### (0) Packages

```
In []: # Run this cell to make sure the mdof module is installed.
!pip install mdof

In [1]: # Import the required packages.
import numpy as np
from pathlib import Path
import matplotlib.pyplot as plt
import mdof
```

#### (1) Load Files

```
In [2]: # Path to the directory where files are saved
        DATA_DIR = Path("../../uploads/mini_shake_table/Flexible vs. Stiff")
        # Get all the files ending in csv
        files = list(DATA DIR.glob("**/*.csv"))
        # Identify the row with the column labels
        header row = 8
        # Get the column labels
        with open(files[0], "r") as readfile:
            header_keys = readfile.readlines()[header_row-1].split(',')
        # Identify the column indices for each acceleration component
        x index = header keys.index('Acc X')
        y_index = header_keys.index('Acc_Y')
        z_index = header_keys.index('Acc_Z')
        # Populate a dictionary with the data. Each file is its own item.
        data = \{\}
        for file in files:
            filename = f"{file.parent.parent.name} {file.parent.name} - {file.name.s
            data[filename] = np.loadtxt(file,
                                         delimiter=",",
                                         skiprows=header_row, # Get all the rows afte
                                        usecols=[x_index,y_index,z_index] # Get only
```

# Print the filenames used as keys in the data dictionary

```
In []: # We'll use this list of filenames as reference for the options
# of records used in the system identification code down below.

for filename in data.keys():
    print(filename)
```

### (2) Plot the records

- top floor only
- x direction only

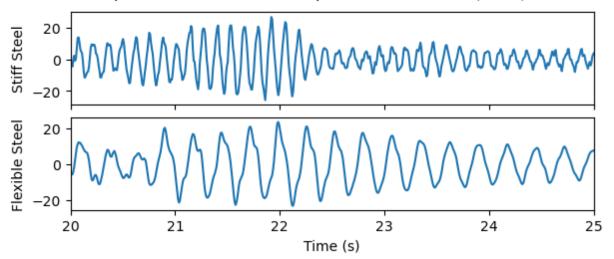
```
In [3]: # Get the time array!
SAMPLE_RATE = 120 #Hz
# If there are sampling_rate samples taken per second, what is the amount of
# That is, how many seconds per sample?
TIME_STEP = 1/SAMPLE_RATE
# Create a function that returns a time array that starts at zero,
# has a length of num_points, and has a step of time_step between
# each point.
# note: the syntax for np.linspace is np.linspace(start,stop,length)
def time_array(time_step, num_points):
    return np.linspace(0,time_step*num_points,num_points)
```

#### Compare the stiff to the flexible three story model.

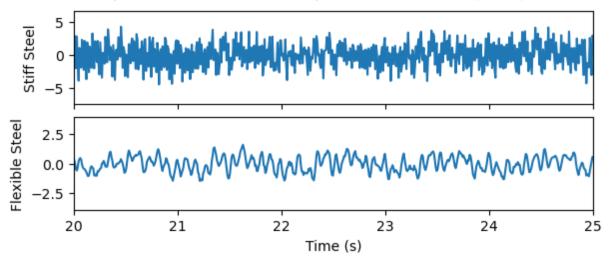
```
In [4]: # Path to a directory where we'll save figures
        OUT_DIR = Path("out/")
        if not OUT DIR.exists():
            OUT DIR.mkdir()
        event_names = ["Northridge", "Sine 1.0", "Sine 1.5", "Sine 2.0", "Bonus Ever
        # Loop through all events
        for event name in event names:
            # Plot the records!
            # Only plot the top floor response.
            # Plot stiff 3 story and flexible 3 story steel side-by-side.
            outputs_stiff = data[f'Stiff 3 story 3rd Floor (Top floor) - ({event_name
            outputs_flex = data[f'Flexible 3 story 3rd Floor (Top floor) - ({event_r
            # Make the records the same length.
            npts = min(outputs stiff.shape[0], outputs flex.shape[0])
            outputs_stiff = outputs_stiff[:npts]
            outputs_flex = outputs_flex[:npts]
            # Construct the time array
            time = time_array(TIME_STEP, npts)
            # Create a figure with two subplots stacked vertically (2 rows, 1 column
            fig,ax = plt.subplots(2, 1, figsize=(6,3), sharex=True, constrained layo
            # Plot the X direction, which is the first column of record
            ax[0].plot(time, outputs_stiff[:,0])
            ax[1].plot(time, outputs_flex[:,0])
            # Labels, limits and title
            ax[0].set ylabel('Stiff Steel')
            ax[1].set_ylabel('Flexible Steel')
            ax[1].set xlabel('Time (s)')
            ax[1].set_xlim((20,25))
            if "Bonus" in event_name:
                ax[1].set_xlim((17,20))
```

fig.suptitle(f"{event\_name} \n Top Floor Acceleration Response, X Direct
# Save the figure
fig.savefig(OUT\_DIR/f"{event\_name}.png")

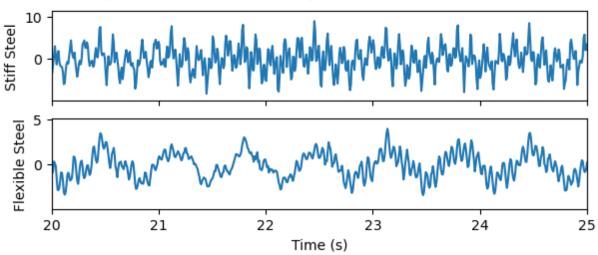
Northridge
Top Floor Acceleration Response, X Direction  $(m/s^2)$ 



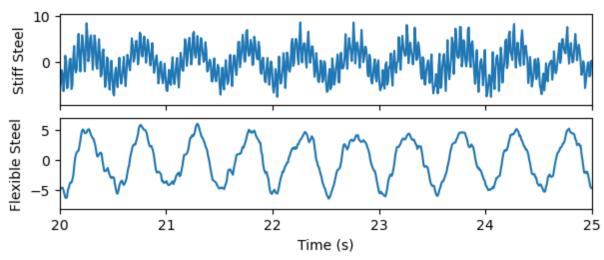
Sine 1.0 Top Floor Acceleration Response, X Direction  $(m/s^2)$ 



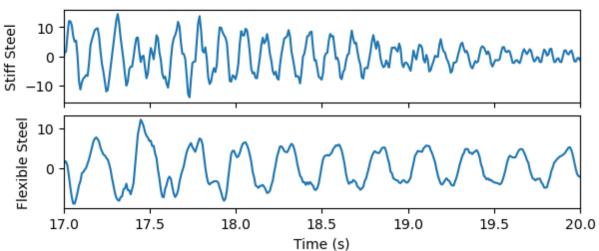
Sine 1.5 Top Floor Acceleration Response, X Direction  $(m/s^2)$ 



Sine 2.0 Top Floor Acceleration Response, X Direction  $(m/s^2)$ 



Bonus Event Top Floor Acceleration Response, X Direction  $(m/s^2)$ 

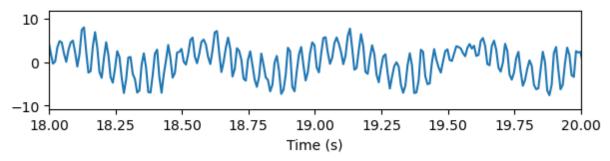


#### Plot the stiff one story model.

Just plot a preview of the Sine Wave record.

```
In [5]: event names = [
            # "El Centro Trial 1", "El Centro Trial 2", "Northridge Trial 1", "North
            "Sine Wave Trial 1",
            # "Sine Wave Trial 2",
        # Loop through all events
        for event name in event names:
            # Plot the records!
            # Only plot the top floor response.
            outputs = data[f'Stiff 1 story Top Floor - ({event name})']
            # Construct the time array
            npts = outputs.shape[0]
            time = time array(TIME STEP, npts)
            # Create a figure
            fig,ax = plt.subplots(figsize=(6,2), sharex=True, constrained_layout=Tru
            # Plot the X direction, which is the first column of record
            ax.plot(time, outputs[:,0])
            # Labels, limits and title
            ax.set xlabel('Time (s)')
            if "Sine" in event name:
                ax.set_xlim((18,20))
            elif "El Centro" in event name:
                ax.set_xlim((8,15))
            else:
                ax.set xlim((5,12))
            fig.suptitle(f"{event name} \n Top Floor Acceleration Response, X Direct
            # Save the figure
            fig.savefig(OUT_DIR/f"{event_name}.png")
```

Sine Wave Trial 1 Top Floor Acceleration Response, X Direction  $(m/s^2)$ 



# (3) Discussion: what can we say about how *frequency of oscillation* varies with *stiffness* of the steel walls?

How does this make sense with the relation,  $\omega_n = \sqrt{\frac{k}{m}}$  ?

Write your observations below.

- Frequency of oscillation increases as stiffness of the steel increases.
- *k* in the equation is stiffness. For stiffer steel, *k* is higher.
- *m* in the equation is mass. mass is constant.
- $\omega_n$  in the equation is natural frequency.
- Therefore, as stiffness increases in the equation, natural frequency increases by a square root relationship with the stiffness.

## (4) Obtain natural frequencies with system identification

#### Unknown system with one input and one output

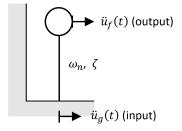
Let's treat these building models as single-degree-of-freedom oscillators with concentrated mass at the top, like the "lollipop" shown below.

This is a very common idealization for structural systems.

There are two parameters that define how this system moves: natural frequency and damping ratio.

parameter	value
$\omega_n$	natural frequency
ζ	damping ratio

These two parameters can be determined experimentally by system identification of the structure's vibrations.



#### 3 Story Stiff Steel

```
In [6]: event_names = ["Northridge", "Sine 1.0", "Sine 1.5", "Sine 2.0", "Bonus Ever
        # Loop through all events
        for event_name in event_names:
            # Load the X component of the bottom sensor as input
            inputs = data[f'Stiff 3 story Ground Floor - ({event name})'][:,0]
            # Don't use the entire time series. Start halfway through the record and
            sixth points = int(inputs.shape[0]/6)
            inputs = inputs[3*sixth points:5*sixth points]
            # Load the top sensor as output
            outputs = data[f'Stiff 3 story 3rd Floor (Top floor) - ({event name})'][
            # Construct the time array
            time = time_array(TIME_STEP, len(inputs))
            # Use the mdof package to perform system identification and determine th
            P, Phi = mdof.modes(inputs, outputs, dt=TIME_STEP)
            # The fundamental natural period is the longest period.
            print(f"Event: {event name} - Fundamental Period: {max(P):.3f} seconds")
       Event: Northridge - Fundamental Period: 0.134 seconds
       Event: Sine 1.0 - Fundamental Period: 0.125 seconds
       Event: Sine 1.5 - Fundamental Period: 0.125 seconds
       Event: Sine 2.0 - Fundamental Period: 0.125 seconds
       Event: Bonus Event - Fundamental Period: 0.128 seconds
```

#### 3 Story Flexible Steel

```
In [7]: event_names = ["Northridge", "Sine 1.0", "Sine 1.5", "Sine 2.0", "Bonus Ever
        # Loop through all events
        for event name in event names:
            # Load the X component of the bottom sensor as input
            inputs = data[f'Flexible 3 story Ground Floor - ({event name})'][:,0]
            # Don't use the entire time series. Start halfway through the record and
            sixth_points = int(inputs.shape[0]/6)
            inputs = inputs[3*sixth points:5*sixth points]
            # Load the top sensor as output
            outputs = data[f'Flexible 3 story 3rd Floor (Top floor) - ({event_name})
            # Construct the time array
            time = time_array(TIME_STEP, len(inputs))
            # Use the mdof package to perform system identification and determine th
            P, Phi = mdof.modes(inputs, outputs, dt=TIME STEP)
            # The fundamental natural period is the longest period.
            realization = mdof.sysid(inputs, outputs)
            print(f"Event: {event_name} - Fundamental Period: {max(P):.3f} seconds")
       Event: Northridge - Fundamental Period: 0.273 seconds
       Event: Sine 1.0 - Fundamental Period: 4.781 seconds
       Event: Sine 1.5 - Fundamental Period: 0.288 seconds
       Event: Sine 2.0 - Fundamental Period: 0.267 seconds
       Event: Bonus Event - Fundamental Period: 0.270 seconds
```

#### 1 Story Stiff Steel

```
In [8]: event_names = ["El Centro Trial 1", "El Centro Trial 2", "Northridge Trial 1
        # Loop through all events
        for event_name in event_names:
            # Load the X component of the bottom sensor as input
            inputs = data[f'Stiff 1 story Ground Floor - ({event_name})'][:,0]
            # Don't use the entire time series. Start halfway through the record and
            sixth points = int(inputs.shape[0]/6)
            inputs = inputs[3*sixth_points:5*sixth_points]
            # Load the top sensor as output
            outputs = data[f'Stiff 1 story Top Floor - ({event name})'][:,0][3*sixth
            # Construct the time array
            time = time_array(TIME_STEP, len(inputs))
            # Use the mdof package to perform system identification and determine th
            P, Phi = mdof.modes(inputs, outputs, dt=TIME_STEP)
            # The fundamental natural period is the longest period.
            realization = mdof.sysid(inputs, outputs)
            print(f"Event: {event_name} - Fundamental Period: {max(P):.3f} seconds")
       Event: El Centro Trial 1 - Fundamental Period: 0.050 seconds
       Event: El Centro Trial 2 - Fundamental Period: 0.049 seconds
       Event: Northridge Trial 1 - Fundamental Period: 0.049 seconds
       Event: Northridge Trial 2 - Fundamental Period: 0.049 seconds
       Event: Sine Wave Trial 1 - Fundamental Period: 0.054 seconds
       Event: Sine Wave Trial 2 - Fundamental Period: 0.050 seconds
```

# (5) Fundamental periods of the three steel models

What is your estimate of the fundamental natural period, in seconds, of the 3 story stiff steel model? of the 3 story flexible steel model? of the 1 story stiff steel model?

Save these values as period\_stiff3, period\_flex3, and period\_flex1.

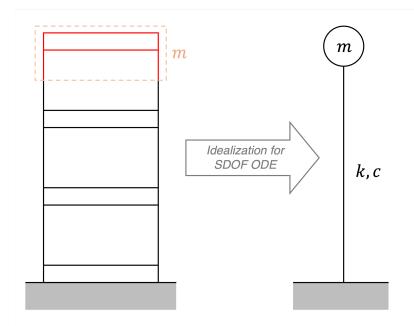
```
In [9]: period_stiff3 = np.mean([0.134,0.125,0.125,0.125,0.128]) # fundamental natur
    period_flex3 = np.mean([0.273,0.288,0.267,0.270]) # fundamental natural peri
    period_stiff1 = np.mean([0.050, 0.049, 0.049, 0.049, 0.054, 0.050]) # fundam
    print(f"{period_stiff3=:.5f}")
    print(f"{period_flex3=:.3f}")
    print(f"{period_stiff1=:.3f}")

    period_stiff3=0.12740
    period_flex3=0.274
    period_stiff1=0.050
```

### (6) Stiffness of each type of steel

Remember the relations,  $\omega_n=\sqrt{\frac{k}{m}}$  and  $T_n=\frac{2\pi}{\omega_n}.$  Note that  $T_n$  is the natural period.

- 1. Derive the relation  $\omega_n=\sqrt{\frac{k}{m}}$  from the governing differential equation of motion for a single degree of freedom (undamped) harmonic oscillator in free vibration,  $m\ddot{u}+ku=0$ .
- 2. Use the relations above, along with the mass of the model materials, to determine the stiffness of each model. Assume the mass is "lumped" at the top floor, and equal to the mass of one polycarbonate block with sensor attached plus a half story of steel wall. Note: the units of stiffness should be a unit of force per unit displacement, such as kg/cm.



Write your answers below, or in a separate document.

1. If 
$$m\ddot{u}+ku=0$$
, then  $u(t)=c_1\cos\omega_n t+c_2\sin\omega_n t$ .

How do we solve for  $\omega_n$  in terms of the values of m and k?

#### [SOLUTION, OPTION A]:

Using the general method for solving a second-order, linear, constant-coefficient ODE:

- Form the characteristic polynomial:  $mr^2 + kr = 0$ .
- Solve the quadratic equation for r:  $r=\pm rac{\sqrt{-4mk}}{2m}$
- Simplify:  $r=\pm i\sqrt{rac{k}{m}}$
- There are two complex roots,  $\alpha+i\beta$ , which means that the solution is of the form  $u(t)=c_1e^{\alpha t}\cos\beta t+c_2e^{\alpha t}\sin\beta t.$

$$ullet$$
 Therefore,  $\overline{\omega_n=eta=\sqrt{rac{k}{m}}}$  .

• (P.S. note that  $\alpha=0$ , so  $e^{\alpha t}=1$  and that term disappears from the solution.)

#### [SOLUTION, OPTION B]:

Since the solution, u(t), of the differential equation is given, we can take its derivative and plug it back into the ODE:

- ODE solution (zeroth derivative):  $u(t) = c_1 \cos \omega_n t + c_2 \sin \omega_n t$
- First derivative:  $\dot{u}(t) = -\omega_n c_1 \sin \omega_n t + \omega_n c_2 \cos \omega_n t$
- Second derivative:  $\ddot{u}(t) = -\omega_n^2 c_1 \cos \omega_n t \omega_n^2 c_2 \sin \omega_n t$
- Plug back into ODE:

$$-m\omega_n^2c_1\cos\omega_nt-m\omega_n^2c_2\sin\omega_nt+kc_1\cos\omega_nt+kc_2\sin\omega_nt=0$$

- Rearrange:  $\backslash \operatorname{red}(k-m\omega_n^2)(c_1\cos\omega_n t + c_2\sin\omega_n t) = 0$
- The term,  $(c_1 \cos \omega_n t + c_2 \sin \omega_n t)$ , can never be zero for all time  $t (\cos \omega_n t)$  and  $\sin \omega_n t$  are "independent"). So, the above equation holds true for all time t if and only if  $(k m\omega_n^2) = 0$ .
- ullet Rearranging,  $\overline{\omega_n = \sqrt{rac{k}{m}}}$
- 2. Start by calculating the "lumped mass."

m = (mass of 1 polycarb block with sensor attached) + 0.5(mass of 6 inches (1 floor) of 2 steel walls).

Then compute k from the relations  $\omega_n=\sqrt{\frac{k}{m}}$  and  $T_n=\frac{2\pi}{\omega_n}$ .

#### [SOLUTION]:

First, calculate the lumped mass.

- Mass of the block with sensor attached =  $0.124 \mathrm{kg}$ .
- 3 Story Stiff model:
  - Weight of steel per unit length: (0.8245 kg 4\*0.124 kg)/(2\*18 in) = 0.00913 kg/in
  - ullet Lumped weight:  $w_{
    m 3st} = 0.124 {
    m kg} + 0.5 (6 {
    m in} * 2 * 0.00913 {
    m kg/in}) = 0.179 {
    m kg}$
- 1 Story Stiff model:
  - Weight of steel per unit length: (0.3589 kg 2 \* 0.124 kg)/(2 \* 6 in) = 0.00924 kg/in
  - ullet Lumped weight:  $w_{1\mathrm{st}} = 0.124\mathrm{kg} + 0.5(6\mathrm{in}*2*0.00924\mathrm{kg/in}) = 0.179\mathrm{kg}$
- Flexible model:
  - Weight of steel per unit length: (0.6045 kg 4 \* 0.124 kg)/(2 \* 18 in) = 0.00301 kg/in

• Lumped weight:  $w_{3\text{fl}} = 0.1240 \text{g} + 0.5(6 \text{in} * 2 * 0.00301 \text{g/in}) = 0.142 \text{kg}$ 

Then compute k.

```
• Rearranging the two equations, k = m(\frac{2\pi}{T})^2.
```

- 3 Story Stiff Steel Stiffness:  $k_{
  m 3st}=0.179 {
  m kg}*rac{1}{981 {
  m cm/s}^2}(rac{2\pi}{0.127 {
  m s}})^2= \boxed{0.44 {
  m kg/cm}}$
- 1 Story Stiff Steel Stiffness:  $k_{1\mathrm{st}}=0.179\mathrm{kg}*rac{1}{981\mathrm{cm/s^2}}(rac{2\pi}{0.050\mathrm{s}})^2=$   $\boxed{2.87\mathrm{kg/cm}}$
- 3 Story Flexible Steel Stiffness:  $k_{3\mathrm{fl}}=0.142\mathrm{kg}*rac{1}{981\mathrm{cm/s^2}}(rac{2\pi}{0.274\mathrm{s}})^2=\boxed{0.08\mathrm{kg/cm}}$

**Note**: due to rounding, the stiffnesses may be off by up to 0.01 kg/cm.

```
In [10]: m_block = 0.1240 # kilograms
         w_st3 = (0.8245-4*m_block)/(2*18) # kilograms per inch
         print(f"Weight of 3 story stiff steel per unit length: {w st3:.5f} kg/in")
         w_st3_lumped = m_block + 0.5*(6*2*w_st3) # kilograms
         print(f"Lumped weight of 3 story stiff steel model: {w_st3_lumped:.3f} kg")
         k st3 = (w st3 lumped/981)*(2*np.pi/period stiff3)**2 # kilograms per centin
         print(f"Stiffness, k, of 3 story stiff steel model: {k_st3:.3f} kg/cm")
         w_st1 = (0.3589-2*m_block)/(2*6) # kilograms per inch
         print(f"\nWeight of 1 story stiff steel per unit length: {w_st1:.5f} kg/in")
         w_st1_lumped = m_block + 0.5*(6*2*w_st1) # kilograms
         print(f"Lumped weight of 1 story stiff steel model: {w_st1_lumped:.3f} kg")
         k st1 = (w st1 lumped/981)*(2*np.pi/period stiff1)**2 # kilograms per centin
         print(f"Stiffness, k, of 1 story stiff steel model: {k_st1:.3f} kg/cm")
         w fl3 = (0.6045-4*m block)/(2*18) # kilograms per inch
         print(f"\nWeight of 3 story flexible steel per unit length: {w_fl3:.5f} kg/i
         w_fl3_lumped = m_block + 0.5*(6*2*w_fl3) # kilograms
         print(f"Lumped weight of 3 story flexible steel model: {w fl3 lumped:.3f} kd
         k_fl3 = (w_fl3_lumped/981)*(2*np.pi/period_flex3)**2 # kilograms per centime
         print(f"Stiffness, k, of 3 story flexible steel model: {k_fl3:.3f} kg/cm")
        Weight of 3 story stiff steel per unit length: 0.00913 kg/in
        Lumped weight of 3 story stiff steel model: 0.179 kg
        Stiffness, k, of 3 story stiff steel model: 0.443 kg/cm
        Weight of 1 story stiff steel per unit length: 0.00924 kg/in
        Lumped weight of 1 story stiff steel model: 0.179 kg
        Stiffness, k, of 1 story stiff steel model: 2.869 kg/cm
        Weight of 3 story flexible steel per unit length: 0.00301 kg/in
        Lumped weight of 3 story flexible steel model: 0.142 kg
        Stiffness, k, of 3 story flexible steel model: 0.076 kg/cm
```