

SHYFEM  
Finite Element Model for Coastal Seas  
  
User Manual

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# Chapter 1

## Overview

### 1.1 What is it

The finite element program *SHYFEM* is a program package that can be used to resolve the hydrodynamic equations in lagoons, coastal seas, estuaries and lakes. The program uses finite elements for the resolution of the hydrodynamic equations. These finite elements, together with an effective semi-implicit time resolution algorithm, makes this program especially suitable for application to a complicated geometry and bathymetry.

This version of the program *SHYFEM* resolves the depth integrated shallow water equations. It is therefore recommended for the application of very shallow basins or well mixed estuaries. Storm surge phenomena can be investigated also. This two-dimensional version of the program is not suited for the application to baroclinic driven flows or large scale flows where the Coriolis acceleration is important.

Finite elements are superior to finite differences when dealing with complex bathymetric situations and geometries. Finite differences are limited to a regular outlay of their grids. This will be a problem if only parts of a basin need high resolution. The finite element method has an advantage in this case allowing more flexibility with its subdivision of the system in triangles varying in form and size.

This model is especially adapted to run in very shallow basins. It is possible to simulate shallow water flats, i.e., tidal marshes that in a tidal cycle may be covered with water during high tide and then fall dry during ebb tide. This phenomenon is handled by the model in a mass conserving way.

Finite element methods have been introduced into hydrodynamics since 1973 and have been extensively applied to shallow water equations by numerous authors [7, 15, 9, 8, 10]. The model presented here [16, 17] uses the mathematical formulation of the semi-implicit algorithm that decouples the solution of the water levels and velocity components from each other leading to smaller systems to solve. Models of this type have been presented from 1971 on by many authors [11, 4, 1].

## Chapter 2

# Installing SHYFEM

### 2.1 Downloading and unpacking

The source code of the model is provided in a file named `shyfem-7_0_6.tar.gz` or similar, depending on the version of the code. In this case the version is 7\_0\_6. The file can be downloaded from the SHYFEM web-site<sup>1</sup>, or by contacting directly its authors.

Once you have downloaded the model distribution, move the file to the directory in which you want to install the model and unpack the distribution. In the following we will assume that the file is in your home directory and your home directory is called `/home/model`. However, any other directory works as well. To unpack the distribution in your home directory, move there and run the command:

```
cd /home/model
tar -xzf shyfem-7_0_6.tar.gz
```

At this point a new folder named `/home/model/shyfem-7_0_6` has been created. This directory is the root of the SHYFEM model. All other commands given in this chapter assume that you are in this directory. Therefore, before reading on, please move into this directory:

```
cd /home/model/shyfem-7_0_6
```

### 2.2 Needed software

The source code is composed mainly of Fortran 77 files, but files written in C, Fortran 90, Perl and Shell scripts are also present.

In order to use the model you have to compile it in a Linux Operating System. Several software products must be present in order to be able to compile the model. Please refer to the documentation of your Linux distribution for installing these programs.

- The package `make` is required for compilation.
- The `perl` interpreter, the `bash` shell and the `gcc` C compiler are necessary for compiling.
- A Fortran 77 and 90 compiler. Supported compilers are the Gnu `g77` and `g90` compilers, the new Gnu compiler `gfortran`, the Intel Fortran compiler `ifort` or the Portland group `pgf90` Fortran compiler.

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<sup>1</sup><http://www.ismar.cnr.it/shyfem/>

Please note that you might already have everything available in your Linux distribution, with the exception maybe of the Fortran compiler.

To find out what software is installed on your computer and what you still have to install you can run the following command:

```
make check_software
```

If you get something like `bash: make: command not found`, then you do not have `make` installed. Please first install the `make` command and then run the command again.

The output of the command will show you what software you will still have to install. The software is divided into different sections. The first section is needed software, which you will not be able to do without. The next section is recommended software, which you really should install, but for compilation and running you will not necessarily need it. The last section is software which is optional, but which makes life easier.

You can always run `make check_software` again to check if the software had been successfully installed. When you are satisfied with the output you can go to the next section.

Please note that you have to carry out the steps in this section only the first time you install the model. If you install a new version of SHYFEM software you can skip these steps.

## 2.3 Installation

Before compiling it is advisable to install some files for a simpler usage of the model. As long as you only want to run a simulation, this step is not strictly necessary. But if you will run some scripts of the distribution, these scripts will not work properly if you do not install the model.

In order to install the model, you should run

```
make install
```

This command will do the following:

- It hardcodes the installation directory in all scripts of the model so only programs of the installed version will be executed.
- It inserts a symbolic link `shyfem` from the home directory to the root of the SHYFEM installation.
- It inserts a small snippet of code into the initialization files `.bashrc` `.bash_profile` `.profile` that are in your home directory. This will adjust your path to point to the SHYFEM directory and gives you access to some administrative commands.

After this command you will find the original files that have been changed in your home directory saved with a trailing number (e.g., `.profile.35624` or similar). If you encounter problems, just substitute back these files.

In order that your new settings will take effect you will have to log out and log in again. Alternatively, you will have to execute one of the files `.bashrc` `.bash_profile` `.profile` that have actually changed. If `.bashrc` has been changed, then run `. ~/.bashrc`.

If you do not want to run the installation routine, you should at least manually insert a symbolic link to the root of the SHYFEM model. Otherwise some of the commands and shell scripts will not work properly. This can be done with the command

```
cd
ln -s /home/model/shyfem-7_0_6 shyfem
```

from your home directory. If there is already such a link existing you will first have to delete it (`rm shyfem`).

You should also think about the possibility to add the fembin path to your default paths to have the main utility commands always available. To do this open your `.bashrc` file in the home directory and add the following lines at the end of the file:

```
SHYFEMDIR=$HOME/fem
PATH=$PATH:$SHYFEMDIR/fembin
export SHYFEMDIR PATH
```

However, the same effect can be achieved more easily by using the abovementioned command `make install`.

If you ever want to uninstall the model, you can do it with the command `make uninstall`. This will delete the symbolic link, cancel the hard links in the model scripts and restore the systemfiles `.bashrc` `.bash_profile` `.profile` to their original content.

Please note that you will still have to delete manually the model directory. This can be done with the command `rm -rf /home/model/shyfem-7.0.6`). In this way, however, changes to the code you have made will be lost.

## 2.4 Compilation

In order to compile the model you will first have to adjust some settings in the `Rules.make` file. Assuming that you are already in the SHYFEM root directory (in our case it would be `/home/model/shyfem-7.0.6`), open the file `Rules.make` with a text editor. In this file the following options can be set:

- **Parameters.** In this section you have to set the maximum number of nodes (`NKNDIM`) and elements (`NELDIM`) used by your grids. You might also have to set also the maximum number of elements attached to a node (`NGRDIM`) and the maximum bandwidth (`MBWDIM`) of the z-level matrix. You can find these numbers when you create the basin file with `vpgrd`. Finally you have to specify the maximum number of vertical levels (`NLVDIM`). It is advisable to set this value close to the desired number of vertical levels, since it affects the model speed performance. So, if you want to run the model in 2D mode, please set `NLVDIM` to 1. For all other possible parameter settings please have a look at `param.h`. However, you should never directly change this file. Always make changes to the parameters in the `Rules.make` file.
- **Compiler.** Set the compiler you want to use. Please see also the section on needed software and the one on compatibility problems to learn more about this choice.
- **Parallel compilation.** Some parts of the code are parallelized with OpenMP statements. Here you can set if you want to use it or not. All supported compilers (except `g77`) accept OpenMP statements.
- **Solver for matrix solution.** There are three different solvers implemented. The `GAUSS` solver is the most robust and best tested solver, but it is quite slow. The `PARDISO` solver needs an external library available at the Intel web-site<sup>2</sup>, that can be freely downloaded for non-commercial use. The `Pardiso` solver is parallelized, but it seems to be a little slower than the `SPARSKIT` solver. The `SPARSKIT` solver is the recommended solver, since it seems to be the fastest one. However, if you are ever in doubt about your results you might want to revert back to the `GAUSS` solver and check the results.

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<sup>2</sup><http://software.intel.com/en-us/articles/code-downloads/>



- `NetCDF` library. If you want output files in NetCDF format you need the NetCDF library.
- `GOTM` library. The GOTM turbulence model is already included in the code. However, a newer and better tested version is available as an external module. In order to use it please set this variable to true. This is the recommended choice. You will need a Fortran 90 compiler to enable this choice.
- `Ecological` module. This option allows for the inclusion of an ecological module into the code. Choices are between `EUTRO`, `ERSEM` and `AQUABC`. Please refer to information given somewhere else on how to run these programs.
- `Compiler` options. Here several sections are present, one for each supported compiler. Normally it should not be necessary to change anything beyond this point.

Once you have set all these options you can start compilation with

```
make clean
make fem
```

This should compile everything. In case of a compilation error you will find some messages during compilation and also at the bottom of the output, where a check is run to see, if the main routines have been compiled.

Please remember that you will always have to run the commands above when you change settings in the `Rules.make` file. If you only change something in the code, or if you only change dimension parameters, it might be enough to run only `make fem`, which only compiles the necessary files. However, if you are in doubt, it is always a good idea to run `make clean` or `make cleanall` before compiling, in order to start from a clean state.

## 2.5 Compatibility problems

The SHYFEM program is designed to work with most of the compilers that are available. Normally there should be no problems with compatibility. However you have to keep in mind some points that are listed below.

The supported compilers for SHYFEM are

- **g77** This is the old GNU compiler. It is Fortran 77 only. It is also a little outdated and is not supported anymore by the major distributions. If you do not need software that relies on Fortran 90 it is still possible to use it. However, GOTM and some ecological modules are relying on Fortran 90 and cannot be used with the `g77` compiler.
- **gfortran** This is the actual GNU compiler. It also supports Fortran 90. This is the compiler that you will find in recent distributions.
- **ifort** This is the INTEL compiler. You will have to download and install it on your own. It is very efficient and normally faster than the GNU compiler.
- **pgf90** This is the Portland group compiler. It is a commercial compiler. It creates very efficient code.

Whatever compiler you choose to use, you have to set your choice in the `Rules.make` file. Even if not desirable, different compilers can give you slightly different results in the computations. This is due to the different optimizations enabled and maybe a different treatment of accuracy and round off. Other compatibility issues are the following.

- With `g77` and `ifort` it is possible to open the same file in read only mode more than once. This is useful, e.g., if you have two open boundaries, but you want to prescribe the same value on these two boundaries. With `gfortran` or `pgf90` you cannot do this. A file, even in read only mode, can be opened only once. In the above example you therefore have to copy the input file to a new name (duplicate it) and then prescribe the two different files as boundary conditions.
- With `gfortran` it is very difficult to decide if a file is formatted or unformatted. Some modules allow the use of either formatted or unformatted input files, where the check on the file type is made via software. In case of `gfortran` this may not work reliably. The only solution to this problem is to specify the file type directly in the code.
- Objects generated during compilation and libraries used in linking are normally not compatible between compilers. What this means is that, when you switch compiler, you will have to recompile everything with `make cleanall; make fem`. Otherwise you will encounter errors during the linkage process.
- Unformatted files are normally not portable between different compilers. You normally cannot use a basin file created with programs compiled with one compiler together with a program compiled with another compiler. The same is true for unformatted data files (initial conditions, wind and meteo forcing, etc.).

If you have problems reading a basin file, try `basinf`. If this is not working chances are high that you have the problem described above. In case of unformatted data files the diagnosis is not so easy. In any case, you can solve the problem recompiling all programs with the commands `make cleantotal; make fem` and then re-creating all unformatted files with the newly compiled programs. In case of the basin file, you will have to run the pre-processor on the grid again.

If you have obtained unformatted data files from others, then there is really no easy solution to this problem. Exchanging unformatted files between different computers and compilers is never a good idea.

- A similar problem exists if you switch files between different architectures (32 bit and 64 bits), even if created with the same compiler. These files are normally not portable.
- Nan values (Not a Number) are treated differently between different compilers. Nan values are created if a not well defined operation is executed (divide by 0 or square root of a negative number). All compilers above (except `pgf90`) treat Nans to be not comparable to any number. This means that a logical expression `a.eq.a` is always false if `a` is a Nan. However the `pgf90` compiler treats Nans to be comparable to any other number. So, an expression like `a.ne.a` will evaluate to true. SHYFEM includes code to handle these problems gracefully, but incompatibilities might still show up.
- In parallel execution you might get a segmentation fault during execution. This is normally due to limited stack size. You can change the behavior by increasing the stack size (`ulimit -s unlimited`) on the console before running the program. Compilers may behave differently. Please see also the section on parallel execution in the file `Rules.make`.

## 2.6 Summary

In summary, the following steps have to be carried out before you will be able to run the model:

- Get the distribution and unpack it in a place of your choice.
- Move to the root of the distribution (e.g., `cd /home/model/shyfem-7_0_6`).
- Check if all software is available (`make check_software`). This step has to be done only the first time you install SHYFEM on a computer.
- Install the model (`make install`). This has to be done every time you get and install a new version of the model.
- Adjust options in `Rules.make`. This has to be done every time you change options (compiler, parallel execution, etc.). After this you have to run also `make cleanall`.
- Adjust dimension parameters in `Rules.make`. This has to be done the first time and every time you change application (basin, etc.) to adapt the dimensions to the new problem. You might also run `make cleanall` after this step, but it is not required.
- Compile the programs with `make fem` and have a look at the error messages.

Moreover, below a summary of administrative commands is given that are available in SHYFEM.

<code>make version</code>	shows version of distribution
<code>make clean</code>	deletes objects and executables from a previous compilation
<code>make cleanall</code>	same as <code>make clean</code> but also deletes compiled libraries
<code>make fem</code>	compiles SHYFEM
<code>make doc</code>	makes this manual ( <code>femdoc/shyfem.pdf</code> )
<code>make check_software</code>	checks the availability of installed software
<code>make check_compilation</code>	checks if all programs have been compiled
<code>make changed</code>	finds files that are changed with respect to the original distribution
<code>make changed_zip</code>	zips files that are changed with respect to the original distribution to the file <code>changed_zip.zip</code>
<code>make install</code>	installs SHYFEM
<code>make uninstall</code>	uninstalls SHYFEM

Finally, if you have installed the model with `make install`, the following utility commands are available

<code>shyfemdir</code>	shows information about actual SHYFEM settings
<code>shyfemdir fem_dir</code>	sets <code>fem_dir</code> to be the new default SHYFEM version
<code>shyfeminstall</code>	shows information about original SHYFEM installation
<code>shyfemcd</code>	moves into root of actual SHYFEM directory

## Chapter 3

# Preprocessing: The numerical grid

### 3.1 Overview

Before you can start using the model you have to create a numerical grid. This step is more difficult for models that work on unstructured grids (like finite element models) than for finite difference models, where often it is enough to have a regular gridded bathymetry to start running simulations.

This chapter describes the steps that you have to take in order to be able to create a numerical grid for SHYFEM. If you are in the happy position to already have a numerical finite element grid, then you can jump ahead to the section on how to transform to/from other grid formats. You might still have to interpolate a bathymetry onto the numerical grid, so you will have to refer to the section on interpolating bathymetry. In any case, at the end you will have to create a (unformatted) basin file, so that SHYFEM is able to read in the information.

The steps that have to be carried out to create a numerical grid are

1. obtain raw digital data of the coastline and the bathymetry
2. convert the digital data into a format GRD that is read by the provided routines
3. prepare the coastline
4. create the numerical grid with an automatic grid generator
5. regularize the grid
6. interpolate bathymetry onto the created grid
7. create the basin file (the finite element version of the grid file)

As mentioned above, some of the steps can be skipped if you already have a finite element grid. If you already have a grid with bathymetry information you can jump to point 7. However, you will have to convert your grid into the GRD format used by SHYFEM. The GRD format is documented in the appendix. For some of the most common unstructured grid formats routines are available to convert between these formats and the GRD format. In any case, the GRD format is quite easy to parse and write, so you might be able to write a transformation routine yourself.

In the following a description is given what you have to do if you start from scratch. Please refer to the section on other programs to create a grid for conversion routines.

## 3.2 Converting to GRD format

Data files with boundary line and bathymetry should be given. These files have to be transformed into GRD files, that can be read and manipulated with the programs `mesh` and `grid`. Examples of how to do so can be found in `coast.pl` and `ldb.pl` for the coastline and `bathy.pl` for the bathymetry points.

```
coast.pl mpcoast.dat > coast.grd
bathy.pl mpbathy.dat > depth.grd
```

Please note that the coordinates for the GRD files should be always in meters. Therefore, if you have your coordinates in other units, you have to adjust the conversion routines in order to create the new coordinates in meters.

Please note that UTM coordinates are in meters, so UTM coordinates are fine. However, since UTM coordinates are normally huge numbers, there might be an accuracy problem when you try to create the grid. If this happens, you should first shift your UTM coordinates so that the origin of your new coordinate system coincides with the central point of your grid. This translation can be done using the program `grd_transl.pl`.

Other transformation routines are:

- `dx2grd.pl` Transforms a grid from DXF (Autocad) to GRD format. This is still experimental.
- `kml2grd.pl` Transforms a grid from the Google Earth format KML to GRD format.
- `xyz2grd.pl` Transforms a simple list of nodes to GRD format. Every line contains 3 values  $(x,y,z)$  or two values  $(x,y)$ , when the information on depth is missing.

Please note that for SHYFEM depth values have to be positive. If your files have depth values as negative numbers, you will have to invert them. You can use the command

```
grd_modify.pl -depth_invert grd-file
```

to achieve this task.

## 3.3 Boundary line: smoothing and reducing

With the routine `grid` the coastline can be viewed. However, normally the line needs some post-processing. It might either have resolution which is too high, island might show up as open lines etc..

It is important that there is one closed boundary line that defines the whole domain of the computation. If you have an open coastline, please close the line with the routine `grid` at the places where you want your open boundary to be.

Once this domain boundary line has been defined, care has to be taken that the lines inside this domain, which denote islands, are closed.

Finally the resolution of the boundary lines (coast and islands) have to be adjusted. If the coastline is left as it is you might have a much too high resolution along the boundaries. This is due to the fact that the meshing algorithm does not discard any points given to it. This means that all boundary nodes are used for the meshing. Therefore, if you have a very high resolution boundary line, you will get many elements along the boundary and relatively little elements (depending on the number of internal points) in the inside of the basin.

Smoothing and reduction of the boundary lines can be done with the routine `reduce`. The command is

```
reduce -s sigma -r reduct coast.grd
```

Here `sigma` specifies the length scale for the smoothing operator and `reduct` is the length scale below which points may be deleted. Both values have to be given in the same units of the coordinates of the file `coast.grd`, so normally meters. The smoothed file can be found in `smooth.grd` and the subsequently reduced file in `reduct.grd`.

If there are some points in the boundary line that should not be smoothed they can be given a depth value of -1. This is a flag that indicates that the position of these points will not be touched.

### 3.4 Other information

Construct a background grid

=====

If you want a grid with a uniform solution all over, then you are already in a position to run the meshing algorithm. You just say: `"mesh -I2000 coast-new.grd"` and then the constructed mesh will be in `final.grd`. The number 2000 means that you want approx. 2000 internal points in the domain. You may adjust this number to your needs.

However, you will normally want to have different resolution in the domain (high at the inlets of lagoons, at interesting sites like harbours etc.). Then you have to construct a background grid that gives an indication to the meshing algorithm what kind of resolution is needed in what area.

You open the coastline with grid and construct elements that cover the parts or all of the domain. The areas where no background grid exists will use the (constant) resolution of the domain computed by the routine `mesh` using the total number of internal nodes (2000 in this example).

Where a background grid exists the model uses the depth values at the element vertices (nodes) to compute a new value for this resolution. The depth value acts like a factor that multiplies the constant overall resolution to obtain a local resolution. So, for example, constructing a background grid and setting all depth values to 1 would not change the resolution at all from a situation without background grid. A factor higher than 1 increases the resolution and one smaller than 1 decreases it. Therefore, in areas where resolution should be higher than average you can set it to 2 or higher, and in other areas, where you want lower resolution, you can set it to 0.5 or lower. All nodes of the background grid need to have a depth (resolution) value. Inside each background element the resolution is interpolated between the three nodes (vertices).

In order to distinguish the background grid from the elements that are constructed by the meshing routine, they must become a unique element type. You can set it to a value that is not used for other elements (99 is a good choice). All elements of the background grid must have this element type.

Please extract the background grid from the grid file you just have constructed by running `exgrd: "exgrd -LS coast-new.grd"`. The file "new.grd" contains only the background grid. Rename it to something more useful (`mv new.grd bck1.grd`). You are then ready to start the meshing algorithm.

```
manually construct background grid using coast-new.grd
delete coastline (leave only background elements in file)
exgrd -LS coast-new.grd
set depth at nodes for resolution
set type in elements to 99
rename to bck1.grd
mv new.grd bck1.grd
```

Meshing of the basin  
=====

The meshing algorithm is called `mesh`. Please see "`mesh -h`" for help of the command line options. The most important are:

```
-I2000 use aprox. 2000 internal nodes for the domain
-g99 element type of background grid is 99
```

With this parameters the call to `mesh` would be  
"`mesh -I2000 -g99 coast-new bck1`".  
The created mesh can be found in `final.grd`.

Please note that you can specify more than one file for the coast line, so you could keep the domain line and the island lines in separate files. You can also have different background grid for different areas in different files. So a call like this is also possible:  
"`mesh -I2000 -g99 coast island1 island2 bck1 bck2 bck3`".

After the meshing please have a look at the result (`final.grd`). If you need more overall resolution, increase the number of internal points (here 2000). If you need more resolution in the background grid, open the background file and increase the factor (depth) value where needed. You might also need other areas with a background grid. Once you are satisfied with the result please save it to a more meaningful name.

```
mesh -I2000 -g99 coast-new bck1
mv final.grd mesh1.grd
```

Adjust elements for regularity  
=====

After the creation of the mesh, the grid is still not good enough for usage in a finite element model. This is due to the fact that the grid is too irregular. Therefore a program has to be applied that regularizes the grid.

The program is called `regularize`. It must be given the input grid file

(irregular) and creates a new one with much more regular characteristics.  
The program has to be called as:  
"regularize mesh1.grd mesh2.grd".  
In this case the new regular grid is in mesh2.grd.

Interpolate bathymetry  
=====

To interpolate bathymetry, a grd file with single points containing depth values has to be available. This file, together with the basin onto which the bathymetry has to be interpolated, has to be specified for the program basbathy. The simplest call is:

```
basbathy mesh2 bathy
```

where bathy.grd is the grd file with the bathymetry values and mesh2 is the basin for which to interpolate the bathymetry. Different types of interpolation can be used. Please run "basbathy -h" for more options.

The new grd file will be in "new.grd".

```
basbathy mesh2 bathy  
mv new.grd mesh3.grd
```

Create basin for FEM model (bandwidth optimization)  
=====

Before proceeding to the simulations we must first create a representation of the basin suitable for the finite element model.

In order to create the finite element representation of the grid, please run "vpgrd mesh3". This creates a file mesh3.bas. This is a binary file suitable for being read by the finite element model.

```
vpgrd mesh3
```

## 3.5 Creating the basin file

The pre-processing routine `vp` is used to generate an optimized version of the file that describes the basin where the main program is to be run. In the following a short introduction in using this program is given.

### 3.5.1 The pre-processing routine `vp`

The main routine `hp` reads the basin file generated by the pre-processing routine `vp` and uses it as the description of the domain where the hydrodynamic equations have to be solved. The program `vp` is started by typing `vp` on the command line. From this point on the program is interactive, asking you about the basin file name and other options. Please



follow the online instructions.

The routine `vp` reads a file of type GRD. This type of file can be generated and manipulated by the program `grid` which is not described here. In short, the file GRD consists of nodes and elements that describe the geometrical layout of the basin. Moreover, the elements have a type and a depth.

The depth is needed by the main program `hp` to run the model. The type of the element is used by `hp` to determine the friction parameter on the bottom, since this parameter may be assigned differently, depending on the various situations of the bottom roughness.

This file GRD is read by `vp` and transformed into an unformatted file BAS. It is this file that is then read by the main routine `hp`. Therefore, if the name of the basin is `lagoon`, then the file GRD is called `lagoon.grd` and the output of the pre-processing routine `vp` is called `lagoon.bas`.

The program `vp` normally uses the depths assigned to the elements in the file GRD to determine the depth of the finite elements to use in the program `hp`. In the case that these depth values are not complete, and that all nodes have depths assigned in the GRD file, the nodal values of the depths are used and interpolated onto the elements. However, if also these nodal depth values are incomplete or are missing altogether, the program terminates with an error.

### 3.5.2 Optimization of the bandwidth

The main task of routine `vp` is the optimization of the internal numbering of the nodes and elements. Re-numbering the elements is just a mere convenience. When assembling the system matrix the contribution of one element after the other has to be added to the system matrix. If the elements are numbered in terms of lowest node numbers, then the access of the nodal pointers is more regular in computer memory and paging is more likely to be inhibited.

However, re-numbering the nodes is absolutely necessary. The system matrix to be solved is of band-matrix type. I.e., non-zero entries are all concentrated along the main diagonal in a more or less narrow band. The larger this band is, the larger the amount of cpu time spent to solve the system. The time to solve a band matrix is of order  $n \cdot m^2$ , where  $n$  is the size of the matrix and  $m$  is the bandwidth. Note that  $m$  is normally much smaller than  $n$ .

If the nodes are left with the original numbering, it is very likely that the bandwidth is very high, unless the nodes in the file GRD are by chance already optimized. Since the bandwidth  $m$  is entering the above formula quadratically, the amount of time spent solving the matrix will be prohibitive. E.g., halving the bandwidth will speed up computations by a factor of 4.

The bandwidth is equal to the maximum difference of node numbers in one element. It is therefore important to re-number the nodes in order to minimize this number. However, there exist only heuristic algorithms for the minimization of this number.

The two main algorithms used in the routine `vp` are the Cuthill McGee algorithm and the algorithm of Rosen. The first one, starting from one node, tries to number all neighbors in a greedy way, optimizing only this step. From the points numbered in this step, the next neighbors are numbered.

This procedure is tried from more than one node, possibly from all boundary nodes. The numbering resulting from this algorithm is normally very good and needs only slight enhancements to be optimum.

Once all nodes are numbered, the Rosen algorithm tries to exchange these node numbers, where the highest difference can be found. This normally gives only a slight improvement of the bandwidth. It has been seen, however, that, if the node numbers coming out from the Cuthill McGee algorithm are reversed, before feeding them into the Rosen algorithm, the results tend to be slightly better. This step is also performed by the program.

All these steps are performed by the program without intervention by the operator, if the automatic optimization of bandwidth is chosen in the program `vp`. The choices are to not

perform the bandwidth optimization at all (GRD file has already optimized node numbering), perform it automatically or perform it manually. It is suggested to always perform automatic optimization of the bandwidth. This choice will lead to a nearly optimum numbering of the nodes and will be by all means good results.

If, however, you decide to do a manual optimization, please follow the online instructions in the program.

### **3.5.3 Internal and external node numbering**

As explained above, the elements and nodes of the basin are re-numbered in order to optimize the bandwidth of the system matrix and so the execution speed of the program.

However, this re-numbering of the node and elements is transparent to the user. The program keeps pointers from the original numbering (external numbers) to the optimized numbering (internal numbers). The user has to deal only with external numbers, even if the program uses internally the other number system.

Moreover, the internal numbers are generated consecutively. Therefore, if there are a total of 4000 nodes in the system, the internal nodes run from 1 to 4000. The external node numbers, on the other side, can be anything the user likes. They just must be unique. This allows for insertion and deletion of nodes without having to re-number over and over again the basin.

The nodes that have to be specified in the input parameter file use again external numbers. In this way, changing the structure of the basin does not at all change the node and element numbers in the input parameter file. Except in the case, where modifications actually touch nodes and elements that are specified in the parameter file.

## Chapter 4

# Running SHYFEM

In the following an overview is given on running the model SHYFEM. The model needs a parameter input file that is read on standard input. Moreover, it needs some external files that are specified in this parameter input file. The model produces several external files with the results of the simulation. Again, the name of this files can be influenced by the parameter input file

### 4.1 How to run: the Parameter Input File (STR)

The model reads one input file that determines the behavior of the simulation. All possible parameters can and must be set in this file. If other data files are to be read, here is the place where to specify them.

The model reads this parameter file from standard input. Thus, if the model binary is called `hp` and the parameter file `param.str`, then the following line starts the simulation

```
ht < param.str
```

and runs the model.

#### 4.1.1 The General Structure of the Parameter Input File

The input parameter file is the file that guides program performance. It contains all necessary information for the main routine to execute the model. Nearly all parameters that can be given have a default value which is used when the parameter is not listed in the file. Only some time parameters are compulsory and must be present in the file.

The format of the file looks very like a namelist format, but is not dependent on the compiler used. Values of parameters are given in the form : `name = value` or `name = 'text'`. If `name` is an array the following format is used :

```
name = value1 , value2, ... valueN
```

The list can continue on the following lines. Blanks before and after the equal sign are ignored. More then one parameter can be present on one line. As separator blank, tab and comma can be used.

Parameters, arrays and data must be given in between certain sections. A section starts with the character `$` followed by a keyword and ends with `$end`. The `$keyword` and `$end` must not contain any blank characters and must be the first non blank characters in the line. Other characters following the keyword on the same line separated by a valid separator are ignored.

Several sections of data may be present in the input parameter file. Further ahead all sections are presented and the possible parameters that can be specified are explained. The

```

$title
    benchmark test for test lagoon
    bench
    venlag
$end

$para
    itanf = 0   itend = 86400   idt = 300
    ireib = 5   czdef = 2.5E-3
    dragco = 2.5E-3
$end

$bound1
    kbound = 73 74 76
    boundn = 'levels1.dat'
$end

$bound2
    kbound = 150 157 97 101
    boundn = 'levels1.dat'
$end

$name
    wind='win18sep.win'
$end

    next are tide gauges used in calibration

$extra ----- tide gauges for calibration -----
13,133,99,259,328,772,419,1141,1195,1070,1064,942,468,1154
73,74,76,353,350,349,1374,1154,1160,1161,408,409,786,795
$end

```

Figure 4.1: Example of a parameter input file (STR file)

sequence in which the sections appear is of no importance. However, the first section must always be section `\$title`, the section that determines the name of simulation and the basin file to use and gives a one line description of the simulation.

Lines outside of the sections are ignored. This gives the possibility to comment the parameter input file.

Figure 4.1 shows an example of a typical input parameter file and the use of the sections and definition of parameters.

## 4.2 Basic usage

This section explains typical usage of the model. It will show how the model can be run doing basic 2D hydrodynamic simulations, simulate a passive tracer, compute T/S, use the Coriolis force and apply wind forcing. More advanced usages of the model, like 3D simulations and the use of the turbulence module will be presented later. This section is conceived as a simple HOWTO document. For the exact meaning and usage of the single parameters, please see the section on input parameters.

To run a simulation, two things are needed. The first is the description of the basin and the

```

$title
    benchmark test for test lagoon
    bench
    venlag
$end

$para
    itanf = 0  itend = 86400  idt = 300
$end

```

Figure 4.2: Example of a basic parameter input file (STR file)

numerical grid, which must be prepared beforehand and then must be compiled in a form that the model can use. How this has been done has already been described in the chapter dealing with preprocessing.

The second thing that is needed is a description of the simulation and the forcings that have to be applied. This is done through a parameter input file. Here we call it *STR* file, because historically these files always ended with an extension of *.str*. However, any extension can be used.

#### 4.2.1 Minimal simulation

A basic version of an *STR* file can be found in 4.2. In fact, it is so basic, it really does not do anything. Here only the compulsory parameters have been inserted. These are:

- An introductory section `$title` where on three lines the following information is given:
  1. A description of the run. This can be any text that fits on one line.
  2. The name of the simulation. This name is used for all files that the simulation produces. These files differ from each other only by their extension.
  3. The name of the basin. This is the basin file without the extension *.bas*.
- A section `$para` that contains all necessary parameters for the simulation to be run. The only compulsory parameters are the ones that specify the start of the simulation `itanf`, its end `itend` and its time step `idt`.

In order to be more helpful, some more information must be added to the *STR* file. As an example let's have a look on 4.1. Here we have added two parameters that deal with the type of friction to be used. `ireib` specifies the bottom friction formulation, here through a simple quadratic bulk formula. (For the exact meaning of the parameters, please refer to the appendix where all parameters are listed.) The parameter `czdef` specifies the value to use for the bottom drag coefficient.

#### 4.2.2 Boundary conditions

In order to have a more meaningful simulation, we need to specify boundary conditions. In this section we will deal with the open boundary conditions, e.g., the conditions at the place where the basin communicates with other water bodies. For lagoons it would be the inlets.

For every boundary condition one section `$bound` must be specified. Since you can have more than one open boundary you must specify also the number of your boundary, e.g.,

\$bound1, \$bound2 etc. Inside every section you can then specify the various parameters that characterize your boundary.

Basically there two types of open boundary conditions. Either the water level or the discharges (fluxes) can be specified. The parameter that decides the type of boundary is `ibtyp`. A value of one indicates water levels, instead a value of 2 or 3 indicates fluxes. If you specify discharges entering at the border of the domain, `ibtyp = 2` should be specified. Otherwise, if there are internal sources in the basin then `ibtyp = 3` must be used. If you do not define this parameter, a value of 1 will be used and water levels will be specified.

The only compulsory parameter in this section is the list of boundary nodes. You do this with the parameter `kbound`. In the case of `ibtype` 1 or 2 at least two nodes must be specified, in order to give an extension of the boundary. The numeration of the boundary nodes must be consecutive and with the basin on its left side when going along the boundary nodes. In the case of `ibtyp = 3` even a single point can be given.

The boundary values you want to give are normally specified through a file with a time series. You give the name of the file that contains the time series with the parameter `boundn`. An example with two boundaries can again be found in Fig. 4.1. Here water levels are prescribed and the values for the water levels are read from a file `levels1.dat`.

If the values on the boundary you want to impose can be described through a simple sinus function, you can also give the boundary values specifying the parameters for the sinus function. An example of a water level boundary with a tide of  $\pm 70cm$  and a period of 12 hours (semi-diurnal) is given in Fig. 4.3. Note that `zref` gives the average water level of the boundary. If you specify `ampli=0` you get a constant boundary value of `zref`.

```
$bound1
    ibtyp = 1    kbound = 23 25 28
    ampli = 0.70 period = 43200 phase = 10800 zref = 0.
$end
```

Figure 4.3: Example of a boundary with regular sinusoidal water levels. The pahse of 10800 (3 hours) makes sure that the simulation starts at slack tide when the basin is completely full.

## 4.3 Advanced usage

### 4.3.1 Variable time step

Generally SHYFEM is run with a fixed time step given by the parameter `idt`. This choice is acceptable when the model runs in unconditionally stable conditions (ie. linear simulation, no horizontal viscosity).

The introduction of the advective terms (`ilin=0`) or horizontal viscosity (`ahpar` greater 0) can introduce instabilities. To be sure that the model runs in stable conditions, it must be assured that the Courant Number is smaller than 1. Please note that only in the case of advection we should call this number the Courant number. However, we will continue to use the term Courant number for all stability related issues.

In the case of advection the Courant number is defined as

$$Cou = \frac{v\Delta t}{\Delta x} \quad (4.1)$$

where  $v$  is the current speed,  $\Delta t$  the time step and  $\Delta x$  the element size. For finite elements, due to the triangular grid, this expression is slightly more complicated. As can be seen, lowering the time step will bring the Courant number below the limit of 1.

To keep the Courant Number under the limit it is necessary to adapt the time step at every computation. The variable timestep is computed introducing in the STR file in the `$para` section the parameters `itsplt`, `coumax` and `idtsyn`.

`coumax` gives the limit of the Courant number. This is normally 1, but since no exact stability limit can be derived for the non-linear advective terms, another value can be specified. If instabilities arise, a slightly lower value than 1 (0.9) can be tried.

`itsplt` decides about the time step splitting. If this value is 0, the time step will be kept constant at its initial value. A value of 1 divides the initial time step into (possibly) equal parts, but makes sure that at the end of the micro time steps one complete macro time step has been executed. The last mode `itsplt = 2` does not care about the macro time step, but always uses the biggest time step possible. In this case it is not assured that after some micro time steps a macro time step will be recovered. Please note that the initial macro time step `idt` will never be exceeded.

Finally, the parameter `idtsyn` is only used in case of `itsplt = 2`. This parameter makes sure that after a time of `idtsyn` the time step will be synchronized to this time. Therefore, setting `idtsyn = 3600` means that there will be a time stamp every hour, even if the model has to take one very small time step in order to reach that time.

An example of how to set the variable time stepping scheme is shown in Fig. 4.4. Here the Courant number is lowered to 0.9 and the variable time step is synchronized every 3600 seconds (1 hour).

```
$para
    coumax = 0.9    itsplt = 2    idtsyn = 3600
$end
```

Figure 4.4: Example of variable time step settings. The time step is synchronized at every hour, and the Courant number is lowered to 0.9.

### 4.3.2 3D computations

The basic way to run the model is in 2D, computing for each element of the grid one value for the whole water column. All the variables are computed in the center of the layer, halfway down the total depth. Deeper basins or highly variable bathymetry can require for the correct reproduction of the velocities, temperature and salinity the need for 3D computation.

The 3D computation is performed on the basis of  $z$  layers. In this representation each layer horizontally has constant depth over the whole basin, but vertically the layer thickness may vary between different layers. However, the first layer (surface layer) is of varying thickness because of the water level variation, and the last layer of an element might be only partially present due to the bathymetry.

Layers are counted from the the surface layer (layer 1) down to the maximum layer, depending again on the local depth. Therefore, elements (and nodes) normally have a different total number of layers from one to each other. This is opposed to sigma layers where the number of total layers is constant all over the basin, but the thickness of each layer varies between different elements.

In order to use layers for 3D computations a new section `$layers` has to be introduced into the STR file, where the sequence of depth values of the bottom of the layers has to be declared. Please, make sure that in the file `Rules.make`, the number of allowed levels `nlvdim` is greater or equal than the ones actually used in the STR file. Layer depths must be declared in increasing order. An example of a `$layer` section is given in figure 4.5. Please note that the maximum depth of the basin in the example must not exceed 20 m.

A specific treatment for the bottom layer has to be carried out. In fact, if the model runs on basins with variable bathymetry, for each element there will be a different total number of

```

$layers
2 4 6 8 10 13 16 20
$end

```

Figure 4.5: Example of section `$layers`. The maximum depth of the basin is 20 meters. The first 5 layers have constant thickness of 2 m, while the last three vary between 3 and 4 m.

layers. The bathymetric value normally does not coincide with one of the layer depths, and therefore the last layer must be treated separately.

To declare how to treat the last layer two parameters have to be inserted in the `$para` section. The first is `hlvmin`, the minimum depth, expressed as a percentage with respect to the full layer depth, ranging between 0 and 1. This is the fraction that the last layer must have in order to be maintained as a separate layer. The second parameter is `ilytyp` and it defines the kind of adjustment done on the last layer. If it is set to 0 no adjustment is done, if it is set to 1 the depth of the last layer is adjusted to the one declared in the `STR` file (full layer change). If it is 2 the adjustment to the previous layer is done only if the fraction of the last layer is smaller than `hlvmin` (change of depth). If it is 3 (default) the bathymetric depth is kept and added to the last but one layer. Therefore with a value of 0 or 3 the total depth will never be changed, whereas with the other levels the total depth might be adjusted.

As an example, take the layer definition of Fig. 4.5. Let `hlvmin` be set to 0.5, and let an element have a depth of 6.5 m. The total number of layers is 4, where the first 3 have each a thickness of 2 m and the last layer of this element (layer 4) is 0.5 m. However, the nominal thickness of layer 4 is 2 m and therefore its relative thickness is 0.25 which is smaller than `hlvmin`. With `ilytyp=0` no adjustment will be done and the total number of layers in this element will be 4 and the last layer will have a thickness of 0.5 m. With `ilytyp=1` the total number of layers will be changed to 3 (all of them with 2 m thickness) and the total depth will be adjusted to 6 m. The same will happen with `ilytyp=2`, because the relative thickness in layer 4 is smaller than `hlvmin`. Finally, with `ilytyp=3` the total number of layers will be changed to 3 but the remaining depth of 0.5 m will be added to layer 3 that will become 2.5 m.

In the case the element has a depth of 7.5 m, the relative thickness is now 0.75 and greater than `hlvmin`. In this case, with `ilytyp=0, 2 and 3` no adjustment will be done and the total number of layers in this element will be 4 and the last layer will have a thickness of 1.5 m. With `ilytyp=1` the total number of layers will be kept as 4 but the total depth will be adjusted to 8 m. This will make all layers equal to 2 m thickness.

The introduction of layers requires also to define the values of vertical eddy viscosity and eddy diffusivity. In any case a value of these two parameters has to be set if the 3D run is performed. This could be done by setting a constant value of the parameters `vistur` (vertical viscosity) and `diftur` (vertical diffusivity). In this case possible values are between  $1 \cdot 10^{-2}$  and  $1 \cdot 10^{-5}$ , depending on the stability of the water column. Higher values ( $1 \cdot 10^{-2}$ ) indicate higher stability and a stronger barotropic behavior.

The other possibility is to compute the vertical eddy coefficients through a turbulence closure scheme. This usage will be described in the section on turbulence.

### 4.3.3 Baroclinic terms

The baroclinic pressure gradient term permits to compute the variation of velocity due to the horizontal gradients of temperature and/or salinity. These gradients act on the horizontal variation of density.

If the variations of temperature and salinity and the baroclinic pressure gradient term has to be computed, the parameter `ibarcl` (in section `$para`) must be set different from 0.



Setting `ibarcl` to a value different from 0 will simulate the transport and diffusion of temperature and salinity in the basin. A value of 1 will compute the full baroclinic pressure terms. A value of 2 will do diagnostic simulations. This means that baroclinic pressure terms are still included in the hydrodynamic equations, but temperature and salinity will not be computed but will be read from a file. Finally for `ibarcl=3` temperature and salinity will be computed but no baroclinic pressure term will be used. In this case the hydrodynamic equations and the equations for temperature and salinity are decoupled and there is no feed back from the density field to the currents.

In any case, if temperature and salinity are computed, first they must be initialized either with constant values or with variable 3D matrices. In the first case the reference values have to be imposed in `temref` and `salref`. An example of this type of simulation is given in Fig. 4.6.

If the temperature and salinity are given as 3D matrices files, they must be provided in the `$name` section, giving the file names in `tempin` and `salin`. In case of diagnostic simulations the matrices of temperature and salinity have to be provided in the files named `tempd` and `saltd` and data must be available for the whole period of simulation.

```
$para
    ibarcl = 1    temref = 18.    salref = 35.
$end
```

Figure 4.6: Example of baroclinic simulation. The initial values for temperature and salinity are set to 18 C and 35.

#### 4.3.4 Turbulence

In the Reynolds equations turbulent eddy diffusivities and viscosities are introduced into the equations that must be parameterized and given some value. Moreover SHYFEM assumes the hydrostatic approximation. Therefore, there is the need to parameterize the non-hydrostatic effects. These are considered sub-scale processes which are mainly of convective nature.

Vertical eddy viscosities and diffusivities have to be defined if there is the intent to model the turbulence effects. These vertical eddy viscosities and diffusivities can be set to constant values, defining `vistur` and `diftur` in the `$para` section. There is also the opportunity to compute, at each timestep, variable values of them, using the turbulence closure module.

The parameter that has to be set in order to choose the turbulence scheme is `iturb` in the `$para` section.. If `iturb=0` the vertical eddy viscosity and eddy diffusivity are set constant (default 0) and must be defined in `vistur` and `diftur`.

If `iturb=1` the turbulence closure scheme applied is the  $k - \epsilon$  model. If `iturb=2` the GOTM turbulence closure module is used. In this case the file `gotmturb.nml` must be provided that sets all necessary parameters. This file must be declared in the section `$name` for the item `gotmpa`.

A default `gotmturb.nml` file is provided and it allows the computation of the vertical eddy viscosity and eddy diffusivity by means of the GOTM  $k - \epsilon$  model. More information on the GOTM turbulence closure module can be found in the GOTM Manual <sup>1</sup>.

If the turbulence module should be used, a value of `iturb=2` is recommended. An example of the settings for the turbulence closure scheme is given in Fig. 4.7.

<sup>1</sup><http://www.gotm.net/index.php?go=documentation>

```

$para
    iturb = 2
$end
$name
    gotmpa = 'gotmturb.nml'
$end

```

Figure 4.7: Example of turbulence settings. The GOTM module for the turbulence closure is used. The parameters are contained in file gotmturb.nml.

### 4.3.5 Sediment transport

The sediment transport module calculates sediment transport for currents only or combined waves and currents over either cohesive and non-cohesive sediments. The core of this module is derived by the SEDTRANS05 sediment transport model, which is here coupled with SHYFEM. The sediment transport module computes the erosion and deposition rates at every mesh node and determines the sediment volume that is injected into the water column. After this step the sediments are advected with the transport and diffusion module described above. The module update every time step the characteristics of the bottom in terms of grainsize composition and sediment density.

The sediment transport module is activated by setting `isedi = 1` and `sedgrs` in the section `sedtr`. For more details about the parameters see the Appendix C.

For more information about the sediment transport module please refer to Neumeier et al. [12] and Ferrarin et al. [6].

#### Sediment transport formulation

For the non-cohesive sediment there are two transport mechanisms, the bedload and the suspended transport, while for the cohesive sediment is assumed to exist only the suspended transport. Then we use the sediment continuity equation for the non-cohesive bedload transport and the transport and diffusion equation to describe the transport of the suspended sediment, both cohesive and non-cohesive. For the bedload component a direct advection scheme is used. Five methods are proposed to predict sediment transport for non-cohesive sediments. The methods of Brown [3], Yalin [22] and Van Rijn [19] predict the bedload transport. The methods of Engelund and Hansen [5] and Bagnold [2] predict the total load transport.

Different approaches have been used to compute the sediment flux between water column and bottom for cohesive and non-cohesive sediment. For cohesive sediments the model computes erosion and deposition rates, while for non-cohesive the net sediment flux between bottom and water column is computed as the difference between the equilibrium concentration and the existing concentration in the lower level. Here the source (erosion occurs, flux from the bottom to the water column) term has been taken as explicit, whereas the sink term as semi-implicit (deposition occurs, flux from the water column to the bottom). This approach permits to avoid negative concentration due to deposition higher than the sediment mass present in the water column.

The vertical mixing coefficient has been calculated using analytical expressions given by [19] for both the case of current related and wave related turbulent mixing.

#### Bed representation

The bed module is designed to have spatially different characteristics, such as grainsize composition, sediment density and critical stress for erosion. The bed is subdivided in several layers and levels. Each layer is considered homogeneous, well mixed and charac-

terized by its own grain size distribution (fraction of each class of sediment considered). On the level are defined the dry bulk density  $\rho_{dry}$  and the critical stress for erosion  $\tau_{ce}$ ; it is assumed that these variables vary linearly between two levels. These characteristics could vary spatially in the domain.

At each location the uppermost layer has to be always greater or equal to the surficial active, or mixed, layer that is available for suspension. Sediment below the active layer is unavailable resuspension until the active layer moves downward either because erosion has occurred, or it has thickened due to increase shear stress. As active layer is considered the bottom roughness height considered as the sum of the grain roughness, the bedload roughness and the bedform (ripple) roughness.

Multiple sand grain size classes are considered to behave independently. At each location the average grain size (based on the sediment fractions) is used to compute the bed roughness and critical shear velocities. Modification of the bed elevation and to the grain size distribution are updated at each time step based on the net erosion and deposition. For each size class, the volume of sediment removed from the bed during any time step is limited by the amount available in the active layer. In this way the model takes into account time-dependent and spatial sediment distribution and bed armouring.

### **Sand-mud mixture**

The morphological behaviour of estuaries and lagoons often depend on non-cohesive as well as cohesive sediments. Prediction the distribution of sediments in these environments, characterized by zonation of sand, mud and mixed deposits, is of crucial importance for sustainable management and development of such systems.

Based on laboratory and field experiments several researchers identified a transition from non-cohesive to cohesive behaviour at increasing mud content in a sand bed. A sand bed with small amount of mud already shows increased resistance against erosion. Above a critical mud content ( $\% < 0.063$  mm), the bed behaved cohesively. The critical mud content depend on the history of the bed and on the geochemical properties of the sand and mud and could varies from 3-20 %. Such a wide range clearly demonstrate that the parameters governing the erosion behaviour of sand-mud mixture, are not fully understood yet. For this reason is crucial to estimate the critical mud or clay content from laboratory and field experiments.

Below the critical mud content the sediments particles are eroded as non-cohesive sediments. It is assumed that the sediments in suspension always behave independently, either if flocculation processes of the thinner particles could trap sand grains into the floc.

Moreover sand increases the binding between the clay particles and results in a more compact and dense matrix which is more resistant to erosion. For non-consolidated cohesive bed adding sand to mud increase the erosion resistance because of the increased density and consolidation rate. The improved critical stress for erosion due to sand particles is taken into consideration increasing the dry density with the sand fraction.

### **Sediment model output**

The sediment transport model writes the following output:

- erosion/deposition [m], 2D variable 80 in file SED
- average grain size of first bottom layer [m], 2D variable 81 in file SED
- bed shear stress [Pa] of first bottom layer, 2D variable 82 in file SED
- updated node depth [m], 2D variable 83 in file SED
- total bedload transport [kg/ms], 2D variable 84 in file SED

- total suspended concentration [kg/m<sup>3</sup>], 3D variable 85 in file SCO

The time step and start time for writing to file SED and SCO are defined by the parameters `idtcon` and `itmcon` in the `para` section. These parameter are the same used for writing tracer concentration, salinity and water temperature. If `idtcon` is not defined, then the sediment model does not write any results.

#### 4.3.6 Coupling to the wave model

SHYFEM could be coupled with the spectral wind wave unstructured model WWMIII or computes wave characteristics using empirical prediction equations.

This empirical wave module is used to calculate the wave height and period from wind speed, fetch and depth using the EMPIRICAL PREDICTION EQUATIONS FOR SHALLOW WATER [18].

WWMIII is not provided in the SHYFEM distribution. The coupling of SHYFEM with WWMIII is done through the FIFO PIPE mechanism. The numerical mesh need to be converted to the GR3 format using the `bas2wwm` program. WWMIII needs its own input parameter file (`wwminput.nml`). The use of the coupled SHYFEM-WWMIII model require additional software which is not described here.

The wave module writes in the WAV file the following output:

- significant wave height [m], variable 31
- mean wave perios [s], variable 32
- mean wave direction [deg], variable 33

The time step and start time for writing to file WAV are defined by the parameters `idtcon` and `itmcon` in the `para` section. These parameter are the same used for writting tracer concentration, salinity and water temperature. If `idtcon` is not defined, then the wave module does not write any results. The wave results can be plotted using `plots -wav`.

In case of SHYFEM-WWMIII coupling several variables are exchanged between the two models:

- SHYFEM sends to WWMIII:
  - surface velocities
  - water level
  - bathymetry e number of vertical layers
  - 3D layer depths
  - wind components\*\*
- SHYFEM reads from WWMIII:
  - gradient of the radiation stresses
  - significant wave heigh
  - mean period
  - significant wave direction
  - wave supported stress
  - peak period
  - wave lenght
  - orbital velocity
  - stokes velocities

- wind drag coefficient
- wave pressure
- wave dissipation
- wind components\*\*

\*\*Wind could be either read from SHYFEM or WWMIII, see parameter `iwave` in Appendix C. For more information about WWMIII and its coupling with SHYFEM please refer to Roland et al. [13] and Ferrarin et al. [6].

## Appendix A

# Hydrodynamic equations and resolution techniques

### A.1 Equations and Boundary Conditions

The equations used in the model are the well known vertically integrated shallow water equations in their formulation with water levels and transports.

$$\frac{\partial U}{\partial t} + gH \frac{\partial \zeta}{\partial x} + RU + X = 0 \quad (\text{A.1})$$

$$\frac{\partial V}{\partial t} + gH \frac{\partial \zeta}{\partial y} + RV + Y = 0 \quad (\text{A.2})$$

$$\frac{\partial \zeta}{\partial t} + \frac{\partial U}{\partial x} + \frac{\partial V}{\partial y} = 0 \quad (\text{A.3})$$

where  $\zeta$  is the water level,  $u, v$  the velocities in  $x$  and  $y$  direction,  $U, V$  the vertical integrated velocities (total or barotropic transports)

$$U = \int_{-h}^{\zeta} u \, dz \quad V = \int_{-h}^{\zeta} v \, dz$$

$g$  the gravitational acceleration,  $H = h + \zeta$  the total water depth,  $h$  the undisturbed water depth,  $t$  the time and  $R$  the friction coefficient. The terms  $X, Y$  contain all other terms that may be added to the equations like the wind stress or the nonlinear terms and that need not be treated implicitly in the time discretization. following treatment.

The friction coefficient has been expressed as

$$R = \frac{g\sqrt{u^2 + v^2}}{C^2 H} \quad (\text{A.4})$$

with  $C$  the Chezy coefficient. The Chezy term is itself not retained constant but varies with the water depth as

$$C = k_s H^{1/6} \quad (\text{A.5})$$

where  $k_s$  is the Strickler coefficient.

In this version of the model the Coriolis term, the turbulent friction term and the nonlinear advective terms have not been implemented.

At open boundaries the water levels are prescribed. At closed boundaries the normal velocity component is set to zero whereas the tangential velocity is a free parameter. This corresponds to a full slip condition.

## A.2 The Model

The model uses the semi-implicit time discretization to accomplish the time integration. In the space the finite element method has been used, not in its standard formulation, but using staggered finite elements. In the following a description of the method is given.

### A.2.1 Discretization in Time - The Semi-Implicit Method

Looking for an efficient time integration method a semi-implicit scheme has been chosen. The semi-implicit scheme combines the advantages of the explicit and the implicit scheme. It is unconditionally stable for any time step  $\Delta t$  chosen and allows the two momentum equations to be solved explicitly without solving a linear system.

The only equation that has to be solved implicitly is the continuity equation. Compared to a fully implicit solution of the shallow water equations the dimensions of the matrix are reduced to one third. Since the solution of a linear system is roughly proportional to the cube of the dimension of the system the saving in computing time is approximately a factor of 30.

It has to be pointed out that it is important not to be limited with the time step by the CFL criterion for the speed of the external gravity waves

$$\Delta t < \frac{\Delta x}{\sqrt{gH}}$$

where  $\Delta x$  is the minimum distance between the nodes in an element. With the discretization described below in most parts of the lagoon we have  $\Delta x \approx 500\text{m}$  and  $H \approx 1\text{m}$ , so  $\Delta t \approx 200$  sec. But the limitation of the time step is determined by the worst case. For example, for  $\Delta x = 100\text{ m}$  and  $H = 40\text{ m}$  the time step criterion would be  $\Delta t < 5\text{ sec}$ , a prohibitive small value.

The equations (1)-(3) are discretized as follows

$$\frac{\zeta^{n+1} - \zeta^n}{\Delta t} + \frac{1}{2} \frac{\partial(U^{n+1} + U^n)}{\partial x} + \frac{1}{2} \frac{\partial(V^{n+1} + V^n)}{\partial y} = 0 \quad (\text{A.6})$$

$$\frac{U^{n+1} - U^n}{\Delta t} + gH \frac{1}{2} \frac{\partial(\zeta^{n+1} + \zeta^n)}{\partial x} + RU^{n+1} + X = 0 \quad (\text{A.7})$$

$$\frac{V^{n+1} - V^n}{\Delta t} + gH \frac{1}{2} \frac{\partial(\zeta^{n+1} + \zeta^n)}{\partial y} + RV^{n+1} + Y = 0 \quad (\text{A.8})$$

With this time discretization the friction term has been formulated fully implicit,  $X, Y$  fully explicit and all the other terms have been centered in time. The reason for the implicit treatment of the friction term is to avoid a sign inversion in the term when the friction parameter gets too high. An example of this behavior is given in Backhaus [1].

If the two momentum equations are solved for the unknowns  $U^{n+1}$  and  $V^{n+1}$  we have

$$U^{n+1} = \frac{1}{1 + \Delta t R} \left( U^n - \Delta t gH \frac{1}{2} \frac{\partial(\zeta^{n+1} + \zeta^n)}{\partial x} - \Delta t X \right) \quad (\text{A.9})$$

$$V^{n+1} = \frac{1}{1 + \Delta t R} \left( V^n - \Delta t gH \frac{1}{2} \frac{\partial(\zeta^{n+1} + \zeta^n)}{\partial y} - \Delta t Y \right) \quad (\text{A.10})$$

If  $\zeta^{n+1}$  were known, the solution for  $U^{n+1}$  and  $V^{n+1}$  could directly be given. To find  $\zeta^{n+1}$  we insert (A.9) and (A.10) in (A.6). After some transformations (A.6) reads

$$\zeta^{n+1} = (\Delta t/2)^2 \frac{g}{1 + \Delta t R} \left( \frac{\partial}{\partial x} \left( H \frac{\partial \zeta^{n+1}}{\partial x} \right) + \frac{\partial}{\partial y} \left( H \frac{\partial \zeta^{n+1}}{\partial y} \right) \right)$$

$$\begin{aligned}
&= \zeta^n + (\Delta t/2)^2 \frac{g}{1 + \Delta t R} \left( \frac{\partial}{\partial x} (H \frac{\partial \zeta^n}{\partial x}) + \frac{\partial}{\partial y} (H \frac{\partial \zeta^n}{\partial y}) \right) \\
&- (\Delta t/2) \left( \frac{2 + \Delta t R}{1 + \Delta t R} \right) \left( \frac{\partial U^n}{\partial x} + \frac{\partial V^n}{\partial y} \right) \\
&+ \frac{\Delta t^2}{2(1 + \Delta t R)} \left( \frac{\partial X}{\partial x} + \frac{\partial Y}{\partial y} \right)
\end{aligned} \tag{A.11}$$

The terms on the left hand side contain the unknown  $\zeta^{n+1}$ , the right hand contains only known values of the old time level. If the spatial derivatives are now expressed by the finite element method a linear system with the unknown  $\zeta^{n+1}$  is obtained and can be solved by standard methods. Once the solution for  $\zeta^{n+1}$  is obtained it can be substituted into (A.9) and (A.10) and these two equations can be solved explicitly. In this way all unknowns of the new time step have been found.

Note that the variable  $H$  also contains the water level through  $H = h + \zeta$ . In order to avoid the equations to become nonlinear  $\zeta$  is evaluated at the old time level so  $H = h + \zeta^n$  and  $H$  is a known quantity.

### A.2.2 Discretization in Space - The Finite Element Method

While the time discretization has been explained above, the discretization in space has still to be carried out. This is done using staggered finite elements. With the semi-implicit method described above it is shown below that using linear triangular elements for all unknowns will not be mass conserving. Furthermore the resulting model will have propagation properties that introduce high numeric damping in the solution of the equations. For these reasons a quite new approach has been adopted here. The water levels and the velocities (transports) are described by using form functions of different order, being the standard linear form functions for the water levels but stepwise constant form functions for the transports. This will result in a grid that resembles more a staggered grid in finite difference discretizations.

#### Formalism

Let  $u$  be an approximate solution of a linear differential equation  $L$ . We expand  $u$  with the help of basis functions  $\phi_m$  as

$$u = \phi_m u_m \quad m = 1, K \tag{A.12}$$

where  $u_m$  is the coefficient of the function  $\phi_m$  and  $K$  is the order of the approximation. In case of linear finite elements it will just be the number of nodes of the grid used to discretize the domain.

To find the values  $u_m$  we try to minimize the residual that arises when  $u$  is introduced into  $L$  multiplying the equation  $L$  by some weighting functions  $\Psi_n$  and integrating over the whole domain leading to

$$\int_{\Omega} \Psi_n L(u) d\Omega = \int_{\Omega} \Psi_n L(\phi_m u_m) d\Omega = u_m \int_{\Omega} \Psi_n L(\phi_m) d\Omega \tag{A.13}$$

If the integral is identified with the elements of a matrix  $a_{nm}$  we can write (A.13) also as a linear system

$$a_{nm} u_m = 0 \quad n = 1, K \quad m = 1, K \tag{A.14}$$

Once the basis and weighting functions have been specified the system may be set up and (A.14) may be solved for the unknowns  $u_m$ .





Figure A.1: a) form functions in domain      b) domain of influence of node  $i$

### Staggered Finite Elements

For decades finite elements have been used in fluid mechanics in a standardized manner. The form functions  $\phi_m$  were chosen as continuous piecewise linear functions allowing a subdivision of the whole area of interest into small triangular elements specifying the coefficients  $u_m$  at the vertices (called nodes) of the triangles. The functions  $\phi_m$  are 1 at node  $m$  and 0 at all other nodes and thus different from 0 only in the triangles containing the node  $m$ . An example is given in the upper left part of Fig. 1a where the form function for node  $i$  is shown. The full circle indicates the node where the function  $\phi_i$  take the value 1 and the hollow circles where they are 0.

The contributions  $a_{nm}$  to the system matrix are therefore different from 0 only in elements containing node  $m$  and the evaluation of the matrix elements can be performed on an element basis where all coefficients and unknowns are linear functions of  $x$  and  $y$ .

This approach is straightforward but not very satisfying with the semi-implicit time stepping scheme for reasons explained below. Therefore an other way has been followed in the present formulation. The fluid domain is still divided in triangles and the water levels are still defined at the nodes of the grid and represented by piecewise linear interpolating functions in the internal of each element, i.e.

$$\zeta = \zeta_m \phi_m \quad m = 1, K$$

However, the transports are now expanded, over each triangle, with piecewise constant (non continuous) form functions  $\psi_n$  over the whole domain. We therefore write

$$U = U_n \psi_n \quad n = 1, J$$

where  $n$  is now running over all triangles and  $J$  is the total number of triangles. An example of  $\psi_n$  is given in the lower right part of Fig. 1a. Note that the form function is constant 1 over the whole element, but outside the element identically 0. Thus it is discontinuous at the element borders.

Since we may identify the center of gravity of the triangle with the point where the transports  $U_n$  are defined (contrary to the water levels  $\zeta_m$  which are defined on the vertices of the triangles), the resulting grid may be seen as a staggered grid where the unknowns are defined on different locations. This kind of grid is usually used with the finite difference method. With the form functions used here the grid of the finite element model resembles very much an Arakawa B-grid that defines the water levels on the center and the velocities on the four vertices of a square.

Staggered finite elements have been first introduced into fluid mechanics by Schoenstadt [14]. He showed that the un-staggered finite element formulation of the shallow water

equations has very poor geostrophic adjustment properties. Williams [20, 21] proposed a similar algorithm, the one actually used in this paper, introducing constant form functions for the velocities. He showed the excellent propagation and geostrophic adjustment properties of this scheme.

### The Practical Realization

The integration of the partial differential equation is now performed by using the subdivision of the domain in elements (triangles). The water levels  $\zeta$  are expanded in piecewise linear functions  $\phi_m$ ,  $m = 1, K$  and the transports are expanded in piecewise constant functions  $\psi_n$ ,  $n = 1, J$  where  $K$  and  $J$  are the total number of nodes and elements respectively. As weighting functions we use  $\psi_n$  for the momentum equations and  $\phi_m$  for the continuity equation. In this way there will be  $K$  equations for the unknowns  $\zeta$  (one for each node) and  $J$  equations for the transports (one for each element).

In all cases the consistent mass matrix has been substituted with the their lumped equivalent. This was mainly done to avoid solving a linear system in the case of the momentum equations. But it was of use also in the solution of the continuity equation because the amount of mass relative to one node does not depend on the surrounding nodes. This was important especially for the flood and dry mechanism in order to conserve mass.

### Finite Element Equations

If equations (A.9,A.10,A.11) are multiplied with their weighting functions and integrated over an element we can write down the finite element equations. But the solution of the water levels does actually not use the continuity equation in the form (A.11), but a slightly different formulation. Starting from equation (A.6), multiplied by the weighting function  $\Phi_M$  and integrated over one element yields

$$\int_{\Omega} \Phi_N (\zeta^{n+1} - \zeta^n) d\Omega + \left(\frac{\Delta t}{2}\right) \int_{\Omega} \left( \Phi_N \frac{\partial(U^{n+1} + U^n)}{\partial x} + \Phi_N \frac{\partial(V^{n+1} + V^n)}{\partial y} \right) d\Omega = 0$$

If we integrate by parts the last two integrals we obtain

$$\int_{\Omega} \Phi_N (\zeta^{n+1} - \zeta^n) d\Omega - \left(\frac{\Delta t}{2}\right) \int_{\Omega} \left( \frac{\partial \Phi_N}{\partial x} (U^{n+1} + U^n) + \frac{\partial \Phi_N}{\partial y} (V^{n+1} + V^n) \right) d\Omega = 0$$

plus two line integrals, not shown, over the boundary of each element that specify the normal flux over the three element sides. In the interior of the domain, once all contributions of all elements have been summed, these terms cancel at every node, leaving only the contribution of the line integral on the boundary of the domain. There, however, the boundary condition to impose is exactly no normal flux over material boundaries. Thus, the contribution of these line integrals is zero.

If now the expressions for  $U^{n+1}, V^{n+1}$  are introduced, we obtain a system with again only the water levels as unknowns

$$\begin{aligned} \int_{\Omega} \Phi_N \zeta^{n+1} d\Omega &+ (\Delta t/2)^2 \alpha g \int_{\Omega} H \left( \frac{\partial \Phi_N}{\partial x} \frac{\partial \zeta^{n+1}}{\partial x} + \frac{\partial \Phi_N}{\partial y} \frac{\partial \zeta^{n+1}}{\partial y} \right) d\Omega \\ &= \int_{\Omega} \Phi_N \zeta^n d\Omega + (\Delta t/2)^2 \alpha g \int_{\Omega} H \left( \frac{\partial \Phi_N}{\partial x} \frac{\partial \zeta^n}{\partial x} + \frac{\partial \Phi_N}{\partial y} \frac{\partial \zeta^n}{\partial y} \right) d\Omega \\ &+ (\Delta t/2)(1 + \alpha) \int_{\Omega} \left( \frac{\partial \Phi_N}{\partial x} U^n + \frac{\partial \Phi_N}{\partial y} V^n \right) d\Omega \\ &- (\Delta t^2/2) \alpha \int_{\Omega} \left( \frac{\partial \Phi_N}{\partial x} X + \frac{\partial \Phi_N}{\partial y} Y \right) d\Omega \end{aligned} \quad (A.15)$$

Here we have introduced the symbol  $\alpha$  as a shortcut for

$$\alpha = \frac{1}{1 + \Delta t R}$$

The variables and unknowns may now be expanded with their basis functions and the complete system may be set up.

### A.2.3 Mass Conservation

It should be pointed out that only through the use of this staggered grid the semi-implicit time discretization may be implemented in a feasible manner. If the Galerkin method is applied in a naive way to the resulting equation (A.11) (introducing the linear form functions for transports and water levels and setting up the system matrix), the model is not mass conserving. This may be seen in the following way (see Fig. 1b for reference). In the computation of the water level at node  $i$ , only  $\zeta$  and transport values belonging to triangles that contain node  $i$  enter the computation (full circles in Fig. 1b). But when, in a second step, the barotropic transports of node  $j$  are computed, water levels of nodes that lie further apart from the original node  $i$  are used (hollow circles in Fig. 1b). These water levels have not been included in the computation of  $\zeta_i$ , the water level at node  $i$ . So the computed transports are actually different from the transports inserted formally in the continuity equation. The continuity equation is therefore not satisfied.

These contributions of nodes lying further apart could in principle be accounted for. In this case not only the triangles  $\Omega_i$  around node  $i$  but also all the triangles that have nodes in common with the triangles  $\Omega_i$  would give contributions to node  $i$ , namely all nodes and elements shown in Fig. 1b. The result would be an increase of the bandwidth of the matrix for the  $\zeta$  computation disadvantageous in terms of memory and time requirements.

Using instead the approach of the staggered finite elements, actually only the water levels of elements around node  $i$  are needed for the computation of the transports in the triangles  $\Omega_i$ . In this case the model satisfies the continuity equation and is perfectly mass conserving.

### A.2.4 Inter-tidal Flats

Part of a basin may consist of areas that are flooded during high tides and emerge as islands at ebb tide. These inter-tidal flats are quite difficult to handle numerically because the elements that represent these areas are neither islands nor water elements. The boundary line defining their contours is wandering during the evolution of time and a mathematical model must reproduce this features.

For reasons of computer time savings a simplified algorithm has been chosen to represent the inter-tidal flats. When the water level in at least one of the three nodes of an element falls below a minimum value (5 cm) the element is considered an island and is taken out of the system. It will be reintroduced only when in all three nodes the water level is again higher than the minimum value. Because in dry nodes no water level is computed anymore, an estimate of the water level has to be given with some sort of extrapolation mechanism using the water nodes nearby.

This algorithm has the advantage that it is very easy to implement and very fast. The dynamical features close to the inter-tidal flats are of course not well reproduced but the behavior of the method for the rest of the lagoon gave satisfactory results.

In any case, since the method stores the water levels of the last time step, before the element is switched off, introducing the element in a later moment with the same water levels conserves the mass balance. This method showed a much better performance than the one where the new elements were introduced with the water levels taken from the extrapolation of the surrounding nodes.

## Appendix B

# File formats

### B.1 GRD file

format of input file for grid utility

=====

legend :

n item number (node, element, line)  
t type  
d depth  
x,y coordinates  
ntot number of following nodes  
n1,n2 node numbers

=====

format of lines in input file :

comment:

0 [anything]

node:

1 n t x y [d]

element:

2 n t ntot n1 n2 n3 ... [d]

line:

3 n t ntot n1 n2 ... [d]

=====

comment :

lines may be split at any point, except before optional argument  
 d must not be on separate line  
 if line is split, the continuation line(s) must start with a blank  
 blank lines can be inserted as needed  
 if d is not specified -999. will be used (flag)  
 use t=0 if you do not know what to use  
 n must be unique for every item type  
 item numbers need not be consecutive  
 the sequence of items is not important, nodes can be mixed with elements/lines  
 the minimum number of nodes for element items is 3  
 the minimum number of nodes for line items is 2  
 element items should have all nodes unique  
 line items with the same first and last node are considered closed

=====

example 1 :

0 example of one line

1 11 0 10. 10.  
 1 12 0 20. 20.

3 7 0 2 11 12

#-----

example 2 :

0 example of one element with continuation line

1 11 0 10. 10.  
 1 12 0 20. 20.  
 1 15 0 10. 20.

2 7 0 3  
   11 12 15

=====

# Appendix C

## Parameter list

### C.1 Parameter list for the SHYFEM model

#### C.1.1 Section \$title

This section must be always the first section in the parameter input file. It contains only three lines. An example is given in figure C.1.

```
$title
    free one line description of simulation
    name_of_simulation
    name_of_basin
$end
```

Figure C.1: Example of section \$title

The first line of this section is a free one line description of the simulation that is to be carried out. The next line contains the name of the simulation. All created files will use this name in the main part of the file name with different extensions. Therefore the hydrodynamic output file (extension out) will be named `name_of_simulation.out`. The last line gives the name of the basin file to be used. This is the pre-processed file of the basin with extension bas. In our example the basin file `name_of_basin.bas` is used.

The directory where this files are read from or written to depends on the settings in section \$name. Using the default the program will read from and write to the current directory.

#### C.1.2 Section \$para

This section defines the general behavior of the simulation, gives various constants of parameters and determines what output files are written. In the following the meaning of all possible parameters is given.

Note that the only compulsory parameters in this section are the ones that chose the duration of the simulation and the integration time step. All other parameters are optional.

**Compulsory time parameters** This parameters are compulsory parameters that define the period of the simulation. They must be present in all cases.

itanf	Start of simulation. (Default 0)
itend	End of simulation.

idt                      Time step of integration.

**Output parameters** The following parameters deal with the output frequency and start time to external files. The content of the various output files should be looked up in the appropriate section.

The default for the time step of output of the files is 0 which means that no output file is written. If the time step of the output files is equal to the time step of the simulation then at every time step the output file is written. The default start time of the output is `itanf`, the start of the simulation.

idtout                  Time step and start time for writing to file OUT, the file containing the  
itmout                  general hydrodynamic results.

idtext                  Time step and start time for writing to file EXT, the file containing hy-  
itmext                  drodynamic data of extra points. The extra points for which the data is  
written to this file are given in section `extra` of the parameter file.

idtrst                  Time step for writing the restart file (extension RST). No restart file is  
written with `idtrst` equal to 0. A negative value is also possible for  
the time step. In this case the time step used is `-idtrst`, but the file is  
overwritten every time. It therefore contains always only the last written  
restart record. The special value of `idtrst = -1` will write only the last  
time step of the simulation in the restart file. This is useful if you want  
to start another simulation from the last output. (Default 0)

itmrst                  Start time for writing the restart file. If not given it is the beginning of  
the simulation.

itrst                   Time to use for the restart. If a restart is performed, then the file name  
containing the restart data has to be specified in `restrt` and the time  
record corresponding to `itrst` is used in this file. A value of -1 is  
also possible. In this case the last record in the restart file is used for  
the restart and the simulation starts from this time. Be aware that this  
option changes the parameter `itanf` to the time of the last record found  
in `restrt`.

ityrst                  Type of restart. If 0 and the restart file is not found the program will  
exit with an error. Otherwise the program will simply continue with a  
cold start. If `ityrst` is 1 and the given time record is not found in the  
file it will exit with error. If it is 2 it will initialize all values from the  
first time record after `itrst`. Therefore, the value of 2 will guarantee  
that the program will not abort and continue running, but it might not  
be doing what you intended. (Default 0)

idtres                  Time step and start time for writing to file RES, the file containing resid-  
itmres                  ual hydrodynamic data.

idtrms                  Time step and start time for writing to file RMS, the file containing  
itmrms                  hydrodynamic data of root mean square velocities.

idtfllx                  Time step and start time for writing to file FLX, the file containing dis-  
itmflx                  charge data through defined sections. The transects for which the dis-  
charges are computed are given in section `flux` of the parameter file.

<code>idtv</code>	Time step and start time for writing to file VOL, the file containing
<code>itmvol</code>	volume information of areas defined by transects. The transects that are used to compute the volumes are given in section <code>volume</code> of the parameter file.
<code>netcdf</code>	This parameter chooses output in NetCDF format if <code>netcdf</code> is 1, else the format is unformatted fortran files. (Default 0)

**General time and date parameters** A time and date can be assigned to the simulation. These values refer to the time 0 of the FEM model. The format for the date is YYYYMMDD and for the time HHMMSS. You can also give a time zone if your time is not referring to GMT but to another time zone such as MET.

`date` The real date corresponding to time 0. (Default 0) `time` The real time corresponding to time 0. (Default 0) `tz` The time zone you are in. This is 0 for GMT, 1 for MET and 2 for MEST (MET summer time). (Default 0)

**Model parameters** The next parameters define the inclusion or exclusion of certain terms of the primitive equations.

<code>ilin</code>	Linearization of the momentum equations. If <code>ilin</code> is different from 0 the advective terms are not included in the computation. (Default 1)
<code>itlin</code>	This parameter decides how the advective (non-linear) terms are computed. The value of 0 (default) uses the usual finite element discretization over a single element. The value of 1 choses a semi-lagrangian approach that is theoretically stable also for Courant numbers higher than 1. It is however recommended that the time step is limited using <code>itsplt</code> and <code>coumax</code> described below. (Default 0)
<code>iclin</code>	Linearization of the continuity equation. If <code>iclin</code> is different from 0 the depth term in the continuity equation is taken to be constant. (Default 0)

The next parameters allow for a variable time step in the hydrodynamic computations. This is especially important for the non-linear model (`ilin=0`) because in this case the criterion for stability cannot be determined a priori and in any case the time integration will not be unconditionally stable.

The variable time steps allows for longer basic time steps (here called macro time steps) which have to be set in `idt`. It then computes the optimal time step (here micro time step) in order to not exceed the given Courant number. However, the value for the macro time step will never be exceeded.

<code>itsplt</code>	Type of variable time step computation. If this value is 0, the time step will kept constant at its initial value. A value of 1 devides the initial time step into (possibly) equal parts, but makes sure that at the end of the micro time steps one complete macro time step has been executed. The last mode <code>itsplt = 2</code> does not care about the macro time step, but always uses the biggest time step possible. In this case it is not assured that after some micro time steps a macro time step will be recovered. Please note that the initial macro time step will never be exceeded. (Default 0)
---------------------	--



<code>coumax</code>	Normally the time step is computed in order to not exceed the Courant number of 1. However, in some cases the non-linear terms are stable even for a value higher than 1 or there is a need to achieve a lower Courant number. Setting <code>coumax</code> to the desired Courant number achieves exactly this effect. (Default 1)
<code>idtsyn</code>	In case of <code>itsplt = 2</code> this parameter makes sure that after a time of <code>idtsyn</code> the time step will be synchronized to this time. Therefore, setting <code>idtsyn = 3600</code> means that there will be a time stamp every hour, even if the model has to take one very small time step in order to reach that time. This parameter is useful only for <code>itsplt = 2</code> and its default value of 0 does not make any synchronization.
<code>idtmin</code>	This variable defines the smallest time step possible when time step splitting is enabled. Normally the smallest time step is 1 second. But when dealing with a lot of wet and drying in areas then sometimes it is useful to take out elements that limit the time step too much. In the case that <code>idtmin</code> is set to a value greater than 1 the program will switch off temporarily elements that are responsible for such a small time step. (Default 0)

These parameters define the weighting of time level in the semi-implicit algorithm. With these parameters the damping of gravity or Rossby waves can be controlled. Only modify them if you know what you are doing.

<code>azpar</code>	Weighting of the new time level of the transport terms in the continuity equation. (Default 0.5)
<code>ampar</code>	Weighting of the new time level of the pressure term in the momentum equations. (Default 0.5)
<code>afpar</code>	Weighting of the new time level of the Coriolis term in the momentum equations. (Default 0.5)
<code>avpar</code>	Weighting of the new time level of the non-linear advective terms in the momentum equations. (Default 0.0)

The next parameters define the weighting of time level for the vertical stress and advection terms. They guarantee the stability of the vertical system. For this reason they are normally set to 1 which corresponds to a fully implicit discretization. Only modify them if you know what you are doing.

<code>atpar</code>	Weighting of the new time level of the vertical viscosity in the momentum equation. (Default 1.0)
<code>adpar</code>	Weighting of the new time level of the vertical diffusion in the scalar equations. (Default 1.0)
<code>aapar</code>	Weighting of the new time level of the vertical advection in the scalar equations. (Default 1.0)

**Coriolis parameters** The next parameters define the parameters to be used with the Coriolis terms.

<code>icor</code>	If this parameter is 0, the Coriolis terms are not included in the computation. A value of 1 uses a beta-plane approximation with a variable Coriolis parameter $f$ , whereas a value of 2 uses an f-plane approximation where the Coriolis parameter $f$ is kept constant over the whole domain. (Default 0)
<code>dlat</code>	Average latitude of the basin. This is used to compute the Coriolis parameter $f$ . If not given the latitude in the basin file is used. If given the value of <code>dlat</code> in the input parameter file effectively substitutes the value given in the basin file. This parameter is not used if spherical coordinates are used ( <code>isphe=1</code> ). (Default 0)
<code>isphe</code>	If 0 a cartesian coordinate system is used, if 1 the coordinates are in the spherical system (lat/lon). Please note that in case of spherical coordinates the Coriolis term is always included in the computation, even with <code>icor = 0</code> . If you really do not want to use the Coriolis term, then please set <code>icor = -1</code> . The default is -1, which means that the type of coordinate system will be determined automatically.

**Depth parameters** The next parameters deal with handling depth values of the basin.

<code>href</code>	Reference depth. If the depth values of the basin and the water levels are referred to mean sea level, <code>href</code> should be 0 (default value). Else this value is subtracted from the given depth values. For example, if <code>href = 0.20</code> then a depth value in the basin of 1 meter will be reduced to 80 centimeters.
<code>hzmin</code>	Minimum total water depth that will remain in a node if the element becomes dry. (Default 0.01 m)
<code>hzoff</code>	Total water depth at which an element will be taken out of the computation because it becomes dry. (Default 0.05 m)
<code>hzon</code>	Total water depth at which a dry element will be re-inserted into the computation. (Default 0.10 m)
<code>hmin</code>	Minimum water depth (most shallow) for the whole basin. All depth values of the basin will be adjusted so that no water depth is shallower than <code>hmin</code> . (Default is no adjustment)
<code>hmax</code>	Maximum water depth (deepest) for the whole basin. All depth values of the basin will be adjusted so that no water depth is deeper than <code>hmax</code> . (Default is no adjustment)

**Bottom friction** The friction term in the momentum equations can be written as  $Ru$  and  $Rv$  where  $R$  is the variable friction coefficient and  $u, v$  are the velocities in  $x, y$  direction respectively. The form of  $R$  can be specified in various ways. The value of `ireib` is choosing between the formulations. In the parameter input file a value  $\lambda$  is specified that is used in the formulas below.

ireib	Type of friction used (default 0):  <b>0</b> No friction used  <b>1</b> $R = \lambda$ is constant  <b>2</b> $\lambda$ is the Strickler coefficient. In this formulation $R$ is written as $R = \frac{g}{C^2} \frac{ u }{H}$ with $C = k_s H^{1/6}$ and $\lambda = k_s$ is the Strickler coefficient. In the above formula $g$ is the gravitational acceleration, $ u $ the modulus of the current velocity and $H$ the total water depth.  <b>3</b> $\lambda$ is the Chezy coefficient. In this formulation $R$ is written as $R = \frac{g}{C^2} \frac{ u }{H}$ and $\lambda = C$ is the Chezy coefficient.  <b>4</b> $R = \lambda/H$ with $H$ the total water depth  <b>5</b> $R = \lambda \frac{ u }{H}$
czdef	The default value for the friction parameter $\lambda$ . Depending on the value of ireib the coefficient $\lambda$ is describing linear friction, constant drag coefficient or a Chezy or Strickler form of friction (default 0).
iczv	Normally $R$ is evaluated at every time step ( $iczv = 1$ ). If for some reason this behavior is not desirable, $iczv = 0$ evaluates the value of $R$ only before the first time step, keeping it constant for the rest of the simulation. (default 1)

The value of  $\lambda$  may be specified for the whole basin through the value of czdef. For more control over the friction parameter it can be also specified in section area where the friction parameter depending on the type of the element may be varied. Please see the paragraph on section area for more information.

**Physical parameters** The next parameters describe physical values that can be adjusted if needed.

rowass	Average density of sea water. (Default $1025 \text{ kg m}^{-3}$ )
roluft	Average density of air. (Default $1.225 \text{ kg m}^{-3}$ )
grav	Gravitational acceleration. (Default $9.81 \text{ m s}^{-2}$ )

**Wind parameters** The next two parameters deal with the wind stress to be prescribed at the surface of the basin.

The wind data can either be specified in an external file (ASCII or binary) or directly in the parameter file in section wind. The ASCII file or the wind section contain three columns, the first giving the time in seconds, and the others the components of the wind speed. Please see below how the last two columns are interpreted depending on the value of iwtype. For the format of the binary file please see the relative section. If both a wind file and section wind are given, data from the file is used.

The wind stress is normally computed with the following formula

$$\tau^x = \rho_a c_D |u| u^x \quad \tau^y = \rho_a c_D |u| u^y \quad (\text{C.1})$$

where  $\rho_a, \rho_0$  is the density of air and water respectively,  $u$  the modulus of wind speed and  $u^x, u^y$  the components of wind speed in  $x, y$  direction. In this formulation  $c_D$  is a dimensionless drag coefficient that varies between  $1.5 \cdot 10^{-3}$  and  $3.2 \cdot 10^{-3}$ . The wind speed is normally the wind speed measured at a height of 10 m.

`iwtype` The type of wind data given (default 1):

- 0** No wind data is processed
- 1** The components of the wind is given in [m/s]
- 2** The stress ( $\tau^x, \tau^y$ ) is directly specified
- 3** The wind is given in speed [m/s] and direction [degrees]. A direction of  $0^\circ$  specifies a wind from the north,  $90^\circ$  a wind from the east etc.
- 4** As in 3 but the speed is given in knots

`itdrag` Formula to compute the drag coefficient.

- 0** constant value given in `dragco`.
- 1** Smith and Banke (1975) formula
- 2** Large and Pond (1981) formula
- 3** Spatio/temporally varying in function of wave. Need the coupling with WWMIII.
- 4** Spatio/temporally varying in function of heat flux. Only for `iheat` = 6.

(Default 0)

`dragco` Drag coefficient used in the above formula. Please note that in case of `iwtype` = 2 this value is of no interest, since the stress is specified directly. (Default 2.5E-3) `wsmax` Maximum wind speed allowed in [m/s]. This is in order to avoid errors if the wind data is given in a different format from the one specified by `iwtype`. (Default 50) `wslim`

Limit maximum wind speed to this value [m/s]. This provides an easy way to exclude strong wind gusts that might blow up the simulation. Use with caution. (Default -1, no limitation)

**Parameters for 3d** The next parameters deal with the layer structure in 3D.

`dzreg` Normally the bottom of the various layers are given in section `$levels`. If only a regular vertical grid is desired then the parameter `dzreg` can be used. It specifies the spacing of the vertical layers in meters. (Default is 0, which means that the layers are specified explicitly in `$levels`.)

The last layer (bottom layer) is treated in a special way. Depending on the parameter `ilytyp` there are various cases to be considered. A value of 0 leaves the last layer as it is, even if the thickness is very small. A value of 1 will always eliminate the last layer, if it has not full layer thickness. A value of 2 will do the same, but only if the last layer is smaller than `hlvmin` (in units of fraction). Finally, a value of 3 will add the last layer to the layer above, if its layer thickness is smaller than `hlvmin`.

`ilytyp` Treatment of last (bottom) layer. 0 means no adjustment, 1 deletes the last layer, if it is not a full layer, 2 only deletes it if the layer thickness is less than `hlvmin`, and 3 adds the layer thickness to the layer above if it is smaller than `hlvmin`. Therefore, 1 and 2 might change the total depth and layer structure, while 3 only might change the layer structure. The value of 1 will always give you full layers at the bottom.

hlvmin      Minimum layer thickness for last (bottom) layer used when `ilytyp` is 2 or 3. The unit is fractions of the nominal layer thickness. Therefore, a value of 0.5 indicates that the last layer should be at least half of the full layer.

The next parameters deal with vertical diffusivity and viscosity.

diftur      Vertical turbulent diffusion parameter for the tracer. (Default 0)

difmol      Vertical molecular diffusion parameter for the tracer. (Default 1.0e-06)

vistur      Vertical turbulent viscosity parameter for the momentum. (Default 0)

vismol      Vertical molecular viscosity parameter for the momentum. (Default 1.0e-06)

dhpar      Horizontal diffusion parameter (general). (Default 0)

The next parameters deal with the control of the scalar transport and diffusion equation. You have possibility to prescribe the tvd scheme desired and to limit the Courant number.

itvd      Type of the horizontal advection scheme used for the transport and diffusion equation. Normally an upwind scheme is used (0), but setting the parameter `itvd` to a value greater than 0 choses a TVD scheme. A value of 1 will use a TVD scheme based on the average gradient, and a value of 2 will use the gradient of the upwind node (recommended). This feature is still experimental, so use with care. (Default 0)

itvdv      Type of the vertical advection scheme used for the transport and diffusion equation. Normally an upwind scheme is used (0), but setting the parameter `itvd` to 1 choses a TVD scheme. This feature is still experimental, so use with care. (Default 0)

rstol      Normally the internal time step for scalar advection is automatically adjusted to produce a Courant number of 1 (marginal stability). You can set `rstol` to a smaller value if you think there are stability problems. (Default 1)

**Temperature and salinity** The next parameters deal with the transport and diffusion of temperature and salinity. Please note that in order to compute T/S the parameter `ibarcl` must be different from 0. In this case T/S advection is computed, but may be selectively turned off setting one of the two parameters `itemp` or `isalt` explicitly to 0.

itemp      Flag if the computation on the temperature is done. A value different from 0 computes the transport and diffusion of the temperature. (Default 1)

isalt      Flag if the computation on the salinity is done. A value different from 0 computes the transport and diffusion of the salinity. (Default 1)

The next parameters set the initial conditions for temperature and salinity. Both the average value and a stratification can be specified.

temref      Reference (initial) temperature of the water in centigrade. (Default 0)

salref      Reference (initial) salinity of the water in psu (practical salinity units) or ppt. (Default 0)

`tstrat` Initial temperature stratification in units of [C/km]. A positive value indicates a stable stratification. (Default 0)

`sstrat` Initial salinity stratification in units of [psu/km]. A positive value indicates a stable stratification. (Default 0)

The next parameters deal with horizontal diffusion of temperature and salinity. These parameters overwrite the general parameter for horizontal diffusion `dhpar`.

`thpar` Horizontal diffusion parameter for temperature. (Default 0)

`shpar` Horizontal diffusion parameter for salinity. (Default 0)

**Concentrations** The next parameters deal with the transport and diffusion of a conservative substance. The substance is dissolved in the water and acts like a tracer.

`iconz` Flag if the computation on the tracer is done. A value different from 0 computes the transport and diffusion of the substance. If greater than 1 `iconz` concentrations are simulated. (Default 0)

`conref` Reference (initial) concentration of the tracer in any unit. (Default 0)

`contau` If different from 0 simulates decay of concentration. In this case `contau` is the decay rate (e-folding time) in days. (Default 0)

`chpar` Horizontal diffusion parameter for the tracer. This value overwrites the general parameter for horizontal diffusion `dhpar`. (Default 0)

**Output for scalars** The next parameters define the output frequency of the computed scalars (temperature, salinity, generic concentration) to file.

`idtcon` Time step and start time for writing to file CON (concentration) and  
`itmcon` NOS (temperature and salinity).

**Wave parameters** There are two parameters that must be set in the `para` section for computing wave field:

`iwave` Type of wave model and coupling procedure (default 0):

- 0** No wave model called
- 1** The parametric wave model is called (see file `subwave.f`)
- >=2** The spectral wave model WWMIII is called
- 2** ... wind from SHYFEM, radiation stress formulation
- 3** ... wind from SHYFEM, vortex force formulation
- 4** ... wind from WWMIII, radiation stress formulation
- 5** ... wind from WWMIII, vortex force formulation

When the vortex force formulation is chosen the wave-supported surface stress is subtracted from the wind stress, in order to avoid double counting of the wind forcing in the flow model. Moreover, the use of the wave-depended wind drag coefficient could be adopted setting `itdrag` = 3.

dtwave      Time step for coupling with WWMIII. Needed only for *iwave* > 1 (default 0).

### C.1.3 Section \$sedtr

This section defines the parameters for the sediment transport module.

**Parameters in the sedtr section** The following parameters activate the sediment transport module and define the sediment grainsize classes to be simulated.

isedi      Flag if the computation on the sediment is done:

**0** Do nothing (default)

**1** Compute sediment transport

sedgrs      Sediment grainsize class vector [mm]. Values have to be ordered from the finest to the more coarse.  
example: sedgrs = 0.1 0.2 0.3 0.4

sedref      Initial sediment reference concentration [kg/m<sup>3</sup>] (Default 0).

sedhpar      Sediment diffusion coefficient (Default 0).

adjtime      Time for sediment initialization [s]. The sediment model takes an initialization time in which the system goes to a quasi steady state. When *t* = adjtime the bed evolution change in the output is reset. Keep in mind that adjtime has to be chosen case by case in function of the morphology and the parameters used for the simulation (Default 0).

percin      Initial sediment distribution [0,1] for each grainsize class. The sum of percin must be equal to 1.  
example: percin = 0.25 0.25 0.25 0.25  
If percin is not selected the model imposes equal percentage for each grainsize class (percin = 1/nrs). In case of spatial differentiation of the sediment distribution set a number of percin equal to the number of grainsize classes per the number of area types.  
example: percin = 0.25 0.25 0.25 0.25  
0.20 0.20 0.30 0.30  
0.45 0.15 0.15 0.15

tauin      Initial dry density or TAUC. In function of the value:  
0-50 : critical erosion stress (Pa)  
>50 : dry bulk density of the surface (kg/m<sup>3</sup>).  
In case of spatial differentiation set a number of tauin equal to the number of area type.  
example: tauin = 0.9 1.4 2.5 1.1

sedp      File containing spatially varying initial sediment distribution. Values are in percentage of each class.

sedt      File containing spatially varying initial critical erosion stress (Pa) or dry bulk density (kg/m<sup>3</sup>).

sedcon	File containing the additional constants used in sediment model. These parameters are usually set to the indicated default values, but can be customized for each sediment transport simulation. The full parameter list together with their default value and brief description is reported in Table C.1. Most of the parameter, especially the ones for the cohesive sediments, have been calibrated for the Venice Lagoon. For more information about these parameters please refer to Neumeier et al. [12] and Ferrarin et al. [6].
--------	---

**Additional parameters** The full parameter list is reported in Table C.1. An example of the settings for the `sedcon` file is given in Fig. 4.7. Please note that is not necessary to define all parameters. If not defined the default value is imposed.

```
IOPT = 3
SURFPOR = 4
DOCOMPACT = 1
```

Figure C.2: Example of the secon file.

#### C.1.4 Section \$name

In this sections names of directories or input files can be given. All directories default to the current directory, whereas all file names are empty, i.e., no input files are given.

**Directory specification** This parameters define directories for various input and output files.

basdir	Directory where basin file BAS resides. (Default .)
datdir	Directory where output files are written. (Default .)
tmpdir	Directory for temporary files. (Default .)
defdir	Default directory for other files. (Default .)

**File names** The following strings enable the specification of files that account for initial conditions or forcing.

zinit	File with initial water level distribution.
wind	File with wind data. The file may be either formatted or unformatted. For the format of the unformatted file please see the section where the WIN file is discussed. The format of formatted ASCII file is in standard time-series format, with the first column containing the time in seconds and the next two columns containing the wind data. The meaning of the two values depend on the value of the parameter <code>iwtype</code> in the <code>para</code> section.
rain	File with rain data. This file is a standard time series with the time in seconds and the rain values in mm/day. The values may include also evaporation. Therefore, also negative values (for evaporation) are permitted.



qflux	File with heat flux data. This file must be in a special format to account for the various parameters that are needed by the heat flux module to run. Please refer to the information on the file qflux.
restrt	Name of the file if a restart is to be performed. The file has to be produced by a previous run with the parameter idtrst different from 0. The data record to be used in the file for the restart must be given by time itrst.
gotmpa	Name of file containing the parameters for the GOTM turbulence model (iturb = 1).
saltin	Name of file containing initial conditions for salinity
tempin	Name of file containing initial conditions for temperature
conzin	Name of file containing initial conditions for concentration

### C.1.5 Section \$bound

These parameters determine the open boundary nodes and the type of the boundary: level or flux boundary. At the first the water levels are imposed, on the second the fluxes are prescribed.

There may be multiple sections bound in one parameter input file, describing all open boundary conditions necessary. Every section must therefore be supplied with a boundary number. The numbering of the open boundaries must be increasing. The number of the boundary must be specified directly after the keyword bound, such as bound1 or bound 1.

kbound      Array containing the node numbers that are part of the open boundary. The node numbers must form one contiguous line with the domain (elements) to the left. This corresponds to an anti-clockwise sense. At least two nodes must be given.

ibtyp      Type of open boundary.

**0** No boundary values specified

**1** Level boundary. At this open boundary the water level is imposed and the prescribed values are interpreted as water levels in meters. If no value for ibtyp is specified this is the default.

**2** Flux boundary. Here the discharge in  $\text{m}^3 \text{s}^{-1}$  has to be prescribed.

**3** Internal flux boundary. As with ibtyp = 2 a discharge has to be imposed, but the node where discharge is imposed can be an internal node and need not be on the outer boundary of the domain. For every node in kbound the volume rate specified will be added to the existing water volume. This behavior is different from the ibtyp = 2 where the whole boundary received the discharge specified.

**4** Momentum input. The node or nodes may be internal. This feature can be used to describe local acceleration of the water column. The unit is force / density [ $\text{m}^4 \text{s}^{-2}$ ]. In other words it is the rate of volume [ $\text{m}^3 \text{s}^{-1}$ ] times the velocity [m/s] to which the water is accelerated.

`igual` If the boundary conditions for this open boundary are equal to the ones of boundary `i`, then setting `igual = i` copies all the values of boundary `i` to the actual boundary. Note that the value of `igual` must be smaller than the number of the actual boundary, i.e., boundary `i` must have been defined before.

The next parameters give a possibility to specify the file name of the various input files that are to be read by the model. Values for the boundary condition can be given at any time step. The model interpolates in between given time steps if needed. The grade of interpolation can be given by `intpol`.

All files are in ASCII and share a common format. The file must contain two columns, the first giving the time of simulation in seconds that refers to the value given in the second column. The value in the second column must be in the unit of the variable that is given. The time values must be in increasing order. There must be values for the whole simulation, i.e., the time value of the first line must be smaller or equal than the start of the simulation, and the time value of the last line must be greater or equal than the end of the simulation.

`boundn` File name that contains values for the boundary condition. The value of the variable given in the second column must be in the unit determined by `ibtyp`, i.e., in meters for a level boundary, in  $\text{m}^3 \text{s}^{-1}$  for a flux boundary and in  $\text{m}^4 \text{s}^{-2}$  for a momentum input.

`zfact` Factor with which the values from `boundn` are multiplied to form the final value of the boundary condition. E.g., this value can be used to set up a quick sensitivity run by multiplying all discharges by a factor without generating a new file. (Default 1)

`levmin`  
`levmax` A point discharge normally distributes its discharge over the whole water column. If it is important that in a 3D simulation the water mass discharge is concentrated only in some levels, the parameters `levmin` and `levmax` can be used. They indicate the lowest and deepest level over which the discharge is distributed. Default values are 0, which indicate that the discharge is distributed over the whole water column. Setting only `levmax` distributes from the surface to this level, and setting only `levmin` distributes from the bottom to this level.

`conzn`  
`tempn`  
`saltn` File name that contains values for the respective boundary condition, i.e., for concentration, temperature and salinity. The format is the same as for file `boundn`. The unit of the values given in the second column must be the ones of the variable, i.e., arbitrary unit for concentration, centigrade for temperature and psu (per mille) for salinity.

`vel3dn` File name that contains current velocity values for the boundary condition. The format is the same as for file `tempn` but it has two variables: current velocity in x and current velocity in y. Velocity can be nudged or imposed depending on the value of `tnudge` (mandatory). The unit is [m/s].

`tnudge` Relaxation time for nudging of boundary velocity. For `tnudge = 0`, velocities are imposed, for `tnudge > 0`, velocities are nudged. The default is -1 which means do nothing. Unit is [s]. (Default -1)

The next variables specify the name of the boundary value file for different modules. Please refer to the documentation of the single modules for the units of the variables.

`bio2dn` File name that contains values for the ecological module (EUTRO-WASP).

sed2dn	File name that contains values for the sediment transport module. The unit of the values given in the second and following columns (equal to the number of defined grainsize in parameter <code>sedgrs</code> ).
mud2dn	File name that contains values for the fluid mud module.
lam2dn	File name that contains values for the fluid mud module (boundary condition for the structural parameter, to be implemented).
dmf2dn	File name that contains values for the fluid mud module (boundary conditions for the advection of flocsizes, to be implemented).
tox3dn	File name that contains values for the toxicological module.
intpol	Order of interpolation for the boundary values read through files. Use for 1 for stepwise (no) interpolation, 2 for linear and 4 for cubic interpolation. The default is linear interpolation, except for water level boundaries ( <code>ibtyp=1</code> ) where cubic interpolation is used.

The next parameters can be used to impose a sinusoidal water level (tide) or flux at the open boundary. These values are used if no boundary file `boundn` has been given. The values must be in the unit of the intended variable determined by `ibtyp`.

ampli	Amplitude of the sinus function imposed. (Default 0)
period	Period of the sinus function. (Default 43200, 12 hours)
phase	Phase shift of the sinus function imposed. A positive value of one quarter of the period reproduces a cosine function. (Default 0)
zref	Reference level of the sinus function imposed. If only <code>zref</code> is specified ( <code>ampli = 0</code> ) a constant value of <code>zref</code> is imposed on the open boundary.

With the next parameters a constant value can be imposed for the variables of concentration, temperature and salinity. In this case no file with boundary values has to be supplied. The default for all values is 0, i.e., if no file with boundary values is supplied and no constant is set the value of 0 is imposed on the open boundary. A special value of -999 is also allowed. In this case the value imposed is the ambient value of the parameter close to the boundary.

conz	Constant boundary values for concentration, temperature and salinity respectively. If these values are set no boundary file has to be supplied. (Default 0)
temp	
salt	

The next two values are used for constant momentum input. This feature can be used to describe local acceleration of the water column. The values give the input of momentum in x and y direction. The unit is force / density ( $\text{m}^4 \text{s}^{-2}$ ). In other words it is the rate of volume ( $\text{m}^3 \text{s}^{-1}$ ) times the velocity (m/s) to which the water is accelerated.

These values are used if boundary condition `ibtyp = 4` has been chosen and no boundary input file has been given. If the momentum input is varying then it may be specified with the file `boundn`. In this case the file `boundn` must contain three columns, the first for the time, and the other two for the momentum input in x,y direction.

umom	Constant values for momentum input. (Default 0)
vmom	

### C.1.6 Section `$wind`

In this section the wind data can be given directly without the creation of an external file. Note, however, that a wind file specified in the `name` section takes precedence over this section. E.g., if both a section `wind` and a wind file in `name` is given, the wind data from the file is used.

The format of the wind data in this section is the same as the format in the ASCII wind file, i.e., three columns, with the first specifying the time in seconds and the other two columns giving the wind data. The interpretation of the wind data depends on the value of `iwtype`. For more information please see the description of `iwtype` in section `para`.

### C.1.7 Section `$extra`

In this section the node numbers of so called “extra” points are given. These are points where water level and velocities are written to create a time series that can be elaborated later. The output for these “extra” points consumes little memory and can be therefore written with a much higher frequency (typically the same as the integration time step) than the complete hydrodynamic output. The output is written to file `EXT`.

The node numbers are specified in a free format on one or more lines. An example can be seen in figure 4.1. No keywords are expected in this section.

### C.1.8 Section `$flux`

In this section transects are specified through which the discharge of water is computed by the program and written to file `FLX`. The transects are defined by their nodes through which they run. All nodes in one transect must be adjacent, i.e., they must form a continuous line in the FEM network.

The nodes of the transects are specified in free format. Between two transects one or more `O`'s must be inserted. An example is given in figure C.3.

```
$flux
1001 1002 1004 0
35 37 46 0 0 56 57 58 0
407
301
435 0 89 87
$end
```

Figure C.3: Example of section `$flux`

The example shows the definition of 5 transects. As can be seen, the nodes of the transects can be given on one line alone (first transect), two transects on one line (transect 2 and 3), spread over more lines (transect 4) and a last transect.

## C.2 Parameter list for the post processing routines

The format of the parameter input file is the same as the one for the main routine. Please see this section for more information on the format of the parameter input file.

Some sections of the parameter input file are identical to the sections used in the main routine. For easier reference we will repeat the possible parameters of these section here.

### C.2.1 Section \$title

This section must be always the first section in the parameter input file. It contains only three lines. An example has been given already in figure C.1.

The only difference with respect to the \$para section of the main routine is the first line. Here any description of the output can be used. It is just a way to label the parameter file. The other two line with the name of simulation and the basin are used to open the files needed for plotting.

### C.2.2 Section \$para

These parameters set generic values for the plot.

Some of the parameters set coordinates in the plot. For example, the values  $x_0$ ,  $y_0$  and  $x_1$ ,  $y_1$  indicate the actual plotting area, which can be bigger or smaller than the extension of the numerical grid.

Normally, values have to be in meters (the same as the coordinates in the numerical grid). However, also relative coordinates can be used. If all values given are in the range between -1 and +2, then these values are interpreted as relative coordinates. Therefore,  $x$  coordinates of 0 indicate the left border, and 1 the right border. The upper left quarter of the domain can be chosen with  $(x_0, y_0) = (0, 0.5)$  and  $(x_1, y_1) = (0.5, 1)$ .

$x_0$  Lower left corner of the plotting area. (Default is whole area)  
 $y_0$

$x_1$  Upper right corner of the plotting area. (Default is whole area)  
 $y_1$

The next values give the position, where the legend (scale bar and true north) is plotted. This legend will only be plotted if the coordinates are not geographical (lat/lon) but cartesian.

$x_{0leg}$  Lower left corner of the area where the legend is plotted.  
 $y_{0leg}$

$x_{1leg}$  Upper right corner of the area. where the legend (north and scale) is  
 $y_{1leg}$  plotted.

$lblank$  The legend is plotted over a white rectangle. Sometimes this blanking is not desirable. If you do not want to have a white box below the legend set  $lblank$  to 0. (Default 1)

$cislnd$  It is possible to plot all islands in grey color. Setting  $cislnd$  to a value between 0 (black) and 1 (white) will achieve this. A negative value will not fill islands with grey color. (Default -1)

$dxygrd$  Grid size if the results are interpolated on a regular grid. A value of 0 does not use a regular grid but the original finite element grid for plotting. (Default 0)

$typ1s$  Typical length scale to be used when scaling velocity or transport arrows. If  $dxygrd$  is given this length is used and  $typ1s$  is not used. If not given it is computed from the basin parameters. (Default 0)

$typ1sf$  Additional factor to be used with  $typ1s$  to determine the length of the maximum or reference vector. This is the easiest way to scale the velocity arrows with an overall factor. (Default 1)

<code>velref</code>	Reference value to be used when scaling arrows. If given, a vector with this value will have a length of <code>typls*typls</code> on the map, or, in case <code>dxmgrd</code> is given, <code>dxmgrd*typls</code> . If not set the maximum value of the velocity/transport will be used as <code>velref</code> . (Default 0)
<code>velmin</code>	Minimum value for which an arrow will be plotted. With this value you can eliminate small arrows in low dynamic areas. (Default 0)
<code>isphe</code>	If 0 a cartesian coordinate system is used, If 1 the coordinates are in the spherical system (lat/lon). Among other, this indicates that the <i>x</i> -coordinates will be multiplied by a factor that accounts for the visual deformation using lat/lon coordinates. The default is -1, which means that the type of coordinate system will be determined automatically. (Default -1)
<code>reggrd</code>	If different from 0 it plots a regular grid over the plot for geographical reference. The value of <code>reggrd</code> gives the spacing of the regular grid lines. The units must be according to the units used for the coordinates. With value of -1 the regular grid is determined automatically. (Default -1)
<code>regdst</code>	This value gives the number of intervals that are used to sub-divide the grid given by <code>reggrd</code> with a black and white scale around the plot. If 0 it tries to determine automatically the sub-intervals (2 or 4). A value of -1 does not plot the subgrid scale. (Default 0)
<code>reggry</code>	If plotting the regular overlay grid this gives the grey value used for the grid. 0 is black, and 1 is white. A value of 1 does not plot the overlay grid, but still writes the labels. (Default 0.5)
<code>bndlin</code>	Name of file that gives the boundary line that is not part of the finite element domain. The file must be in BND format. You can use the program <code>grd2bnd.pl</code> to create the file from a GRD file. (Default is no file)
<code>ioverl</code>	Create overlay of velocity vectors on scalar value. With the value of 0 no overlay is created, 1 creates an overlay with the velocity speed. The value of 2 overlays vertical velocities 3 water levels and 4 overlays bathymetry.(Default 0)
<code>inorm</code>	Normally the horizontal velocities are plotted in scale. The value of <code>inorm</code> can change this behavior. A value of 1 normalizes velocity vectors (all vectors are the same length), whereas 2 scales from a given minimum velocity <code>velmin</code> . Finally, the value of 3 uses a logarithmic scale. (Default 0)

### C.2.3 Section `$color`

The next parameters deal with the definition of the colors to be used in the plot. A color bar is plotted too.

<code>icolor</code>	Flag that determines the type of color table to be used. 0 stands for gray scale, 1 for HSB color table. Other possible values are 2 (from white to blue), 3 (from white to red), 4 (from blue over white to red). Values 5 and 6 indicate non-linear HSB color tables. (Default 0)
---------------------	---

isoval	Array that defines the values for the isolines and colors that are to be plotted. Values given must be in the unit of the variable that will be plotted, i.e., meters for water levels etc.
color	Array that gives the color indices for the plotting color to be used. Ranges are from 0 to 1. The type of the color depends on the variable <code>icolor</code> . For the gray scale table 0 represents black and 1 white. Values in between correspond to tones of gray. For the HSB color table going from 0 to 1 gives the color of the rainbow. There must be one more value in <code>color</code> than in <code>isoval</code> . The first color in <code>color</code> refers to values less than <code>isoval(1)</code> , the second color in <code>color</code> to values between <code>isoval(1)</code> and <code>isoval(2)</code> . The last color in <code>color</code> refers to values greater than the last value in <code>isoval</code> .
x0col y0col	Lower left corner of the area where the color bar is plotted.
xlcol ylcol	Upper right corner of the area where the color bar is plotted.
cblank	The color bar is plotted over a white rectangle. Sometimes this blanking is not desirable. If you do not want to have a white box below the legend set <code>cblank</code> to 0. (Default 1)
faccol	Factor for the values that are written to the color bar legend. This enables you, e.g., to give water level results in mm ( <code>faccol</code> = 1000). (Default 1)
ndccol	Decimals after the decimal point for the values written to the color bar legend. Use the value -1 to not write the decimal point. A value of 0 automatically computes the number of decimals needed. (Default 0)
legcol	Text for the description of the color bar. This text is written above the color bar.
It is not necessary to give all values for isolines and colors above. A faster way is to give only the minimum and maximum values and fix the number of isovalues to be used.	
niso	Total number of isolines to use. (Default is <code>nisodf</code> )
nisodf	Default number of isolines to use. (Default 5)
colmin colmax	Minimum and maximum color index used. Defaults are 0.1 and 0.9 respectively. The value of <code>colmax</code> can be smaller than <code>colmin</code> which inverts the color index used.
valmin valmax	Minimum and maximum value for isovalues to be used. There is no default.
rfiso	Defines function to be used to compute intermediate values between <code>valmin</code> and <code>valmax</code> . If 0 or 1 the values are linearly interpolated. Else they are computed by $y = x^n$ where $n$ is <code>rfiso</code> and $x = \frac{v-v_{min}}{v_{max}-v_{min}}$ . Values for <code>rfiso</code> greater than 0 capture higher detail in the lower values, whereas values less than 1 do the opposite. (Default 0)
dval	Difference of values between isolines. If this value is greater than 0 the values for isolines and the total number of isolines are computed automatically using also <code>valmin</code> and <code>valmax</code> . (Default 0)

Since there is a great choice of combinations between the parameters, we give here the following rules how the values for colors and isolines are determined.

If colors are given in array `color`, they are used, else `colmin` and `colmax` or their respective defaults are used to determine the color bar. If `isoval` is given it is used, else `valmin` and `valmax` are used. If `valmin` and `valmax` are not given they are computed every time for each plot and the minimum and maximum value in the basin are used. In any case, if `isoval` is specified the total number of isovalues is known and `niso` is ignored. However, if `isoval` is not given then first `dval` is used to decide how many isovalues to plot, and if `dval` is 0 then the `niso` and finally `nisodf` is used.

Other parameters that can be changed are the following.

<code>nisomx</code>	Maximum for <code>niso</code> allowed. This is especially useful when the value for <code>niso</code> is determined automatically. It avoids you to plot 1000 isolines due to wrong settings of <code>dval</code> . However, if you want to use 50 isovalues then just set <code>niso</code> and <code>nisomx</code> to 50. (Default 20)
<code>ntick</code>	Number of values to be written in color bar. If <code>niso</code> is high the labels on the color bar become unreadable. Therefore you can use <code>ntick</code> to write only some of the values to the color bar. For example, if <code>valmin</code> is 0 and <code>valmax</code> is 5 and you use many isolines, then setting <code>ntick</code> to 6 would give you labels at values 0,1,2,3,4,5. If <code>ntick</code> is 0 then all labels are written. (Default 0)
<code>isolin</code>	Normally the isolines are not drawn on the plot, just the colors are used to show the value in the different parts of the plot. A value different from 0 plots also the isolines. In this case <code>isolin</code> gives the number of isolines to be plotted. A good choice is to make this equal to <code>ntick</code> , so that the isolines correspond to the values written on the colorbar. For compatibility, a value of 1 plots all isolines. (Default 0)
<code>isoinp</code>	Normally inside elements the values are interpolated. Sometimes it is usefull to just plot the value of the node without interpolation inside the element. This can be accomplished by setting <code>isoinp=0</code> . (Default 1)
<code>bgray</code>	Gray value used for the finite element grid when plotting the bathymetry. (Default 0.8)
<code>bsgray</code>	Gray value used to plot the finite element grid over a scalar or velocity plot. This is basically useful for debugging reasons. The default is to not plot the grid (Default -1.0)

## C.2.4 Section `$arrow`

The next parameters deal with the reference arrow that is plotted in a legend. The arrow regards the plots where the velocity or the transport is plotted.

<code>x0arr</code> <code>y0arr</code>	Lower left corner of the area where the reference arrow is plotted.
<code>xlarr</code> <code>ylarr</code>	Upper right corner of the area where the reference arrow is plotted.
<code>ablank</code>	The arrow legend is plotted over a white rectangle. Sometimes this blanking is not desireable. If you do not want to have a white box below the legend set <code>ablank</code> to 0. (Default 1)



<code>facvel</code>	Factor for the value that is written to the arrow legend for the velocity. This enables you, e.g., to give velocities in mm/s ( <code>facvel = 1000</code> ). (Default 1)
<code>ndcvel</code>	Decimals after the decimal point for the values written to the arrow legend. Use the value <code>-1</code> to not write the decimal point. (Default 2)
<code>legvel</code>	Text for the description of the arrow legend. This text is written above the arrow.
<code>arrvel</code>	Length of arrow in legend (in velocity units). If not given the arrow length will be computed automatically. (Default 0)
<code>sclvel</code>	Additional factor to be used for the arrow in the legend. When the arrow length will be computed automatically, this parameter gives the possibility to change the length of the reference vector. This is an easy way to scale the velocity arrow with an overall factor. Not used if <code>arrvel</code> is given. (Default 1)

### C.2.5 Section `$legend`

In this section annotations in the plots can be given. The section consists of a series of lines that must contain the following information:

The first value is a keyword that specifies what has to be plotted. Possible values are `text`, `line`, `vect`, `rect`, `circ` and also `wid` and `col`. These correspond to different types of information that is inserted into the plot such as text, line, vector, rectangle or circle (filled or just outline). Moreover, the color and line width of the pen can be controlled by with `wid` and `col`.

In case of `text` the starting position (lower left corner) is given, then the point size of the font and the text that is inserted. `line` needs the starting and end point of the line. The same with `vect`, but in this case also the relative tip size must be given as a final parameter. `rect` needs the coordinates of the lower left corner and upper right corner of the rectangle. It also needs the color used for the filling of the rectangle (0-1) or the flag `-1` which only draws the outline of the rectangle without filling it. `circ` needs the center point and the radius and a fill color (see rectangle). Finally `wid` needs the relative width of the line and `col` the stroke color used when plotting lines.

A small example of an annotation that explains the above parameters would be:

```
$legend
text 30500 11800      15 'Chioggia'    #text, 15pt
line 30500 11800 35000 15000          #line
vect 30500 11800 35000 15000 0.1      #arrow, tipsize 0.1
rect 30500 11800 35000 15000 0.1      #rectangle, fill color 0.1
rect 30500 11800 35000 15000 -1       #rectangle (outline, no fill)
circ 30500 11800 5000 -1              #circle (outline, no fill)
wid 5                                 #set line width to 5
col 0.5                              #set color to 0.5
$end
```

There is also an old way to specify the legend that does not use keywords. However, this way is deprecated and unsupported and is therefore not described anymore in this manual.

### C.2.6 Section `$legvar`

In this section variable fields like the date and wind vectors may be inserted into the plot.

A time and date can be assigned to the simulation results. These values refer to the time 0 of the FEM model. The format for the date is YYYYMMDD and for the time HHMMSS. You can also give a time zone if your time is not referring to GMT but to another time zone such as MET. Please note that you have to give this information only if the simulation does not contain it already. Normally, this information is already assigned during the simulation runs.

date	The real date corresponding to time 0. (Default 0)
time	The real time corresponding to time 0. (Default 0)
tz	The time zone you are in. This is 0 for GMT, 1 for MET and 2 for MEST (MET summer time). (Default 0)
tzshow	The time zone you want to show your results. If your time zone is GMT (0) and you want to show the results referred to MET (+1) set this to +1. Please note that you have to set this variable only if you want to show results in a different time zone than the one given in tz. (Default 0)

The information of date and time may be written to the plot. This is done with the following parameters.

xdate	Starting point for the date text (lower left corner).
ydate	
sdate	Point size of the text. (Default 18)
idate	Output mode. If 0 no date is written to the plot, else the date and time is written. (Default 0)

Wind data can be used to insert a wind vector into the figure. This is useful because in the case of variable wind the direction and speed of the wind that was blowing in the moment of the plot is shown.

Since only one wind vector can be plotted, the wind data must consist of one value for each time. The same ASCII file that is used in the STR file can be used.

xwind	Starting point where the wind arrow is plotted.
ywind	
iwtype	Type of wind data. The same as the one in the STR file. If this parameter is 0 then no wind vector is plotted. (Default 0)
lwwind	Line width of the wind vector. (Default 0.1)
scwind	Scaling parameter of the wind vector. This depends on the size of your plot. If your wind is 10 m/s and you want the vector to stretch over a distance of 5 km on the plot then you have to choose the value of 500 ( $10 \cdot 500 = 5000$ ) for scwind. (Default 1)
wfile	Name of the file containing the wind data. This may be the same file than the one used in the STR file to run the program.

The wind vector is also given a text legend with the speed of the wind written out. The next parameters decide where and how this information is put.

xtwind	Starting point for the legend text (lower left corner).
ytwind	
stwind	Point size of the text. (Default 18)

wtext	Text used for the legend (Default 'Wind speed')
wunit	Unit for the wind speed (Default 'm/s')

### **C.2.7 Section** \$name

**Directory specification** This parameters define directories for various input and output files.

basdir	Directory where basin file BAS resides. (Default .)
datdir	Directory where output files are written. (Default .)
tmpdir	Directory for temporary files. (Default .)
defdir	Default directory for other files. (Default .)

Table C.1: Additional parameter for the sediment transport model to be set in the sedcon file.

Name	Default value	Description
CSULVA	159.4	Coefficient for the solid transmitted stress by Ulva
TMULVA	1.054d-3	Threshold of motion of Ulva (Pa)
TRULVA	0.0013	Threshold of full resuspension of Ulva (Pa)
E0	1.95d-5	Minimum erosion rate
RKERO	5.88	Erosion proportionality coefficient
WSCLAY	5.0	Primary median Ws class (in the range 1:NBCONC)
CDISRUPT	0.001	Constant for turbulent floc disruption during erosion
CLIM1	0.1	Lower limit for flocculation (kg/m3)
CLIM2	2.0	Limit between simple and complex flocculation (kg/m3)
KFLOC	0.001	Constant K for flocculation equation
MFLOC	1.0	Constant M for flocculation equation
RHOCLAY	2600.0	Density of clay mineral
CTAUDEP	1.0	Scaling factor for TAUCD
PRS	0.0	Resuspension probability (range 0-1)
RHOMUD	50.0	Density of the freshly deposited mud
DPROFA	470.0	Constants for density profile
DPROFB	150.0	A : final deep density
DPROFC	0.015	Define the shape (in conjunction with B and C)
DPROFD	0.0	Aux parameter for density profile
DPROFE	0.0	Aux parameter for density profile
CONSOA	1d-5	time constant of consolidation
TEROA	6d-10	Constant for erosion threshold from density
TEROB	3.0	Aux parameter for erosion threshold from density
TEROC	3.47	Aux parameter for erosion threshold from density
TEROD	-1.915	Aux parameter for erosion threshold from density
KCOES	0.15	Fraction of mud for sediment to be cohesive
CDRAGRED	-0.0893	Constant for the drag reduction formula
Z0COH	2.0D-4	Bed roughness length for cohesive sediments
FCWCOH	2.2D-3	Friction factor for cohesive sediments
LIMCOH	0.063	Limit of cohesive sediment grainsize [mm]
SMOOTH	1.0	Smoothing factor for morphodynamic
ANGREP	32.0	Angle of repose
IOPT	5	Sediment bedload transport formula option number
MORPHO	1.0	Morphological acceleration factor
RHOSED	2650.0	Sediment grain density
POROS	0.4	Bed porosity [0,1]
SURFPOR	0.6	Bed porosity of freshly deposited sand [0,1]
DOCOMPACT	0.0	If not zero, call COMPACT routine

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