

# Secondary Beams

Beam ID	Start Point	End Point	Span	Mmax	Vmax
20	(18,4,3)	(18,8,3)	4	4.65376	4.65376
19	(18,0,3)	(18,4,3)	4	4.65376	4.65376
2	(0,4,3)	(0,8,3)	4	4.65376	4.65376
1	(0,0,3)	(0,4,3)	4	4.65376	4.65376

## Design Limit state:

Combo: 1.2D+1.4L

Md: 4.65376 t.m

Vd: 4.65376 ton

## Service Limit State

Combo: LIVE

Span: 4 m

Load: -1 t/m'

## Design Checks

### 1-Check Local Buckling

$dw/tw = 32.39 < 81.97814749472366 \Rightarrow$  Compact Web

$c/tf = 4.81 < 10.908903091817557 \Rightarrow$  Compact Flange

### 2-Check Lateral Torsional Buckling

$Lu_{act} = 0 \text{ m} < Lu_{max} = 154.91933384829667 \text{ m} \Rightarrow$  Supported (No LTB)

### 3-Check Bending Stress

Section: IPE 240

$f_{act} = 1.4363456790123459 \text{ t/cm}^2 < F_b = 1.536 \text{ t/cm}^2$

### 4-Check Shear Stress

$q_{act} = 0.31275268817204305 \text{ t/cm}^2 < q_{all} = 0.84 \text{ t/cm}^2$

### 5-Check Deflection

$d_{act} = 0.4080466805402538 \text{ cm} < d_{all} = 1.3333333333333333 \text{ cm}$

Beam ID	Start Point	End Point	Span	Mmax	Vmax
16	(14,4,3)	(14,8,3)	4	9.25376	9.25376
15	(14,0,3)	(14,4,3)	4	9.25376	9.25376

14	(12,4,3)	(12,8,3)	4	9.25376	9.25376
13	(12,0,3)	(12,4,3)	4	9.25376	9.25376
12	(10,4,3)	(10,8,3)	4	9.25376	9.25376
11	(10,0,3)	(10,4,3)	4	9.25376	9.25376
17	(16,0,3)	(16,4,3)	4	9.25376	9.25376
9	(8,0,3)	(8,4,3)	4	9.25376	9.25376
8	(6,4,3)	(6,8,3)	4	9.25376	9.25376
7	(6,0,3)	(6,4,3)	4	9.25376	9.25376
6	(4,4,3)	(4,8,3)	4	9.25376	9.25376
5	(4,0,3)	(4,4,3)	4	9.25376	9.25376
4	(2,4,3)	(2,8,3)	4	9.25376	9.25376
3	(2,0,3)	(2,4,3)	4	9.25376	9.25376
18	(16,4,3)	(16,8,3)	4	9.25376	9.25376
10	(8,4,3)	(8,8,3)	4	9.25376	9.25376

#### Design Limit state:

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Combo: 1.2D+1.4L

Md: 9.25376 t.m

Vd: 9.25376 ton

#### Service Limit State

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Combo: LIVE

Span: 4 m

Load: -2 t/m'

#### Design Checks

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##### 1-Check Local Buckling

$dw/tw = 37.87 < 81.97814749472366 \Rightarrow$  Compact Web

$c/tf = 5.64 < 10.908903091817557 \Rightarrow$  Compact Flange

##### 2-Check Lateral Torsional Buckling

$Lu_{act} = 0 \text{ m} < Lu_{max} = 206.55911179772892 \text{ m} \Rightarrow$  Supported (No LTB)

##### 3-Check Bending Stress

Section: IPE 330

$fact = 1.2978625525946703 \text{ t/cm}^2 < F_b = 1.536 \text{ t/cm}^2$

#### 4-Check Shear Stress

$q_{act} = 0.3738892929292929 \text{ t/cm}^2 < q_{all} = 0.84 \text{ t/cm}^2$

#### 5-Check Deflection

$d_{act} = 0.2697198958881202 \text{ cm} < d_{all} = 1.3333333333333333 \text{ cm}$

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## Main Beams

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Beam ID	Start Point	End Point	Span	Mmax	Vmax
9	(12,8,3)	(18,8,3)	6	18.70246	9.38372
8	(6,8,3)	(12,8,3)	6	18.70246	9.38372
7	(0,8,3)	(6,8,3)	6	18.70246	9.38372
3	(12,0,3)	(18,0,3)	6	18.70246	9.38372
2	(6,0,3)	(12,0,3)	6	18.70246	9.38372
1	(0,0,3)	(6,0,3)	6	18.70246	9.38372

#### Design Limit state:

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Combo: 1.2D+1.4L

Md: 18.70246 t.m

Vd: 9.38372 ton

#### Service Limit State

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Combo: LIVE

Span: 6 m

Load: -1.3333333333333333 t/m'

#### Design Checks

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##### 1-Check Local Buckling

$d_w/t_w = 41.66 < 81.97814749472366 \Rightarrow$  Compact Web

$c/t_f = 5.19 < 10.908903091817557 \Rightarrow$  Compact Flange

##### 2-Check Lateral Torsional Buckling

$L_{uact} = 0 \text{ m} < L_{umax} = 245.28894525980309 \text{ m} \Rightarrow$  Supported (No LTB)

##### 3-Check Bending Stress

Section: IPE 450

$f_{act} = 1.2468306666666666 \text{ t/cm}^2 < F_b = 1.536 \text{ t/cm}^2$

##### 4-Check Shear Stress

$q_{act} = 0.2218373522458629 \text{ t/cm}^2 < q_{all} = 0.84 \text{ t/cm}^2$

#### 5-Check Deflection

$d_{act} = 0.31755440765517823 \text{ cm} < d_{all} = 2 \text{ cm}$

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Beam ID	Start Point	End Point	Span	Mmax	Vmax
6	(12,4,3)	(18,4,3)	6	37.209979999999995	18.63748
5	(6,4,3)	(12,4,3)	6	37.209979999999995	18.63748
4	(0,4,3)	(6,4,3)	6	37.209979999999995	18.63748

#### Design Limit state:

Combo: 1.2D+1.4L

Md: 37.209979999999995 t.m

Vd: 18.63748 ton

#### Service Limit State

Combo: LIVE

Span: 6 m

Load: -2.6666666666666665 t/m'

#### Design Checks

##### 1-Check Local Buckling

$d_w/t_w = 43.36 < 81.97814749472366 \Rightarrow$  Compact Web

$c/t_f = 4.79 < 10.908903091817557 \Rightarrow$  Compact Flange

##### 2-Check Lateral Torsional Buckling

$L_{uact} = 0 \text{ m} < L_{umax} = 271.1088342345192 \text{ m} \Rightarrow$  Supported (No LTB)

##### 3-Check Bending Stress

Section: IPE 550

$f_{act} = 1.5249991803278686 \text{ t/cm}^2 < F_b = 1.536 \text{ t/cm}^2$

##### 4-Check Shear Stress

$q_{act} = 0.3052822276822277 \text{ t/cm}^2 < q_{all} = 0.84 \text{ t/cm}^2$

##### 5-Check Deflection

$d_{act} = 0.31925761961518817 \text{ cm} < d_{all} = 2 \text{ cm}$

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