



Seat No

King Mongkut's University of Technology Thonburi

Final Examination
Semester 2 Academic Year 2014

CVE 341: Steel and Timber Design

Date: 18th May 2015

Time 9:00 – 12:00

Instructions :

1. The exam has **5** questions in **14** pages. Total points are **50** points with each question not of equal points.
2. Read the questions carefully and strictly follow instruction.
3. Textbooks and written materials **are allowed** in the examination room.
4. A calculator is allowed.
5. Write your name on every page.
6. Perform your work in the examination paper.

Examiner: Associate Professor Sutat Leelataviwat
Assistant Professor Aphinat Ashakul
Tel. 02-470-9148

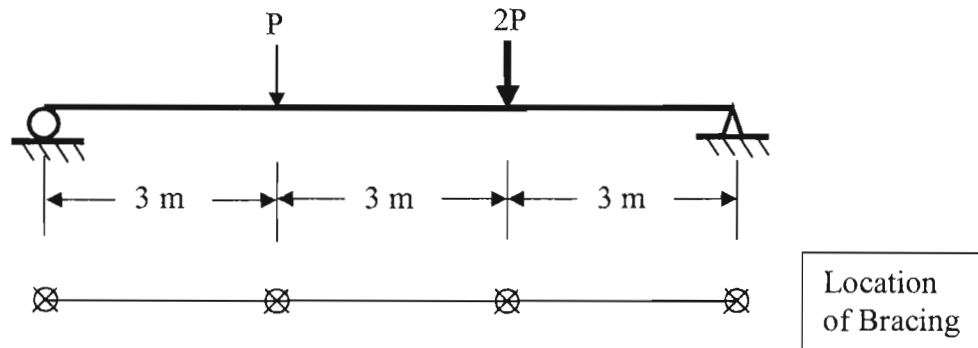
This examination paper has been approved by the Department of Civil Engineering

**Associate Professor Dr. Sutat Leelataviwat
Head of the Civil Engineering Department**

Student Name & I.D. _____

1. Investigate beam H500x200x89.6 kg/m SM520 ($F_y = 3,700 \text{ kg/cm}^2$) shown to determine how much a concentrated load P the beam can carry. Self-weight can be neglected. The beam has stiffeners at all locations where concentrated loads are applied; therefore, all local failure can be neglected. There is no need to consider serviceability. Use ASD (10 Points)

H500x200x89.6 kg/m: $b_f = 200 \text{ mm}$, $t_f = 16 \text{ mm}$, $d = 500 \text{ mm}$, $t_w = 10 \text{ mm}$, $k = 36 \text{ mm}$, $S_x = 1,910 \text{ cm}^3$, $Z_x = 2,180 \text{ cm}^3$

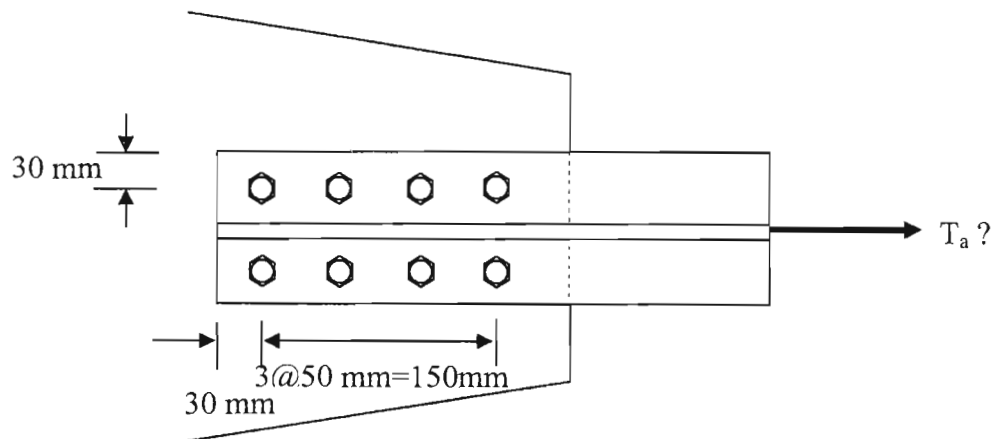


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2. Check the tensile strength of the bolted connection connecting T150x150x16.0 kg/m SM400 to the gusset plate as shown. Bolts used are 8-M16 A325N. Assume that the gusset plate is adequate. Use ASD method (10 Points)

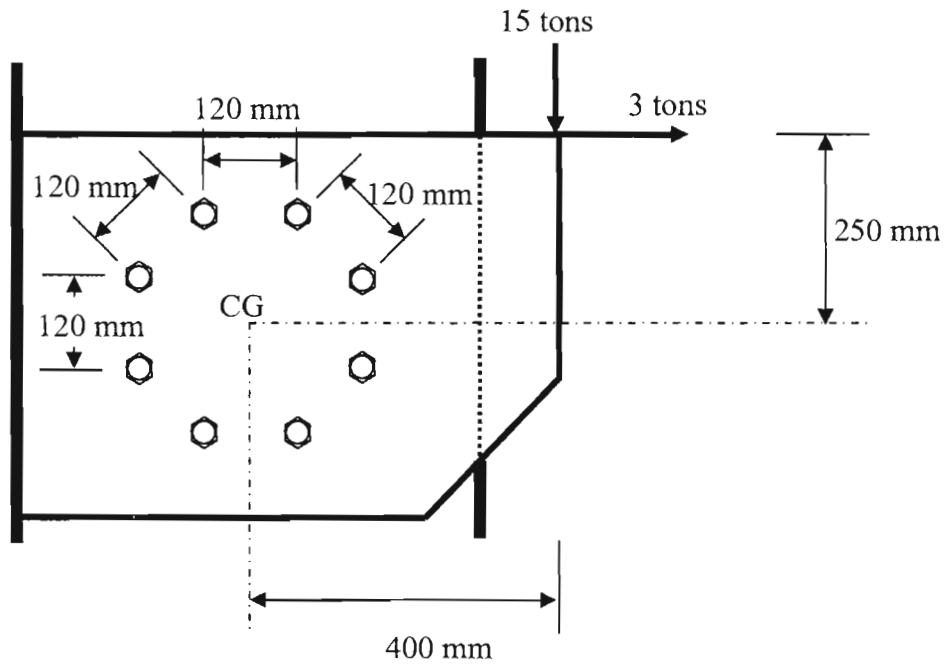
T150x150x16.0 kg/m: $b_f = 149$ mm, $t_f = 8$ mm, $d = 149$ mm, $t_s = 5.5$ mm, $\bar{y} = 3.26$ cm, $A_g = 20.4$ cm²



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3. According to Feng-shui, a 8-bolted connection is required for a lucky-charm gusset to carry the load as shown. Design the bolt size for A325N bolts to carry the load as illustrated. Use LRFD (10 Points)



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4) A steel column (H350x350x12x19 – 137 kg/m) carries the loads as shown below. The member is a part of a braced system, and has supports in the weak direction at supports and mid-height. Use LRFD method (10 Points);

- (a) Find the available flexural strength ($\phi_b M_n$) of the member.
- (b) Find the available axial compressive strength ($\phi_c P_n$) of the member.
- (c) Find the required strength (P_r and M_r) including the second order effect.
- (d) Check the adequacy of the member as a beam-column member.

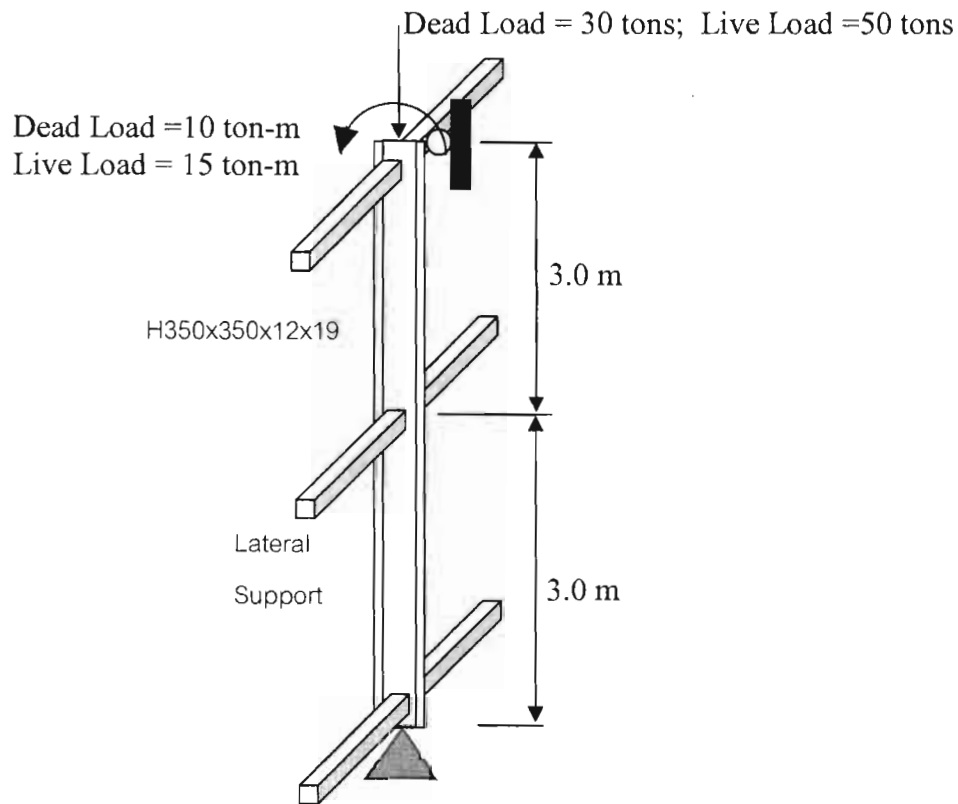


Figure 1

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5) A timber floor framing around a stairway opening is shown below. The left ends of the short joists are supported by a header. The floor self-weight is assumed to be equal to 40 kg/m^2 for design purpose and the live load is 100 kg/m^2 . Assume dense timber with a specific gravity of 0.90. Deflection limit is set at $L/240$. (10 Points)

- Design the size of the header to support the given load. Check all the limit states.
- If the header is connected at its ends to the joist (header-to-joist connection) using 2-4" nails. Determine if this is sufficient. If not, find the required number and size of the nails.

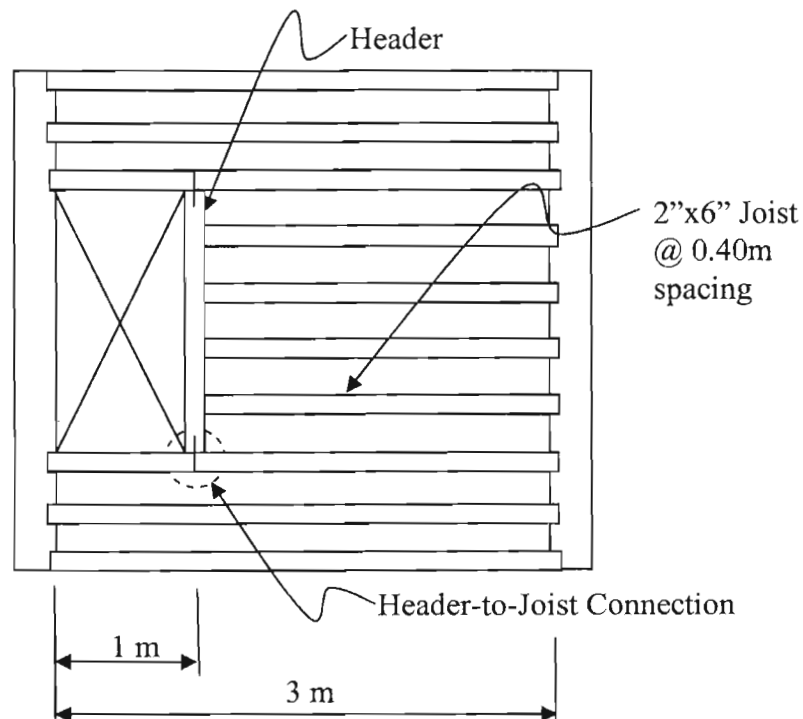


Figure 2

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BEAM FORMULAS

	LOADING	MOMENT	SHEAR	DEFLECTION
		$M_C = M_F = \frac{2PL}{5} \quad M_D = M_E = \frac{3PL}{5}$	$R_A = R_B = 2P$	$d_{max.} = \frac{63PL^3}{1000EI}$
		<p>When n is odd, $M_{max.} = \frac{(n^2 - 1) PL}{8n}$</p> <p>When n is even, $M_{max.} = n \cdot PL/8$</p>	$R_A = R_B = (n-1)P/2$	<p>When n is odd $d_{max.} = \frac{PL^3}{192EI} \left[n - \frac{1}{n} \right] \left[3 - \frac{1}{2} \left(1 - \frac{1}{n^2} \right) \right]$</p> <p>When n is even $d_{max.} = \frac{PL^3}{192EI} \cdot n \left[3 - \frac{1}{2} \left(1 + \frac{4}{n^2} \right) \right]$</p>
		$M_C = M_E = \frac{3PL}{8} \quad M_D = \frac{PL}{2}$	$R_A = R_B = \frac{3P}{2}$	$d_{max.} = \frac{19PL^3}{384EI}$
		$M_C = M_F = \frac{PL}{4} \quad M_D = M_E = \frac{PL}{2}$	$R_A = R_B = 2P$	$d_{max.} = \frac{41PL^3}{768EI}$