

Group - 2

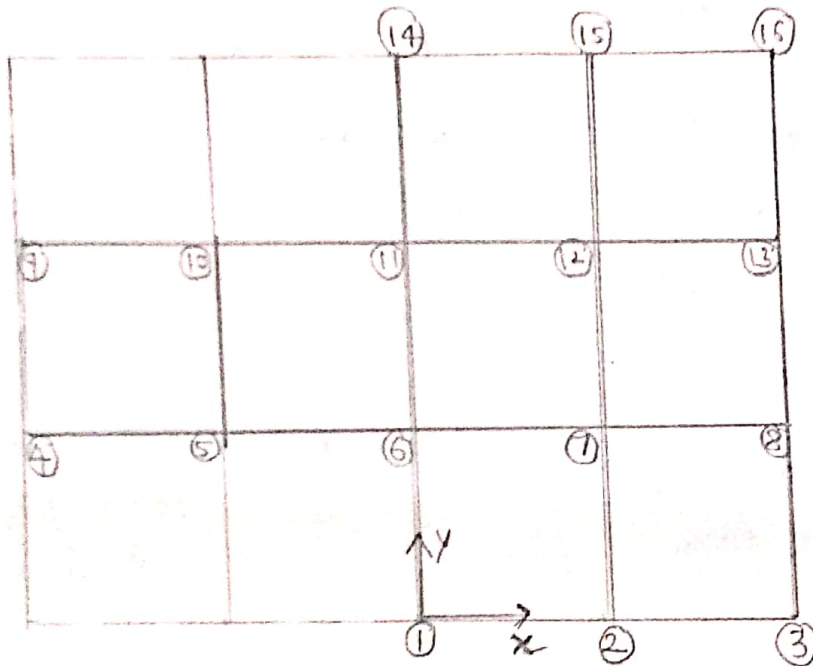
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MATLAB SUPPORTING FILE

Joint Equivalent load calculation

Joint numbering



Same way we will proceed for upper floor

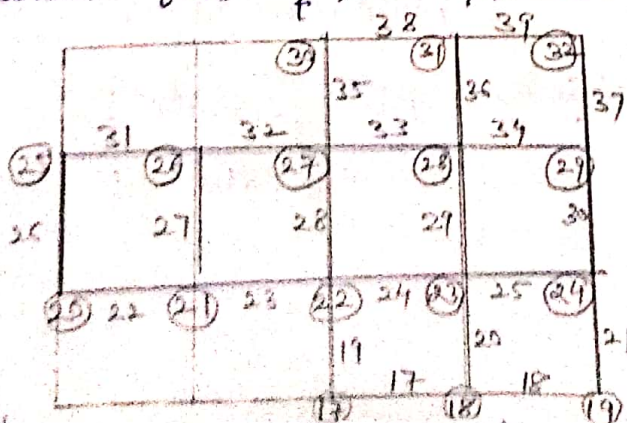
$$\text{Total no. of joints} = 16 \times 5 = 80$$

$$\text{Total no. of member} = 156$$

$$\text{Total no. of degree of freedom} = 80 \times 6 = 480$$

Numbering of members

Numbering start from bottom column at origin & no. all columns first & then proceed to beam.



Node no. & member
no. of ground floor
beam

Equivalent load for Joint

Sample for Node 17 { in figure provided }

$$V_1 = \text{along } x \text{ axis (Global)} = 1.125 \text{ KN} \left\{ EQ_x = \frac{\text{Load}}{\text{No. of members/joints}} = \frac{18}{16} = 1.125 \right.$$

$$V_2 = \text{along Global } y \text{ axis} = 1.125 \text{ KN} \quad EQ_y = EQ_u$$

$$\begin{aligned} V_3 &= \frac{wL}{2} (\text{of beam 17}) + \frac{wL}{2} (\text{of beam 19}) \\ &= 3.37 \text{ KN} + 5.76 \text{ KN} \\ &= 9.13 \text{ KN} \end{aligned}$$

$$M_3 = 0$$

$$M_1 = +wL^2/12 \text{ (along } x \text{ axis)}$$

$$M_1 = 2.68 \text{ KN}\cdot\text{m}$$

$$M_2 = -\frac{wL^2}{12} \text{ (-ve } y)$$

$$= -1.67 \text{ KN}\cdot\text{m}$$

Assumption

- i) No diaphragm constraint was considered
- ii) Value of E, G, J, I_x, I_y & coordinates of node were taken from SAP.

Important point

Name of file containing member parameters is
(MEMBERTABLE.XLSX ,

Name of file containing Joint parameters is
(JOINT.XLSX ,

Dead load slab

All in kN/m

A				
		2.464	4.328	2.464
B	2.0125	2.0125	4.312	4.312
	2.415	5.635	4.83	4.328
C	2.0125	2.0125	4.312	4.312
		2.464	4.328	2.464
D				

Live loads

ON Roof

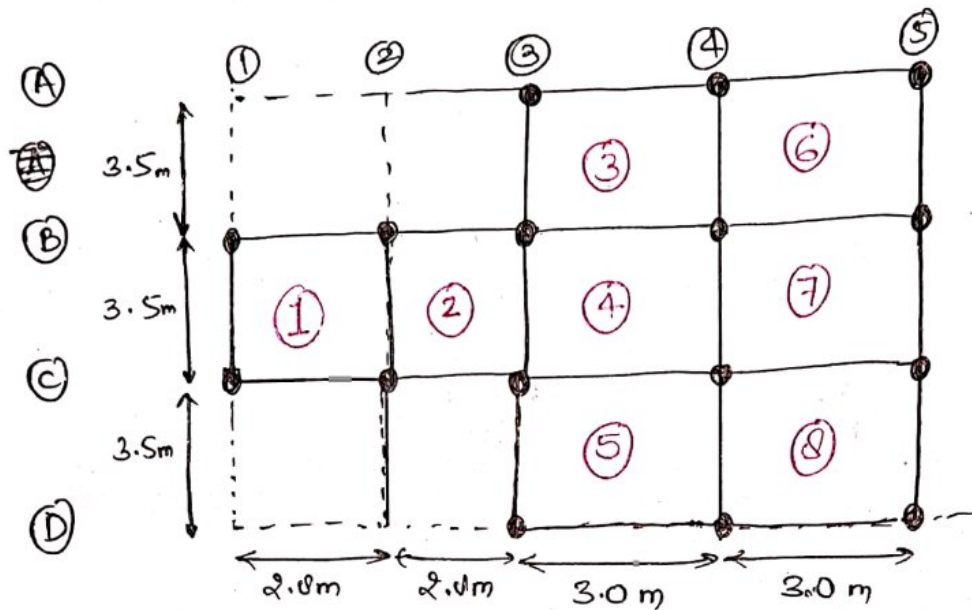
		1.125	1.125	
				1.2857
	1.05	1.05		2.5714
			2.25	2.25
1.26		2.52	2.54	2.57
	1.05	1.05	2.25	2.25
			1.28	2.57
				1.28
		1.125	1.125	

On floor

		2.25	2.25	
		2.57	5.14	2.57
	2.1	2.1	4.5	4.5
	2.52	5.04	5.07	5.14
	2.1	2.1	4.5	4.5
			2.57	5.14
				2.57
		2.25	2.25	

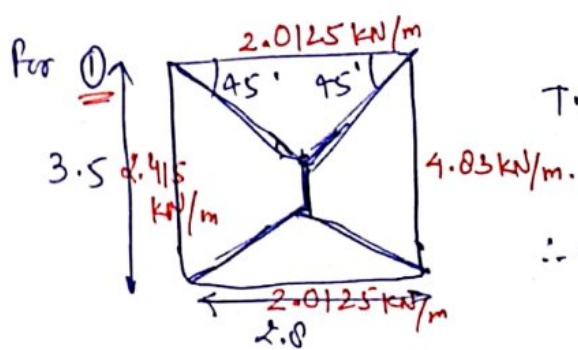
2.0125 kN/m

LOAD CALCULATIONS BY 'TRIBUTARY METHOD'



Tributary Area is a loaded area that contributes to the (T.A.) load on the member supporting that area.

eg: The area from the centre b/w 2 beams to the centre of the next two beams for the full span is the load on the centre beam.



$$T.A. \text{ of } \Delta = \frac{1}{2} \times 2.8 \times 1.4 = 1.96 \text{ m}^2$$

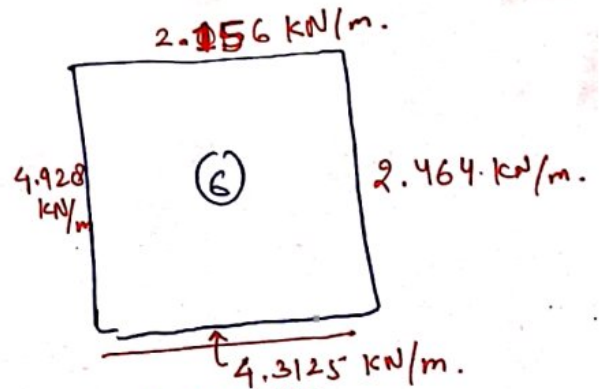
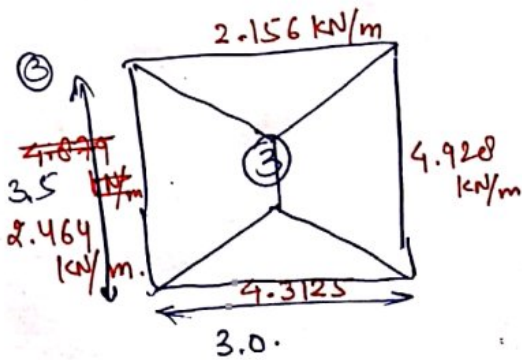
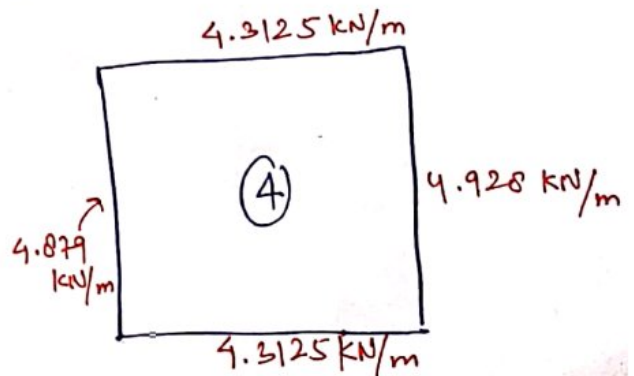
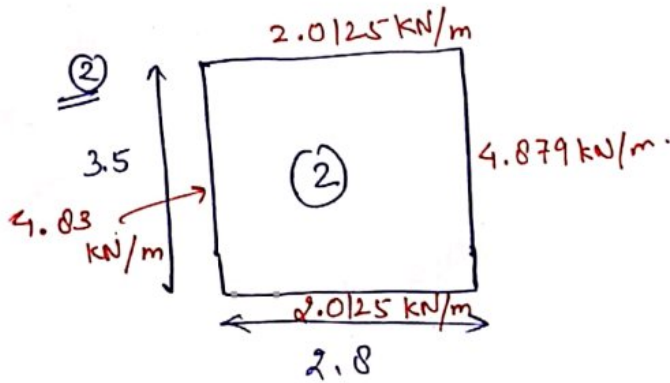
$$\therefore udl = \frac{25 \times 1.96 \times 0.115}{2.8} \rightarrow \text{thickness of slab.}$$

$$= 2.0125 \text{ kN/m}$$

$$T.A. \text{ of } \Delta = \frac{1}{2} \times (0.7 + 3.5) \times 1.4 = 2.94$$

Using symmetry:

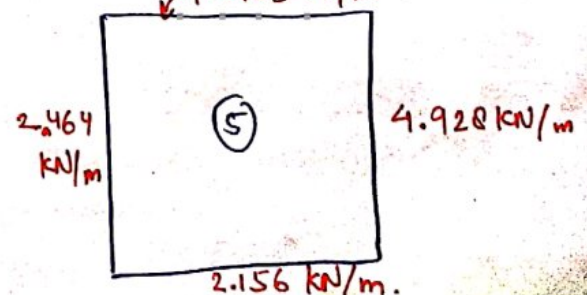
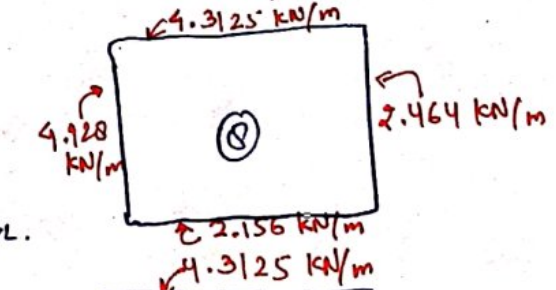
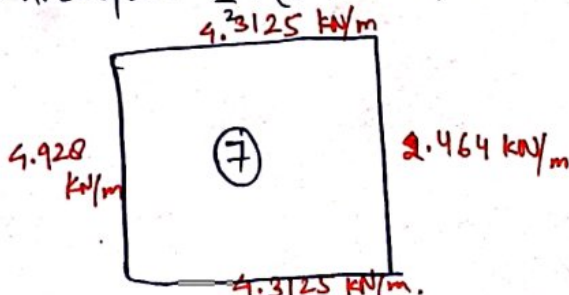
$$\therefore udl = \frac{25 \times 2.94 \times 0.115}{3.5} = 2.415 \text{ kN/m}$$



$$T.A. = \frac{1}{2} \times 1.5 \times 3 = 2.25$$

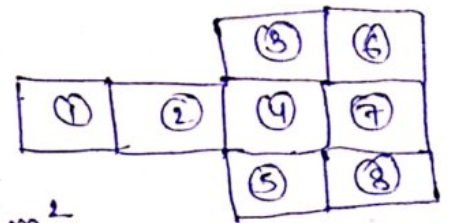
$$udl = \frac{25 \times 2.25 \times 0.115}{3} = 2.156$$

$$T. \text{ Area of } \Delta = \frac{1}{2} \times (0.5 + 3.5) \times 1.5 = 3 \text{ m}^2$$



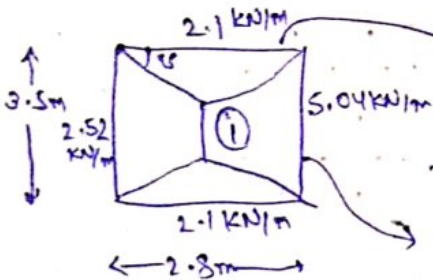
Load calculation for Live Load

For every floor except the roof



For 1

$$TA \text{ of } \Delta = \frac{1}{2} \times 2.8 \times 1.4 = 1.96 \text{ m}^2$$



$$UDL = \frac{(1.96 \times 3 \text{ kPa})}{2.8 \text{ (m)}} = 2.1 \text{ kN/m}$$

$$TA \text{ of } \triangle = \frac{1}{2} \times (0.7 + 3.5) \times 1.4 = 2.94 \text{ m}^2$$

$$UDL = \frac{(2 \times 2.94) \text{ (m}^2) \times 3 \text{ (kPa)}}{3.5 \text{ (m)}} = 5.04 \text{ kN/m}$$

Using Symmetry

