

1.

We first assume all the particles is under laminar flow condition and use Re parameter to readjusts the equation due to different flow type, then

$$Re = \frac{\rho v_s d_p}{\mu} \quad \text{and} \quad v_s = \frac{g(\rho_p - \rho_w)d_p^2}{18\mu}$$

where Re = Reynolds number, dimensionless

$\mu$  = dynamic viscosity,  $N \cdot s/m^2$  or  $kg/m \cdot s = 1.307 \times 10^{-3} kg/m \cdot s$

Then, in terms of different Re (Clark, 1996), readjusts our equation to fit different conditions

$$Cd = \frac{Re}{24} \quad (Re < 2) = \frac{18.5}{Re^{0.6}} \quad (2 < Re < 500) = 0.44 \quad (500 < Re < 2 \times 10^5)$$

$$v_{re} = \sqrt{\frac{4g(\rho_p - \rho_w)d_p}{3C_d\rho_w}} \quad \text{and} \quad Re_{re} = \frac{\rho v_s d_p}{\mu}$$

Make a table:

Particle	Re(laminar flow)	Actual Flow Type	Re <sub>re</sub>	v <sub>re</sub>
A	0.07	Laminar Flow	0.040	0.002
B	2.01	Turbulent Flow	1.939	0.005
C	0.43	Laminar Flow	0.423	0.002
D	1.72	Laminar Flow	1.068	0.015
E	2.07	Turbulent Flow	1.562	0.013

\* detailed calculation process by Matlab programming

2.

$$V_f = 20 \text{ cm/s} = 0.2 \text{ m/s} \quad \text{And} \quad a = 4 \text{ m}, b = 0.6 \text{ m}, c = 0.9 \text{ m}$$

$$v_s = \frac{b}{t} = \frac{b}{a/V_f} = \frac{0.6}{4/0.2} = 0.03 \text{ m/s}$$

$$\text{Then } OR = \frac{a}{c/v_s} = \frac{4}{0.9/0.03} = 0.13 \text{ m/s}$$

$$OR = \frac{h_o}{\tau} = \frac{h_o Q}{h_o A} = \frac{Q}{A} = \frac{OR \times S}{A} =$$

3.

$$OR = \frac{h_o}{\tau} = \frac{h_o Q}{h_o A} = \frac{Q}{A} = 50.47 \text{ m/d}$$

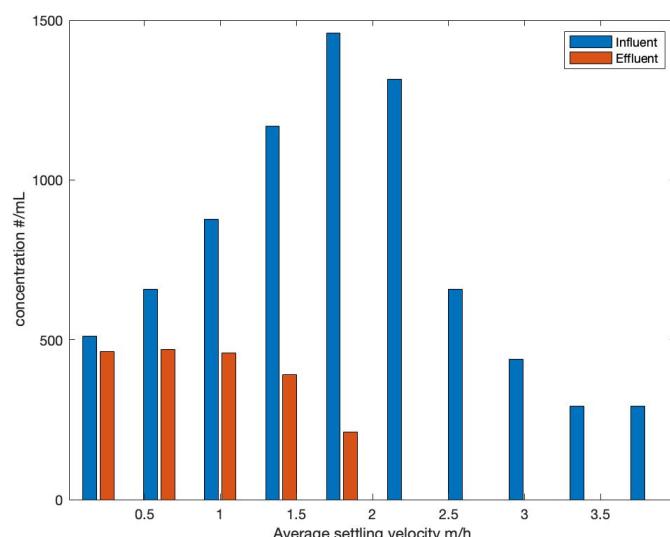
$$\text{Removal efficiency} = \frac{\sum N_i^0 \times (1 - \frac{v_i}{OR})}{N}$$

where N is the total particles amount and vi is the average settling velocity

Settling Velocity	Average Velocity	Number of Influent	Fraction of Removed	Number of Removed	Number in Effluent
0 - 0.4	0.2	511	0.10	49	462
0.4-0.8	0.6	657	0.29	187	470
0.8-1.2	1.0	876	0.48	417	459
1.2-1.6	1.4	1168	0.67	778	390
1.6-2.0	1.8	1460	0.86	1250	210
2.0-2.4	2.2	1314	1.00	1314	0
2.4-2.8	2.6	657	1.00	657	0
2.8-3.2	3.0	438	1.00	438	0
3.2-3.6	3.4	292	1.00	292	0
3.6-4.0	3.8	292	1.00	292	0

\* detailed calculation process by Matlab programming

$$\text{Removal efficiency} = \frac{\sum N_i^0 \times (1 - \frac{v_i}{OR})}{N} = 74 \%$$



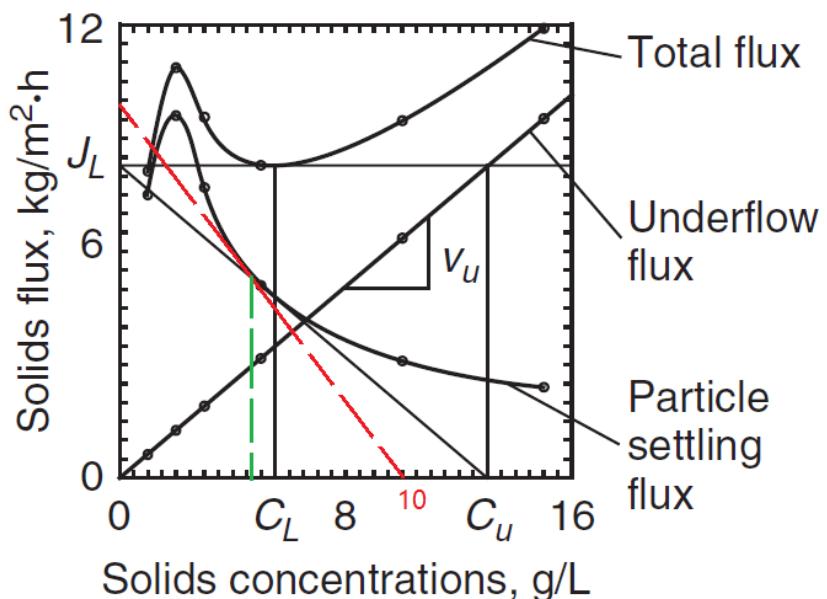
4.

Influent flow rate = 3000 m<sup>3</sup> / h

Influent solid concentration = 500 mg / L

Underflow solid concentration = 10000mg / L

Determine  $J_L$  and  $C_L$ . From the data plotted on Fig. 10-8b and an underflow solids concentration of  $C_u = 10000$  mg/L, the gravity flux is determined by drawing a line from the x axis at a solids concentration of 10,000 mg/L to the y axis such that it is tangent to the solids flux curve and intersects the y axis. The point at which the line intersects the y axis is the limiting gravity flux and is equal to  $9.90 \text{ kg/m}^2 \cdot \text{h}$ . The value for  $C_L$  can also be determined by drawing a vertical line from the tangent point to the x axis and is equal to 4600 mg/L.



Then

$$J_L = 9.9 \text{ kg/m}_2 \cdot \text{h}$$

$$C_L = 4600 \text{ mg/L}$$

$$Q_u = \frac{Q_i C_i}{C_u} = \frac{3000 \text{ m}^3/\text{h} \times 500 \text{ mg/L}}{10000 \text{ mg/L}} = 150 \text{ m}^3/\text{h}$$

5.

**Determine the number of basins:** Two basins would satisfy the minimum requirement for maintenance purposes. According to the small flow in the requirement, two basins will be selected.

**SIZE:** The basin width will be governed by the standard size of sludge removal equipment. The standard maximum width of the chain-and-flight sludge collector is 6 m, so basin widths in increments of 3 m will be considered, starting with 6 m. Water depths from 3 to 5 m are appropriate, according to the design criteria listed in Table 10-4. As previously mentioned, deeper basins are recommended over shallower basins, so a depth of 4 m will be selected.

**Determine the basin area:** I assume The water is coagulated with alum and the alum floc was measured to have a settling velocity of 2.2 m/h at 10°C (50°F). The dynamic viscosity of water at 10°C is 0.00131 kg/m s and the density of water is 999.7 kg/m<sup>3</sup>.

$$A = \frac{Q}{v_c} = \frac{0.1736 m^3/s}{2.2 m/h \times \frac{1 h}{3600 s}} = 284.1 m^2$$

$$ratio = \frac{A}{2 \times 3m} \times \frac{1}{3m} = 4.0$$

The length-to-width ratio is greater than the minimum recommendation of 4:1 to 5:1.

**Check the various design parameters listed in Table 10-4**

$$Q_{max} = \frac{(6 \times A \times 4) \times 2 basins}{OR \times \frac{1 h}{3600 s}} = 21.82 h$$

6.