

# Secondary Beams

Beam ID	Start Point	End Point	Span	Mmax	Vmax
22	(6,4,6)	(6,8,6)	4	2.0537599999999996	2.0537599999999996
21	(6,0,6)	(6,4,6)	4	2.0537599999999996	2.0537599999999996
2	(0,4,6)	(0,8,6)	4	2.0537599999999996	2.0537599999999996
1	(0,0,6)	(0,4,6)	4	2.0537599999999996	2.0537599999999996

## Design Limit state:

Combo: 1.2D+1.4L

Md: 2.0537599999999996 t.m

Vd: 2.0537599999999996 ton

## Service Limit State

Combo: LIVE

Span: 4 m

Load: -0.5 t/m'

## Design Checks

### 1-Check Local Buckling

$dw/tw = 29.65 < 81.97814749472366 \Rightarrow$  Compact Web

$c/tf = 4.56 < 10.908903091817557 \Rightarrow$  Compact Flange

### 2-Check Lateral Torsional Buckling

$Lu_{act} = 0 \text{ m} < Lu_{max} = 129.09944487358058 \text{ m} \Rightarrow$  Supported (No LTB)

### 3-Check Bending Stress

Section: IPE 200

$fact = 1.0586391752577318 \text{ t/cm}^2 < F_b = 1.536 \text{ t/cm}^2$

### 4-Check Shear Stress

$q_{act} = 0.18337142857142852 \text{ t/cm}^2 < q_{all} = 0.84 \text{ t/cm}^2$

### 5-Check Deflection

$d_{act} = 0.40909834724267713 \text{ cm} < d_{all} = 1.3333333333333333 \text{ cm}$

Beam ID	Start Point	End Point	Span	Mmax	Vmax
18	(16,4,6)	(16,8,6)	4	4.05376	4.05376
17	(16,0,6)	(16,4,6)	4	4.05376	4.05376

20	(18,4,6)	(18,8,6)	4	4.05376	4.05376
3	(2,0,6)	(2,4,6)	4	4.05376	4.05376
4	(2,4,6)	(2,8,6)	4	4.05376	4.05376
5	(4,0,6)	(4,4,6)	4	4.05376	4.05376
6	(4,4,6)	(4,8,6)	4	4.05376	4.05376
7	(6,0,6)	(6,4,6)	4	4.05376	4.05376
8	(6,4,6)	(6,8,6)	4	4.05376	4.05376
9	(8,0,6)	(8,4,6)	4	4.05376	4.05376
10	(8,4,6)	(8,8,6)	4	4.05376	4.05376
19	(18,0,6)	(18,4,6)	4	4.05376	4.05376
12	(10,4,6)	(10,8,6)	4	4.05376	4.05376
13	(12,0,6)	(12,4,6)	4	4.05376	4.05376
14	(12,4,6)	(12,8,6)	4	4.05376	4.05376
15	(14,0,6)	(14,4,6)	4	4.05376	4.05376
16	(14,4,6)	(14,8,6)	4	4.05376	4.05376
11	(10,0,6)	(10,4,6)	4	4.05376	4.05376

#### Design Limit state:

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Combo: 1.2D+1.4L

Md: 4.05376 t.m

Vd: 4.05376 ton

#### Service Limit State

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Combo: LIVE

Span: 4 m

Load: -1 t/m'

#### Design Checks

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##### 1-Check Local Buckling

$dw/tw = 32.39 < 81.97814749472366 \Rightarrow$  Compact Web

$c/tf = 4.81 < 10.908903091817557 \Rightarrow$  Compact Flange

##### 2-Check Lateral Torsional Buckling

$Lu_{act} = 0 \text{ m} < Lu_{max} = 154.91933384829667 \text{ m} \Rightarrow$  Supported (No LTB)

##### 3-Check Bending Stress

Section: IPE 240

fact= 1.2511604938271603 t/cm<sup>2</sup> < Fb= 1.536 t/cm<sup>2</sup>

#### 4-Check Shear Stress

qact= 0.27243010752688174 t/cm<sup>2</sup> < qall= 0.84 t/cm<sup>2</sup>

#### 5-Check Deflection

dact= 0.4080466805402538 cm < dall= 1.3333333333333333 cm

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## Main Beams

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Beam ID	Start Point	End Point	Span	Mmax	Vmax
9	(14,8,6)	(20,8,6)	6	8.30246	4.18372
7	(0,8,6)	(6,8,6)	6	8.30246	4.18372
3	(14,0,6)	(20,0,6)	6	8.30246	4.18372
1	(0,0,6)	(6,0,6)	6	8.30246	4.18372

#### Design Limit state:

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Combo: 1.2D+1.4L

Md: 8.30246 t.m

Vd: 4.18372 ton

#### Service Limit State

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Combo: LIVE

Span: 6 m

Load: -0.6666666666666666 t/m'

#### Design Checks

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##### 1-Check Local Buckling

dw/tw= 36.23 < 81.97814749472366 => Compact Web

c/tf= 5.68 < 10.908903091817557 => Compact Flange

##### 2-Check Lateral Torsional Buckling

Luact= 0 m < Lumax= 193.64916731037084 m => Supported (No LTB)

##### 3-Check Bending Stress

Section: IPE 300

fact= 1.4905673249551166 t/cm<sup>2</sup> < Fb= 1.536 t/cm<sup>2</sup>

##### 4-Check Shear Stress

qact= 0.19641877934272303 t/cm<sup>2</sup> < qall= 0.84 t/cm<sup>2</sup>

### 5-Check Deflection

dact= 0.6408065618591935 cm < dall= 2 cm

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Beam ID	Start Point	End Point	Span	Mmax	Vmax
6	(14,4,6)	(20,4,6)	6	16.409979999999997	8.23748
4	(0,4,6)	(6,4,6)	6	16.409979999999997	8.23748
8	(6,8,6)	(14,8,6)	8	16.5616	6.253919999999999
2	(6,0,6)	(14,0,6)	8	16.5616	6.253919999999999

### Design Limit state:

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Combo: 1.2D+1.4L

Md: 16.5616 t.m

Vd: 6.253919999999999 ton

### Service Limit State

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Combo: LIVE

Span: 8 m

Load: -0.75 t/m'

### Design Checks

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#### 1-Check Local Buckling

dw/tw= 40.24 < 81.97814749472366 => Compact Web

c/tf= 5.35 < 10.908903091817557 => Compact Flange

#### 2-Check Lateral Torsional Buckling

Luact= 0 m < Lumax= 232.379000772445 m => Supported (No LTB)

#### 3-Check Bending Stress

Section: IPE 400

fact= 1.4277241379310344 t/cm<sup>2</sup> < Fb= 1.536 t/cm<sup>2</sup>

#### 4-Check Shear Stress

qact= 0.1818 t/cm<sup>2</sup> < qall= 0.84 t/cm<sup>2</sup>

#### 5-Check Deflection

dact= 0.8235027690280609 cm < dall= 2.6666666666666665 cm

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Beam ID	Start Point	End Point	Span	Mmax	Vmax
5	(6,4,6)	(14,4,6)	8	32.77664	12.33456

## Design Limit state:

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Combo: 1.2D+1.4L

Md: 32.77664 t.m

Vd: 12.33456 ton

## Service Limit State

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Combo: LIVE

Span: 8 m

Load: -1.5 t/m'

## Design Checks

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### 1-Check Local Buckling

$dw/tw = 43.36 < 81.97814749472366 \Rightarrow$  Compact Web

$c/tf = 4.79 < 10.908903091817557 \Rightarrow$  Compact Flange

### 2-Check Lateral Torsional Buckling

$Lu_{act} = 0 \text{ m} < Lu_{max} = 271.1088342345192 \text{ m} \Rightarrow$  Supported (No LTB)

### 3-Check Bending Stress

Section: IPE 550

$f_{act} = 1.343304918032787 \text{ t/cm}^2 < F_b = 1.536 \text{ t/cm}^2$

### 4-Check Shear Stress

$q_{act} = 0.20204029484029482 \text{ t/cm}^2 < q_{all} = 0.84 \text{ t/cm}^2$

### 5-Check Deflection

$d_{act} = 0.5675691015381122 \text{ cm} < d_{all} = 2.6666666666666665 \text{ cm}$

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