

## King Mongkut's University of Technology Thonburi

#### Midterm Examination Semester 2 Academic Year 2014

### **CVE 341: Steel and Timber Design**

Date: 27<sup>th</sup> February 2015 Time 9:00 – 12:00

#### Instructions:

- 1. The exam has 4 questions in 10 pages. Total points are 30 points with each question not of equal points.
- 2. Read the questions carefully and strictly follow instruction.
- 3. Textbooks and written materials are allowed in the examination room.
- 4. A calculator is allowed.
- 5. Write your name on every page.
- 6. Perform your work in the examination paper.

Examiner: Assistant Professor Aphinat Ashakul

Tel. 02-470-9148

This examination paper has been approved by the Department of Civil Engineering

Associate Professor Dr. Sutat Leelataviwat Head of the Civil Engineering Department

- 1. From the given floor plan, carry out the following task (12 Points):
- a) Check the adequacy of the  $H400x200x66.0\ kg/m\ SM400$  used as a secondary beam as shown.
- b) Design the lightest H section for beam B1 without considering serviceability.

Use the following criteria:

All beams are to be SM400 steel

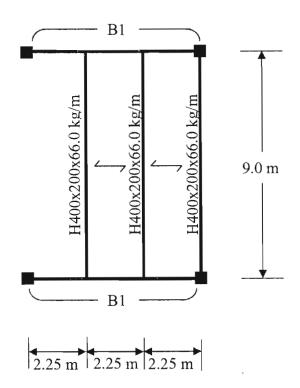
DL + LL of the floor is  $800 \text{ kg/m}^2$ 

Consider beam's self-weight in all the calculation

No local failure mode is to be considered.

Use ASD method

 $\label{eq:hammer} \begin{array}{ll} \textbf{H400x200x66.0 kg/m:} \;\; b_f = 200 \; mm, \; t_f = 13 \; mm, \; d = 400 \; mm, \; t_w = 8.0 \; mm, \\ k = 29.0 \; mm, \; Z_x = 1,330 \; cm^3, \; S_x = 1,190 \; cm^3 \end{array}$ 

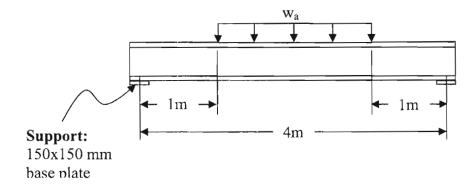


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2. How much uniform load  $w_a$  can the H250x125x25.7 kg/m SM400 carry? Use the clear span in the calculation. The base plate size is 250x250 mm. The self-weight of the beam is negligible. (10 Points)

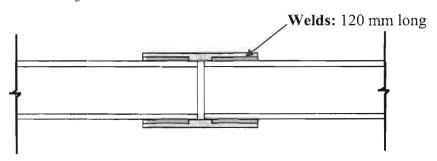
**H250x125x25.7 kg/m**:  $b_f = 124$  mm,  $t_f = 8.0$  mm, d = 248 mm,  $t_w = 5.0$  mm, k = 20.0 mm,  $Z_x = 319$  cm<sup>3</sup>,  $S_x = 285$  cm<sup>3</sup>



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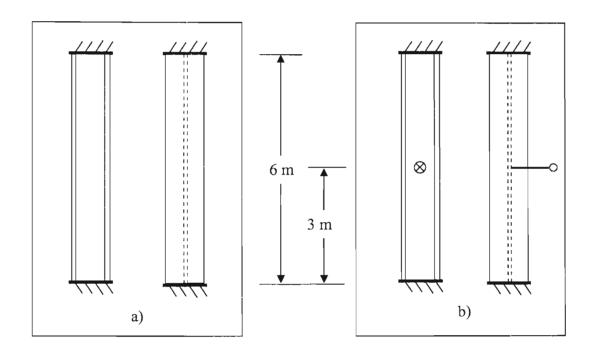
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3. The spliced H150x150x31.5 kg/m SM520 ( $F_y$  = 3,700 kg/cm<sup>2</sup>,  $F_u$  = 5,300 kg/cm<sup>2</sup>) is to be used to transfer the service tensile force equal to 60 tons. The length of the welds used on each spliced plate is 120 mm long. Check whether or not this section can carry this load. Assume that the connection itself is sufficient. (4 Points) H150x150x31.5 kg/m:  $b_f$  = 150 mm,  $t_f$  = 10 mm, d = 150 mm,  $t_w$  = 7.0 mm, d = 40.1 cm<sup>2</sup>



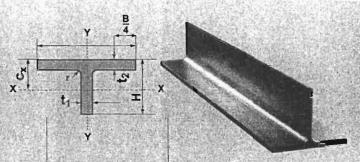
- 4. For the 6-m fixed-fixed H200x200x56.2 kg/m SM400 column shown, answer the following questions (4 Points):
  - a) How much compressive force can this column carry?
- b) If a bracing is added in the middle to reduce the unbraced length in the weak axis direction, what K would you use for the calculation of  $F_{cr}$  in weak axis? Why? Answer briefly.

**H200x200x56.2 kg/m**:  $b_f = 204$  mm,  $t_f = 12$  mm, d = 200 mm,  $t_w = 12$  mm  $r_x = 8.35$  cm,  $r_y = 4.88$  cm,  $A_g = 71.5$  cm<sup>2</sup>



Student Name & I.D.	

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# **CUT BEAMS**

(Cutting service is available upon request) | CUT BEAM | DIMENSION

Nominal Size	Weight	Sectional Dimension			Sectional	Moment of Inertia		Radius of Gyration		Modulus of Section		Center of Gravity		
		н	В	t <sub>1</sub>	t <sub>2</sub>	r	Area	t	L	į.		Z <sub>x</sub>	Z <sub>y</sub>	C <sub>x</sub>
mm	kg/m	mm	mm	mm	mm	mm	cm <sup>2</sup>	cm⁴	cm⁴	cm	cm	cm <sup>3</sup>	cm <sup>3</sup>	em
50x50	4.70	50.0	50	5.0	7	8	5.93	11.8	7.4	1.41	1.12	3.17	3.0	1.28
50x100	8.60	50.0	100	6.0	8	10	10.95	16.1	66.9	1.21	2.47	4.03	13.4	1.00
62.5x125	11.90	62.5	125	6.5	9	10	15.16	35.0	147.0	1.52	3.11	6.91	23.5	1.19
75x75	7.00	75.0	75	5.0	7	8	8.925	42.6	24.7	2.18	1.66	7.46	6.6	1.79
75x100	10.55	74.0	100	6.0	9	11	13.42	51.7	75.3	1.96	2.37	8.84	15.1	1.55
75x150	15.75	75.0	150	7.0	10	11	20.07	66.4	282.0	1.82	3.75	10.80	37.6	1.37
87.5x175		87.5	175	7.5	11	12	25.61	115.0	492.0	2.12	4.38	15.90	56.2	1.55
	<b>9</b> .10	99.0	99	4.5	7	11	11.59	93.8	56.8	2.84	2.21	12.10	11.5	2.14
100x100	10.65	100.0	100	5.5	8	11	13.58	114.0	67.0	2.90	2.22	14.80	13.4	2.29
100x150	15.30	97.0	150	6.0	9	13	19.51	125.0	254.0	2.53	3.61	15.80	33.8	1.79
	24.95	100.0	200	8.0	12	13	31.77	184.0	801.0	2.41	5.02	22.30	80.1	1.73
100x200	* 28.10	100.0	204	12.0	12	13	35.77	256.0	851.0	2.67	4.88	32.40	83.4	2.09
	* 32.85	104.0	202	10.0	16	13	41.85	251.0	1,100.0	2.45	5.13	29.50	109.0	1.91
125x125	<b>Ø</b> 12.85	124.0	124	5.0	8	12	16.34	208.0	127.0	3.57	2.79	21.30	20.5	2.68
120X120	14.80	125.0	125	6.0	9	12	18.83	248.0	147.0	3.63	2.79	25.60	23.5	2.78
125x175	22.05	122.0	175	7.0	11	16	28.12	289.0	492.0	3.20	4.18	29.10	56.3	2.27
	<b>Ø</b> 32.20	122.0	252	11.0	11	16	41.03	445.0	1,470.0	3.29	5.98	45.30	117,0	2.39
125x250	* 33.25	124.0	249	8.0	13	16	42.35	364.0	1,670.0		6.29	34.90	134.0	1.98
1203200	36.20	125.0	250	9.0	14	. 16	46.09	412.0	1,820.0	2.99	6.29	39.50	146.0	2.08
	* 41.10	125.0	255	14.0	14	16	52.34	589.0	1,940.0	3.36	6.09	59.40	152.0	2.58
150x150	<b>Ø</b> 16.00	149.0	149	5.5	8	13	20.40	393.0	221.0	4.39	3.29	33.80	29.7	3.26
1302130	18.35	150.0	150	6.5	9	13	23.39	464.0	254.0	4.45	3.29	40.00	33.8	3.41
150x200	28.40	147.0	200	8.0	12	18	36.19	572.0	802.0	3.97	4.71	48.20	80.2	2.83
130,200	* 32.70	149.0	201	9.0	14	18	41.68	662.0	949.0	3.99	4.77	55.20	94.4	2.91
	<b>Q</b> 42.25	147.0	302	12.0	12	18	53.83	858.0	2,760.0		7.16	72.30	183.0	3.84
	* 43.50	149.0	299	9.0	14	18	55.40	715.0	3,120.0		7.51	57.00	209.0	2.36
150x300		150.0	300	10.0	15	18	59.89	798.0	3,380.0	_	7.51	63.70	225.0	2.47
	<b>*</b> 53.00	150.0	305	15.0	15	18	67.39		3,550.0	_	7.26	92.50	233.0	2.03
	* 53.00	152.0	301	11.0	17	18	.67.41	903.0	3,870.0		7.57	71.40	257.0	2.55
	<b>©</b> 20.70	173.0	174	6.0	9	14	26.34	679.0	396.0	5.08	3.88	50.00	45.5	3.71
175x175		175.0	175	7.0	11	14	31.57	815.0	492.0	5.08	3.95	59.30	56.2	3.75
	* 28.90	177.0	176	8.0	13	14	36.84	955.0	590.0	5.09	4.01	68.80	67.0	3.82
175x250	* 34.60	168.0	249	8.0	12	20	44.08	881.0	1,540.0		5.92	64.00	124.0	3.02
	39.85	170.0	250	9.0	14	20	50.76	1,020.0		-	6.00	73.10	146.0	3.09
,	* 53.00	169.0	351	13.0	13	20	67.63	1,420	4,690.0		8.33	104.00	-	3.21
4== 0=0	<b>Ø</b> 57.50	172.0		10.0	16	20	73.00	1,230	5,620.0		8.78	84.70	323.0	2.67
175x350	Marie Control of the	172.0	354	16.0	16	20	83.32	1,800	5,920.0		8.43	131.00		3.40
	68.50	175.0	350	12.0	19	20	86.94	1,520	6,790.0		8.84	104.00		2.86
	* 78.00	175.0	_	19.0	19	20	99.19	2,200	7,220.0	_	8.53	158.00		3.59
	<b>©</b> 28.30	198.0	199	7.0	11	16	36.08	1,190	723.0	_	4.48	76.40	72.7	4.17
200x200		200.0	200	8.0	13	16	42.06	1,400	868.0		4.54	88.60	86.8	4.23
	* 37.75	202.0		9.0	15	16	48.08		1,015.0		4.59	101.00	*	4.31
200x300	* 47.15	193.0	-	9.0	14	22	60.05		3,120.0		7.21	95.50	209.0	
	53.50	195.0	300	10.0	16	22	67.98	1,730.0	3,600.0	5.05	7.28	108.00	240.0	0.41

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