

Secondary Beams

Beam ID	Start Point	End Point	Span	Mmax	Vmax
4	(6,0,3)	(6,5,3)	5	11.959000000000001	9.5672
3	(4,0,3)	(4,5,3)	5	11.959000000000001	9.5672
2	(2,0,3)	(2,5,3)	5	11.959000000000001	9.5672
1	(0,0,3)	(0,5,3)	5	11.959000000000001	9.5672

Design Limit state:

Combo: 1.2D+1.4L

Md: 11.959000000000001 t.m

Vd: 9.5672 ton

Service Limit State

Combo: LIVE

Span: 5 m

Load: -1 t/m'

Design Checks

1-Check Local Buckling

$dw/tw = 38.65 < 81.97814749472366 \Rightarrow$ Compact Web

$c/tf = 5.38 < 10.908903091817557 \Rightarrow$ Compact Flange

2-Check Lateral Torsional Buckling

$Lu_{act} = 0 \text{ m} < Lu_{max} = 219.46905628508696 \text{ m} \Rightarrow$ Supported (No LTB)

3-Check Bending Stress

Section: IPE 360

$f_{act} = 1.3228982300884957 \text{ t/cm}^2 < F_b = 1.536 \text{ t/cm}^2$

4-Check Shear Stress

$q_{act} = 0.3321944444444444 \text{ t/cm}^2 < q_{all} = 0.84 \text{ t/cm}^2$

5-Check Deflection

$d_{act} = 0.23818365186681106 \text{ cm} < d_{all} = 1.6666666666666667 \text{ cm}$

Main Beams

Beam ID	Start Point	End Point	Span	Mmax	Vmax
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2	(0,5,3)	(6,5,3)	6	19.3293400000000002	9.6971600000000002
1	(0,0,3)	(6,0,3)	6	19.3293400000000002	9.6971600000000002

Design Limit state:

Combo: 1.2D+1.4L

Md: 19.3293400000000002 t.m

Vd: 9.6971600000000002 ton

Service Limit State

Combo: LIVE

Span: 6 m

Load: -0.8333333333333334 t/m'

Design Checks

1-Check Local Buckling

$dw/tw = 41.66 < 81.97814749472366 \Rightarrow$ Compact Web

$c/tf = 5.19 < 10.908903091817557 \Rightarrow$ Compact Flange

2-Check Lateral Torsional Buckling

$Lu_{act} = 0 \text{ m} < Lu_{max} = 245.28894525980309 \text{ m} \Rightarrow$ Supported (No LTB)

3-Check Bending Stress

Section: IPE 450

$f_{act} = 1.2886226666666667 \text{ t/cm}^2 < F_b = 1.536 \text{ t/cm}^2$

4-Check Shear Stress

$q_{act} = 0.22924728132387714 \text{ t/cm}^2 < q_{all} = 0.84 \text{ t/cm}^2$

5-Check Deflection

$d_{act} = 0.1984715047844864 \text{ cm} < d_{all} = 2 \text{ cm}$
