

# 2015-AE-40-52

EE24BTECH11003 - Akshara Sarma Chennubhatla

- 1) The directional derivative of the field  $u(x, y, z) = x^2 - 3yz$  in the direction of the vector  $(\mathbf{i} + \mathbf{j} - 2\mathbf{k})$  at point  $(2, -1, 4)$  is \_\_\_\_\_
- 2) The composition of an air-entrained concrete is given below:

Water :  $184\text{kg}/\text{m}^3$

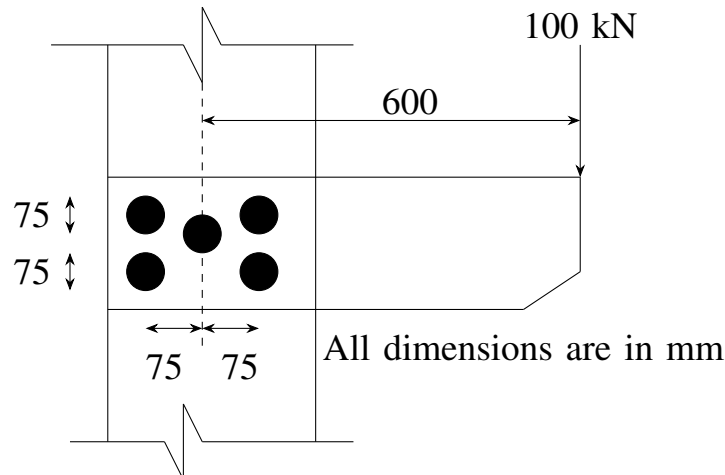
Ordinary Portland Cement (OPC) :  $368\text{kg}/\text{m}^3$

Sand :  $606\text{kg}/\text{m}^3$

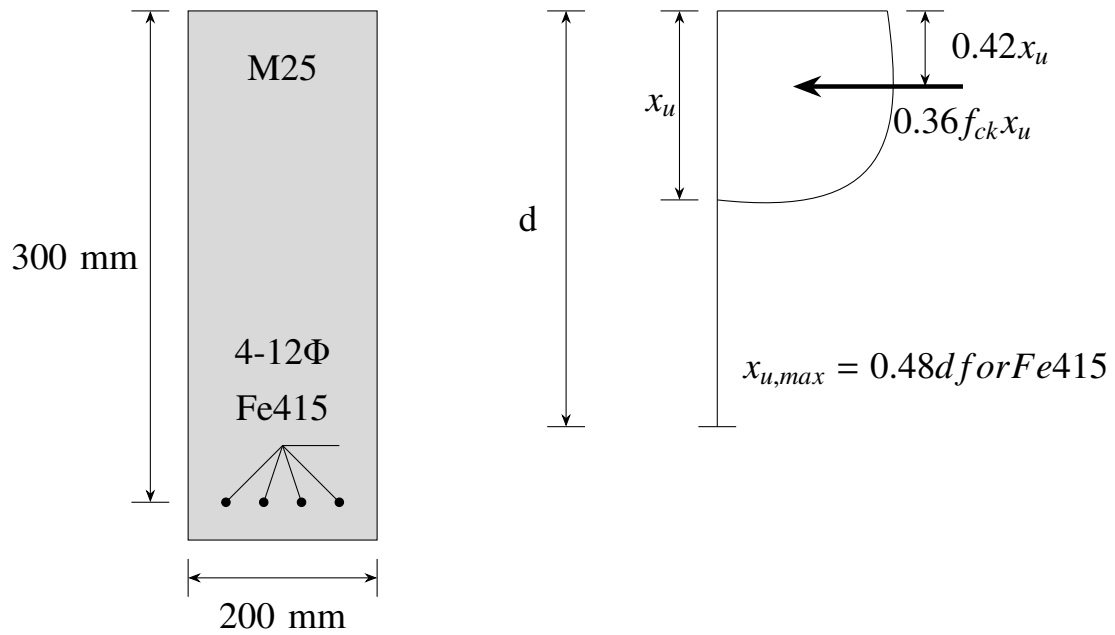
Coarse aggregate :  $1155\text{kg}/\text{m}^3$

Assume the specific gravity of OPC, sand and coarse aggregate to be 3.14, 2.67 and 2.74, respectively. The air content is \_\_\_\_\_ litres/ $\text{m}^3$ .

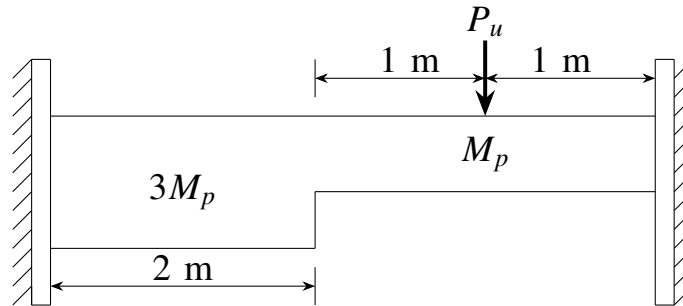
- 3) A bracket plate connected to a column flange transmits a load of 100 kN as shown in the following figure. The maximum force for which the bolts should be designed is \_\_\_\_\_ kN.



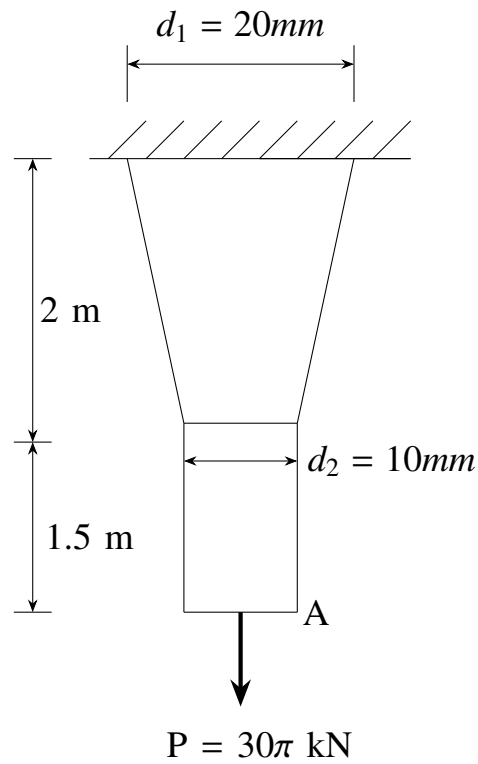
- 4) Consider the singly reinforced beam section given below (left figure). The stress block parameters for the cross-section from IS:456 – 2000 are also given below (right figure). The moment of resistance for the given section by the limit state method is \_\_\_\_\_ kN-m.



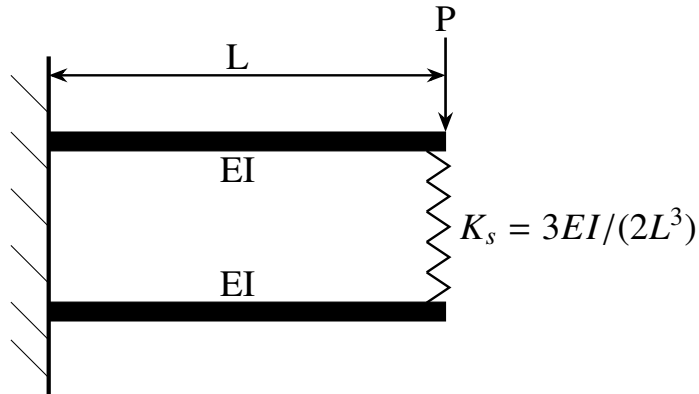
- 5) For formation of collapse mechanism in the following figure, the minimum value of  $P_u$  is  $xM_p/L$ .  $M_p$  and  $3M_p$  denote the plastic moment capacities of beam section as shown in this figure. The value of  $c$  is \_\_\_\_\_.



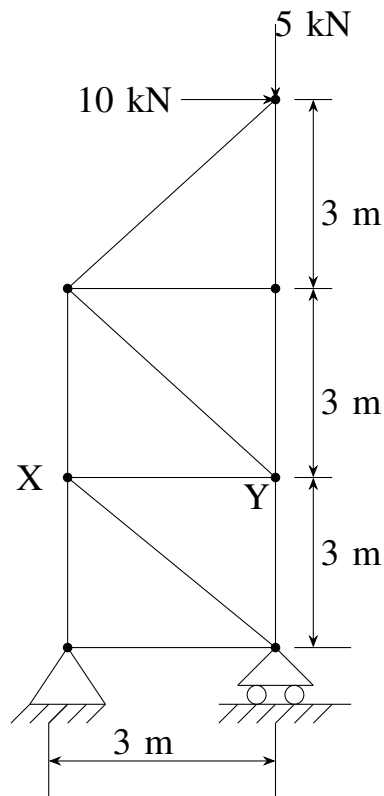
- 6) A tapered circular rod of diameter varying from 20 mm to 10 mm is connected to another uniform circular rod of diameter 10 mm as shown in the figure. Both bars are made of same material with the modulus of elasticity,  $E = 2 \times 10^5$  MPa. When subjected to a load  $P = 30\pi$  kN, the deflection at point A is \_\_\_\_\_ mm.



- 7) Two beams are connected by a linear spring as shown in the following figure. For a load  $P$  as shown in the figure, the percentage of the applied load  $P$  carried by the spring is \_\_\_\_\_.

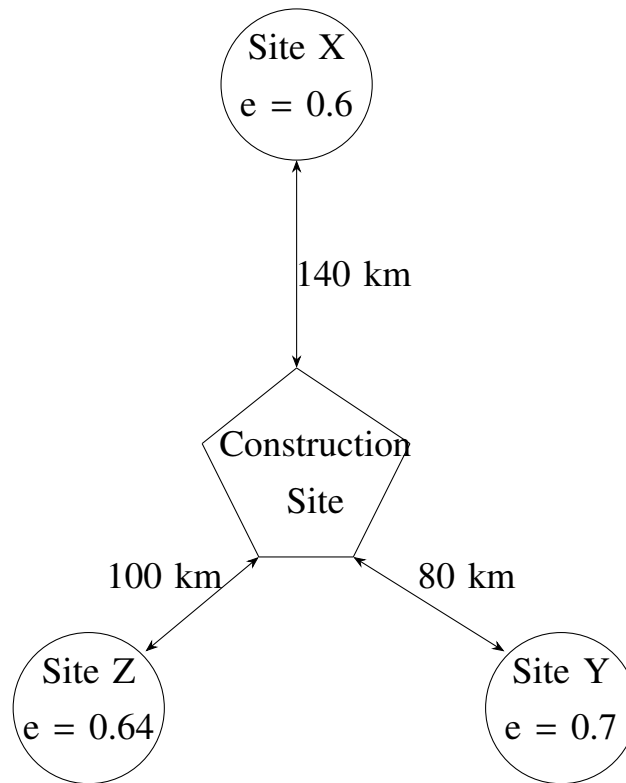


- 8) For the 2D truss with the applied loads shown below, the strain energy in the member  $XY$  is \_\_\_\_\_ kN-m. For the member  $XY$ , assume  $AE = 30 \text{ kN}$ , where  $A$  is cross-section area and  $E$  is the modulus of elasticity.

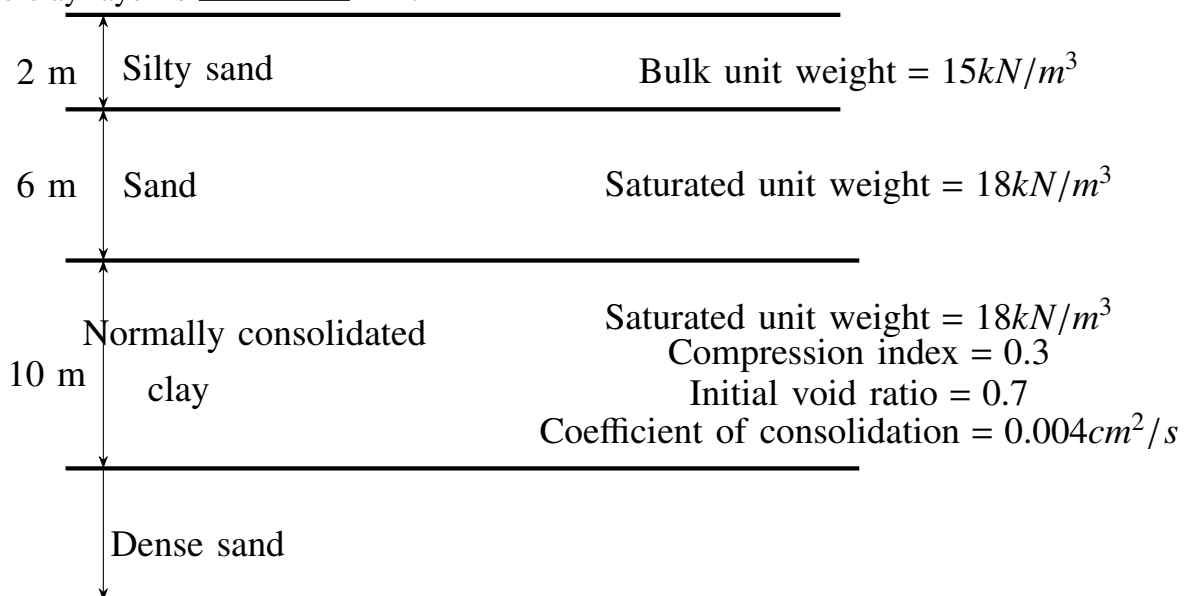


- 9) An earth embankment is to be constructed with compacted cohesionless soil. The volume of the embankment is  $5000\text{ m}^3$  and the target dry unit weight is  $16.2\text{ kN/m}^3$ . Three nearby sites, see the figure below, have been identified from where the required soil can be transported to the construction site. The void ratios ( $e$ ) of different sites are shown in the figure. Assume the specific gravity of soil to be 2.7 for all three sites. If the cost of transportation per km is twice the cost of excavation per  $\text{m}^3$  of borrow pits, which site would you choose as the most economic solution? (Use unit weight of water =  $10\text{ kN/m}^3$ )

- Site X
- Site Y
- Site Z
- Any of the sites



- 10) A water tank is to be constructed on the soil deposit shown in the figure below. A circular footing of diameter 3 m and depth of embedment 1 m has been designed to support the tank. The total vertical load to be taken by the footing is 1500 kN. Assume the unit weight of water as  $10 \text{ kN/m}^3$  and the load dispersion pattern as  $2V : 1H$ . The expected settlement of the tank due to primary consolidation of the clay layer is \_\_\_\_\_ mm.



- 11) A 20 m thick clay layer is sandwiched between a silty sand layer and a gravelly sand layer. The layer experiences 30 mm settlement in 2 years.

Given:

$$T_v = \begin{cases} \frac{\pi}{4} \left( \frac{U}{100} \right)^2 & \text{for } U \leq 60\% \\ 1.781 - 0.933 \log_{10} (100 - U) & U > 60\% \end{cases}$$

where  $T_v$  is the time factor and  $U$  is the degree of consolidation in %.

If the coefficient of consolidation of the layer is  $0.003 \text{ cm}^2/\text{s}$ , the deposit will experience a total of

50 mm settlement in the next \_\_\_\_\_ years.

- 12) A non-homogeneous soil deposit consists of a silt layer sandwiched between a fine-sand layer at top and a clay layer below. Permeability of the silt layer is 10 times the permeability of the clay layer and one-tenth of the permeability of the sand layer. Thickness of the silt layer is 2 times the thickness of the sand layer and two-third of the thickness of the clay layer. The ratio of equivalent horizontal and equivalent vertical permeability of the deposit is \_\_\_\_\_.
- 13) A square footing, 2 m x 2 m, is subjected to an inclined point load,  $P$  as shown in the figure below. The water table is located below the base of the footing. Considering one-way eccentricity, the net safe load carrying capacity of the footing for a factor of safety of 3.0 is \_\_\_\_\_ kN.

The following factors may be used:

Bearing capacity factors:  $N_q = 33.3$ ,  $N_\gamma = 37.16$ ; Shape factors:  $F_{qs} = F_{\gamma s} = 1.314$ ; Depth factors:  $F_{qd} = F_{\gamma d} = 1.113$ ; Inclination factors:  $F_{qi} = 0.444$ ,  $F_{\gamma i} = 0.02$

