

DATE ACCEPTED: 12-1-1979

CONTRACTOR'S NAME: [blank]
ENGINEER'S NAME: [blank]
DATE: [blank]
SHEET NO.: [blank]

ALL MEASUREMENTS ON THIS PROJECT
SHALL BE IN ACCORDANCE WITH THE
LATEST EDITION OF THE TEXAS
SURVEYING AND MAPPING ACT
AND THE RULES AND REGULATIONS
OF THE TEXAS SURVEYING BOARD.

EQUATION: NONE

EXCEPTION: NONE

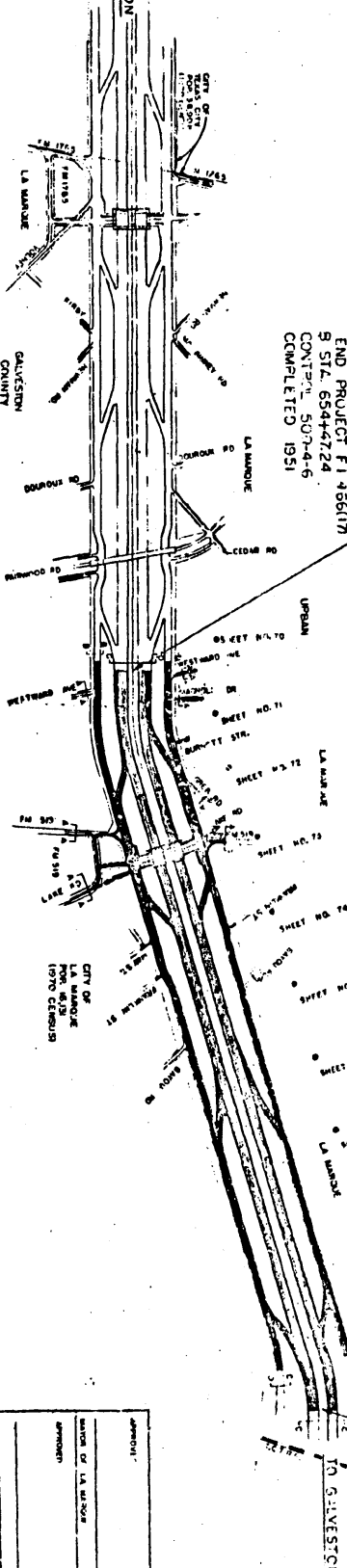
RAILROAD CROSSING: NONE

LAYOUT SCALE: 1 IN. = 1000 FT. ALONG &
1 IN. = 200 FT. PERPENDICULAR TO &

DELIVERY POINTS FOR MATERIALS SHALL
BE ARRANGED BY THE CONTRACTOR.

STATE HIGHWAY DEPARTMENT
9-19-79
[Signature]
[Signature]

APPROVED: [Signature]
DATE: [blank]



BIG PROJECT 14-1166D10
R STA 654+47.24
CONTROL 500-4-55
END PROJECT 14-156117
R STA 654+47.24
CONTROL 500-4-6
COMPLETED 1951

500-4-55
Comp 1 1980

FROM 0.6 MILE NORTHWEST OF FM 519, SOUTH TO 0.6 MILE
NORTHWEST OF T.C.T. RAILROAD
GRADING, STRUCTURES, C&G STAB BASE, ASPH STAB BASE, CONC
PAV, CONC PAV WIDENING & OVERLAY, STORM SEWER, CONC MED BARR,
FE 60 FE, CONC FLEX BASE, 11th CONC PAV, CONC CRSE SUP
PAV, FLEX BASE, LVL, PILING, CIVIL CONTROL SIGNALS,
SIGNING, DELINEATION, PAV MARKING, & SAFETY LIGHTING

INTERSTATE HIGHWAY 45
GALVESTON COUNTY

CONTROL	500-4-55	URBAN PROJECT
ROADWAY	12.7531	2.421 MI.
BRIDGE	4214	0.002 MI.

FEDERAL AID PROJECT

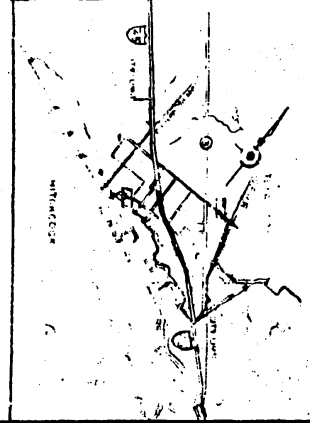
1.65-1(166) D10

SCALE: 1 IN. = 50 FT.

NET LENGTH OF PROJECT: 3232.87 FT. = 2.50 MI.

SEE SHEET 500-4-55 FOR SUMMARY OF
FIELD CHANGES & EXTRA WORK ORDERS

END PROJECT 14-156117
R STA 654+47.24
CONTROL 500-4-6
COMPLETED 1951

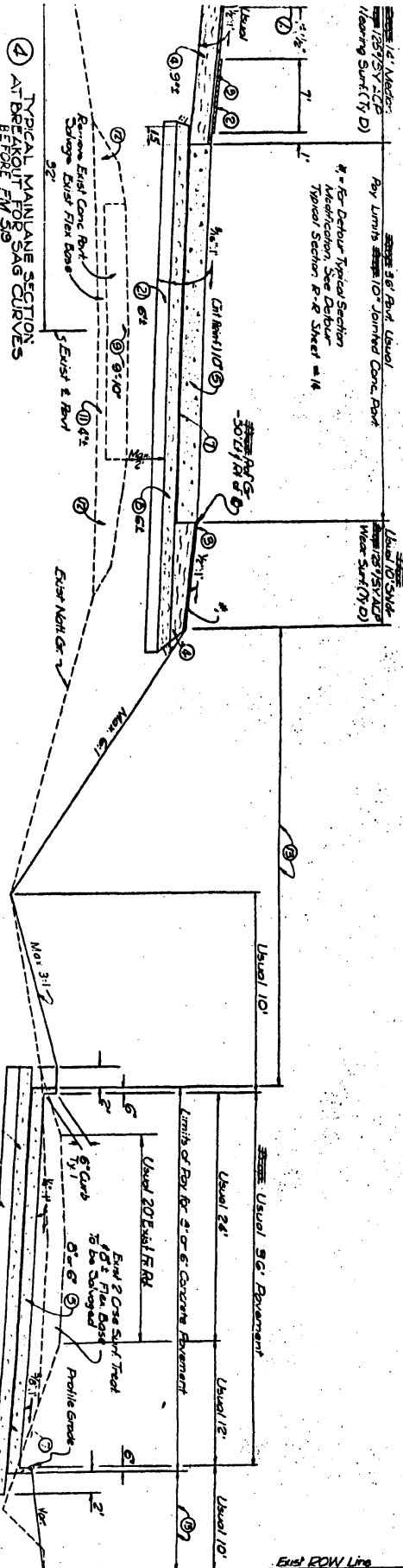


STATE OF TEXAS STATE HIGHWAY DEPARTMENT PLANS OF PROPOSED FINAL STATE HIGHWAY IMPROVEMENT

7441
07
Comp 12-8-80
Recpt 12-2-80

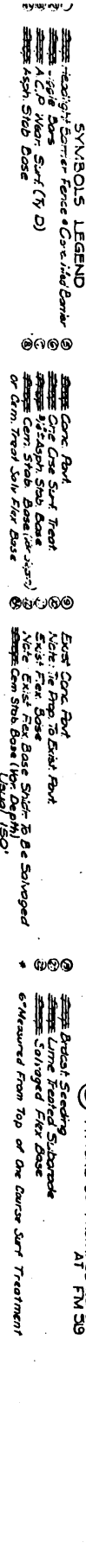
DATE	BY	REVISION
12-8-80	[Signature]	1
12-2-80	[Signature]	2

Usual 150'

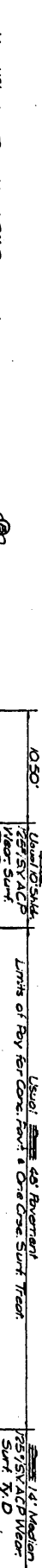


4 TYPICAL MAINLANE SECTION AT BRIDGE AND SAG CURVES BEFORE FM 519

5 TYPICAL 26' FRONTAGE ROAD SECTION WITH CURB AND GUTTER AT FM 519



5 & 6 3/4" x 11" x 15"



5 & 6 3/4" x 11" x 15"



5 & 6 3/4" x 11" x 15"



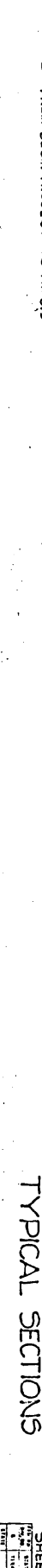
5 & 6 3/4" x 11" x 15"



5 & 6 3/4" x 11" x 15"



5 & 6 3/4" x 11" x 15"



5 & 6 3/4" x 11" x 15"



5 & 6 3/4" x 11" x 15"

Usual 150'

12' Median
25' (5' 11" D)
Weaving Surf (11' D)

* For Detail Typical Section
Modification See Detail
Typical Section R-2 Sheet #14

Note: For Detail Typical Section
Modification See Sheet #14
Detail Typical Section R-2
Sheet #14

- SYMBOLS LEGEND**
- ① Headlight Dome Fence Concrete Median Barrier
 - ② Headlight Dome Fence Concrete Median Barrier
 - ③ Headlight Dome Fence Concrete Median Barrier
 - ④ Headlight Dome Fence Concrete Median Barrier
 - ⑤ Headlight Dome Fence Concrete Median Barrier
 - ⑥ Headlight Dome Fence Concrete Median Barrier
 - ⑦ Headlight Dome Fence Concrete Median Barrier
 - ⑧ Headlight Dome Fence Concrete Median Barrier
 - ⑨ Headlight Dome Fence Concrete Median Barrier
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 - ⑫ Headlight Dome Fence Concrete Median Barrier
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 - ⑭ Headlight Dome Fence Concrete Median Barrier
 - ⑮ Headlight Dome Fence Concrete Median Barrier
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 - ⑰ Headlight Dome Fence Concrete Median Barrier
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 - ⑲ Headlight Dome Fence Concrete Median Barrier
 - ⑳ Headlight Dome Fence Concrete Median Barrier
 - ㉑ Headlight Dome Fence Concrete Median Barrier
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 - ㉟ Headlight Dome Fence Concrete Median Barrier
 - ㊱ Headlight Dome Fence Concrete Median Barrier
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 - ㊳ Headlight Dome Fence Concrete Median Barrier
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 - ㊿ Headlight Dome Fence Concrete Median Barrier

Exist ROW Line

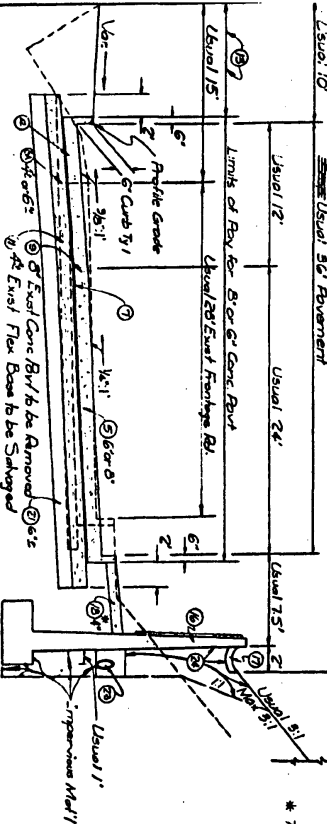
⑧ TYPICAL 36' MAINTENANCE HIGHWAY SECTION
AT FM 519
AND FROM FM 519 TO END PROJECT

Usual 150' ROW to B

Usual 1' Impermeable Wall

* To be Used of Ret Wall Areas Only

③ TYPICAL 47' CURB AND GUTTER FRONTAGE ROAD
SECTION WITH RETAINING WALLS
AT FM 519

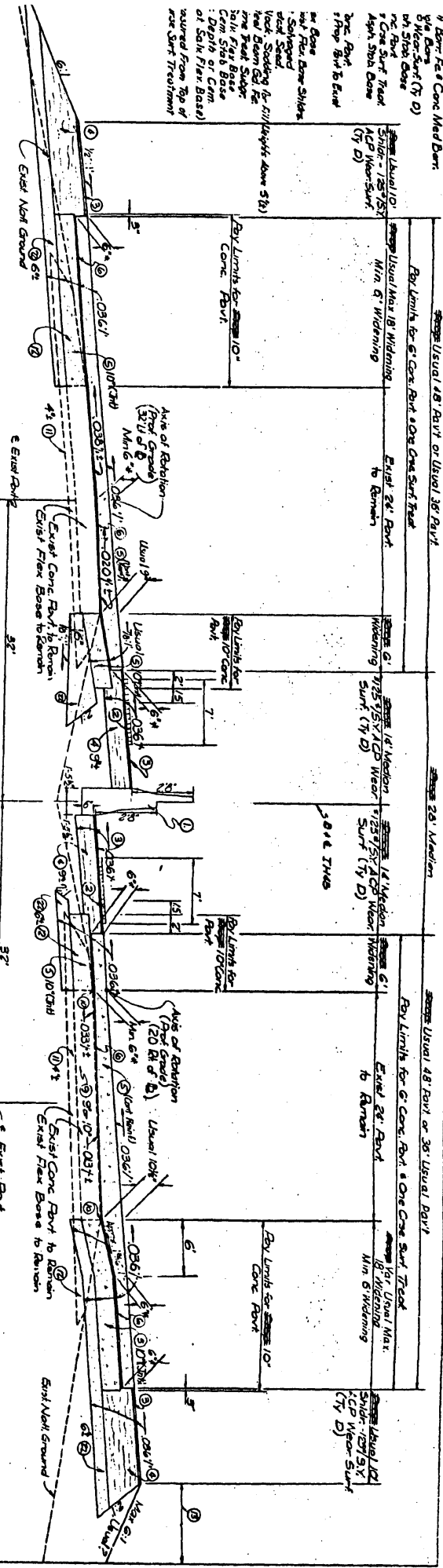


Inset #1

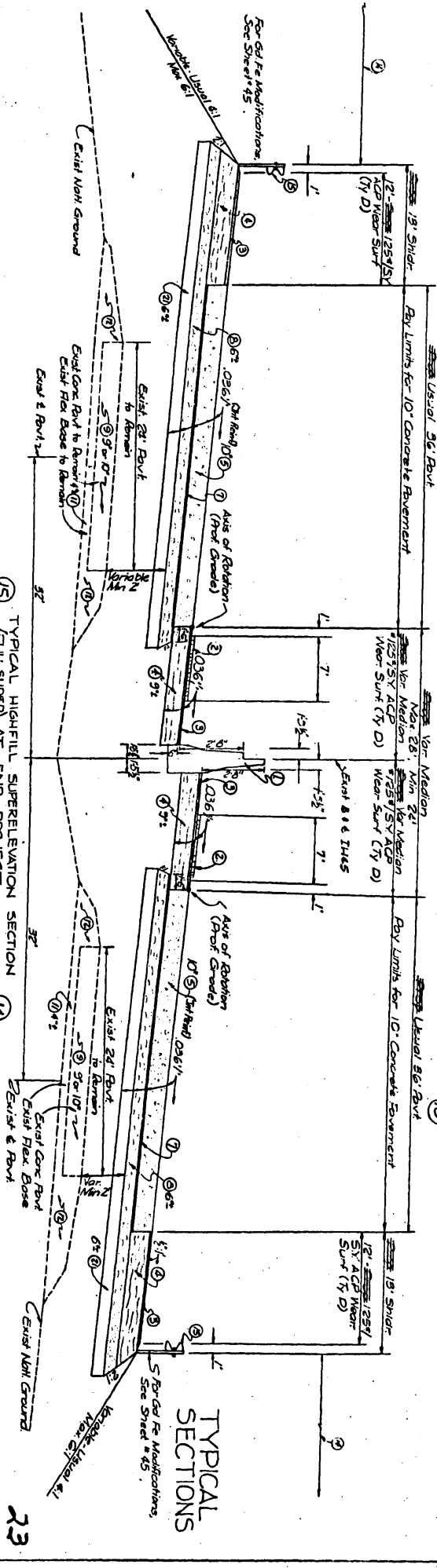
TYPICAL SECTIONS

JOBS LEGEND

1. For Fe & Conc Med Base
 2. For Fe & Conc Med Base
 3. For Fe & Conc Med Base
 4. For Fe & Conc Med Base
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 100. For Fe & Conc Med Base

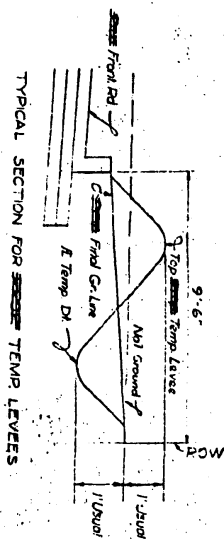


17) TYPICAL WIDENING SUPERELEVATION SECTION (FULL SUPER)



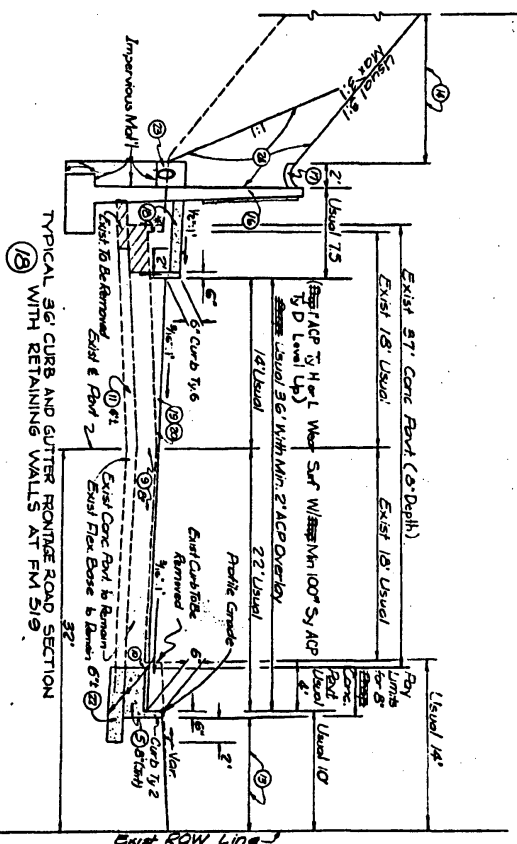
15) TYPICAL HIGHFILL SUPERELEVATION SECTION 14

TYPICAL SECTIONS



TYPICAL SECTION FOR ~~FREE~~ TEMP. LEVELS

STATION	TO	STATION
② 574 659+00 LY		② 510 666+92 LY
② 570 669+00 LY		② 510 675+30 LY
		② 509 727+40 LY

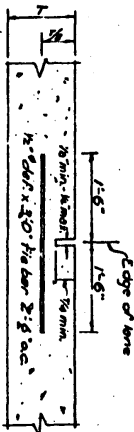


18) TYPICAL 36" CURB AND GUTTER FRONTAGE ROAD SECTION WITH RETAINING WALLS AT FM 519.

* To Be Used At Retaining Wall Areas Only

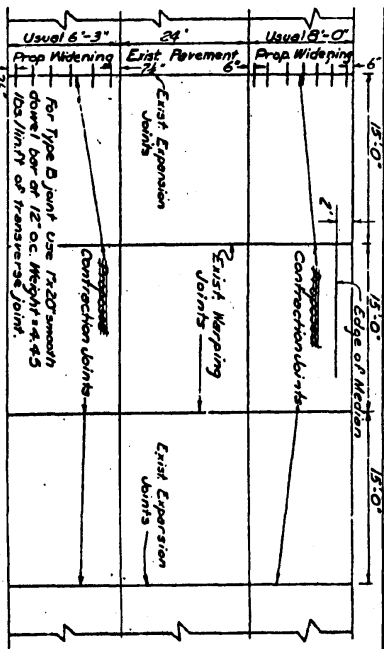
- [illegible]

TRANSVERSE CONTRACTION JOINTS



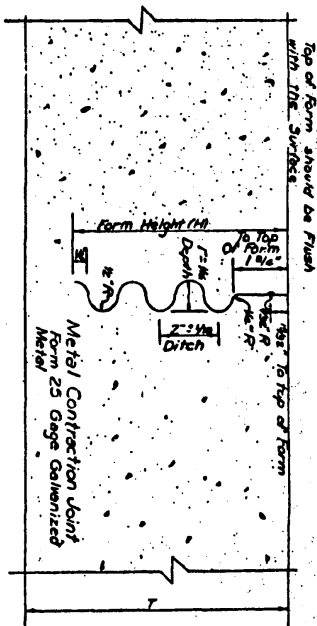
TYPE 1- CONSTRUCTION
(Tongue and Groove with Tie bar)

TYPE 2 - GROOVED
(Formed with Tie Bars)



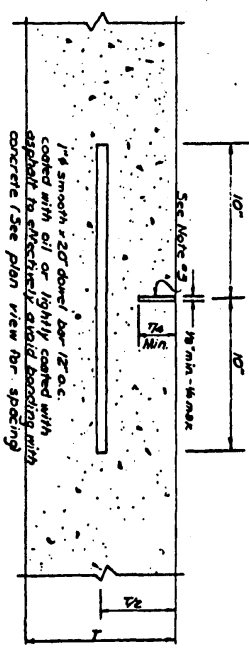
PLAN VIEW OF LONGITUDINAL AND TRANSVERSE CONTRACTION JOINTS

TRANSVERSE CONTRACTION JOINTS



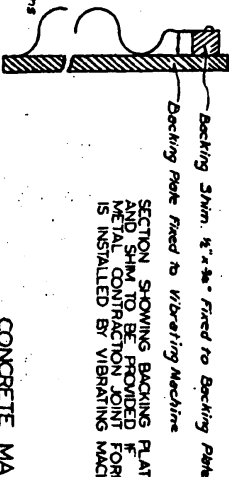
TYPE A (CORRUGATED METAL)

Note: Permissible Fabrication Tolerance: The semicircular segments shall be connected at a common point of tangency on the vertical axis of the joint or by a segment not exceeding one fourth of an inch (6.35 mm) in length at right angles to and symmetrical with the vertical axis of the joint.



TYPE B (GROOVED)
(Formed with Coated Dome)

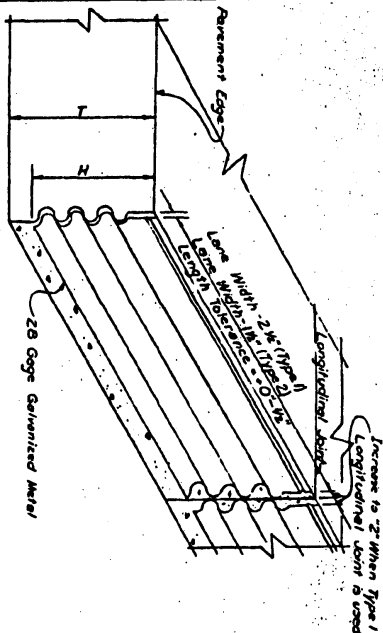
coated with oil or lightly coated with asphalt to effectively avoid bonding with concrete (See plan view for spacing)



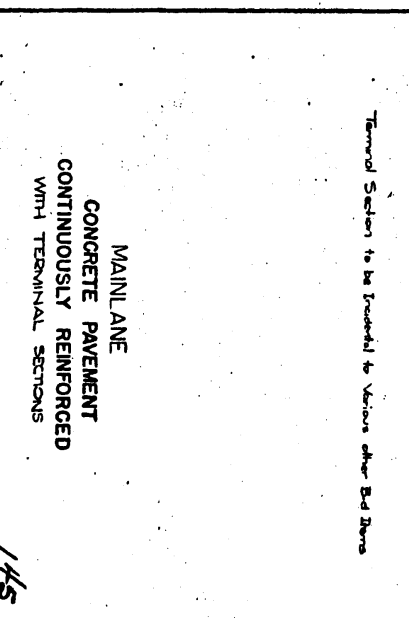
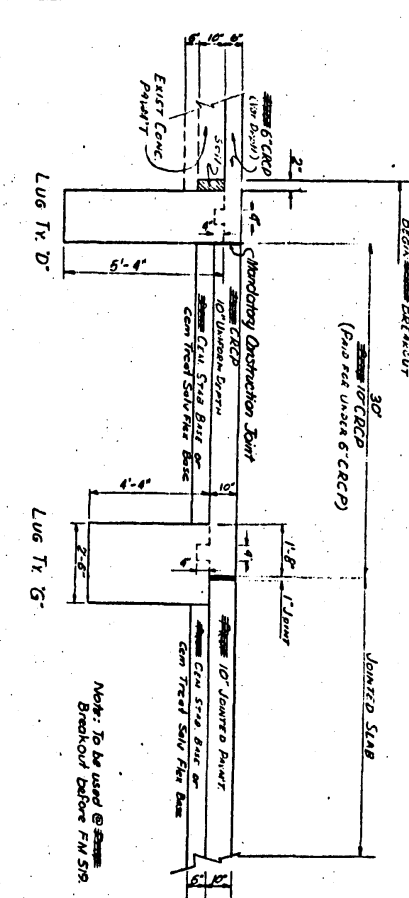
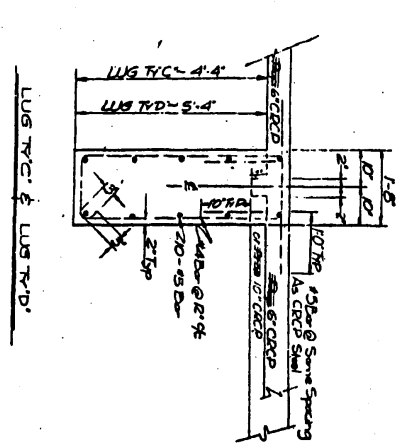
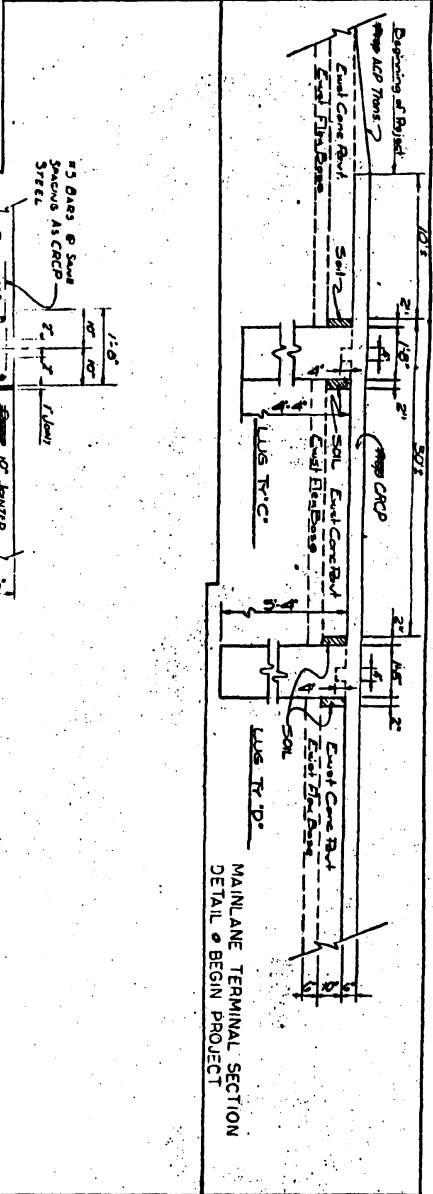
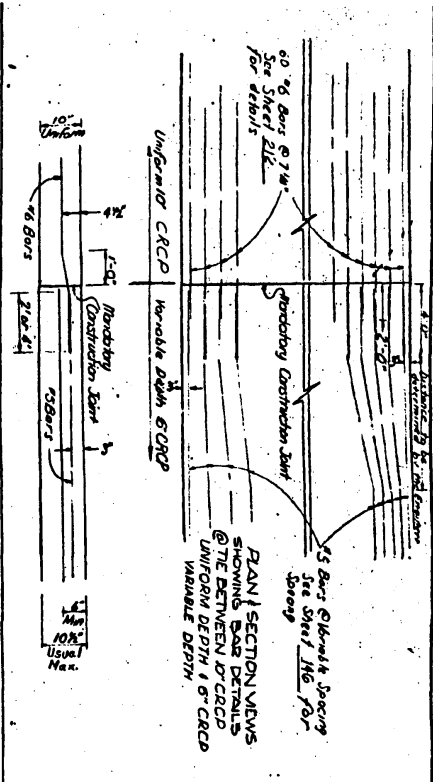
CONCRETE MAIN LANE PAVEMENT WIDENING DETAILS CONTRACTION DESIGN

[illegible]

OBLOUE SECTION SHOWING METAL
CONTRACTION JOINT FORM IN PLACE

[illegible]

Increase to "2" when Type 1 Longitudinal Joint is used



CRCP Reinforcement
Longitudinal Bars to be positioned 3" from surface of pavement.
See Terminal Anchorage Standard for details not shown here.
Reinforcing Steel size and placement not shown shall conform to 6' CRCP pavement shown elsewhere in the plans.

NOTE: Terminal Anchorage Steel for CRCP (Variable Depth) to be 6' 10' Bars from Top of CRCP (Variable Depth).

Terminal Section to be Truncated to Various other Bar Spacing

MAINLINE
CONCRETE PAVEMENT
CONTINUOUSLY REINFORCED
WITH TERMINAL SECTIONS

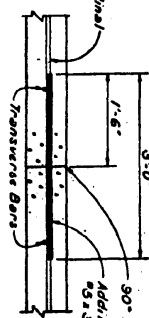
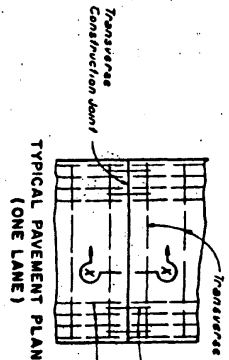
SHEET 3 OF 8 SHEETS

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68	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.67
69	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.68
70	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.69
71	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.70
72	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.71
73	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.72
74	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.73
75	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.74
76	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.75
77	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.76
78	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.77
79	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.78
80	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.79
81	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.80
82	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.81
83	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.82
84	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.83
85	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.84
86	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.85
87	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.86
88	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.87
89	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.88
90	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.89
91	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.90
92	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.91
93	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.92
94	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.93
95	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.94
96	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.95
97	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.96
98	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.97
99	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.98
100	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.99
101	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.00
102	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.01
103	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.02
104	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.03
105	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.04
106	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.05
107	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.06
108	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.07
109	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.08
110	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.09
111	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.10
112	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.11
113	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.12
114	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.13
115	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.14
116	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.15
117	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.16
118	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.17
119	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.18
120	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.19
121	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.20
122	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.21
123	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.22
124	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.23
125	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.24
126	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.25
127	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.26
128	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.27
129	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.28
130	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.29
131	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.30
132	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.31
133	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.32
134	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.33
135	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.34
136	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.35
137	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.36
138	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.37
139	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.38
140	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.39
141	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.40
142	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.41
143	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.42
144	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.43
145	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.44
146	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.45
147	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.46
148	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.47
149	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.48
150	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.49
151	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.50
152	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.51
153	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.52
154	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.53
155	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.54
156	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.55
157	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.56
158	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.57
159	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.58
160	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.59
161	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.60
162	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.61
163	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.62
164	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.63
165	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.64
166	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.65
167	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.66
168	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.67
169	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.68
170	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.69
171	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.70
172	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.71
173	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.72
174	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.73
175	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.74
176	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.75
177	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.76
178	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.77
179	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.78
180	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.79
181	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.80
182	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.81
183	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.82
184	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.83
185	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.84
186	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.85
187	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.86
188	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.87
189	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.88
190	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.89
191	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.90
192	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.91
193	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.92
194	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.93
195	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.94
196	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.95
197	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.96
198	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.97
199	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.98
200	10/1/85	J. J. J.	J. J. J.	J. J. J.	2.99

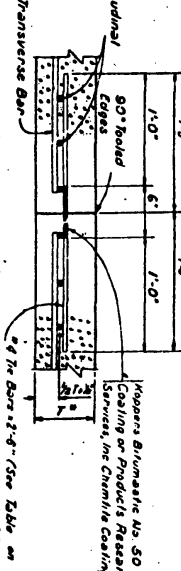
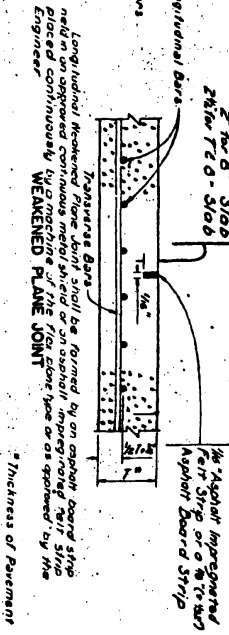
NO.	DATE	BY	CHKD.	APP.	REVISION
1	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.0
2	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.1
3	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.2
4	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.3
5	10/1/85	J. J. J.	J. J. J.	J. J. J.	1.4
6	10/1/85	J. J. J.	J.		

OVERLAY SECTION - VARIABLE DEPTH

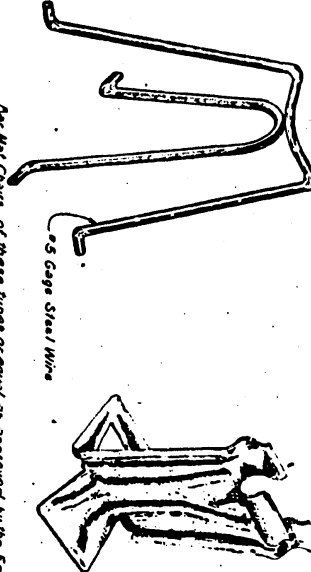
FREEWAY PAVEMENT PLAN



TRANSVERSE CONSTRUCTION JOINT



CONSTRUCTION JOINT
SHOWING PAVEMENT TIE BAR
LONGITUDINAL JOINTS



be used. Chairs will not be required, unless otherwise directed by engineer. Boltonizing of chairs will not be required.

SUGGESTED CHAIR DETAILS

CONCRETE PAVEMENT DETAILS CONTINUOUSLY REINFORCED STEEL BARS

[illegible]

DETAILS AS TO PRESENT WORK, TOLERANCES, AND GROWING EDGE SHALL BE AS SHOWN ELSEWHERE IN THE PLANS OR DIRECTED BY THE ENGINEER.

[illegible][illegible]

LEGAL NOTES

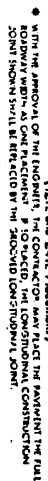
L-Bars @ 3'-4" (Carry thru)
 Longitudinal (Weakened Plane) $\frac{1}{2}$ 4 Bars @ 3'-0" (Carry thru)
 (Used with Constr. Jt. only)

* If longitudinal Meatened Plane Join is used to accommodate 36-0° width manulitic placement, omit tie bar and carry transverse steel through joint.

IT SHALL BE RESPONSIBILITY OF THE CONTRACTOR TO TAKE ALL NECESSARY PRECAUTIONS TO INSURE THAT THE FINAL POSITION OF THE STEEL IS WITHIN TOLERANCE OF $\frac{1}{4}$ " HORIZONTAL & VERTICALLY UP & DOWN FROM THE THEORETICAL POSITIONS SHOWN WITH A MIN 2" FROM BOTTOM OF STEEL TO BOTTOM OF SLAB.

[illegible]

NOT LESS THAN 6 LBS. THE LAPS OF THE LONGITUDINAL STEEL SHALL BE WELDED. THE FACES OF THE LAPS SHALL BE GRIND FLAT. THE JOINTS SHALL BE REINFORCED WITH STEEL BARS. THE JOINTS SHALL BE CLEANED SO THAT ADEQUATE BOND WILL DEVELOP BETWEEN NEW AND OLD CONCRETE. THE JOINTS SHALL BE PROTECTED BY A PROTECTIVE COATING. THE REQUIRED PLAIN THICKNESS OF MAINLINE PAVEMENT FOR DETERMINING THE PAVEMENT FOR INTERIOR PAVEMENT THICKNESS SHALL BE CONTINUED TO MEAN OF FOR THE FULL WIDTH OF THE SLAB, RESPECTIVELY. OF THE PROPOSED MAINLINE WIDTH OF PAVEMENT.



TWO LANE PAVEMENT PLAN
(24 ft. Placement) -



**TYPICAL SECTION
(24ft. Placement)**

*LANE WIDTHS ARE FOR ILLUSTRATIVE PURPOSES ONLY AND SHOULD NOT BE USED IF IN CONFLICT WITH TYPICAL CROSS SECTIONS SHOWN ELSEWHERE IN THE PLANS.

[illegible][illegible][illegible]

IF THE LEAVE-OUT IS LESS THAN 6' LONG THE
OF THE FREE END SHALL BE CLEANED SO THAT
RETE.

[illegible]

BASED UPON 1 FOOT PAYMENT LINGERS FOR THE WIDTH INDICATED
ALL TRANSVERSE STEEL IS 4 BARS AT 36" CENTERS

STEEL BARS

MAINLANES 10' CRCP

SHEET 5 OF 8 SHEETS

BY	DATE	PROJECT NO.	SHEET NO.
CA 24	1/2 1958	145-1(146) 010	147
CA 25	1/2 1958		
CA 26	1/2 1958		
CA 27	1/2 1958		
CA 28	1/2 1958		
CA 29	1/2 1958		
CA 30	1/2 1958		
CA 31	1/2 1958		
CA 32	1/2 1958		
CA 33	1/2 1958		
CA 34	1/2 1958		
CA 35	1/2 1958		
CA 36	1/2 1958		
CA 37	1/2 1958		
CA 38	1/2 1958		
CA 39	1/2 1958		
CA 40	1/2 1958		
CA 41	1/2 1958		
CA 42	1/2 1958		
CA 43	1/2 1958		
CA 44	1/2 1958		
CA 45	1/2 1958		
CA 46	1/2 1958		
CA 47	1/2 1958		
CA 48	1/2 1958		
CA 49	1/2 1958		
CA 50	1/2 1958		
CA 51	1/2 1958		
CA 52	1/2 1958		
CA 53	1/2 1958		
CA 54	1/2 1958		
CA 55	1/2 1958		
CA 56	1/2 1958		
CA 57	1/2 1958		
CA 58	1/2 1958		
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CA 60	1/2 1958		
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CA 62	1/2 1958		
CA 63	1/2 1958		
CA 64	1/2 1958		
CA 65	1/2 1958		
CA 66	1/2 1958		
CA 67	1/2 1958		
CA 68	1/2 1958		
CA 69	1/2 1958		
CA 70	1/2 1958		
CA 71	1/2 1958		
CA 72	1/2 1958		
CA 73	1/2 1958		
CA 74	1/2 1958		
CA 75	1/2 1958		
CA 76	1/2 1958		
CA 77	1/2 1958		
CA 78	1/2 1958		
CA 79	1/2 1958		
CA 80	1/2 1958		
CA 81	1/2 1958		
CA 82	1/2 1958		
CA 83	1/2 1958		
CA 84	1/2 1958		
CA 85	1/2 1958		
CA 86	1/2 1958		
CA 87	1/2 1958		
CA 88	1/2 1958		
CA 89	1/2 1958		
CA 90	1/2 1958		
CA 91	1/2 1958		
CA 92	1/2 1958		
CA 93	1/2 1958		
CA 94	1/2 1958		
CA 95	1/2 1958		
CA 96	1/2 1958		
CA 97	1/2 1958		
CA 98	1/2 1958		
CA 99	1/2 1958		
CA 100	1/2 1958		

REVISED DEC. 1, 1969

2. STEEL SCHEDULE REVISED SHOWING POLANDS PER SQUARE YARD OF STEEL FOR 24" AND 17" PLACEMENT WIDTHS TO CONFORM TO REVISED TRANSVERSE STEEL SPACING.

REVISED DEC. 1, 1969

REVISED DEC. 1, 1969