = \min(T_{dg}, T_{dn}, T_{db})$ $= \min(227.27, 229.94, 200.26)$ $= 200.26$	Pass
Slenderness	$\frac{K * L}{r} \leq 400$	$\frac{K * L}{r} = \frac{1 * 5000.0}{15.8}$ $= 316.46$	Pass
Utilization Ratio	$Utilization \ Ratio \leq 1$	$Utilization \ Ratio = \frac{F}{T_d} = \frac{200.0}{200.26}$ $= 1.0$	
Axial Load Considered (kN)	$A_{cmin} = 0.3 * A_c$ $= 0.3 * 227.27$ $= 68.18$ $A_{cmax} = 227.27$	$A = 200.0$	Pass

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## 2.4 Bolt Checks

Check	Required	Provided	Remarks
Diameter (mm)	Bolt Quantity Optimisation	$d = 14$	
Hole Diameter (mm)		$d_0 = 15$	
Grade	Bolt Grade Optimisation	3.6	
Bolt Ultimate Strength (N/mm <sup>2</sup> )		$f_{ub} = 330.0$	
Bolt Yield Strength (N/mm <sup>2</sup> )		$f_{yb} = 190.0$	
Nominal Stress Area (mm <sup>2</sup> )		$A_{nb} = 115$ (Ref IS 1367 – 3 (2002))	
Kb		$k_b = \min\left(\frac{e}{3 * d_0}, \frac{p}{3 * d_0} - 0.25, \frac{f_{ub}}{f_u}, 1.0\right)$ $= \min\left(\frac{30}{3 * 15.0}, \frac{45}{3 * 15.0} - 0.25, \frac{330.0}{410}, 1.0\right)$ $= \min(0.67, 0.75, 0.8, 1.0)$ $= 0.67$	
Shear Capacity (kN)		$V_{dsb} = \frac{f_{ub} n_n A_{nb}}{\sqrt{3} \gamma_{mb}}$ $= \frac{330.0 * 1 * 115}{\sqrt{3} * 1.25}$ $= 17.53$	
Bearing Capacity (kN)		$V_{dpb} = \frac{2.5 k_b d t f_u}{\gamma_{mb}}$ $= \frac{2.5 * 0.67 * 14.0 * 3.0 * 410}{1.25}$ $= 23.07$	
Capacity (kN)		$V_{db} = \min(V_{dsb}, V_{dpb})$ $= \min(17.53, 23.07)$ $= 17.53$	
No of Bolts	$R_u = \sqrt{V_u^2 + A_u^2}$ $n_{trial} = R_u / V_{bolt}$ $R_u = \frac{\sqrt{0.0^2 + 200.0^2}}{17.53}$ $= 12$	$n = 12$	
No of Columns		$n_c = 6$	
No of Rows		$n_r = 2$	
Min. Pitch (mm)	$p/g_{min} = 2.5 d$ $= 2.5 * 14.0 = 35.0$	45	Pass

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Check	Required	Provided	Remarks
Max. Pitch (mm)	$p/g_{max} = \min(32 t, 300 \text{ mm})$ $= \min(32 * 3.0, 300 \text{ mm})$ $= 96.0$ <i>where,</i> $t = \min(8.0, 8.0)$	45	Pass
Min. Gauge (mm)	$p/g_{min} = 2.5 d$ $= 2.5 * 14.0 = 35.0$	35	Pass
Max. Gauge (mm)	$p/g_{max} = \min(32 t, 300 \text{ mm})$ $= \min(32 * 3.0, 300 \text{ mm})$ $= 96.0$ <i>where,</i> $t = \min(8.0, 8.0)$	35	Pass
Min. End Distance (mm)	$e/e'_{min} = [1.5 \text{ or } 1.7] * d_0$ $= 1.7 * 15.0 = 25.5$	30	Pass
Max. End Distance (mm)	$e/e'_{max} = 12 t \varepsilon$ $\varepsilon = \sqrt{\frac{250}{f_y}}$ $e/e'_{max} = 12 * 3.0 * \sqrt{\frac{250}{250}}$ $= 36.0$	30	Pass
Min. Edge Distance (mm)	$e/e'_{min} = [1.5 \text{ or } 1.7] * d_0$ $= 1.7 * 15.0 = 25.5$	32.4	Pass
Max. Edge Distance (mm)	$e/e'_{max} = 12 t \varepsilon$ $\varepsilon = \sqrt{\frac{250}{f_y}}$ $e/e'_{max} = 12 * 3.0 * \sqrt{\frac{250}{250}}$ $= 36.0$	32.4	Pass
Bolt Capacity post Long Joint (kN)	<i>if</i> $l \geq 15 * d$ <i>then</i> $V_{rd} = \beta_{ij} * V_{db}$ <i>if</i> $l < 15 * d$ <i>then</i> $V_{rd} = V_{db}$ <i>where,</i> $l = ((nc \text{ or } nr) - 1) * (p \text{ or } g)$ $\beta_{ij} = 1.075 - l/(200 * d)$ <i>but</i> $0.75 \leq \beta_{ij} \leq 1.0$	$l = ((nc \text{ or } nr) - 1) * (p \text{ or } g)$ $= (6 - 1) * 45 = 225$ $= (2 - 1) * 35 = 35$ $l = 225$ $15 * d = 15 * 14.0 = 210.0$ <i>since,</i> $l \geq 15 * d$ <i>then</i> $V_{rd} = \beta_{ij} * V_{db}$ $\beta_{ij} = 1.075 - 225/(200 * 14.0) = 0.99$ $V_{rd} = 0.99 * 17.53 = 17.35$	
Capacity (kN)	16.67	17.35	Pass

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## 2.5 Gusset Plate Checks

Check	Required	Provided	Remarks
Min.Height (mm)		$H = 1 * Depth + clearance$ $= (1 * 125) + 30.0$ $= 155$	
Min.Length (mm)	5000.0	$L = (nc - 1) * p + 2 * e$ $= (6 - 1) * 45 + (2 * 30)$ $= 285$	Pass
Thickness (mm)		$t_p = 8.0$	
Tension Yielding Capacity (kN)		$T_{dg} = \frac{l * t * f_y}{\gamma_{mo}}$ $= \frac{1 * 125 * 8.0 * 250}{1.1}$ $= 227.27$	
Tension Rupture Capacity (kN)		$T_{dn} = \frac{0.9 * A_n * f_u}{\gamma_{m1}}$ $= \frac{0.9 * (125 - 2 * 15.0) * 8.0 * 410}{1.25}$ $= 224.35$	
Block Shear Capacity (kN)		$T_{db1} = \frac{A_{vg} f_y}{\sqrt{3} \gamma_{m0}} + \frac{0.9 A_{tn} f_u}{\gamma_{m1}}$ $T_{db2} = \frac{0.9 * A_{vn} f_u}{\sqrt{3} \gamma_{m1}} + \frac{A_{tg} f_y}{\gamma_{m0}}$ $T_{db} = \min(T_{db1}, T_{db2}) = 380.65$	
Tension Capacity (kN)	$A = 200.0$	$T_d = \min(T_{dg}, T_{dn}, T_{db})$ $= \min(227.27, 224.35, 380.65)$ $= 224.35$	Pass

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3 3D View

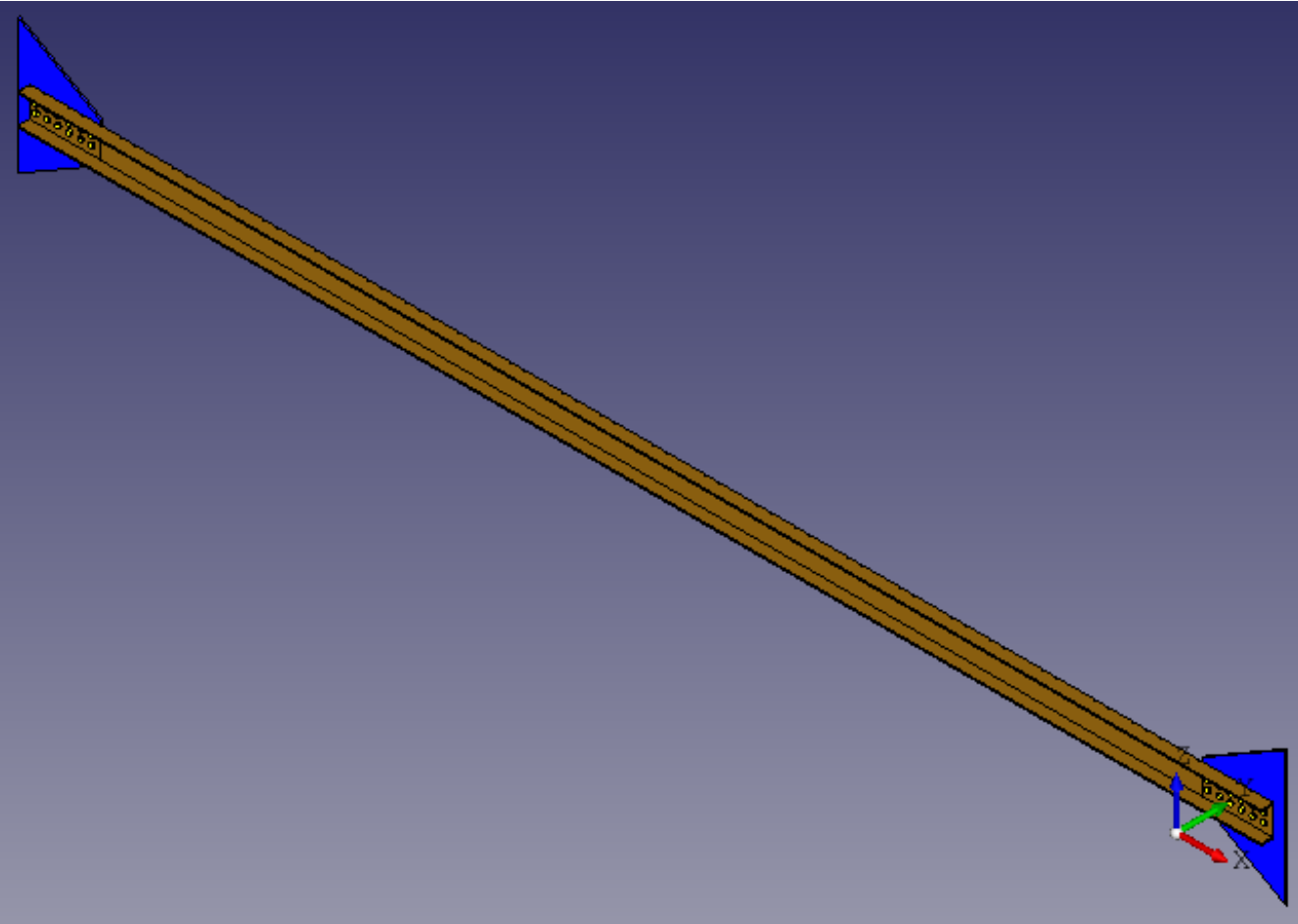


Figure 1: 3D View