

Secondary Beams

Beam ID	Start Point	End Point	Span	Mmax	Vmax
0	(6,3,0)	(6,3,5)	5	5.53875	4.431
0	(0,3,0)	(0,3,5)	5	5.53875	4.431

Design Limit state:

Combo: D+L

Md: 5.53875 t.m

Vd: 4.431 ton

Service Limit State

Combo: LIVE

Span: 5 m

Load: -1 t/m'

Design Checks

1-Check Local Buckling

$dw/tw = 34.73 < 81.97814749472366 \Rightarrow$ Compact Web

$c/tf = 5.3 < 10.908903091817557 \Rightarrow$ Comapct Flange

2-Check Lateral Torsional Buckling

$Lu_{act} = 0 \text{ m} < Lu_{max} = 174.28425057933376 \text{ m} \Rightarrow$ Supported (No LTB)

3-Check Bending Stress

Section: IPE 270

$\sigma_{act} = 1.291083916083916 \text{ t/cm}^2 < \sigma_b = 1.536 \text{ t/cm}^2$

4-Check Shear Stress

$\tau_{act} = 0.24865319865319865 \text{ t/cm}^2 < \tau_{all} = 0.84 \text{ t/cm}^2$

5-Check Deflection

$\delta_{act} = 0.6693001754530252 \text{ cm} < \delta_{all} = 1.6666666666666667 \text{ cm}$

Beam ID	Start Point	End Point	Span	Mmax	Vmax
0	(4,3,0)	(4,3,5)	5	11.0075	8.806000000000001
0	(2,3,0)	(2,3,5)	5	11.0075	8.806000000000001

Design Limit state:

Combo: D+L

Md: 11.0075 t.m

Vd: 8.806000000000001 ton

Service Limit State

Combo: LIVE

Span: 5 m

Load: -2 t/m'

Design Checks

1-Check Local Buckling

$dw/tw = 38.65 < 81.97814749472366 \Rightarrow$ Compact Web

$c/tf = 5.38 < 10.908903091817557 \Rightarrow$ Comapct Flange

2-Check Lateral Torsional Buckling

$Luact = 0 \text{ m} < Lumax = 219.46905628508696 \text{ m} \Rightarrow$ Supported (No LTB)

3-Check Bending Stress

Section: IPE 360

$fact = 1.2176438053097345 \text{ t/cm}^2 < Fb = 1.536 \text{ t/cm}^2$

4-Check Shear Stress

$qact = 0.3057638888888889 \text{ t/cm}^2 < qall = 0.84 \text{ t/cm}^2$

5-Check Deflection

$dact = 0.4763673037336221 \text{ cm} < dall = 1.666666666666667 \text{ cm}$

Main Beams

Beam ID	Start Point	End Point	Span	Mmax	Vmax
0	(0,3,5)	(6,3,5)	6	21.27445	10.6643
0	(0,3,0)	(6,3,0)	6	21.27445	10.6643

Design Limit state:

Combo: D+L

Md: 21.27445 t.m

Vd: 10.6643 ton

Service Limit State

Combo: LIVE

Span: 6 m

Load: -2 t/m'

Design Checks

1-Check Local Buckling

$d_w/t_w = 41.66 < 81.97814749472366 \Rightarrow$ Compact Web

$c/t_f = 5.19 < 10.908903091817557 \Rightarrow$ Compact Flange

2-Check Lateral Torsional Buckling

$L_{uact} = 0 \text{ m} < L_{umax} = 245.28894525980309 \text{ m} \Rightarrow$ Supported (No LTB)

3-Check Bending Stress

Section: IPE 450

$f_{act} = 1.4182966666666668 \text{ t/cm}^2 < F_b = 1.536 \text{ t/cm}^2$

4-Check Shear Stress

$q_{act} = 0.2521111111111111 \text{ t/cm}^2 < q_{all} = 0.84 \text{ t/cm}^2$

5-Check Deflection

$d_{act} = 0.47633161148276737 \text{ cm} < d_{all} = 2 \text{ cm}$
