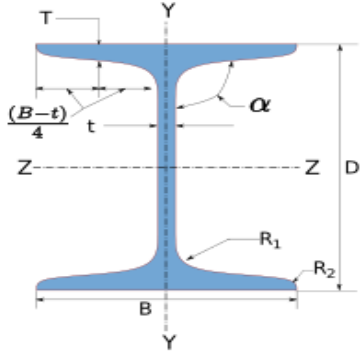


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## 1 Input Parameters

Module		Beam Coverplate Weld Connection		
MainModule		Moment Connection		
Moment(kNm)*		10.0		
Shear(kN)*		10.0		
Axial (kN) *		10.0		
Section				
	Beam Section *		NPB 750x270x146.9	
	Material *		E 250 (Fe 410 W)A	
	Ultimate strength, fu (MPa)		410	
	Yield Strength , fy (MPa)		250	
	Mass	146.87	Iz(mm4)	1645354000.0
	Area(mm2) - A	18710.0	Iy(mm4)	52878600.0
	D(mm)	750.0	rz(mm)	296.6
	B(mm)	265.0	ry(mm)	53.2
	t(mm)	13.2	Zz(mm3)	4387610.0
	T(mm)	17.0	Zy(mm3)	399080.0
	FlangeSlope	90	Zpz(mm3)	5081800.0
	R1(mm)	1.7	Zpy(mm3)	399080.0
	R2(mm)	0.0		
Weld Details				
Weld Type		Fillet		
Type of weld fabrication		Shop Weld		
Material grade overwrite (MPa) Fu		410.0		

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## 2 Design Checks

### 2.1 Member Capacity

Check	Required	Provided	Remarks
Axial Capacity Ac (kN)		$A_c = \frac{A * f_y}{\gamma_{m0} * 1000}$ $= \frac{18710.0 * 250}{1.1 * 1000}$ $= 4252.27$	
Shear Capacity Sc (kN)		$S_c = \frac{A_v * f_y}{\sqrt{3} * \gamma_{mo} * 1000}$ $= \frac{716.0 * 13.2 * 250}{\sqrt{3} * 1.1 * 1000}$ $= 1240.1483799999999$	
Plastic Moment Capacity Pmc (kNm)		$P_{mc} = \frac{\beta_b * Z_p * f_y}{\gamma_{mo} * 1000000}$ $= \frac{1 * 1691765 * 250}{1.1 * 1000000}$ $= 384.49$	
Moment Deformation Criteria Mdc (kNm)		$M_{dc} = \frac{1.5 * Z_e * f_y}{1.1}$ $= \frac{1.5 * 4387610.0 * 250}{1.1}$ $= 1495.78$	
Moment Capacity Mc (kNm)		$M_c = \min(P_{mc}, M_{dc})$ $= \min(384.49, 1495.78)$ $= 384.49$	

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## 2.2 Load Consideration

Check	Required	Provided	Remarks
Axial Load $A_u$ (kN)	$A_{c_{min}} = 0.3 * A_c$ $= 0.3 * 4252.27$ $= 1275.68$	$A_u = \max(A, A_{c_{min}})$ $= \max(10.0, 1275.68)$ $= 1275.68$	Pass
Shear Load $V_u$ (kN)	$S_{c_{min}} = 0.6 * A_c$ $= 0.6 * 1240.15$ $= 744.09$	$V_u = \max(V, V_{c_{min}})$ $= \max(10.0, 744.09)$ $= 744.09$	Pass
Moment Load $M_u$ (kNm)	$M_{c_{min}} = 0.5 * M_c$ $= 0.5 * 384.49$ $= 192.25$	$M_u = \max(M, M_{c_{min}})$ $= \max(10.0, 192.25)$ $= 192.25$	Pass
Forces Carried by Web		$A_w = \text{Axial force in web}$ $= \frac{(D - 2 * T) * t * A_u}{A}$ $= \frac{(750.0 - 2 * 17.0) * 13.2 * 1275.68}{18710.0}$ $= 644.4$ $M_w = \text{Moment in web}$ $= \frac{Z_w * M_u}{Z}$ $= \frac{1691765 * 192.25}{5081800.0}$ $= 64.0$	
Forces Carried by Flange		$A_f = \text{Axial force in flange}$ $= \frac{A_u * B * T}{A}$ $= \frac{1275.68 * 265.0 * 17.0}{18710.0}$ $= 307.16$ $M_f = \text{Moment in flange}$ $= M_u - M_w$ $= 192.25 - 64.0$ $= 128.25$ $F_f = \text{flange force}$ $= \frac{M_f * 1000}{D - T} + A_f$ $= \frac{128.25}{750.0 - 17.0} + 307.16$ $= 482.12$	

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### 2.3 Weld Design Checks

Check	Required	Provided	Remarks
Min Weld Size (mm)	$\text{Thickness of Thicker part}$ $= \max(17.0, 14.0)$ $= 17.0$ <i>IS800 : 2007 cl.10.5.2.3 Table21,</i> $t_{w_{min}} = 5$	12	Pass
Max Weld Size (mm)	$\text{Thickness of Thinner part}$ $= \min(17.0, 14.0) = 14.0$ $t_{w_{max}} = 14.0$	12	Pass
Flange Weld Strength (N/mm)	$\text{Stress} = \frac{F_f * 1000}{F_{rl}}$ $= \frac{482.12 * 1000}{760}$ $= 634.3678780249971$	1590.72	Pass

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## 2.4 Flange Plate Check-Outside/Inside

Check	Required	Provided	Remarks
Min. Plate Height (mm)	50	$b_{fp} = B - 2 * sp$ $= 265.0 - 2 * 17$ $= 230$	Pass
Min. Plate Length (mm)	230	$l_{fp} = [2 * (l_w + 2 * s) + g]$ $= [2 * (2752 * 12) + 10.0]$ $= 610$	Pass
Min. Inner Plate Height (mm)	50	$b_{ifp} = \frac{B - 4 * sp - t_w - 2 * r_1}{2}$ $= \frac{265.0 - 4 * 17 - 13.2 - 2 * 1.7}{2}$ $= 90$	Pass
Max. Inner Plate Height (mm)	$b_{ifp} = \frac{B - 4 * sp - t_w - 2 * r_1}{2}$ $= \frac{265.0 - 4 * 17 - 13.2 - 2 * 1.7}{2}$ $= 90$	90	Pass
Min. Inner Plate Length (mm)	230	$l_{fp} = [2 * (l_w + 2 * s) + g]$ $= [2 * (2752 * 12) + 10.0]$ $= 610$	Pass

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### 3 3D View



Figure 1: 3D View