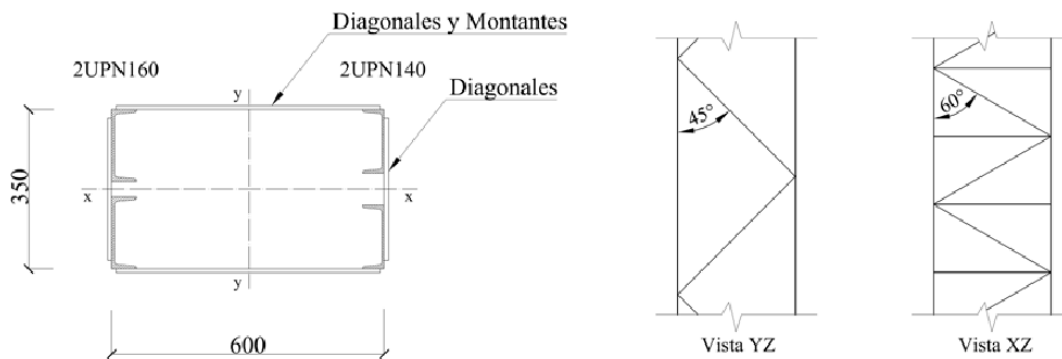


## T.P.N°7: Pandeo

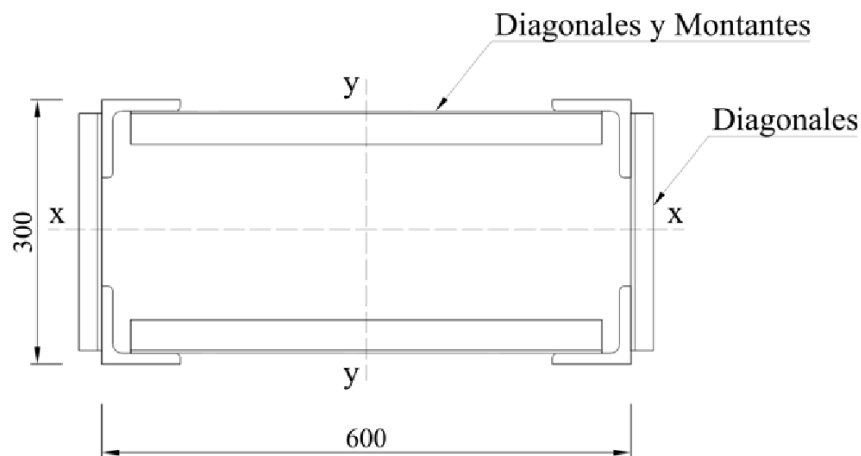
- Se tiene una columna compuesta por perfiles UPN de 35 cm x 60cm, donde se aplica una carga de 1000 kN en el baricentro de la sección. La altura es de 5m y se considera como condición de vínculo empotrado - articulado en el plano YZ y empotrado libre en el plano XZ. Uno de los cordones está formado por 2UPN 160 (unidos por diagonales) y el otro por dos UPN 140 (unidos por diagonales). Ambos cordones se encuentran unidos mediante diagonales y montantes como se indican en el dibujo. El acero a utilizar es F-24. Se requiere:

- Verificar la columna, en caso de no ser así, redimensionar
- Dimensionar los elementos de enlace



**Figura 1:** Columna compuesta por perfiles UPN de 35 cm x 60cm

- Determinar la carga ultima de una columna formada por 4 perfiles ángulos de  $3\frac{1}{2}$ " x  $\frac{1}{2}$ ". La altura es de 6m y se considera como condición de vínculo empotrado - articulado en el plano YZ y empotrado libre en el plano XZ. La unión en el plano YZ son mediante diagonales y la unión en el plano XZ son mediante diagonales y montantes. El acero a utilizar es F-24. Dimensionar los elementos de enlaces.



**Figura 2:** Columna compuesta por perfiles L  $3\frac{1}{2}$ " x  $3\frac{1}{2}$ " x  $\frac{1}{2}$ "

## Solución

1. Resolver una columna compuesta por perfiles UPN de 35 cm x 60cm.

■ Datos

Acero F-24

$E = 200000 MPa$

$f_y = 235 MPa$

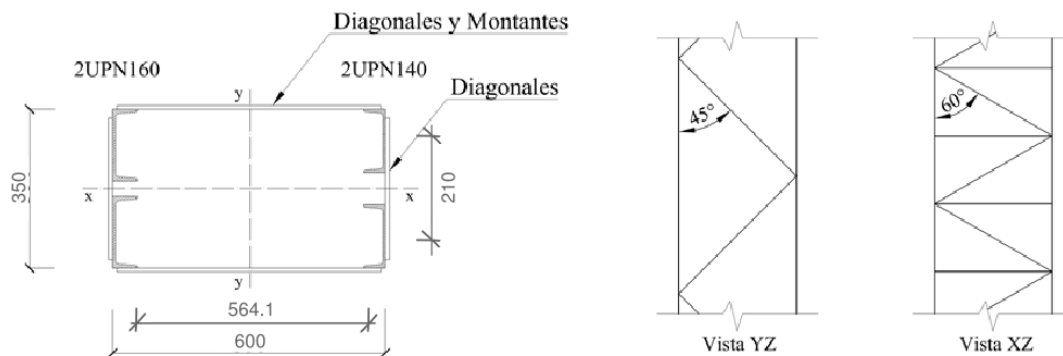
$P_u = 1000 KN$

$L = 500 cm$

$K_{yz} = 0,7$

$K_{xz} = 2$

Perfil	UPN160	UPN140
$I_x =$	$925 cm^4$	$605 cm^4$
$I_y =$	$85,3 cm^4$	$62,7 cm^4$
$r_x =$	$6,21 cm$	$5,45 cm$
$r_y = r_1 =$	$1,89 cm$	$1,75 cm$
$A_g =$	$24 cm^2$	$20,4 cm^2$
$e_x =$	$1,84 cm$	$1,75 cm$



**Figura 3:** Columna compuesta por perfiles UPN de 35 cm x 60cm

- Eje libre x-x  $\rightarrow$  Plano YZ

Condición de sustentación empotrado-articulado  $\Rightarrow K_{yz} = 0,70$

- Momentos de Inercia y Radio de Giro

$$d_1 = \frac{35cm}{2} - 8cm = 9,5cm$$

$$d_2 = \frac{35cm}{2} - 7cm = 10,5cm$$

$$d_3 = \frac{60cm}{2} - e_x = \frac{60cm}{2} - 1,84cm = 28,16cm$$

$$d_4 = \frac{60cm}{2} - e_x = \frac{60cm}{2} - 1,75cm = 28,25cm$$

$$I_{xx} = [(I_{xxUPN160} + A_g \cdot d_1^2) + (I_{xxUPN140} + A_g \cdot d_2^2)] \cdot 2$$

$$I_{xx} = [(925cm^4 + 24cm^2 \cdot (9,5cm)^2) + (605cm^4 + 20,4cm^2 \cdot (10,5cm)^2)] \cdot 2 = \boxed{11890,2cm^4}$$

$$r_x = \sqrt{\frac{I_{xx}}{A_t}} = \sqrt{\frac{11890,2cm^4}{2 \cdot (24cm^2 + 20,4cm^2)}} = \boxed{11,57cm}$$

- Esbeltez modificada

$$\lambda_0 = \frac{K_{yz} \cdot L}{r_x} = \frac{0,70 \cdot 500cm}{11,57cm} = \boxed{30,25}$$

$$\lambda_0 < 200$$

$$30,25 < 200 \quad \checkmark \quad \text{Verifica}$$

$$\alpha = 45^\circ$$

$$h = 2 \cdot d_2 = 2 \cdot 10,5cm = \boxed{21cm}$$

$$a = 2 \cdot \frac{h}{tg\alpha} = 2 \cdot \frac{21cm}{tg(45^\circ)} = \boxed{42cm}$$

$$d = \frac{h}{Sen\alpha} = \frac{21cm}{Sen(45^\circ)} = \boxed{29,70cm}$$

Adopto diagonales de perfil L 1  $\frac{1}{2}$ " x 1  $\frac{1}{2}$ " x 3/16"

$$A_d = 3,46cm^2$$

$$r_1 = 0,72cm$$

$$\lambda_1 = \pi \cdot \sqrt{\frac{2 \cdot A_g \cdot d^3}{n_0 \cdot A_d \cdot a \cdot h^2}}$$

$$n_0 = n^\circ \text{ de planos de celosía} = 2$$

$$\lambda_1 = \pi \cdot \sqrt{\frac{2 \cdot (2 \cdot (24cm^2 + 20,4cm^2)) \cdot (29,70cm)^3}{2 \cdot 3,46cm^2 \cdot 42cm \cdot (21cm)^2}} = \boxed{18,93}$$

$$\lambda_m = \sqrt{\lambda_0^2 + \lambda_1^2} = \sqrt{(30,25)^2 + (18,93)^2} = \boxed{35,68}$$

■ Resistencia de diseño local

$$P_{cm} = \frac{\pi^2 \cdot E \cdot A_g}{\lambda_m^2} \cdot 10^{-1}$$

$$P_{cm} = \frac{\pi^2 \cdot 200000 MPa \cdot 2 \cdot (24cm^2 + 20,4cm^2)}{(35,68)^2} \cdot 10^{-1} = \boxed{13768,38KN}$$

$$e_0 = \frac{K_{yz} \cdot L}{500} = \frac{0,70 \cdot 500cm}{500} = \boxed{0,70cm}$$

$$M_s = \frac{P_u \cdot e_0}{1 - \frac{P_u}{P_{cm}}} \cdot 10^{-2} = \frac{1000KN \cdot 0,70cm}{1 - \frac{1000KN}{13768,38KN}} \cdot 10^{-2} = \boxed{7,55KN.m}$$

$$P_{u1} = \frac{P_u}{n} + \frac{M_s}{n_1 \cdot h} \cdot 10^2$$

$$P_{u1} = \frac{1000KN}{4} + \frac{7,55KN.m}{2 \cdot 21cm} \cdot 10^2 = \boxed{267,97KN}$$

$n = n^\circ$  de barras de la columna armada  $\Rightarrow 4$

$n_1 = n^\circ$  de barras que forman el cordón  $\Rightarrow 2$

$$\lambda_{c1} = \frac{1}{\pi} \cdot \frac{L_1}{r_1} \cdot \sqrt{\frac{f_y}{E}}$$

$$L_1 = a = 42cm$$

$$\lambda_{c1} = \frac{1}{\pi} \cdot \frac{42cm}{1,75cm} \cdot \sqrt{\frac{235MPa}{200000MPa}} = \boxed{0,26}$$

$$\lambda_{c1} < 1,5$$

$$0,26 < 1,5 \quad \checkmark \quad \text{Verifica}$$

$$F_{cr} = 0,658^{\lambda_{c1}^2} \cdot f_y = 0,658^{0,26^2} \cdot 235MPa = \boxed{228,35MPa}$$

$$P_{d1} = 0,85 \cdot F_{cr} \cdot A_g \cdot 10^{-1} = 0,85 \cdot 228,35MPa \cdot 20,40cm^2 \cdot 10^{-1} = \boxed{395,96KN}$$

$$P_{d1} > P_{u1}$$

$$395,96KN > 267,97KN \quad \checkmark \quad \text{Verifica}$$

■ Verificación de las diagonales

Corte  $V_{eu}$

$$V_{eu} = \beta_1 \cdot P_u$$

$$\beta_1 = \frac{\pi}{500} \cdot \left[ \frac{1}{1 - \frac{P_u}{P_{cm}}} \right] = \frac{\pi}{500} \cdot \left[ \frac{1}{1 - \frac{1000KN}{13768,38KN}} \right] = \boxed{0,00678}$$

$$V_{eu} = 0,00678 \cdot 1000KN = \boxed{6,78KN}$$

El esfuerzo que solicita a la diagonal es  $D_u$

$$D_u = \frac{V_{eu}}{2 \cdot \cos \alpha} = \frac{6,78KN}{2 \cdot \cos(45^\circ)} = \boxed{4,79KN}$$

La resistencia de diseño del perfil ángulo es  $R_d$

$$R_d = \phi_c \cdot P_n$$

$$R_d > D_u$$

Se determina el factor de esbeltez adimensional  $\lambda_c$

$$\lambda_c = \frac{1}{\pi} \cdot \frac{K_{yz} \cdot L_1}{r_1} \cdot \sqrt{\frac{f_y}{E}}$$

$$L_1 = d = 29,70cm$$

$$\lambda_c = \frac{1}{\pi} \cdot \frac{0,70 \cdot 29,70cm}{0,72cm} \cdot \sqrt{\frac{235MPa}{200000MPa}} = \boxed{0,32}$$

$$\lambda_c < 1,5$$

$$0,32 < 1,5 \quad \checkmark \quad \text{Verifica}$$

$$F_{cr} = 0,658^{\lambda_c^2} \cdot f_y = 0,658^{0,32^2} \cdot 235MPa = \boxed{225,44MPa}$$

$$P_n = F_{cr} \cdot A_g \cdot 10^{-1} = 225,44MPa \cdot 20,40cm^2 \cdot 10^{-1} = \boxed{78KN}$$

$$R_d = \phi_c \cdot P_n = 0,85 \cdot 78KN = \boxed{66,30KN}$$

$$R_d > D_u$$

$$66,30KN > 4,79KN \quad \checkmark \quad \text{Verifica}$$

- Eje libre y-y  $\rightarrow$  Plano XZ  
Condición de sustentación empotrado-libre  $\Rightarrow K_{xz} = 2$
- Momentos de Inercia y Radio de Giro

$$d_1 = \frac{35cm}{2} - 8cm = 9,5cm$$

$$d_2 = \frac{35cm}{2} - 7cm = 10,5cm$$

$$d_3 = \frac{60cm}{2} - e_x = \frac{60cm}{2} - 1,84cm = 28,16cm$$

$$d_4 = \frac{60cm}{2} - e_x = \frac{60cm}{2} - 1,75cm = 28,25cm$$

$$I_{yy} = [(I_{yyUPN160} + A_g \cdot d_3^2) + (I_{yyUPN140} + A_g \cdot d_4^2)] \cdot 2$$

$$I_{yy} = [(85,3cm^4 + 24cm^2 \cdot (28,16cm)^2) + (62,7cm^4 + 20,4cm^2 \cdot (28,25cm)^2)] \cdot 2$$

$$I_{yy} = \boxed{70920,25cm^4}$$

$$r_y = \sqrt{\frac{I_{yy}}{A_t}} = \sqrt{\frac{70920,25cm^4}{2 \cdot (24cm^2 + 20,4cm^2)}} = \boxed{28,26cm}$$

- Esbeltez modificada

$$\lambda_0 = \frac{K_{xz} \cdot L}{r_y} = \frac{2 \cdot 500cm}{28,26cm} = \boxed{35,39}$$

$$\lambda_0 < 200$$

$$35,39 < 200 \quad \checkmark \quad \text{Verifica}$$

$$\alpha = 60^\circ$$

$$h = d_3 + d_4 = 28,16cm + 28,25cm = \boxed{56,41cm}$$

$$a = \frac{h}{tg\alpha} = \frac{56,41cm}{tg(60^\circ)} = \boxed{32,57cm}$$

$$d = \frac{h}{Sen\alpha} = \frac{56,41cm}{Sen(60^\circ)} = \boxed{65,14cm}$$

Adopto diagonales de perfil L 2" x 2" x 3/16"

$$A_d = 4,72cm^2$$

$$r_1 = 0,97cm$$

$$\lambda_1 = \pi \cdot \sqrt{\frac{A_g \cdot d^3}{n_0 \cdot A_d \cdot a \cdot h^2}}$$

$$n_0 = n^\circ \text{ de planos de celosía} = 2$$

$$\lambda_1 = \pi \cdot \sqrt{\frac{(2 \cdot (24cm^2 + 20,4cm^2)) \cdot (65,14cm)^3}{2 \cdot 4,72cm^2 \cdot 32,57cm \cdot (56,41cm)^2}} = \boxed{15,73}$$

$$\lambda_m = \sqrt{\lambda_0^2 + \lambda_1^2} = \sqrt{(35,39)^2 + (15,73)^2} = \boxed{38,73}$$

■ Resistencia de diseño local

$$P_{cm} = \frac{\pi^2 \cdot E \cdot A_g}{\lambda_m^2} \cdot 10^{-1}$$

$$P_{cm} = \frac{\pi^2 \cdot 200000 MPa \cdot 2 \cdot (24cm^2 + 20,4cm^2)}{(38,73)^2} \cdot 10^{-1} = \boxed{11688,05KN}$$

$$e_0 = \frac{K_{xz} \cdot L}{500} = \frac{2 \cdot 500cm}{500} = \boxed{2cm}$$

$$M_s = \frac{P_u \cdot e_0}{1 - \frac{P_u}{P_{cm}}} \cdot 10^{-2} = \frac{1000KN \cdot 2cm}{1 - \frac{1000KN}{11688,05KN}} \cdot 10^{-2} = \boxed{21,87KN.m}$$

$$P_{u1} = \frac{P_u}{n} + \frac{M_s}{n_1 \cdot h} \cdot 10^2$$

$$P_{u1} = \frac{1000KN}{4} + \frac{21,87KN.m}{2 \cdot 56,41cm} \cdot 10^2 = \boxed{269,39KN}$$

$n = n^\circ$  de barras de la columna armada  $\Rightarrow 4$

$n_1 = n^\circ$  de barras que forman el cordón  $\Rightarrow 2$

$$\lambda_{c1} = \frac{1}{\pi} \cdot \frac{L_1}{r_1} \cdot \sqrt{\frac{f_y}{E}}$$

$$L_1 = a = 32,57cm$$

$$\lambda_{c1} = \frac{1}{\pi} \cdot \frac{32,57cm}{1,75cm} \cdot \sqrt{\frac{235MPa}{200000MPa}} = \boxed{0,20}$$

$$\lambda_{c1} < 1,5$$

$$0,20 < 1,5 \quad \checkmark \quad \text{Verifica}$$

$$F_{cr} = 0,658^{\lambda_{c1}^2} \cdot f_y = 0,658^{0,20^2} \cdot 235MPa = \boxed{230,98MPa}$$

$$P_{d1} = 0,85 \cdot F_{cr} \cdot A_g \cdot 10^{-1} = 0,85 \cdot 230,98MPa \cdot 20,40cm^2 \cdot 10^{-1} = \boxed{400,52KN}$$

$$P_{d1} > P_{u1}$$

$$400,52KN > 269,39KN \quad \checkmark \quad \text{Verifica}$$

■ Verificación de las diagonales

Corte  $V_{eu}$

$$V_{eu} = \beta_1 \cdot P_u$$

$$\beta_1 = \frac{\pi}{500} \cdot \left[ \frac{1}{1 - \frac{P_u}{P_{cm}}} \right] = \frac{\pi}{500} \cdot \left[ \frac{1}{1 - \frac{1000KN}{11688,05KN}} \right] = \boxed{0,00687}$$

$$V_{eu} = 0,00687 \cdot 1000KN = \boxed{6,87KN}$$

El esfuerzo que solicita a la diagonal es  $D_u$

$$D_u = \frac{V_{eu}}{2 \cdot \cos \alpha} = \frac{6,87KN}{2 \cdot \cos(60^\circ)} = \boxed{6,87KN}$$

La resistencia de diseño del perfil ángulo es  $R_d$

$$R_d = \phi_c \cdot P_n$$

$$R_d > D_u$$

Se determina el factor de esbeltez adimensional  $\lambda_c$

$$\lambda_c = \frac{1}{\pi} \cdot \frac{K_{xz} \cdot L_1}{r_1} \cdot \sqrt{\frac{f_y}{E}}$$

$$L_1 = d = 65,14cm$$

$$\lambda_c = \frac{1}{\pi} \cdot \frac{2 \cdot 65,14cm}{0,97cm} \cdot \sqrt{\frac{235MPa}{200000MPa}} = \boxed{1,47}$$

$$\lambda_c < 1,5$$

$$1,47 < 1,5 \quad \checkmark \quad \text{Verifica}$$

$$F_{cr} = 0,658^{\lambda_c^2} \cdot f_y = 0,658^{1,47^2} \cdot 235MPa = \boxed{95,66MPa}$$

$$P_n = F_{cr} \cdot A_g \cdot 10^{-1} = 95,66MPa \cdot 20,40cm^2 \cdot 10^{-1} = \boxed{45,15KN}$$

$$R_d = \phi_c \cdot P_n = 0,85 \cdot 45,15KN = \boxed{38,38KN}$$

$$R_d > D_u$$

$$38,38KN > 4,79KN \quad \checkmark \quad \text{Verifica}$$



2. Determinar la carga última de una columna de 30 cm x 60cm, compuesta por 4 perfiles ángulos L 3 1/2" x 3 1/2" x 1/2".

■ Datos

Acero F-24

$$E = 200000 MPa$$

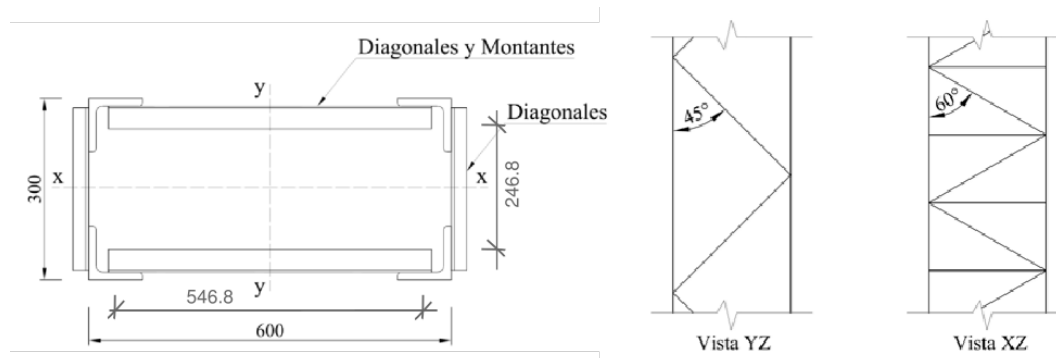
$$f_y = 235 MPa$$

$$L = 600 cm$$

$$K_{yz} = 0,7$$

$$K_{xz} = 2$$

Perfil	L 3 1/2" x 3 1/2" x 1/2"
$I_x = I_y =$	$149,65 cm^4$
$r_x = r_y =$	$2,66 cm$
$r_1 =$	$1,70 cm$
$e_x = e_y =$	$2,66 cm$
$A_g =$	$21,12 cm^2$



**Figura 4:** Columna de 30 cm x 60cm compuesta por perfiles ángulo

- Eje libre x-x  $\rightarrow$  Plano YZ  
Condición de sustentación empotrado-articulado  $\Rightarrow K_{yz} = 0,70$
- Momentos de Inercia y Radio de Giro

$$d_1 = \frac{Lado_y}{2} - e_y$$

$$d_1 = \frac{30cm}{2} - 2,66cm = 12,34cm$$

$$I_{xx} = (I_{xx} + A_g \cdot d_1^2) \cdot 4$$

$$I_{xx} = (149,65cm^4 + 21,12cm^2 \cdot (12,34cm)^2) \cdot 4 = \boxed{13462,84cm^4}$$

$$r_x = \sqrt{\frac{I_{xx}}{A_t}} = \sqrt{\frac{13462,84cm^4}{4 \cdot (21,12cm^2)}} = \boxed{12,62cm}$$

- Esbeltez modificada

$$\lambda_0 = \frac{K_{yz} \cdot L}{r_x} = \frac{0,70 \cdot 600cm}{2,66cm} = \boxed{33,27}$$

$$\lambda_0 < 200$$

$$33,27 < 200 \quad \checkmark \quad \text{Verifica}$$

$$\alpha = 45^\circ$$

$$h = 30cm - 2 \cdot e_y = 30cm - 2 \cdot 2,66cm = \boxed{24,68cm}$$

$$a = 2 \cdot \frac{h}{tg\alpha} = 2 \cdot \frac{24,68cm}{tg(45^\circ)} = \boxed{49,36cm}$$

$$d = \frac{h}{Sen\alpha} = \frac{24,68cm}{Sen(45^\circ)} = \boxed{34,90cm}$$

Adopto diagonales de perfil L 2" x 2" x 1/4"

$$A_d = 6,17cm^2$$

$$r_1 = 0,97cm$$

$$\lambda_1 = \pi \cdot \sqrt{\frac{2 \cdot A_g \cdot d^3}{n_0 \cdot A_d \cdot a \cdot h^2}}$$

$$n_0 = \text{n}^\circ \text{ de planos de celosía} = 2$$

$$\lambda_1 = \pi \cdot \sqrt{\frac{2 \cdot (4 \cdot (21,12cm^2)) \cdot (34,90cm)^3}{2 \cdot 6,17cm^2 \cdot 49,36cm \cdot (24,68cm)^2}} = \boxed{13,82}$$

$$\lambda_m = \sqrt{\lambda_0^2 + \lambda_1^2} = \sqrt{(33,27)^2 + (13,82)^2} = \boxed{36,03}$$

■ Resistencia de diseño local

$$P_{cm} = \frac{\pi^2 \cdot E \cdot A_g}{\lambda_m^2} \cdot 10^{-1}$$

$$P_{cm} = \frac{\pi^2 \cdot 200000 MPa \cdot 4 \cdot (21,12 cm^2)}{(36,03)^2} \cdot 10^{-1} = \boxed{12846,93 KN}$$

$$e_0 = \frac{K_{yz} \cdot L}{500} = \frac{0,70 \cdot 600 cm}{500} = \boxed{0,84 cm}$$

$$\lambda_{c1} = \frac{1}{\pi} \cdot \frac{L_1}{r_1} \cdot \sqrt{\frac{f_y}{E}}$$

$$L_1 = a = 49,36 cm$$

$$\lambda_{c1} = \frac{1}{\pi} \cdot \frac{49,36 cm}{0,97 cm} \cdot \sqrt{\frac{235 MPa}{200000 MPa}} = \boxed{0,32}$$

$$\lambda_{c1} < 1,5$$

$$0,32 < 1,5 \quad \checkmark \quad \text{Verifica}$$

$$F_{cr} = 0,658^{\lambda_{c1}^2} \cdot f_y = 0,658^{0,32^2} \cdot 235 MPa = \boxed{225,33 MPa}$$

$$P_{d1} = 0,85 \cdot F_{cr} \cdot A_g \cdot 10^{-1} = 0,85 \cdot 225,33 MPa \cdot 21,12 cm^2 \cdot 10^{-1} = \boxed{404,52 KN}$$

$$P_{u1} = \frac{P_u}{n} + \frac{P_u \cdot e_0}{1 - \frac{P_u}{P_{cm}}} \cdot 10^{-2} \cdot \frac{1}{n_1 \cdot h} \cdot 10^2$$

$$n = n^\circ \text{ de barras de la columna armada} \Rightarrow 4$$

$$n_1 = n^\circ \text{ de barras que forman el cordón} \Rightarrow 2$$

$$P_{u1} = \frac{P_u}{4} + \frac{P_u \cdot 0,84}{1 - \frac{P_u}{12846,93 KN}} \cdot 10^{-2} \cdot \frac{1}{2 \cdot 24,68 cm} \cdot 10^2$$

Dandole valores a  $P_u$  hasta que se verifique la condición  $P_{d1} > P_{u1}$

$$\boxed{P_u = 1500,36 KN} \Rightarrow P_{u1} = 404 KN$$

$$P_{d1} > P_{u1}$$

$$404,52 KN > 404 KN \quad \checkmark \quad \text{Verifica}$$

■ Verificación de las diagonales

Corte  $V_{eu}$

$$V_{eu} = \beta_1 \cdot P_u$$

$$\beta_1 = \frac{\pi}{500} \cdot \left[ \frac{1}{1 - \frac{P_u}{P_{cm}}} \right] = \frac{\pi}{500} \cdot \left[ \frac{1}{1 - \frac{1500,36KN}{12846,93KN}} \right] = \boxed{0,00711}$$

$$V_{eu} = 0,00711 \cdot 1500,36KN = \boxed{10,67KN}$$

El esfuerzo que solicita a la diagonal es  $D_u$

$$D_u = \frac{V_{eu}}{2 \cdot \cos \alpha} = \frac{10,67KN}{2 \cdot \cos(45^\circ)} = \boxed{7,55KN}$$

La resistencia de diseño del perfil ángulo es  $R_d$

$$R_d = \phi_c \cdot P_n$$

$$R_d > D_u$$

Se determina el factor de esbeltez adimensional  $\lambda_c$

$$\lambda_c = \frac{1}{\pi} \cdot \frac{K_{yz} \cdot L_1}{r_1} \cdot \sqrt{\frac{f_y}{E}}$$

$$L_1 = d = 34,90cm$$

$$\lambda_c = \frac{1}{\pi} \cdot \frac{0,70 \cdot 34,90cm}{0,97cm} \cdot \sqrt{\frac{235MPa}{200000MPa}} = \boxed{0,27}$$

$$\lambda_c < 1,5$$

$$0,27 < 1,5 \quad \checkmark \quad \text{Verifica}$$

$$F_{cr} = 0,658^{\lambda_c^2} \cdot f_y = 0,658^{0,27^2} \cdot 235MPa = \boxed{227,69MPa}$$

$$P_n = F_{cr} \cdot A_g \cdot 10^{-1} = 227,69MPa \cdot 21,12cm^2 \cdot 10^{-1} = \boxed{140,48KN}$$

$$R_d = \phi_c \cdot P_n = 0,85 \cdot 140,48KN = \boxed{119,41KN}$$

$$R_d > D_u$$

$$119,41KN > 7,55KN \quad \checkmark \quad \text{Verifica}$$

- Eje libre y-y  $\rightarrow$  Plano XZ  
Condición de sustentación empotrado-libre  $\Rightarrow K_{xz} = 2$
- Momentos de Inercia y Radio de Giro

$$d_2 = \frac{Lado_x}{2} - e_x$$

$$d_2 = \frac{60cm}{2} - 2,66cm = 27,34cm$$

$$I_{yy} = (I_{yy} + A_g \cdot d_2^2) \cdot 4$$

$$I_{yy} = (149,65cm^4 + 21,12cm^2 \cdot (27,34cm)^2) \cdot 4 = \boxed{63745,34cm^4}$$

$$r_y = \sqrt{\frac{I_{yy}}{A_t}} = \sqrt{\frac{63745,34cm^4}{4 \cdot (21,12cm^2)}} = \boxed{27,47cm}$$

- Esbeltez modificada

$$\lambda_0 = \frac{K_{xz} \cdot L}{r_y} = \frac{2 \cdot 600cm}{27,47cm} = \boxed{43,69}$$

$$\lambda_0 < 200$$

$$43,69 < 200 \quad \checkmark \quad \text{Verifica}$$

$$\alpha = 60^\circ$$

$$h = 60cm - 2 \cdot e_x = 60cm - 2 \cdot 2,66cm = \boxed{54,68cm}$$

$$a = \frac{h}{tg\alpha} = \frac{54,68cm}{tg(60^\circ)} = \boxed{31,57cm}$$

$$d = \frac{h}{Sen\alpha} = \frac{54,68cm}{Sen(60^\circ)} = \boxed{63,14cm}$$

Adopto diagonales de perfil L 2" x 2" x 1/4"

$$A_d = 6,17cm^2$$

$$r_1 = 0,97cm$$

$$\lambda_1 = \pi \cdot \sqrt{\frac{A_g \cdot d^3}{n_0 \cdot A_d \cdot a \cdot h^2}}$$

$$n_0 = n^\circ \text{ de planos de celosía} = 2$$

$$\lambda_1 = \pi \cdot \sqrt{\frac{4 \cdot (21,12cm^2) \cdot (63,14cm)^3}{2 \cdot 6,17cm^2 \cdot 31,57cm \cdot (54,68cm)^2}} = \boxed{13,42}$$

$$\lambda_m = \sqrt{\lambda_0^2 + \lambda_1^2} = \sqrt{(43,69)^2 + (13,42)^2} = \boxed{45,70}$$

■ Resistencia de diseño local

$$P_{cm} = \frac{\pi^2 \cdot E \cdot A_g}{\lambda_m^2} \cdot 10^{-1}$$

$$P_{cm} = \frac{\pi^2 \cdot 200000 MPa \cdot 4 \cdot (21,12 cm^2)}{(45,70)^2} \cdot 10^{-1} = \boxed{7984,24 KN}$$

$$e_0 = \frac{K_{xz} \cdot L}{500} = \frac{2 \cdot 600 cm}{500} = \boxed{2,4 cm}$$

$$\lambda_{c1} = \frac{1}{\pi} \cdot \frac{L_1}{r_1} \cdot \sqrt{\frac{f_y}{E}}$$

$$L_1 = a = 31,57 cm$$

$$\lambda_{c1} = \frac{1}{\pi} \cdot \frac{31,57 cm}{0,97 cm} \cdot \sqrt{\frac{235 MPa}{200000 MPa}} = \boxed{0,20}$$

$$\lambda_{c1} < 1,5$$

$$0,20 < 1,5 \quad \checkmark \quad \text{Verifica}$$

$$F_{cr} = 0,658^{\lambda_{c1}^2} \cdot f_y = 0,658^{0,20^2} \cdot 235 MPa = \boxed{231 MPa}$$

$$P_{d1} = 0,85 \cdot F_{cr} \cdot A_g \cdot 10^{-1} = 0,85 \cdot 231 MPa \cdot 21,12 cm^2 \cdot 10^{-1} = \boxed{414,68 KN}$$

$$P_{u1} = \frac{P_u}{n} + \frac{P_u \cdot e_0}{1 - \frac{P_u}{P_{cm}}} \cdot 10^{-2} \cdot \frac{1}{n_1 \cdot h} \cdot 10^2$$

$$n = n^\circ \text{ de barras de la columna armada} \Rightarrow 4$$

$$n_1 = n^\circ \text{ de barras que forman el cordón} \Rightarrow 2$$

$$P_{u1} = \frac{P_u}{4} + \frac{P_u \cdot 2,4}{1 - \frac{P_u}{7984,24 KN}} \cdot 10^{-2} \cdot \frac{1}{2 \cdot 54,68 cm} \cdot 10^2$$

Dandole valores a  $P_u$  hasta que se verifique la condición  $P_{d1} > P_{u1}$

$$\boxed{P_u = 1494,58 KN} \Rightarrow P_{u1} = 414 KN$$

$$P_{d1} > P_{u1}$$

$$414,68 KN > 414 KN \quad \checkmark \quad \text{Verifica}$$

■ Verificación de las diagonales

Corte  $V_{eu}$

$$V_{eu} = \beta_1 \cdot P_u$$

$$\beta_1 = \frac{\pi}{500} \cdot \left[ \frac{1}{1 - \frac{P_u}{P_{cm}}} \right] = \frac{\pi}{500} \cdot \left[ \frac{1}{1 - \frac{1494,58KN}{7984,24KN}} \right] = \boxed{0,00773}$$

$$V_{eu} = 0,00773 \cdot 1494,58KN = \boxed{11,55KN}$$

El esfuerzo que solicita a la diagonal es  $D_u$

$$D_u = \frac{V_{eu}}{2 \cdot \cos \alpha} = \frac{11,55KN}{2 \cdot \cos(60^\circ)} = \boxed{11,55KN}$$

La resistencia de diseño del perfil ángulo es  $R_d$

$$R_d = \phi_c \cdot P_n$$

$$R_d > D_u$$

Se determina el factor de esbeltez adimensional  $\lambda_c$

$$\lambda_c = \frac{1}{\pi} \cdot \frac{K_{xz} \cdot L_1}{r_1} \cdot \sqrt{\frac{f_y}{E}}$$

$$L_1 = d = 63,14cm$$

$$\lambda_c = \frac{1}{\pi} \cdot \frac{2 \cdot 63,14cm}{0,97cm} \cdot \sqrt{\frac{235MPa}{200000MPa}} = \boxed{1,42}$$

$$\lambda_c < 1,5$$

$$1,42 < 1,5 \quad \checkmark \quad \text{Verifica}$$

$$F_{cr} = 0,658^{\lambda_c^2} \cdot f_y = 0,658^{1,42^2} \cdot 235MPa = \boxed{101MPa}$$

$$P_n = F_{cr} \cdot A_g \cdot 10^{-1} = 101MPa \cdot 21,12cm^2 \cdot 10^{-1} = \boxed{62,32KN}$$

$$R_d = \phi_c \cdot P_n = 0,85 \cdot 62,32KN = \boxed{52,97KN}$$

$$R_d > D_u$$

$$52,97KN > 11,55KN \quad \checkmark \quad \text{Verifica}$$