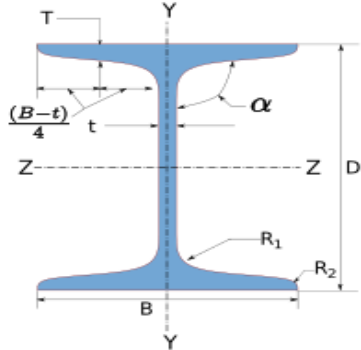


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## 1 Input Parameters

Module		Beam Coverplate Weld Connection		
MainModule		Moment Connection		
Moment(kNm)*		10.0		
Shear(kN)*		10.0		
Axial (kN) *		10.0		
Section				
	Beam Section *		NPB 270x135x30.7	
	Material *		E 250 (Fe 410 W)A	
	Ultimate strength, fu (MPa)		410	
	Yield Strength , fy (MPa)		250	
	Mass	30.73	Iz(mm4)	49173000.0
	Area(mm2) - A	3910.0	Iy(mm4)	3574200.0
	D(mm)	267.0	rz(mm)	112.10000000000001
	B(mm)	135.0	ry(mm)	30.2
	t(mm)	5.5	Zz(mm3)	368340.0
	T(mm)	8.7	Zy(mm3)	52950.0
	FlangeSlope	90	Zpz(mm3)	412490.0
	R1(mm)	1.5	Zpy(mm3)	52950.0
	R2(mm)	0.0		
Weld Details				
Weld Type		Fillet		
Type of weld fabrication		Shop Weld		
Material grade overwrite (MPa) Fu		410.0		

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## 2 Design Checks

### 2.1 Member Capacity

Check	Required	Provided	Remarks
Axial Capacity Ac (kN)		$Ac = \frac{A * f_y}{\gamma_{m0} * 1000}$ $= \frac{3910.0 * 250}{1.1 * 1000}$ $= 888.64$	
Shear Capacity Sc (kN)		$S_c = \frac{A_v * f_y}{\sqrt{3} * \gamma_{mo} * 1000}$ $= \frac{249.6 * 5.5 * 250}{\sqrt{3} * 1.1 * 1000}$ $= 180.13327999999998$	
Plastic Moment Capacity Pmc (kNm)		$Pmc = \frac{\beta_b * Z_p * f_y}{\gamma_{mo} * 1000000}$ $= \frac{1 * 85663 * 250}{1.1 * 1000000}$ $= 19.47$	
Moment Deformation Criteria Mdc (kNm)		$Mdc = \frac{1.5 * Z_e * f_y}{1.1}$ $= \frac{1.5 * 368340.0 * 250}{1.1}$ $= 125.57$	
Moment Capacity Mc (kNm)		$M_c = \min(Pmc, Mdc)$ $= \min(19.47, 125.57)$ $= 19.47$	

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## 2.2 Load Consideration

Check	Required	Provided	Remarks
Axial Load $A_u$ (kN)	$A_{c_{min}} = 0.3 * A_c$ $= 0.3 * 888.64$ $= 266.59$	$A_u = \max(A, A_{c_{min}})$ $= \max(10.0, 266.59)$ $= 266.59$	Pass
Shear Load $V_u$ (kN)	$S_{c_{min}} = 0.6 * A_c$ $= 0.6 * 180.13$ $= 108.08$	$V_u = \max(V, V_{c_{min}})$ $= \max(10.0, 108.08)$ $= 108.08$	Pass
Moment Load $M_u$ (kNm)	$M_{c_{min}} = 0.5 * M_c$ $= 0.5 * 19.47$ $= 9.73$	$M_u = \max(M, M_{c_{min}})$ $= \max(10.0, 9.73)$ $= 10.0$	Pass
Forces Carried by Web		$A_w = \text{Axial force in web}$ $= \frac{(D - 2 * T) * t * A_u}{A}$ $= \frac{(267.0 - 2 * 8.7) * 5.5 * 266.59}{3910.0}$ $= 93.6$ $M_w = \text{Moment in web}$ $= \frac{Z_w * M_u}{Z}$ $= \frac{85663 * 10.0}{412490.0}$ $= 2.08$	
Forces Carried by Flange		$A_f = \text{Axial force in flange}$ $= \frac{A_u * B * T}{A}$ $= \frac{266.59 * 135.0 * 8.7}{3910.0}$ $= 80.08$ $M_f = \text{Moment in flange}$ $= M_u - M_w$ $= 10.0 - 2.08$ $= 7.92$ $F_f = \text{flange force}$ $= \frac{M_f * 1000}{D - T} + A_f$ $= \frac{7.92}{267.0 - 8.7} + 80.08$ $= 110.75$	

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### 2.3 Weld Design Checks

Check	Required	Provided	Remarks
Min Weld Size (mm)	$\text{Thickness of Thicker part}$ $= \max(8.7, 14.0)$ $= 14.0$ <i>IS800 : 2007 cl.10.5.2.3 Table21,</i> $t_{w_{min}} = 5$	7	Pass
Max Weld Size (mm)	$\text{Thickness of Thinner part}$ $= \min(8.7, 14.0) = 8.7$ $t_{w_{max}} = 8.7$	7	Pass
Flange Weld Strength (N/mm)	$\text{Stress} = \frac{F_f * 1000}{F_{rl}}$ $= \frac{110.75 * 1000}{391}$ $= 283.2588987614436$	927.92	Pass

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## 2.4 Flange Plate Check

Check	Required	Provided	Remarks
Min. Plate Height (mm)	50	$b_{fp} = B - 2 * sp$ $= 135.0 - 2 * 15$ $= 105$	Pass
Max. Plate Height (mm)	$b_{fp} = B - 2 * sp$ $= 135.0 - 2 * 15$ $= 105$	105	Pass
Min. Plate Length (mm)	105	$l_{fp} = [2 * (l_w + 2 * s) + g]$ $= [2 * (1502 * 7) + 10.0]$ $= 340$	Pass

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3 3D View



Figure 1: 3D View