

Joints and Doublers

Bill Gran

GRAN Corporation

Joints and Doublers

by

BILL GRAN

President, CEO & Super Genius

GRAN CORPORATION

First Edition

GRAN CORPORATION

2010

Joints and Doublers

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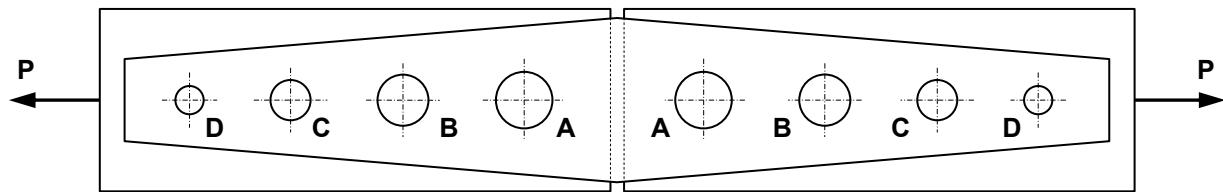
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1.0 Bolt Load Distribution

Elmer F. Bruhn *Analysis and Design of Flight Vehicle Structures* page D1.3

$$P = 100,000 \text{ lb}$$



Shear Strength

ANC-5 *Strength of Metal Aircraft Elements* page 32

Table 2.6111(a) Shear and Tensile Strengths ... of Steel Bolts and Pins

Bolt	AN Bolt	Diameter	Single Shear	Double Shear
A	AN-10	5 / 8	23,000	46,000 lb
B	AN-9	9 / 16	18,700	37,400 lb
C	AN-7	7 / 16	11,250	22,500 lb
D	AN-5	5 / 16	5,750	11,500 lb
				Σ 117,400 lb

Load Distribution

$$P_a = P \frac{P_{sa}}{\sum P_s} = 100,000 \text{ lb} \left(\frac{46,000 \text{ lb}}{117,400 \text{ lb}} \right) = 39,182 \text{ lb}$$

$$P_b = P \frac{P_{sb}}{\sum P_s} = 100,000 \text{ lb} \left(\frac{37,400 \text{ lb}}{117,400 \text{ lb}} \right) = 31,857 \text{ lb}$$

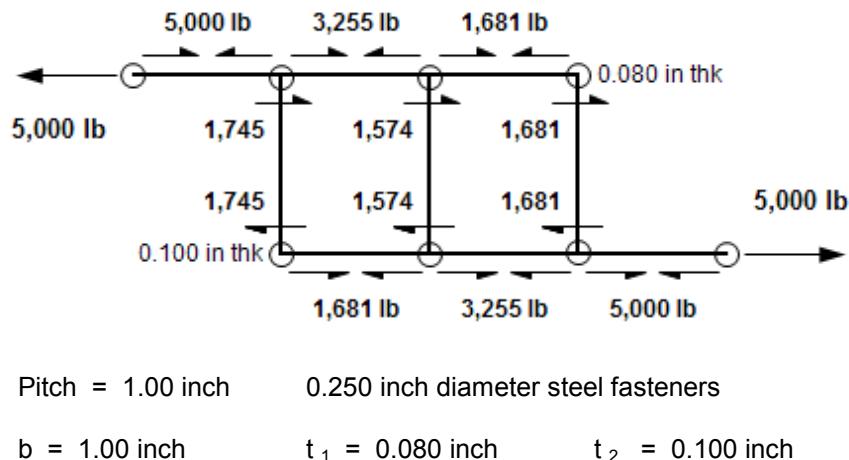
$$P_c = P \frac{P_{sc}}{\sum P_s} = 100,000 \text{ lb} \left(\frac{22,500 \text{ lb}}{117,400 \text{ lb}} \right) = 19,165 \text{ lb}$$

$$P_d = P \frac{P_{sd}}{\sum P_s} = 100,000 \text{ lb} \left(\frac{11,500 \text{ lb}}{117,400 \text{ lb}} \right) = 9,796 \text{ lb}$$

Check

$$P = 39,182 \text{ lb} + 31,857 \text{ lb} + 19,165 \text{ lb} + 9,796 \text{ lb} = 100,000 \text{ lb}$$

2.0 Fastener Flexibility



References

Heimo Huth

Influence of Fastener Flexibility on the Prediction of Load Transfer and Fatigue Life for Multiple-Row Joints

Correct equation is...

$$C = [(t_1 + t_2)/2d]^a (b/n) [1/(t_1 E_1) + 1/(n t_2 E_2) + 1/(2t_1 E_3) + 1/(2n t_2 E_3)]$$

“Zum Einfluss der Nietnachgiebigkeit mehrreihiger Nietverbindungen auf die Lastübertragungs - und Lebensdauervorhersage”

LBF Report No. FB-172, Dissertation, Technische Universität München, Munich, Germany, 1984.

Thanks to analyst64 on the www.eng-tips.com website.

Manford B. Tate and Samuel J. Rosenfeld

NACA TN-1051 *Preliminary Investigation of the Loads Carried by Individual Bolts in Bolted Joints*

Samuel J. Rosenfeld

ASTM STP 927 *Fatigue in Mechanically Fastened Composite and Metallic Joints*

NACA TN-1458 *Analytical and Experimental Investigation of Bolted Joints*

T. Swift *Development of the Fail-Safe Design Features of the DC-10*

William F. McCombs, James C. McQueen, Jeffrey L. Perry

AFFDL-TR-67-184 *Analytical Design Methods for Aircraft Structural Joints*

<http://www.dtic.mil/>

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Data for Calculating Joint Spring Constants

William F. McCombs

Engineering Column Analysis - The Analysis of Compression Members Appendix E, page E1

$$k_{o\ joint} = \frac{1}{\frac{1}{k_1} + \frac{1}{k_2}} = \frac{k_1 k_2}{k_1 + k_2} \quad \text{where } k_n = A_n k_{n\ tabulated}$$

Figure E-1

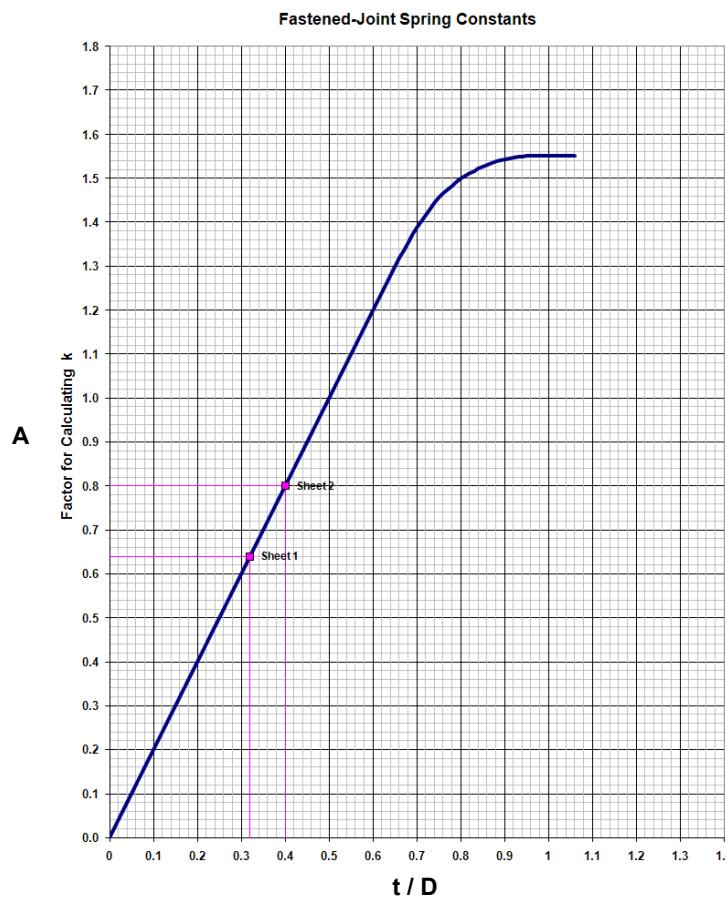


Table E-2

Fsnr Dia	$k_n \times 10^{-6}$		
	D	Alum	Steel
0.125	0.212	4.71	0.715
0.156	0.264	5.88	0.892
0.188	0.317	7.07	1.073
0.250	0.423	9.43	1.431
0.313	0.528	11.80	1.790
0.375	0.633	14.20	2.154
0.438	0.732	16.40	2.488
0.540	0.845	18.90	2.867
0.563	0.952	21.20	3.216
0.625	1.060	23.50	3.565

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Plate Constants

Samuel J. Rosenfeld

NACA TN-1458 *Analytical and Experimental Investigation of Bolted Joints* page 6

$$K = \frac{p}{b t E}$$

p *Fastener Pitch*

b *Plate Width*

t *Plate Thickness*

E *Young's Modulus*

$$K_a = K_b = \frac{p}{b t E} = \frac{1.00 \text{ inch}}{1.00 \text{ inch} (0.080 \text{ inch}) 10.5 \text{ ksi}} = 1.19 \times 10^{-6} \text{ inch/lb}$$

$$K_c = K_d = \frac{p}{b t E} = \frac{1.00 \text{ inch}}{1.00 \text{ inch} (0.100 \text{ inch}) 10.5 \text{ ksi}} = 0.92 \times 10^{-6} \text{ inch/lb}$$

Fastened-Joint Spring Constants

William F. McCombs

Engineering Column Analysis - The Analysis of Compression Members Appendix E, page E1

$$\frac{t_1}{d} = 0.080 \text{ inch} / 0.25 \text{ inch} = 0.32 \quad A = 0.64 \quad k_1' = k_3' = 0.64 (423,000) = 270,720 \text{ lb/inch}$$

$$\frac{t_2}{d} = 0.100 \text{ inch} / 0.25 \text{ inch} = 0.40 \quad A = 0.80 \quad k_2' = 0.80 (423,000) = 338,400 \text{ lb/inch}$$

Three Equations, Three Unknowns

$$\left(\frac{R_1}{k_1'} \right) + R_1 K_a = (P - R_1) K_b + \left(\frac{R_2}{k_2'} \right)$$

$$\left(\frac{R_2}{k_2'} \right) + (R_1 + R_2) K_c = (P - R_1 - R_2) K_d + \left(\frac{R_3}{k_3'} \right)$$

$$R_1 + R_2 + R_3 = P$$

$$6.075 R_1 - 2.955 R_2 = 5,952.4$$

$$1.905 R_1 + 4.860 R_2 - 3.694 R_3 = 4,761.9$$

$$R_1 + R_2 + R_3 = 5,000$$

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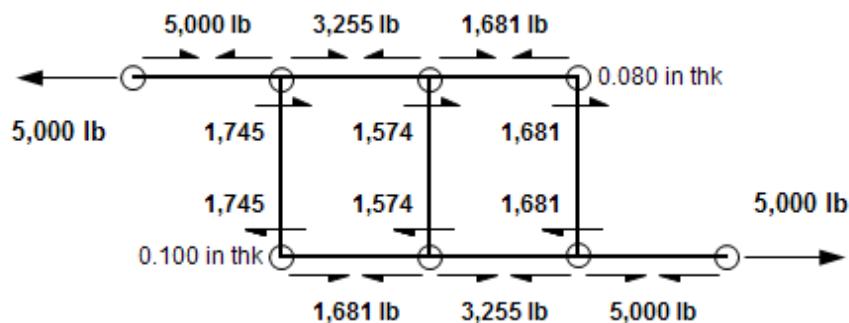
Solve

$$\begin{bmatrix} 6.075 & -2.955 & 0 \\ 1.905 & 4.860 & -3.694 \\ 1 & 1 & 1 \end{bmatrix} \begin{Bmatrix} R_1 \\ R_2 \\ R_3 \end{Bmatrix} = \begin{Bmatrix} 5,952 \\ 4,762 \\ 5,000 \end{Bmatrix}$$

$$\begin{bmatrix} 6.075 & -2.955 & 0 \\ 1.905 & 4.860 & -3.694 \\ 1 & 1 & 1 \end{bmatrix}^{-1} = \begin{bmatrix} 0.125 & 0.043 & 0.159 \\ -0.082 & 0.089 & 0.328 \\ -0.043 & -0.132 & 0.513 \end{bmatrix}$$

$$\begin{Bmatrix} R_1 \\ R_2 \\ R_3 \end{Bmatrix} = \begin{Bmatrix} 5,952 \\ 4,762 \\ 5,000 \end{Bmatrix} \begin{bmatrix} 0.125 & 0.043 & 0.159 \\ -0.082 & 0.089 & 0.328 \\ -0.043 & -0.132 & 0.513 \end{bmatrix} = \begin{Bmatrix} 1,745 \\ 1,574 \\ 1,681 \end{Bmatrix}$$

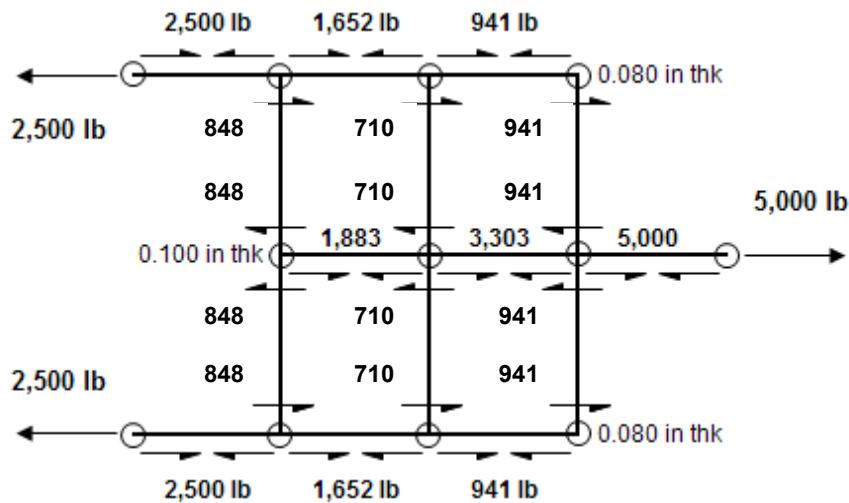
Single Shear



Take this analysis with a block of salt. Compare to $5,000 \text{ lb} / 3 = 1,667 \text{ lb}$ per fastener.

Double Shear

In a similar fashion:



Compare to $5,000 \text{ lb} / 6 = 833 \text{ lb}$

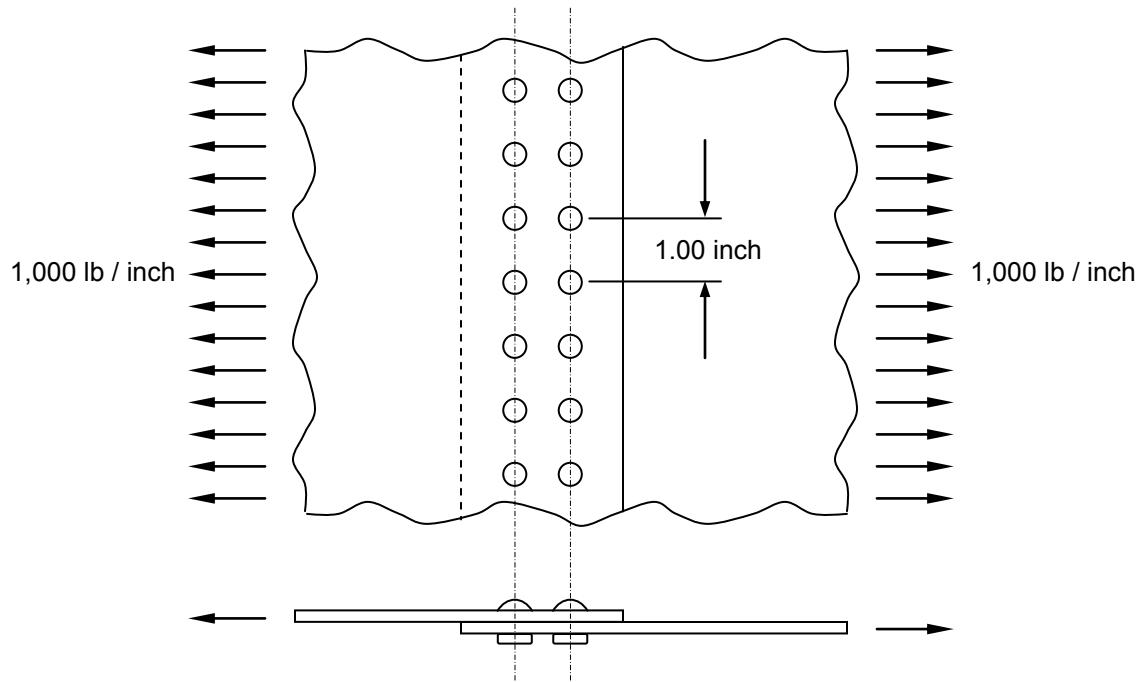
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3.0 Lap Joint

William F. McCombs *Analysis and Design of Flight Vehicle Structures* page D1.24

See page D3.14 Good Practice - Holes in Line Poor Practice - Holes Staggered

Ultimate Load = 869.6 lb / inch (1.15 Fitting Factor) = 1,000 lb / inch



Tension in Sheet

Elmer F. Bruhn *Analysis and Design of Flight Vehicle Structures* Table D1.6, page D1.21

MMPDS-01 *Metallic Materials Properties Development and Standardization* page 8-12

Table 8.1.2(a) Standard Rivet-Hole Drill Sizes and Nominal Hole Diameters

Rivet Size, inch	1/16	3/32	1/8	5/32	3/16	1/4	5/16	3/8
Drill No.	51	41	30	21	11	F	P	W
Nominal Hole Diameter, inch	0.067	0.096	0.1285	0.159	0.191	0.257	0.323	0.386

AD Rivets, Driven AL 2117-T3 5/32 inch diameter Nominal Hole Diameter = 0.159 inch

Ultimate Tension $F_{tu} = 60 \text{ ksi}$ Clad 2024-T3 0.040 inch thick sheet

Net Area $A_{net} = 0.040 \text{ inch} (1.00 \text{ inch} - 0.159 \text{ inch}) = 0.0336 \text{ in}^2$

Tension Allowable $P_{allow} = F_{tu} A_{net} = 60,000 \text{ psi} (0.0336 \text{ in}^2) = 2,018 \text{ lb}$

Sheet Tension Margin of Safety

$$MS = \frac{P_{allow}}{P} - 1 = \frac{2,018 \text{ lb}}{1,000 \text{ lb}} - 1 = 1.02$$

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Shear

MMPDS-01 *Metallic Materials Properties Development and Standardization*

page 8-13

Table 8.1.2(b) Single Shear Strength of Solid Rivets^a

Undriven			Driven		Rivet Designation	Rivet Size								
Rivet Material	F _{su} (ksi)		Rivet Material	F _{su} (ksi)		1/16	3/32	1/8	5/32	3/16	1/4	5/16	3/8	
	Min	Max				Driven Single Shear Strength (lbs) ^c								
5056-H32	24	n/a	5056-H32 ^d	28 ^e	B ^f	99	203	363	556	802	1,450	2,290	3,275	
2117-T4	26	n/a	2117-T3	30 ^e	AD	106	217	389	596	860	1,555	2,455	3,510	
2017-T4	35	42	2017-T3	38 ^e	D	134	275	493	755	1,085	1,970	3,115	4,445	
2024-T4	37	n/a	2024-T31	41 ^e	DD	145	297	532	814	1,175	2,125	3,360	4,795	
7050-T73	41	46	7050-T73 ^d	43 ^e	E ^h	152	311	558	854	1,230	2,230	3,520	5,030	
Monel	49	59	Monel	52 ^e	M	183	376	674	1,030	1,490	2,695	4,260	6,085	
Ti-45Cb	50	59	Ti-45Cb	53 ^e	T	187	384	687	1,050	1,515	2,745	4,340	6,200	
A-286	85	95	A-286 90 ^e	90 ^e	-	317	651	1,165	1,785	2,575	4,665	7,375	10,500	

MMPDS-01 *Metallic Materials Properties Development and Standardization* page 8-15

Table 8.1.2.1(b) Shear Strength Correction Factors for Solid Protruding Head Rivets^a

Thick (inch)	Fastener Diameter (inch)							
	1/16	3/32	1/8	5/32	3/16	1/4	5/16	3/8
	Single-Shear Rivet Strength Factors							
0.016	0.964
0.018	0.981	0.912
0.020	0.995	0.933
0.025	1.000	0.970	0.920
0.032	...	1.000	0.964	0.925
0.036	0.981	0.946	0.912
0.040	0.995	0.964	0.933
0.045	1.000	0.981	0.953
0.050	0.995	0.970	0.920
0.063	1.000	1.000	0.961	0.922	...
0.071	0.979	0.944	0.909
0.080	0.995	0.964	0.933
0.090	1.000	0.981	0.953
0.100	0.995	0.972
0.125	1.000	1.000

^a Sheet thickness is that of the thinnest sheet in single-shear joints and the middle sheet in double-shear joints.

MMPDS-01 Section 8.1.2.1 Protruding-Head Solid Rivet Joints page 8-11

In computing protruding-head rivet design shear strengths, the shear strength values obtained from Table 8.1.2(b) should be multiplied by the correction factors given in Table 8.1.2.1(b). This compensates for the reduction in rivet shear strength resulting from high bearing stresses on the rivet at t/D ratios less than 0.33 for single-shear joints and 0.67 for double-shear joints.

Single Shear Strength

$$P_{\text{allow}} = P_s C = 596 \text{ lb per rivet} (0.964) 2 \text{ rivets} = 1,149 \text{ lb}$$

Single Shear Margin of Safety

$$MS = \frac{P_{\text{allow}}}{P} - 1 = \frac{1,149 \text{ lb}}{1,000 \text{ lb}} - 1 = 0.15$$

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Bearing

Unit Bearing Strength

MMPDS-01 *Metallic Materials Properties Development and Standardization*

page 8-14

Table 8.1.2.1(a) Unit Bearing Strength of Sheet on Rivets $F_{br} = 100$ ksi

Sheet Thickness (inch)	Fastener Diameter (inch)							
	1/16	3/32	1/8	5/32	3/16	1/4	5/16	3/8
Unit Bearing Strength for Indicated Rivet Diameter (lbs)								
0.012	80
0.016	107
0.018	121	173
0.020	134	192
0.025	168	240	321
0.032	214	307	411	509
0.036	241	346	462	572	688
0.040	268	384	514	636	764
0.045	302	432	578	716	860
0.050	335	480	642	795	955	1,285
0.063	422	605	810	1,002	1,203	1,619	2,035	...
0.071	476	682	912	1,129	1,356	1,825	2,293	2,741
0.080	536	768	1,028	1,272	1,528	2,056	2,584	3,088
0.090	603	864	1,156	1,431	1,719	2,313	2,907	3,474
0.100	670	960	1,285	1,590	1,910	2,570	3,230	3,860
0.125	838	1,200	1,606	1,988	2,388	3,212	4,038	4,825
0.160	1,072	1,536	2,056	2,544	3,056	4,112	5,168	6,176
0.190	1,273	1,824	2,442	3,021	3,629	4,883	6,137	7,334
0.250	1,670	2,400	3,210	3,975	4,775	6,425	8,075	9,650

Bearing Factor

William F. McCombs *Analysis and Design of Flight Vehicle Structures* Table D1.8, page D1.22

ANC-5 Strength of Metal Aircraft Elements

Table 3.6111(b) Aluminum Alloy and Plate – Bearing Factors, page 84

Clad 2024-T3 0.040 inch thick A Values Edge Margin = 5 / 16 inch e / D = 2

K = 1.14 Ratio of actual bearing strength of sheet to $F_{br} = 100$ ksi

Bearing Strength

From Table 8.1.2.1(a) Unit Bearing Strength $P_{br} = 636$ lb

$P_{allow} = P_{br} K = 636$ lb per rivet (1.14) 2 rivets = 1,450 lb

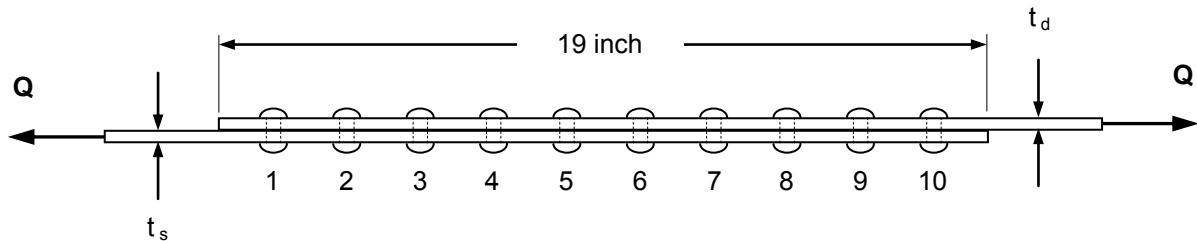
Bearing Margin of Safety

$$MS = \frac{P_{allow}}{P} - 1 = \frac{1,450 \text{ lb}}{1,000 \text{ lb}} - 1 = 0.45$$

4.0 Splice

William F. McCombs, James C. McQueen, Jeffrey L. Perry

AFFDL-TR-67-184 *Analytical Design Methods for Aircraft Structural Joints* pages 28 - 30



Given

$$Q = 8,000 \text{ lb} \quad t_s = 0.20 \text{ inch} \quad t_d = 0.20 \text{ inch} \quad \text{Pitch, } p = 1.90 \text{ inch}$$

$$\text{Width} = 1.84 \text{ inch} \quad \text{Steel, } E = 29 \text{ msi} \quad D = 0.375 \text{ inch} \quad k_F = 800,000 \text{ lb/inch}$$

Data

$$\text{Net Effective Area} \quad A_s = (\text{Width} - 0.80 D) t_s \quad \text{Note: } \cos 40^\circ = 0.766$$

$$A_s = [1.84 \text{ inch} - 0.80 (0.375 \text{ inch})] 0.20 \text{ inch} = 0.308 \text{ in}^2$$

$$A_d = [1.84 \text{ inch} - 0.80 (0.375 \text{ inch})] 0.20 \text{ inch} = 0.308 \text{ in}^2$$

$$k = \frac{k_F}{p} = \frac{800,000 \text{ lb/inch}}{1.90 \text{ inch}} = 421,053 \text{ lb/inch per inch}$$

$$N = \frac{k Q}{A_s E} = \frac{421,052.6 \frac{\text{lb}}{\text{in}} \text{ per inch} (8,000 \text{ lb})}{0.308 \text{ in}^2 (29 \text{ E06 psi})} = 377.118$$

$$M = k \left(\frac{1}{A_s E_s} + \frac{1}{A_d E_d} \right)$$

$$M = 421,052.6 \frac{\text{lb}}{\text{in}} \text{ per inch} \left[\frac{1}{0.308 \text{ in}^2 (29 \text{ E06 psi})} + \frac{1}{0.308 \text{ in}^2 (29 \text{ E06 psi})} \right] = 0.09428$$

$$C_1 = \frac{Q - \frac{N}{M} (1 - e^{-\sqrt{M} L})}{e^{\sqrt{M} L} - e^{-\sqrt{M} L}}$$

$$C_1 = \frac{8,000 \text{ lb} - \frac{377.118}{0.09428} (1 - e^{-19 \sqrt{0.09428}})}{e^{19 \sqrt{0.09428}} - e^{-19 \sqrt{0.09428}}} = \frac{8,000 - 3,988.3}{341.709 - 0.003} = 11.740$$

$$P_n = C_1 (e^{\sqrt{M} x} - e^{-\sqrt{M} x}) + \frac{N}{M} (1 - e^{-\sqrt{M} x})$$

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Fastener Loads

$$P_n = C_1 \left(e^{\sqrt{M}x} - e^{-\sqrt{M}x} \right) + \frac{N}{M} \left(1 - e^{-\sqrt{M}x} \right)$$

$$P_1 = 11.740 \left(e^{1.9\sqrt{0.09428}} - e^{-1.9\sqrt{0.09428}} \right) + \frac{377.118}{0.09428} \left(1 - e^{-1.9\sqrt{0.09428}} \right)$$

$$P_1 = P_{F1} = 1,782.5 \text{ lb}$$

$$P_2 = 11.740 \left(e^{3.8\sqrt{0.09428}} - e^{-3.8\sqrt{0.09428}} \right) + \frac{377.118}{0.09428} \left(1 - e^{-3.8\sqrt{0.09428}} \right)$$

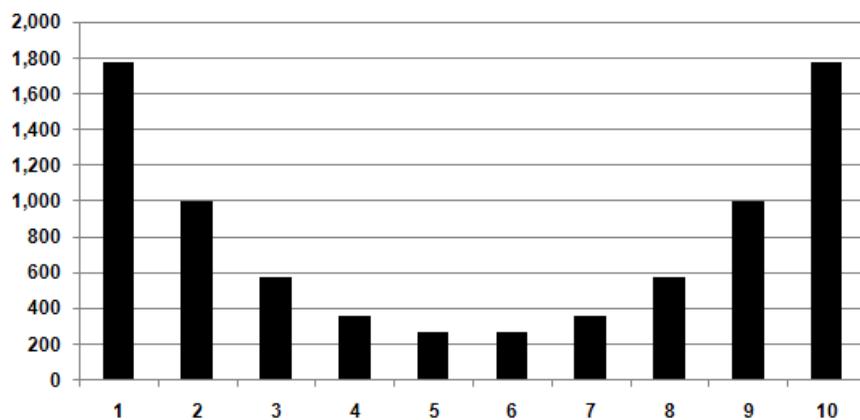
$$P_2 = 2,788.6 \text{ lb} \quad P_{F2} = 2,788.6 \text{ lb} - 1,782.5 \text{ lb} = 1,006.1 \text{ lb}$$

$$P_3 = 11.740 \left(e^{5.7\sqrt{0.09428}} - e^{-5.7\sqrt{0.09428}} \right) + \frac{377.118}{0.09428} \left(1 - e^{-5.7\sqrt{0.09428}} \right)$$

Table

Fastener	x	P _n	P _F	%
n	inch	lb	lb	
1	1.9	1,782.5	1,782	22.3%
2	3.8	2,788.6	1,006	12.6%
3	5.7	3,370.6	582	7.3%
4	7.6	3,732.2	362	4.5%
5	9.5	4,000.0	268	3.3%
6	11.4	4,267.8	268	3.3%
7	13.3	4,629.4	362	4.5%
8	15.2	5,211.4	582	7.3%
9	17.1	6,217.5	1,006	12.6%
10	19.0	8,000.0	1,782	22.3%

Load Distribution Chart



Load in Center Segment

$$P = 11.740 \left(e^{9.5\sqrt{0.09428}} - e^{-9.5\sqrt{0.09428}} \right) + \frac{377.118}{0.09428} \left(1 - e^{-9.5\sqrt{0.09428}} \right)$$

$$P = 4,000 \text{ lb}$$

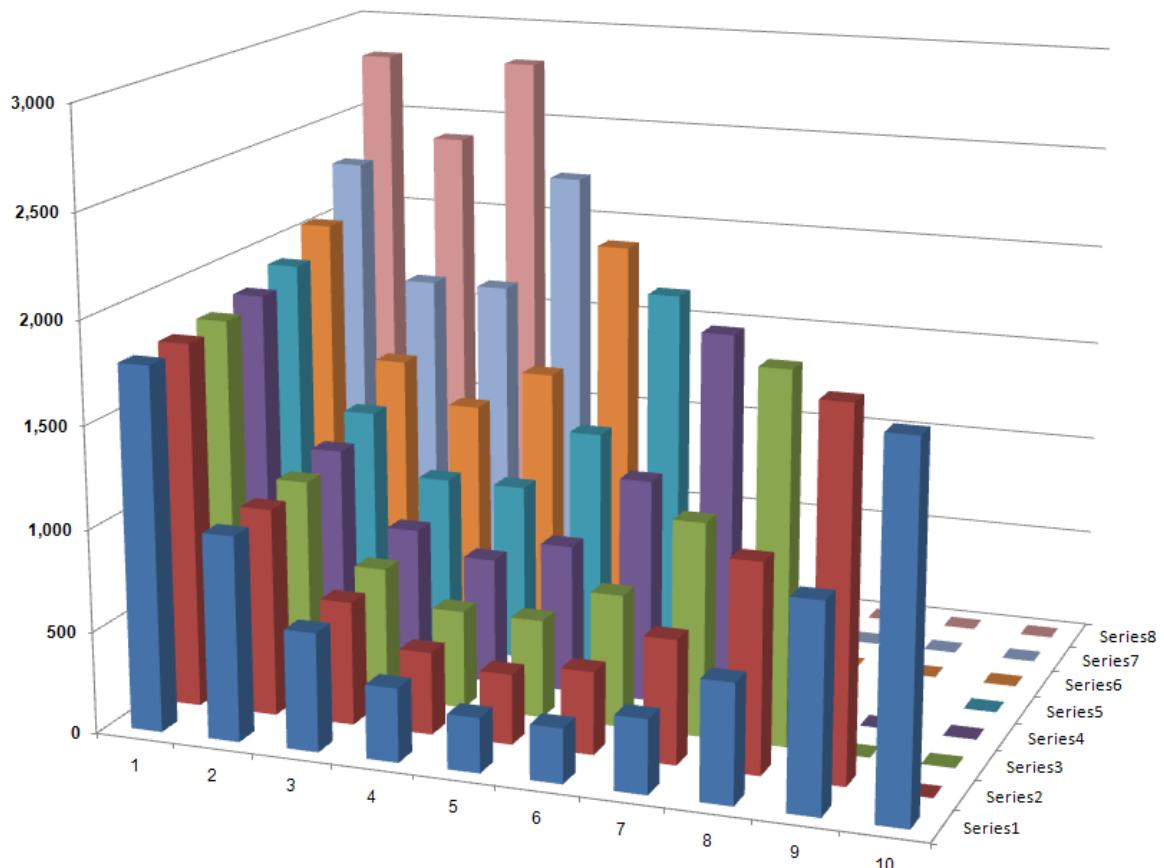
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Compare Splices with Three to Ten Fasteners

Table

Series	1	2	3	4	5	6	7	8
L	19	17.1	15.2	13.3	11.4	9.5	7.6	5.7
C₁	14.675	26.361	47.441	85.663	155.63	285.96	536.78	1,051.4
n	10	9	8	7	6	5	4	3
Fasteners	P_F							
	lb							
1	1,782	1,794	1,815	1,853	1,922	2,050	2,298	2,806
2	1,006	1,022	1,050	1,101	1,194	1,368	1,702	2,388
3	582	607	652	734	884	1,164	1,702	2,806
4	362	405	483	625	884	1,368	2,298	
5	268	345	483	734	1,194	2,050		
6	268	405	652	1,101	1,922			
7	362	607	1,050	1,853				
8	582	1,022	1,815					
9	1,006	1,794						
10	1,782							

Column Chart



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Numerical Method

William F. McCombs, James C. McQueen, Jeffrey L. Perry

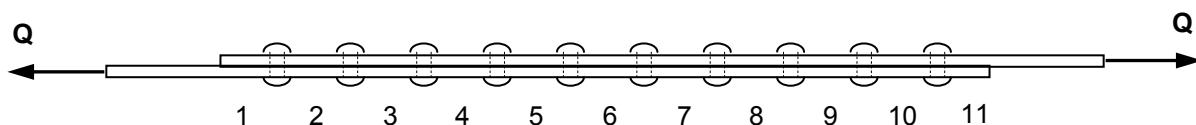
AFFDL-TR-67-184 *Analytical Design Methods for Aircraft Structural Joints* pages 39 to 43

$$k_d = \frac{A_d E_d}{p} = \frac{0.308 \text{ in}^2 (29 \text{ E06 psi})}{1.90 \text{ inch}} = 470,105 \text{ lb/inch per inch}$$

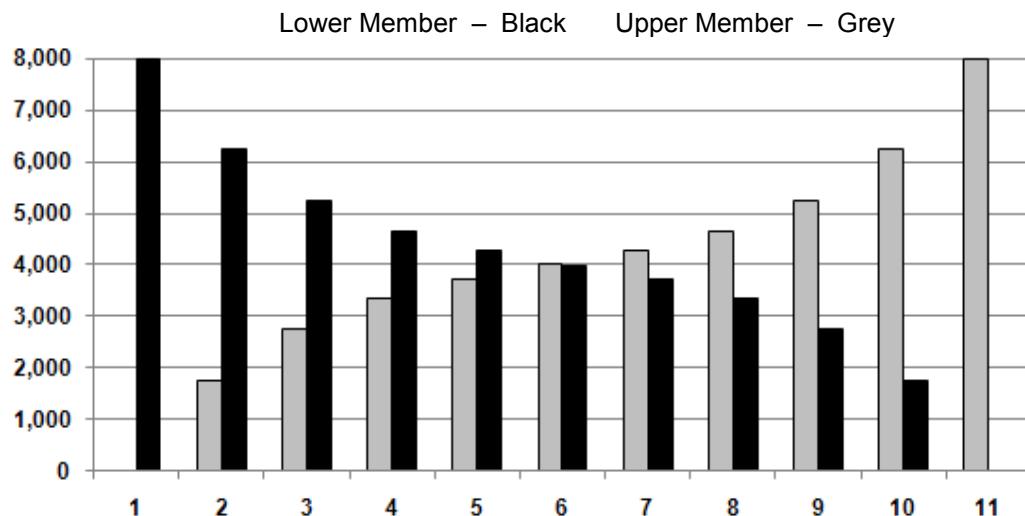
$$k_s = \frac{A_s E_s}{p} = \frac{0.308 \text{ in}^2 (29 \text{ E06 psi})}{1.90 \text{ inch}} = 470,105 \text{ lb/inch per inch}$$

Table

1	2	4	5	6	7	8	9	12	13	14	15	16
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Member Load P_D	Spring Constant k_D	Member Strain $\Delta\delta_D$	Applied Load $q_L + (11)$	Load in Base P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$
n	$(2)_{n-1} - (16)_{n-1}$	$(2) + (3)$	$\times 10^6 \text{ lb/in}$	$(4) * (5)$	$\Sigma (6)$	$\times 10^6 \text{ lb/in}$	$(7) / (8)$	$q_L + (11)$	$(12) - (7)$	$\times 10^6 \text{ lb/in}$	$(13) / (14)$	$(15) - (9)$
1	2,207	2,207	0.80	1,766	1,766	4.70	376	8,000	6,234	4.70	1,326	951
2	1,256	1,256	0.80	1,005	2,771	4.70	589	8,000	5,229	4.70	1,112	523
3	733	733	0.80	587	3,357	4.70	714	8,000	4,643	4.70	988	273
4	460	460	0.80	368	3,725	4.70	792	8,000	4,275	4.70	909	117
5	343	343	0.80	275	4,000	4.70	851	8,000	4,000	4.70	851	0
6	343	343	0.80	275	4,275	4.70	909	8,000	3,725	4.70	792	-117
7	460	460	0.80	368	4,643	4.70	988	8,000	3,357	4.70	714	-273
8	733	733	0.80	587	5,229	4.70	1,112	8,000	2,771	4.70	589	-523
9	1,256	1,256	0.80	1,005	6,234	4.70	1,326	8,000	1,766	4.70	376	-951
10	2,207	2,207	0.80	1,766	8,000							



Load Distribution Chart



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Oversize Hole

William F. McCombs, James C. McQueen, Jeffrey L. Perry

AFFDL-TR-67-184 *Analytical Design Methods for Aircraft Structural Joints* pages 31 to 43

Assume the first fastener hole is "sloppy". Without a 0.003 inch oversize hole ...

$$P_{F1} = \frac{Q}{n/4} \left(\frac{A_d E_d}{A_d E_d + A_s E_s} \right)$$

$$P_{F1} = \frac{Q}{10/4} \left[\frac{0.308 \text{ in}^2 (29 \text{ E06 psi})}{0.308 \text{ in}^2 (29 \text{ E06 psi}) + 0.308 \text{ in}^2 (29 \text{ E06 psi})} \right] = 1,600 \text{ lb}$$

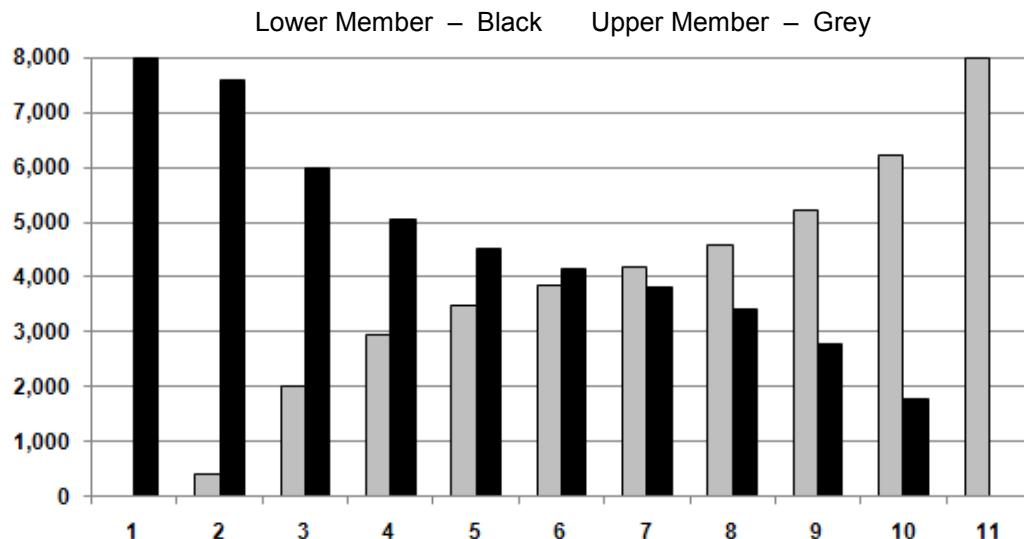
Assume that the first fastener takes half of the 1,600 lb for the first iteration in column six.

Use a trial and error method changing the load in the first fastener (Column 6) until the load in the far end of the upper member (Column 7) is equal to Q, in this case 8,000 pounds.

Table

1	2	3	4	5	6	7	8	9	12	13	14	15	16
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener "Slop" k_F	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Member Load P_D	Spring Constant k_D	Member Strain $\Delta\delta_D$	Applied Load $q_L + (11)$	Load in Base P_S	Spring Constant k_S	Base Strain δ_S	Strain Delta $\delta_S - \delta_D$
n	$(2)_{n-1} - (16)_{n-1}$	$\mu \text{ inch}$	$(2) + (3)$	$x 10^6 \text{ lb/in}$	$(4) * (5)$	$\Sigma (6)$	$x 10^6 \text{ lb/in}$	$(7) / (8)$	$q_L + (11)$	$(12) - (7)$	$x 10^6 \text{ lb/in}$	$(13) / (14)$	$(15) - (9)$
1	3,520	3,000	520	0.80	415.3	415	4.70	88	8,000	7,585	4.70	1,613	1,525
2	1,995	0	1,995	0.80	1,596	2,011	4.70	428	8,000	5,989	4.70	1,274	846
3	1,149	0	1,149	0.80	919	2,930	4.70	623	8,000	5,070	4.70	1,078	455
4	694	0	694	0.80	555	3,485	4.70	741	8,000	4,515	4.70	960	219
5	475	0	475	0.80	380	3,865	4.70	822	8,000	4,135	4.70	880	57
6	418	0	418	0.80	334	4,199	4.70	893	8,000	3,801	4.70	808	-85
7	502	0	502	0.80	402	4,601	4.70	979	8,000	3,399	4.70	723	-256
8	758	0	758	0.80	606	5,208	4.70	1,108	8,000	2,792	4.70	594	-514
9	1,272	0	1,272	0.80	1,017	6,225	4.70	1,324	8,000	1,775	4.70	378	-947
10	2,218	0	2,218	0.80	1,775	8,000							

Load Distribution Chart

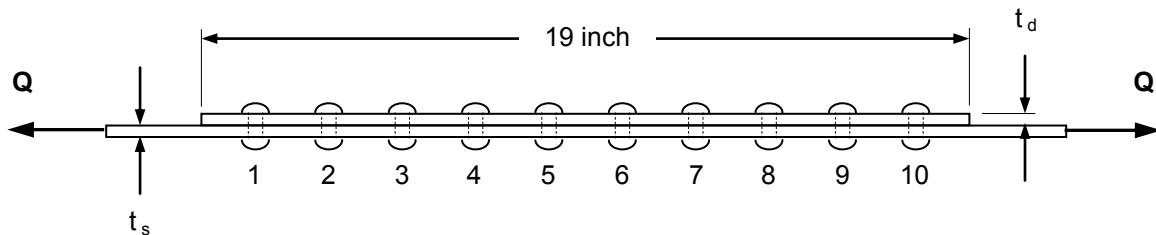


5.0 Doublers

Doubler

William F. McCombs, James C. McQueen, Jeffrey L. Perry

AFFDL-TR-67-184 *Analytical Design Methods for Aircraft Structural Joints* pages 27 and 28



Given

$$Q = 8,000 \text{ lb} \quad t_s = 0.200 \text{ inch} \quad t_d = 0.200 \text{ inch} \quad \text{Pitch, } p = 1.90 \text{ inch}$$

$$\text{Width} = 1.84 \text{ inch} \quad \text{Steel, } E = 29 \text{ ksi} \quad D = 0.375 \text{ inch} \quad k_F = 800,000 \text{ lb/inch}$$

Data

$$\text{Net Effective Area} \quad A_s = (\text{Width} - 0.80 D) t_s \quad \text{Note: } \cos 40^\circ = 0.766$$

$$A_s = [1.84 \text{ inch} - 0.80 (0.375 \text{ inch})] 0.20 \text{ inch} = 0.308 \text{ in}^2$$

$$A_d = [1.84 \text{ inch} - 0.80 (0.375 \text{ inch})] 0.20 \text{ inch} = 0.308 \text{ in}^2$$

$$k = \frac{k_F}{p} = \frac{800,000 \text{ lb/inch}}{1.90 \text{ inch}} = 421,052.6 \frac{\text{lb}}{\text{in}} \text{ per inch}$$

$$N = \frac{k Q}{A_s E} = \frac{421,052.6 \text{ lb/inch per inch} (8,000 \text{ lb})}{0.308 \text{ in}^2 (29 \text{ E06 psi})} = 377.118$$

$$M = k \left(\frac{1}{A_s E_s} + \frac{1}{A_d E_d} \right)$$

$$M = 421,052.6 \text{ lb/inch per inch} \left[\frac{1}{0.308 \text{ in}^2 (29 \text{ E06 psi})} + \frac{1}{0.308 \text{ in}^2 (29 \text{ E06 psi})} \right] = 0.09428$$

$$C_1 = \frac{Q - \frac{N}{M} (1 - e^{-\sqrt{M} L})}{e^{\sqrt{M} L} - e^{-\sqrt{M} L}}$$

$$C_1 = \frac{8,000 \text{ lb} - \frac{377.118}{0.09428} (1 - e^{-19 \sqrt{0.09428}})}{e^{19 \sqrt{0.09428}} - e^{-19 \sqrt{0.09428}}} = \frac{8,000 - 3,988.3}{341.709 - 0.003} = 11.740$$

$$P_n \cong \frac{N}{M} (1 - e^{-\sqrt{M} x})$$

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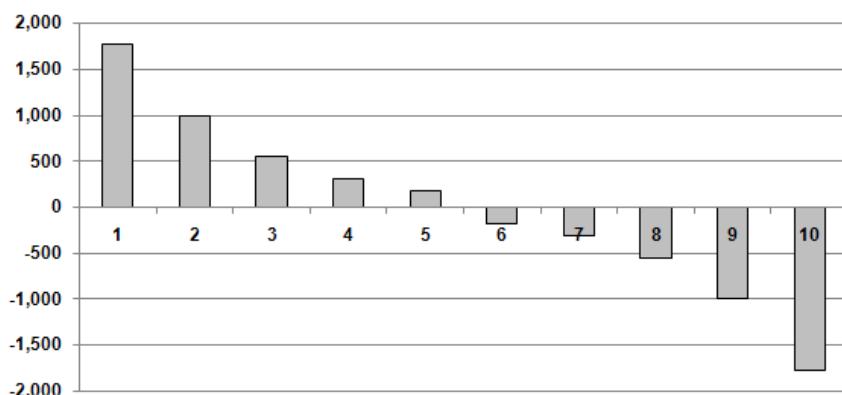
End Fastener Load

$$P_1 \cong \frac{N}{M} \left(1 - e^{-\sqrt{M}x} \right) \cong \frac{377.118}{0.09428} \left(1 - e^{-1.9 \sqrt{0.09428}} \right) = P_{F1} = 1,768 \text{ lb}$$

Table

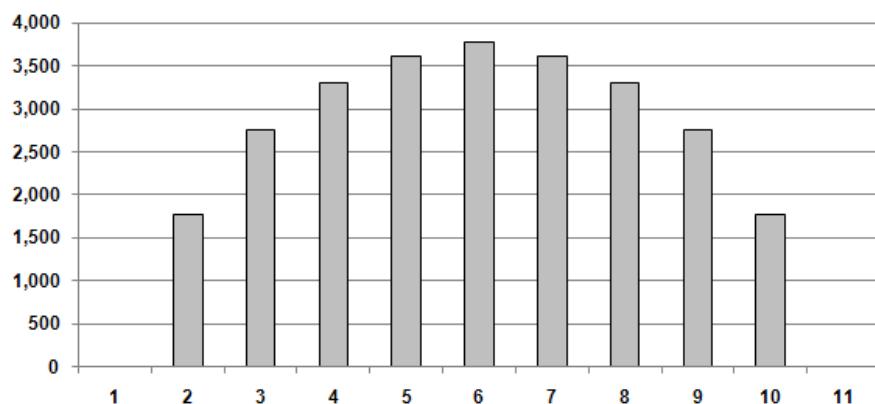
Fastener	x	P _n	P _F	%
n	inch	lb	lb	
1	1.9	1,768.0	1,768	22.1%
2	3.8	2,754.5	987	12.3%
3	5.7	3,305.0	550	6.9%
4	7.6	3,612.2	307	3.8%
5	9.5	3,783.6	171	2.1%
6	11.4	3,783.6	-171	2.1%
7	13.3	3,612.2	-307	3.8%
8	15.2	3,305.0	-550	6.9%
9	17.1	2,754.5	-987	12.3%
10	19.0	1,768.0	-1,768	22.1%

Fastener Loads



Center of Doubler

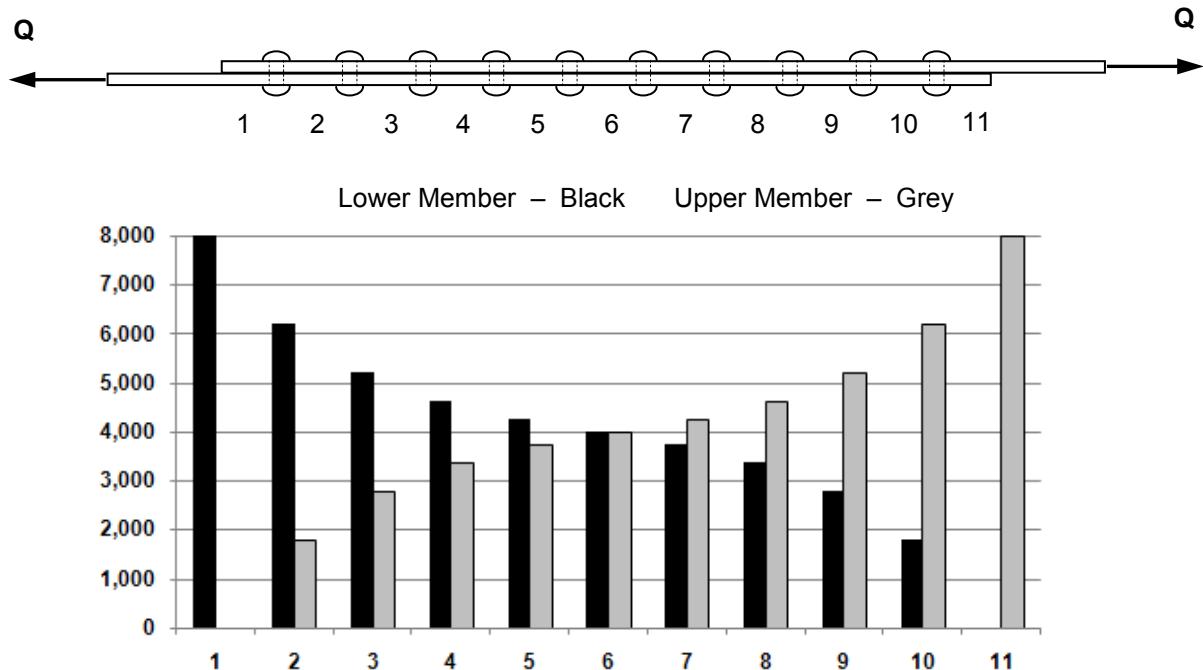
$$P_5 \cong \frac{N}{M} \left(1 - e^{-\sqrt{M}x} \right) \cong \frac{377.118}{0.09428} \left(1 - e^{-9.5 \sqrt{0.09428}} \right) = 3,783.6 \text{ lb}$$



Splice and Doubler Comparison

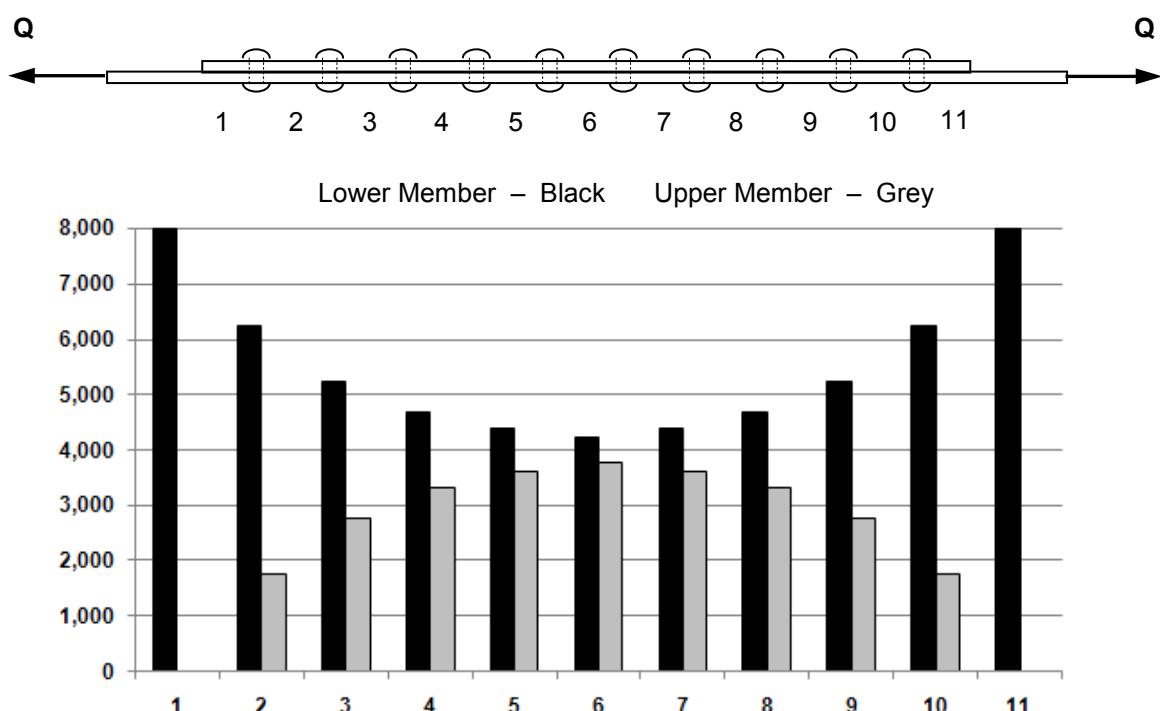
Splice Load Distribution

A splice's function is to transfer a given load. It is kept as short as possible in accomplishing this.
Page 42



Doubler Load Distribution

A doubler's function is to pick up load (and relieve another member). In order to do this efficiently it must have some considerable length, although this is kept to a minimum. Therefore doublers are, by nature, relatively long members compared to splices. Page 42



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Numerical Method I

William F. McCombs, James C. McQueen, Jeffrey L. Perry

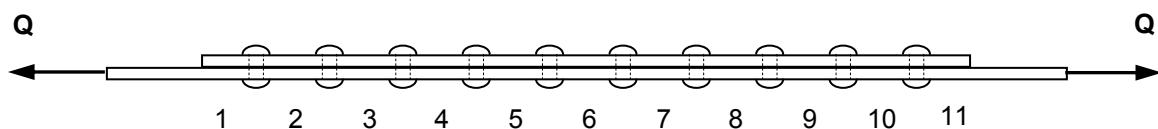
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$$k_d = \frac{A_d E_d}{p} = \frac{0.308 \text{ in}^2 (29 \text{ E06 psi})}{1.90 \text{ inch}} = 470,105 \text{ lb/inch per inch}$$

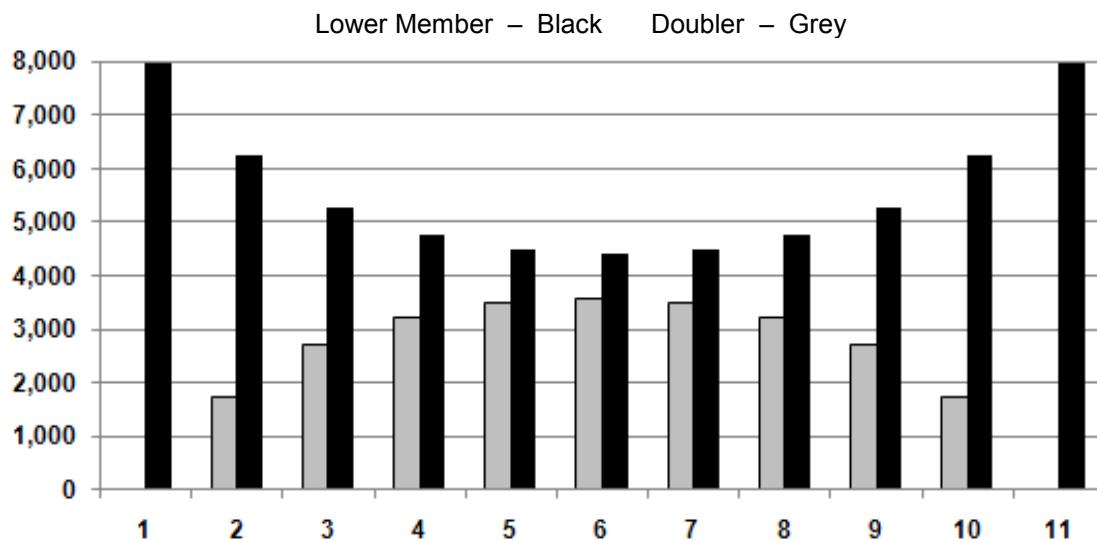
$$k_s = \frac{A_s E_s}{p} = \frac{0.308 \text{ in}^2 (29 \text{ E06 psi})}{1.90 \text{ inch}} = 470,105 \text{ lb/inch per inch}$$

Table

1	2	4	5	6	7	8	9	12	13	14	15	16
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Applied Load $q_L + (11)$	Load in Base P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$
n	$(2)_{n-1} - (16)_{n-1}$	$(2) + (3)$	$\times 10^6 \text{ lb/in}$	$(4) * (5)$	$\Sigma (6)$	$\times 10^6 \text{ lb/in}$	$(7) / (8)$	$q_L + (11)$	$(12) - (7)$	$\times 10^6 \text{ lb/in}$	$(13) / (14)$	$(15) - (9)$
1	2,169	2,169	0.80	1,735	1,735	4.70	369	8,000	6,266	4.70	1,333	964
2	1,205	1,205	0.80	964	2,698	4.70	574	8,000	5,302	4.70	1,128	554
3	651	651	0.80	521	3,219	4.70	685	8,000	4,781	4.70	1,017	332
4	319	319	0.80	255	3,475	4.70	739	8,000	4,525	4.70	963	224
5	95.5	95.5	0.80	76	3,551	4.70	755	8,000	4,449	4.70	946	191
6	-95.5	-95.5	0.80	-76	3,475	4.70	739	8,000	4,525	4.70	963	224
7	-319	-319	0.80	-255	3,219	4.70	685	8,000	4,781	4.70	1,017	332
8	-651	-651	0.80	-521	2,699	4.70	574	8,000	5,301	4.70	1,128	554
9	-1,205	-1,205	0.80	-964	1,735	4.70	369	8,000	6,265	4.70	1,333	964
10	-2,169	-2,169	0.80	-1,735	0	4.70		8,000		4.70		



Load Distribution



Numerical Method II

William F. McCombs, James C. McQueen, Jeffrey L. Perry

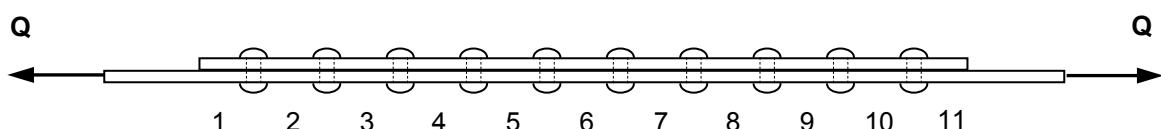
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Using the approximate stiffness equation:

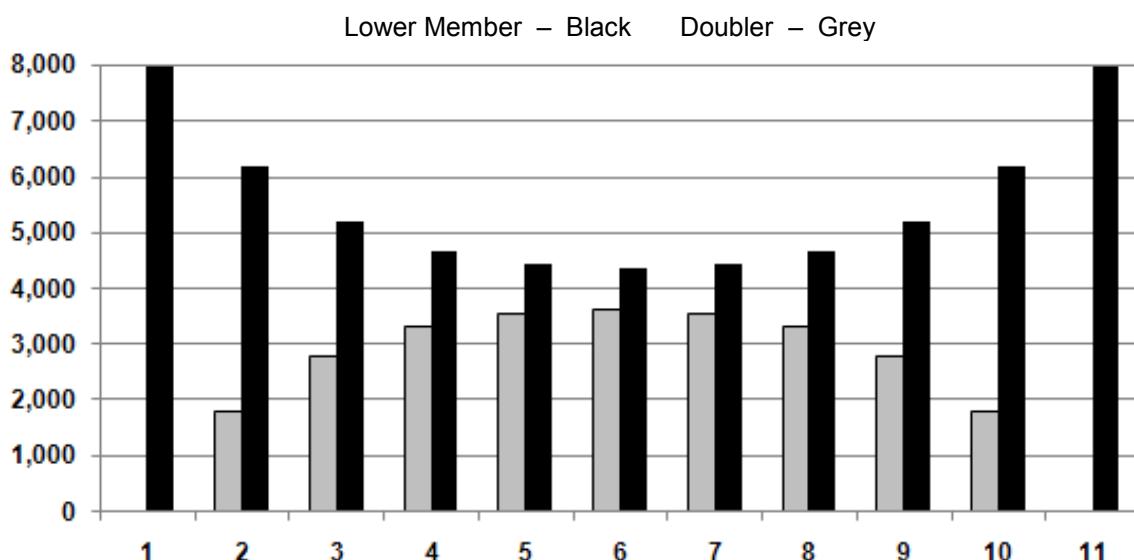
$$k = \frac{k_F}{p} = \frac{800,000 \text{ lb/inch}}{1.90 \text{ inch}} = 421,053 \text{ lb/inch per inch}$$

Table

1	2	4	5	6	7	8	9	12	13	14	15	16
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Applied Load $q_L + (11)$	Load in Base P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$
n	$(2)_{n-1} - (16)_{n-1}$	$(2) + (3)$	$\times 10^6 \text{ lb/in}$	$(4) * (5)$	$\Sigma (6)$	$\times 10^6 \text{ lb/in}$	$(7) / (8)$	$q_L + (11)$	$(12) - (7)$	$\times 10^6 \text{ lb/in}$	$(13) / (14)$	$(15) - (9)$
1	2,260	2,260	0.8	1,809	1,809	4.21	430	8,000	6,191	4.21	1,470	1,041
2	1,219	1,219	0.8	975	2,784	4.21	661	8,000	5,216	4.21	1,239	578
3	642	642	0.8	513	3,297	4.21	783	8,000	4,703	4.21	1,117	334
4	308	308	0.8	246	3,544	4.21	842	8,000	4,456	4.21	1,058	217
5	91.1	91.1	0.8	73	3,616	4.21	859	8,000	4,384	4.21	1,041	182
6	-91.1	-91.1	0.8	-73	3,544	4.21	842	8,000	4,456	4.21	1,058	217
7	-308	-308	0.8	-246	3,297	4.21	783	8,000	4,703	4.21	1,117	334
8	-642	-642	0.8	-513	2,784	4.21	661	8,000	5,216	4.21	1,239	578
9	-1,219	-1,219	0.8	-976	1,808	4.21	429	8,000	6,192	4.21	1,471	1,041
10	-2,260	-2,260	0.8	-1,808	0	4.21		8,000		4.21		



Load Distribution



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Oversize Hole

William F. McCombs, James C. McQueen, Jeffrey L. Perry

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Assume the first fastener hole is "sloppy". Without a 0.003 inch oversize hole ...

$$P_{F1} = \frac{Q}{n/4} \left(\frac{A_d E_d}{A_d E_d + A_s E_s} \right)$$

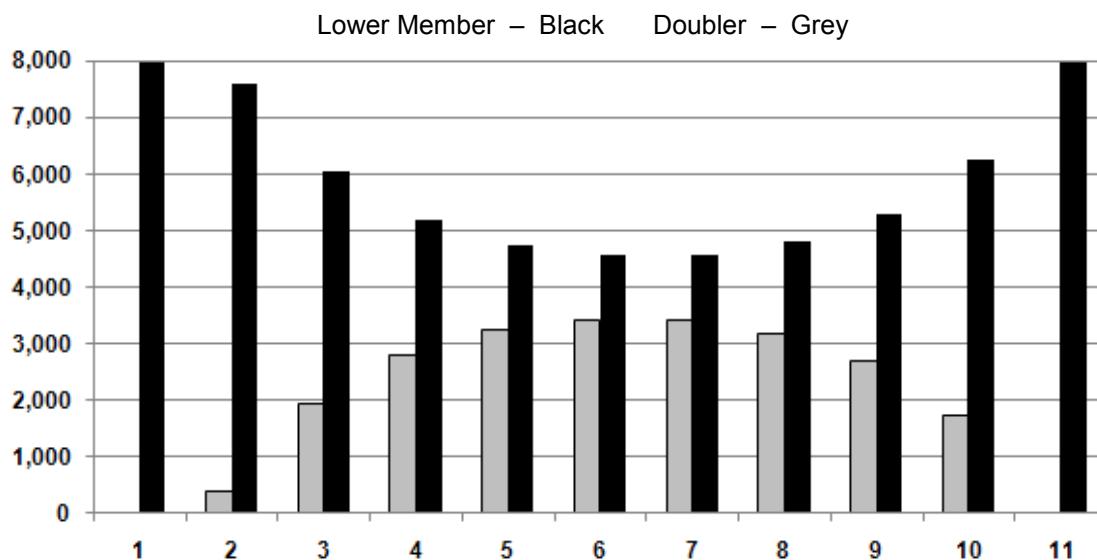
$$P_{F1} = \frac{Q}{10/4} \left[\frac{0.308 \text{ in}^2 (29 \text{ E06 psi})}{0.308 \text{ in}^2 (29 \text{ E06 psi}) + 0.308 \text{ in}^2 (29 \text{ E06 psi})} \right] = 1,600 \text{ lb}$$

Assume that the first fastener takes half of the 1,600 lb for the first iteration. Using a trial and error method change the load in the first fastener (until the load in the far end of the doubler (Column 7) is zero.

Table

1	2	3	4	5	6	7	8	9	12	13	14	15	16
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener "Slop" k_F	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Applied Load $q_L + (11)$	Load in Base P_S	Spring Constant k_S	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$
n	$(2)_{n-1} - (16)_{n-1}$	$\mu \text{ inch}$	$(2) + (3)$	$\times 10^6 \text{ lb/in}$	$(4) * (5)$	$\Sigma (6)$	$\times 10^5 \text{ lb/in}$	$(7) / (8)$	$q_L + (11)$	$(12) - (7)$	$\times 10^6 \text{ lb/in}$	$(13) / (14)$	$(15) - (9)$
1	3,482	3,000	482	0.8	384	384	4.70	82	8,000	7,616	4.70	1,620	1,538
2	1,944	0	1,944	0.8	1,555	1,939	4.70	412	8,000	6,061	4.70	1,289	877
3	1,067	0	1,067	0.8	853	2,792	4.70	594	8,000	5,208	4.70	1,108	514
4	553	0	553	0.8	442	3,234	4.70	688	8,000	4,766	4.70	1,014	326
5	227	0	227	0.8	182	3,416	4.70	727	8,000	4,584	4.70	975	248
6	-21	0	-21	0.8	-17	3,399	4.70	723	8,000	4,601	4.70	979	256
7	-277	0	-277	0.8	-221	3,178	4.70	676	8,000	4,822	4.70	1,026	350
8	-626	0	-626	0.8	-501	2,677	4.70	569	8,000	5,323	4.70	1,132	563
9	-1,189	0	-1,189	0.8	-951	1,725	4.70	367	8,000	6,275	4.70	1,335	968
10	-2,157	0	-2,157	0.8	-1,726	0	4.70		8,000		4.70		

Load Distribution



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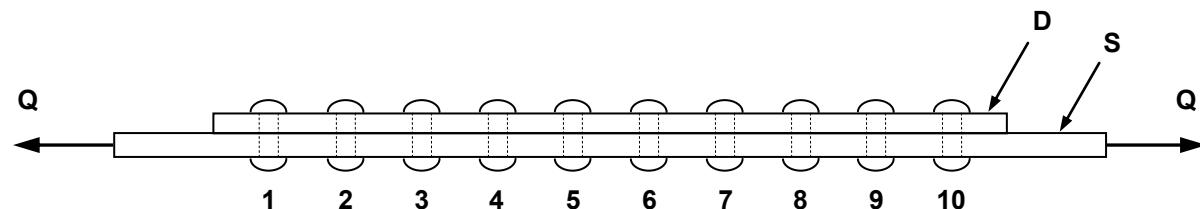
Loads in the Plastic Range

William F. McCombs, James C. McQueen, Jeffrey L. Perry

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Analytical Design Methods for Aircraft Structural Joints

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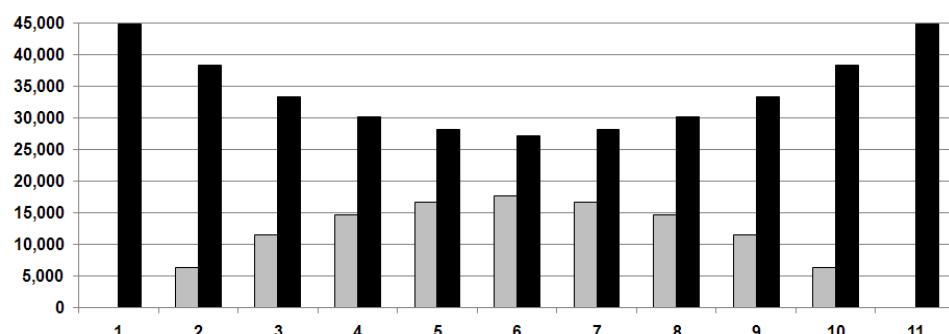
$$Q = 44,800 \text{ lb}$$

$$\text{Pitch} = 1.00 \text{ inch}$$

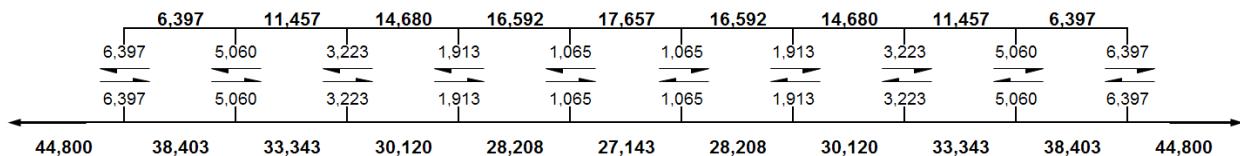
$$K_s = 2,360,000 \text{ lb / inch}$$

$$K_d = 1,920,000 \text{ lb / inch}$$

Load Distribution



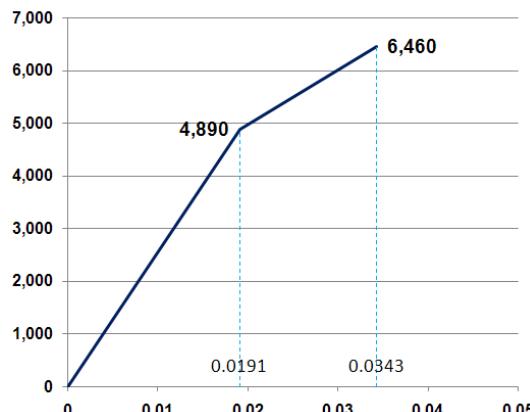
Free Body Diagram



Fastener Load versus Joint Deflection

$$K_{FA} = 4,889.6 \text{ lb / } 0.0191 \text{ inch} = 256,000 \text{ lb / inch}$$

$$K_{FB} = (6459.8 \text{ lb} - 4,889.6 \text{ lb}) / (0.0343 \text{ inch} - 0.0191 \text{ inch}) = 103,300 \text{ lb / inch}$$



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First Iteration

1	2	4	5	6	7	8	9	10	11	12	13	14	15	16
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Intermed Loads $q_a L_n$	Intermed Loads Sum (10)	Applied Load $q_L + (11)$	Load in Base P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$
n	(2) $- (16)_{n-1}$	(2) $+ (3)$	$\times 10^6$ lb/in	(4) * (5)	$\Sigma (6)$	$\times 10^6$ lb/in	(7) / (8)	$q_a L_n$	$\Sigma (10)$	$q_L + (11)$	(12) - (7)	$\times 10^6$ lb/in	(13) / (14)	(15) - (9)
1	30,121	30,121	0.256	7,711	7,711	1.92	4,016	0	0	44,800	37,089	2.36	15,716	11,700
2	18,421	18,421	0.256	4,716	12,427	1.92	6,472	0	0	44,800	32,373	2.36	13,718	7,245
3	11,176	11,176	0.256	2,861	15,288	1.92	7,962	0	0	44,800	29,512	2.36	12,505	4,543
4	6,633	6,633	0.256	1,698	16,986	1.92	8,847	0	0	44,800	27,814	2.36	11,786	2,939
5	3,694	3,694	0.256	946	17,931	1.92	9,339	0	0	44,800	26,869	2.36	11,385	2,046
6	-3,694	-3,694	0.256	-946	17,931	1.92	9,339	0	0	44,800	26,869	2.36	11,385	2,046
7	-6,633	-6,633	0.256	-1,698	16,986	1.92	8,847	0	0	44,800	27,814	2.36	11,786	2,939
8	-11,176	-11,176	0.256	-2,861	15,288	1.92	7,962	0	0	44,800	29,512	2.36	12,505	4,543
9	-18,421	-18,421	0.256	-4,716	12,427	1.92	6,472	0	0	44,800	32,373	2.36	13,718	7,245
10	-30,121	-30,121	0.256	-7,711	7,711	1.92	4,016	0	0	44,800	37,089	2.36	15,716	11,700

Second Iteration

1	2	4	5	6	7	8	9	10	11	12	13	14	15	16
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Intermed Loads $q_a L_n$	Intermed Loads Sum (10)	Applied Load $q_L + (11)$	Load in Base P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$
n	(2) $- (16)_{n-1}$	(2) $+ (3)$	$\times 10^6$ lb/in	(4) * (5)	$\Sigma (6)$	$\times 10^6$ lb/in	(7) / (8)	$q_a L_n$	$\Sigma (10)$	$q_L + (11)$	(12) - (7)	$\times 10^6$ lb/in	(13) / (14)	(15) - (9)
1	14,291	14,291	0.103	1,476	1,476	1.92	769	0	0	16,392	14,915	2.36	6,320	5,551
2	8,740	8,740	0.256	2,237	3,714	1.92	1,934	0	0	16,392	12,678	2.36	5,372	3,438
3	5,302	5,302	0.256	1,357	5,071	1.92	2,641	0	0	16,392	11,320	2.36	4,797	2,155
4	3,147	3,147	0.256	806	5,877	1.92	3,061	0	0	16,392	10,515	2.36	4,455	1,395
5	1,752	1,752	0.256	449	6,325	1.92	3,295	0	0	16,392	10,066	2.36	4,265	971
6	-1,752	-1,752	0.256	-449	6,325	1.92	3,295	0	0	16,392	10,066	2.36	4,265	971
7	-3,147	-3,147	0.256	-806	5,877	1.92	3,061	0	0	16,392	10,515	2.36	4,455	1,395
8	-5,302	-5,302	0.256	-1,357	5,071	1.92	2,641	0	0	16,392	11,320	2.36	4,797	2,155
9	-8,740	-8,740	0.256	-2,237	3,714	1.92	1,934	0	0	16,392	12,678	2.36	5,372	3,438
10	-14,291	-14,291	0.103	-1,476	1,476	1.92	769	0	0	16,392	14,915	2.36	6,320	5,551

Third Iteration

1	2	4	5	6	7	8	9	10	11	12	13	14	15	16
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Intermed Loads $q_a L_n$	Intermed Loads Sum (10)	Applied Load $q_L + (11)$	Load in Base P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$
n	(2) $- (16)_{n-1}$	(2) $+ (3)$	$\times 10^6$ lb/in	(4) * (5)	$\Sigma (6)$	$\times 10^6$ lb/in	(7) / (8)	$q_a L_n$	$\Sigma (10)$	$q_L + (11)$	(12) - (7)	$\times 10^6$ lb/in	(13) / (14)	(15) - (9)
1	2,460	2,460	0.103	254	254	1.92	132	0	0	2,478	2,224	2.36	942	810
2	1,650	1,650	0.103	170	425	1.92	221	0	0	2,478	2,053	2.36	870	649
3	1,001	1,001	0.256	256	681	1.92	355	0	0	2,478	1,797	2.36	761	407
4	594	594	0.256	152	833	1.92	434	0	0	2,478	1,645	2.36	697	263
5	331	331	0.256	85	918	1.92	478	0	0	2,478	1,560	2.36	661	183
6	-331	-331	0.256	-85	918	1.92	478	0	0	2,478	1,560	2.36	661	183
7	-594	-594	0.256	-152	833	1.92	434	0	0	2,478	1,645	2.36	697	263
8	-1,001	-1,001	0.256	-256	681	1.92	355	0	0	2,478	1,797	2.36	761	407
9	-1,650	-1,650	0.103	-170	425	1.92	221	0	0	2,478	2,053	2.36	870	649
10	-2,460	-2,460	0.103	-254	254	1.92	132	0	0	2,478	2,224	2.36	942	810

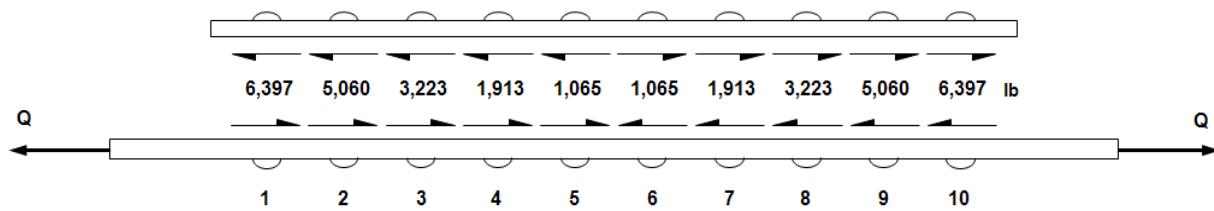
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Internal Loads in the Plastic Range

Table III.3

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17
Load	First Unit Solution	Limiting Load Level	Possible Limiting Ratios	First Loading Increm.	Sum of Loads	Second Unit Solution	Limiting Load Level	Possible Limiting Ratios	Second Loading Increm.	Sum of Loads	Third Unit Solution	Limiting Load Level	Possible Limiting Ratios	Third Loading Increm.	Sum of Loads	Fourth Unit Solution
			r_1	Δ_1	$\Sigma \Delta_1$			r_2	Δ_2	$\Sigma \Delta_2$			r_3	Δ_3	$\Sigma \Delta$	
			(3) / (2)	(4) _{min*} (2)	(5)	(2) - (6)		(8) - (6) (7)	(9) _{min*} (7)	(6) + (10)	(2) - (11)		(13) - (11) (12)	(14) _{min*} (12)	(11) + (13)	(2) - (16)
			k ₁ to k ₅	256,000 lb/in per inch		k ₁ 103,300	k ₂ to k ₅	256,000			k ₁ & k ₂	103,300	k ₃ to k ₅	256,000		
Q _L	44,800	44,800	1.000	28,408	28,408	16,392	44,800	1.000	13,914	42,322	2,478	44,800	1.000	2,478	44,800	0
P _{F1}	7,711	4,889.6	0.634	4,890	4,890	1,476	6,459.8	1.064	1,253	6,143	254	6,459.8	1.248	254	6,397	1,314
P _{F2}	4,716	4,889.6	1.037	2,990	2,990	2,237	4,889.6	0.849	1,899	4,890	170	6,459.8	9.213	170	5,060	-344
P _{F3}	2,861	4,889.6	1.709	1,814	1,814	1,357	4,889.6	2.266	1,152	2,966	256	4,889.6	7,505	256	3,223	-362
P _{F4}	1,698	4,889.6	2.880	1,077	1,077	806	4,889.6	4.733	684	1,761	152	4,889.6	20.575	152	1,913	-215
P _{F5}	946	4,889.6	5.171	600	600	449	4,889.6	9.562	381	980	85	4,889.6	46.159	85	1,065	-120
P _{D5}	16,740			10,615	10,615	5,369			4,558	15,173	918			918	16,090	650
P _{S5}	28,060			17,794	17,794	11,022			9,356	27,150	1,560			1,560	28,710	-650

Fastener Loads



Base and Doubler Loads

Load	P _F	P _D	P _S
			44,800
P _{F1}	6,397	6,397	38,403
P _{F2}	5,060	11,457	33,343
P _{F3}	3,223	14,680	30,120
P _{F4}	1,913	16,592	28,208
P _{F5}	1,065	17,657	27,143
P _{F6}	-1,065	17,657	27,143
P _{F7}	-1,913	16,592	28,208
P _{F8}	-3,223	14,680	30,120
P _{F9}	-5,060	11,457	33,343
P _{F10}	-6,397	6,397	38,403
		0	44,800

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Parabolic WAG Method

If I were to invent my own doubler analysis method for preliminary analysis, I would assume that the load at the center of the base member is equal to the “EA ratio” times the load.

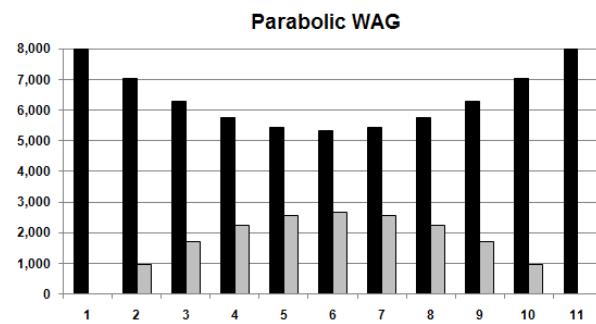
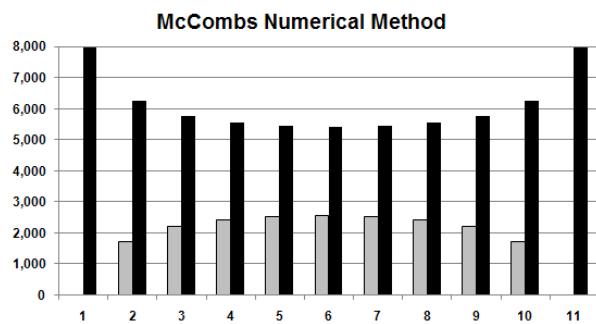
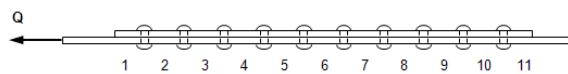
$$\text{Load at Center of Base Member} = Q \left(\frac{E_s A_s}{E_d A_d + E_s A_s} \right)$$

You then have three points of the load distribution curve. Solve for the equation of a parabola. Voilà. Splice analysis could be accomplished in similar fashion.

Stick with William McCombs, Manford Tate and Samuel Rosenfeld.

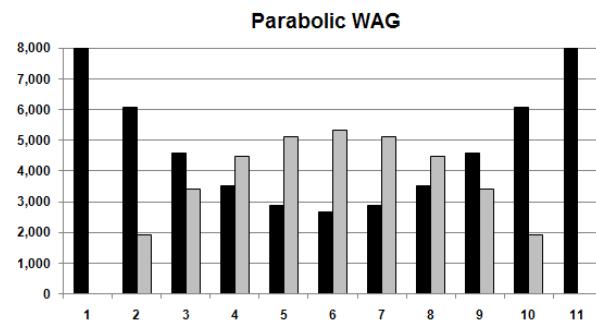
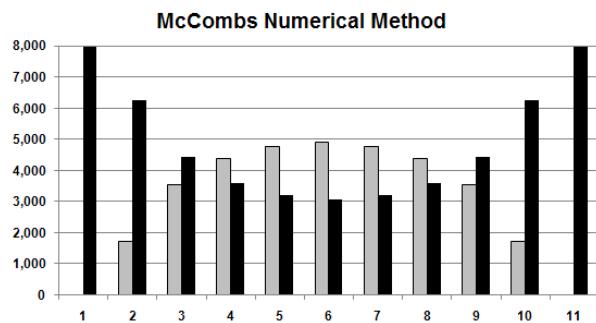
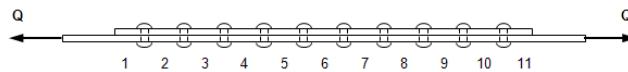
Example 1

$$t_d = 0.100 \text{ inch} \quad t_s = 0.200 \text{ inch}$$



Example 2

$$t_d = 0.200 \text{ inch} \quad t_s = 0.100 \text{ inch}$$



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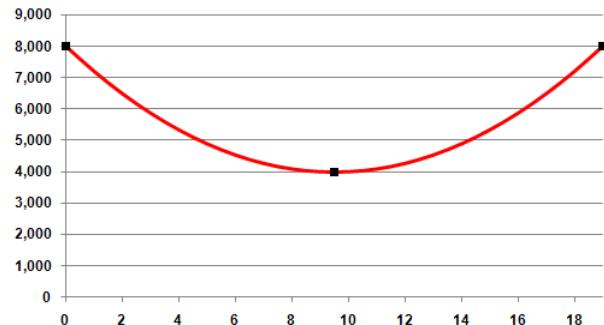
Three Equations, Three Unknowns

DATA

x_1	0	inch	y_1	8,000	lb
x_2	9.50	inch	y_2	4,000	lb
x_3	19.00	inch	y_3	8,000	lb

3 Equations, 3 Unknowns

$$\begin{aligned}y_1 &= a x_1^2 + b x_1 + c & 0 & 8,000 \\y_2 &= a x_2^2 + b x_2 + c & 9.5 & 4,000 \\y_3 &= a x_3^2 + b x_3 + c & 19.0 & 8,000\end{aligned}$$



In Matrix Form

$$\left| \begin{array}{ccc} x_1^2 & x_1 & 1 \\ x_2^2 & x_2 & 1 \\ x_3^2 & x_3 & 1 \end{array} \right| \left| \begin{array}{c} a \\ b \\ c \end{array} \right| = \left| \begin{array}{c} y_1 \\ y_2 \\ y_3 \end{array} \right|$$

Inverse

$$\left| \begin{array}{ccc} 0 & 0 & 1 \\ 90.25 & 9.5 & 1 \\ 361 & 19 & 1 \end{array} \right|^{-1} = \left| \begin{array}{ccc} 0.006 & -0.011 & 0.006 \\ -0.158 & 0.211 & -0.053 \\ 1.000 & 0.000 & 0.000 \end{array} \right|$$

Therefore:

$$\left| \begin{array}{ccc} 0 & 0 & 1 \\ 90.25 & 9.5 & 1 \\ 361 & 19 & 1 \end{array} \right| \left| \begin{array}{c} a \\ b \\ c \end{array} \right| = \left| \begin{array}{c} 8,000 \\ 4,000 \\ 8,000 \end{array} \right|$$

Check

$$\left| \begin{array}{ccc} 0 & 0 & 1 \\ 90.25 & 9.5 & 1 \\ 361 & 19 & 1 \end{array} \right| \left| \begin{array}{c} 0.006 \\ -0.158 \\ 1.000 \end{array} \right| = \left| \begin{array}{ccc} 0.006 & -0.011 & 0.006 \\ -0.158 & 0.211 & -0.053 \\ 1.000 & 0.000 & 0.000 \end{array} \right|$$

Solution

$$\left| \begin{array}{c} a \\ b \\ c \end{array} \right| = \left| \begin{array}{ccc} 0.006 & -0.011 & 0.006 \\ -0.158 & 0.211 & -0.053 \\ 1.000 & 0.000 & 0.000 \end{array} \right| \left| \begin{array}{c} 8,000 \\ 4,000 \\ 8,000 \end{array} \right| = \left| \begin{array}{c} 44.3 \\ -842.1 \\ 8,000 \end{array} \right| \quad \text{Identity Matrix} = \left| \begin{array}{ccc} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{array} \right|$$

$$a = 44.3$$

$$b = -842.1$$

$$c = 8,000$$

$$y = a x^2 + b x + c$$

$$y = 44.32 x^2 + (-842.11) x + 8,000$$

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Multiple Doublers

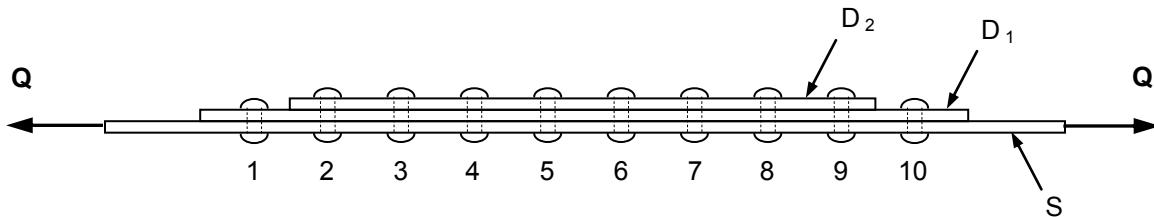
William F. McCombs, James C. McQueen, Jeffrey L. Perry

AFFDL-TR-67-184

Analytical Design Methods for Aircraft Structural Joints

pages 57 thru 63

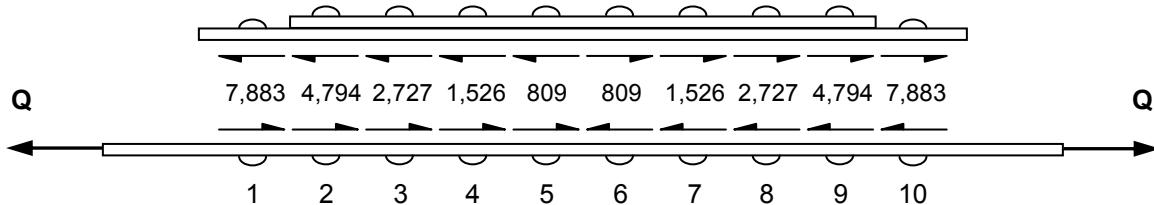
$$Q = 32,000 \text{ lb} \quad \text{Pitch} = 1.00 \text{ inch}$$



$$K_{Fn} = 0.47 \times 10^6 \text{ lb/in per inch} \quad K_{Sn} = K_{D1n} = 2.47 \times 10^6 \quad K_{D2n} = 1.23 \times 10^6$$

See page 59:

- a *Combine the stacked doublers D_1 and D_2 into one member, D , (by adding the k values) as in Figure III.13c. This assumes the fasteners between them to be rigid.*
- b *Determine the corresponding fastener loads between this assumed member, D_1 , and the base structure, S , in the conventional tabular manner. Note the strains, Column 9 of the table.*



- c *Then consider only the two doublers, as they actually exist, to be a structure subjected to the loads of (b) above, applied to the member D_1 , as in Figure III.13d.*
- d *Determine the internal loads for this configuration and loading and also note the strains in the member D_1 , Column 15 of the table. Member D_1 is the "base structure" in this analysis.*
- e *Calculate an effective k_D value for the combined members D_1 and D_2 using the member strains from (b) and (d) above as follows:*

For any segment the effective k_D of the combined members is taken as

$$(k_d)_{eff} = (k_d)_{assumed} \left(\frac{\delta D}{\delta D_1} \right)$$

Repeat steps (b) through (e) until the doubler strains equal the base strains.

A rougher estimate can, of course, be obtained simply by carrying out steps (a) and (b) only one time. This assumes the doublers to be one integral member and therefore results in the fastener loads and the doubler load being larger than they actually are.

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First Iteration

1	2	4	5	6	7	8	9	10	11	12	13	14	15	16
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Intermed Loads $q_a L_n$	Intermed Loads Sum (10)	Applied Load $q_L + (11)$	Load in Base P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$
n	$(2)_{n-1} - (16)_{n-1}$	$(2) + (3)$	$x 10^6$ lb/in	$(4) * (5)$	$\Sigma (6)$	$x 10^6$ lb/in	$(7) / (8)$	$q_a L_n$	$\Sigma (10)$	$q_L + (11)$	$(12) - (7)$	$x 10^6$ lb/in	$(13) / (14)$	$(15) - (9)$
1	16,772	16,772	0.47	7,883	7,883	2.47	3,191	0	0	32,000	24,117	2.47	9,764	6,572
2	10,200	10,200	0.47	4,794	12,677	3.70	3,426	0	0	32,000	19,323	2.47	7,823	4,397
3	5,803	5,803	0.47	2,727	15,404	3.70	4,163	0	0	32,000	16,596	2.47	6,719	2,556
4	3,247	3,247	0.47	1,526	16,930	3.70	4,576	0	0	32,000	15,070	2.47	6,101	1,525
5	1,722	1,722	0.47	809	17,740	3.70	4,795	0	0	32,000	14,260	2.47	5,773	979
6	-1,722	-1,722	0.47	-809	17,740	3.70	4,795	0	0	32,000	14,260	2.47	5,773	979
7	-3,247	-3,247	0.47	-1,526	16,930	3.70	4,576	0	0	32,000	15,070	2.47	6,101	1,525
8	-5,803	-5,803	0.47	-2,727	15,404	3.70	4,163	0	0	32,000	16,596	2.47	6,719	2,556
9	-10,200	-10,200	0.47	-4,794	12,677	3.70	3,426	0	0	32,000	19,323	2.47	7,823	4,397
10	-16,772	-16,772	0.47	-7,883	7,883	2.47	3,191	0	0	32,000	24,117	2.47	9,764	6,572

1	2	4	5	6	7	8	9	10	11	12	13	14	15	16
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Intermed Loads $q_a L_n$	Intermed Loads $\Sigma (10)$	Applied Load $q_L + (11)$	Load in Base P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$
n	$(2)_{n-1} - (16)_{n-1}$	$(2) + (3)$	$x 10^6$ lb/in	$(4) * (5)$	$\Sigma (6)$	$x 10^6$ lb/in	$(7) / (8)$	$q_a L_n$	$\Sigma (10)$	$q_L + (11)$	$(12) - (7)$	$x 10^6$ lb/in	$(13) / (14)$	$(15) - (9)$
1	0	0	0	0	0	0	8,517	7,883	7,883	7,883	7,883	2.47	3,191	-5,325
2	5,325	5,325	0.47	2,503	2,503	1.23	2,036	4,794	12,677	12,677	10,174	2.47	4,119	2,084
3	3,241	3,241	0.47	1,523	4,026	1.23	3,273	2,727	15,404	15,404	11,378	2.47	4,606	1,333
4	1,908	1,908	0.47	897	4,923	1.23	4,002	1,526	16,930	16,930	12,007	2.47	4,861	859
5	1,049	1,049	0.47	493	5,416	1.23	4,403	809	17,740	17,740	12,324	2.47	4,989	586
6	-1,049	-1,049	0.47	-493	5,416	1.23	4,403	-809	17,740	17,740	12,324	2.47	4,989	586
7	-1,908	-1,908	0.47	-897	4,923	1.23	4,002	-1,526	16,930	16,930	12,007	2.47	4,861	859
8	-3,241	-3,241	0.47	-1,523	4,026	1.23	3,273	-2,727	15,404	15,404	11,378	2.47	4,606	1,333
9	-5,325	-5,325	0.47	-2,503	2,503	1.23	2,035	-4,794	12,677	12,677	10,174	2.47	4,119	2,084
10	0	0	0	0	0	0	8,578	-7,883	7,883	7,883	7,883	2.47	3,191	-5,386

Strain and Effective Stiffness

Compare the doubler strains from Column 9 in the first table to base strains in Column 15 of the second table. Use the effective stiffness you calculate in the first iteration for the second calculation and so on.

$$(k_d)_{eff} = (k_d)_{assumed} \left(\frac{\delta D}{\delta D_1} \right)$$

Doubler Strain	Base Strain	Ratio	K	K_{eff}
inch / inch	inch / inch			lb/in per inch
0.00319	0.00319	1.00	2.47	2.470
0.00343	0.00412	0.83	3.70	3.078
0.00416	0.00461	0.90	3.70	3.344
0.00458	0.00486	0.94	3.70	3.483
0.00479	0.00499	0.96	3.70	3.555
0.00479	0.00499	0.96	3.70	3.555
0.00458	0.00486	0.94	3.70	3.483
0.00416	0.00461	0.90	3.70	3.344
0.00343	0.00412	0.83	3.70	3.078
0.00319	0.00319	1.00	2.47	2.470

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Second Iteration

1	2	4	5	6	7	8	9	10	11	12	13	14	15	16
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Intermed Loads $q_a L_n$	Intermed Loads Sum (10)	Applied Load $q_L + (11)$	Load in Base P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$
n	$(2)_{n-1} - (16)_{n-1}$	$(2) + (3)$	$x 10^6$ lb/in	$(4) * (5)$	$\Sigma(6)$	$x 10^6$ lb/in	$(7) / (8)$	$q_a L_n$	$\Sigma(10)$	$q_L + (11)$	$(12) - (7)$	$x 10^6$ lb/in	$(13) / (14)$	$(15) - (9)$
1	16,425	16,425	0.47	7,720	7,720	2,470	3,125	0	0	32,000	24,280	2.47	9,830	6,705
2	9,721	9,721	0.47	4,569	12,289	3,078	3,993	0	0	32,000	19,711	2.47	7,980	3,987
3	5,733	5,733	0.47	2,695	14,983	3,344	4,481	0	0	32,000	17,017	2.47	6,889	2,409
4	3,325	3,325	0.47	1,563	16,546	3,483	4,751	0	0	32,000	15,454	2.47	6,257	1,506
5	1,819	1,819	0.47	855	17,401	3,555	4,894	0	0	32,000	14,599	2.47	5,911	1,017
6	-1,819	-1,819	0.47	-855	17,401	3,555	4,894	0	0	32,000	14,599	2.47	5,911	1,017
7	-3,325	-3,325	0.47	-1,563	16,546	3,483	4,751	0	0	32,000	15,454	2.47	6,257	1,506
8	-5,733	-5,733	0.47	-2,695	14,983	3,344	4,481	0	0	32,000	17,017	2.47	6,889	2,409
9	-9,721	-9,721	0.47	-4,569	12,289	3,078	3,993	0	0	32,000	19,711	2.47	7,980	3,987
10	-16,425	-16,425	0.47	-7,720	7,720	2,470	3,125	0	0	32,000	24,280	2.47	9,830	6,705

1	2	4	5	6	7	8	9	10	11	12	13	14	15	16
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Intermed Loads $q_a L_n$	Intermed Loads $\Sigma(10)$	Applied Load $q_L + (11)$	Load in Base P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$
n	$(2)_{n-1} - (16)_{n-1}$	$(2) + (3)$	$x 10^6$ lb/in	$(4) * (5)$	$\Sigma(6)$	$x 10^6$ lb/in	$(7) / (8)$	$q_a L_n$	$\Sigma(10)$	$q_L + (11)$	$(12) - (7)$	$x 10^6$ lb/in	$(13) / (14)$	$(15) - (9)$
1	4,153	4,153	0	0	0	0	4,153	7,720	7,720	7,720	7,720	2.47	3,125	-1,028
2	5,181	5,181	0.47	2,435	2,435	1.23	1,980	4,569	12,289	12,289	9,853	2.47	3,989	2,009
3	3,172	3,172	0.47	1,491	3,926	1.23	3,192	2,695	14,983	14,983	11,057	2.47	4,477	1,285
4	1,887	1,887	0.47	887	4,813	1.23	3,913	1,563	16,546	16,546	11,733	2.47	4,750	837
5	1,050	1,050	0.47	493	5,306	1.23	4,314	855	17,401	17,401	12,094	2.47	4,896	582
6	-1,050	-1,050	0.47	-493	5,306	1.23	4,314	-855	17,401	17,401	12,094	2.47	4,896	582
7	-1,887	-1,887	0.47	-887	4,813	1.23	3,913	-1,563	16,546	16,546	11,733	2.47	4,750	837
8	-3,172	-3,172	0.47	-1,491	3,926	1.23	3,192	-2,695	14,983	14,983	11,057	2.47	4,477	1,285
9	-5,181	-5,181	0.47	-2,435	2,435	1.23	1,980	-4,569	12,289	12,289	9,853	2.47	3,989	2,009
10	-4,159	-4,159	0	0	0	0	4,147	-7,720	7,720	7,720	7,720	2.47	3,125	-1,022

Strain and Effective Stiffness

$$(k_d)_{eff} = (k_d)_{assumed} \left(\frac{\delta D}{\delta D_1} \right)$$

Doubler Strain	Base Strain	Ratio	K	K _{eff}
inch / inch	inch / inch			lb/in per inch
0.00313	0.00313	1.000	2.470	2.47
0.00399	0.00399	1.001	3.078	3.080
0.00448	0.00448	1.001	3.344	3.347
0.00475	0.00475	1.000	3.483	3.483
0.00489	0.00490	0.999	3.555	3.554
0.00489	0.00490	0.999	3.555	3.554
0.00475	0.00475	1.000	3.483	3.483
0.00448	0.00448	1.001	3.344	3.347
0.00399	0.00399	1.001	3.078	3.080
0.00313	0.00313	1.000	2.470	2.47

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Third Iteration

1	2	4	5	6	7	8	9	10	11	12	13	14	15	16
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Intermed Loads $q_a L_n$	Intermed Loads Sum (10)	Applied Load $q_L + (11)$	Load in Base P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$
n	$(2)_{n=1} - (16)_{n=1}$	$(2) + (3)$	$x 10^6$ lb/in	$(4) * (5)$	$\Sigma (6)$	$x 10^6$ lb/in	$(7) / (8)$	$q_a L_n$	$\Sigma (10)$	$q_L + (11)$	$(12) - (7)$	$x 10^6$ lb/in	$(13) / (14)$	$(15) - (9)$
1	16,427	16,427	0.47	7,721	7,721	2.470	3,126	0	0	32,000	24,279	2.47	9,830	6,704
2	9,723	9,723	0.47	4,570	12,290	3,080	3,990	0	0	32,000	19,710	2.47	7,980	3,990
3	5,733	5,733	0.47	2,695	14,985	3,347	4,477	0	0	32,000	17,015	2.47	6,889	2,412
4	3,321	3,321	0.47	1,561	16,546	3,483	4,750	0	0	32,000	15,454	2.47	6,257	1,506
5	1,815	1,815	0.47	853	17,399	3,554	4,896	0	0	32,000	14,601	2.47	5,911	1,015
6	-1,815	-1,815	0.47	-853	17,399	3,554	4,896	0	0	32,000	14,601	2.47	5,911	1,015
7	-3,321	-3,321	0.47	-1,561	16,546	3,483	4,750	0	0	32,000	15,454	2.47	6,257	1,506
8	-5,733	-5,733	0.47	-2,695	14,985	3,347	4,477	0	0	32,000	17,015	2.47	6,889	2,412
9	-9,723	-9,723	0.47	-4,570	12,290	3,080	3,990	0	0	32,000	19,710	2.47	7,980	3,990
10	-16,427	-16,427	0.47	-7,721	7,721	2.470	3,126	0	0	32,000	24,279	2.47	9,830	6,704

1	2	4	5	6	7	8	9	10	11	12	13	14	15	16
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Intermed Loads $q_a L_n$	Intermed Loads $\Sigma (10)$	Applied Load $q_L + (11)$	Load in Base P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$
n	$(2)_{n=1} - (16)_{n=1}$	$(2) + (3)$	$x 10^6$ lb/in	$(4) * (5)$	$\Sigma (6)$	$x 10^6$ lb/in	$(7) / (8)$	$q_a L_n$	$\Sigma (10)$	$q_L + (11)$	$(12) - (7)$	$x 10^6$ lb/in	$(13) / (14)$	$(15) - (9)$
1	4,154	4,154	0	0	0	0	4,154	7,721	7,721	7,721	7,721	2.47	3,126	-1,028
2	5,182	5,182	0.47	2,435	2,435	1.23	1,980	4,570	12,290	12,290	9,855	2.47	3,990	2,010
3	3,172	3,172	0.47	1,491	3,926	1.23	3,192	2,695	14,985	14,985	11,059	2.47	4,477	1,285
4	1,887	1,887	0.47	887	4,813	1.23	3,913	1,561	16,546	16,546	11,733	2.47	4,750	837
5	1,049	1,049	0.47	493	5,306	1.23	4,314	853	17,399	17,399	12,093	2.47	4,896	582
6	-1,049	-1,049	0.47	-493	5,306	1.23	4,314	-853	17,399	17,399	12,093	2.47	4,896	582
7	-1,887	-1,887	0.47	-887	4,813	1.23	3,913	-1,561	16,546	16,546	11,733	2.47	4,750	837
8	-3,172	-3,172	0.47	-1,491	3,926	1.23	3,192	-2,695	14,985	14,985	11,059	2.47	4,477	1,285
9	-5,182	-5,182	0.47	-2,435	2,435	1.23	1,980	-4,570	12,290	12,290	9,855	2.47	3,990	2,010
10	25,173	25,173	0	0	0	0	33,481	-7,721	7,721	7,721	7,721	2.47	3,126	-30,355

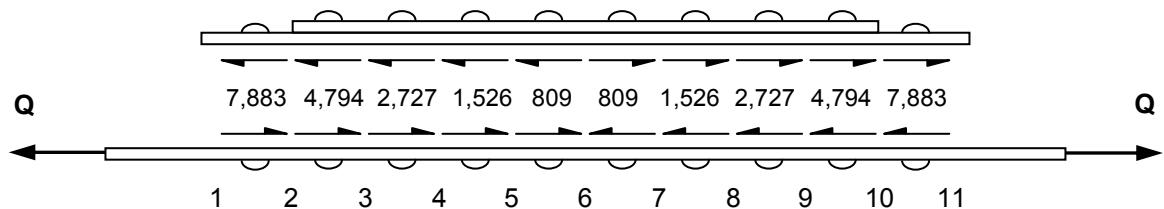
Strain and Effective Stiffness

$$(k_d)_{eff} = (k_d)_{assumed} \left(\frac{\delta D}{\delta D_1} \right)$$

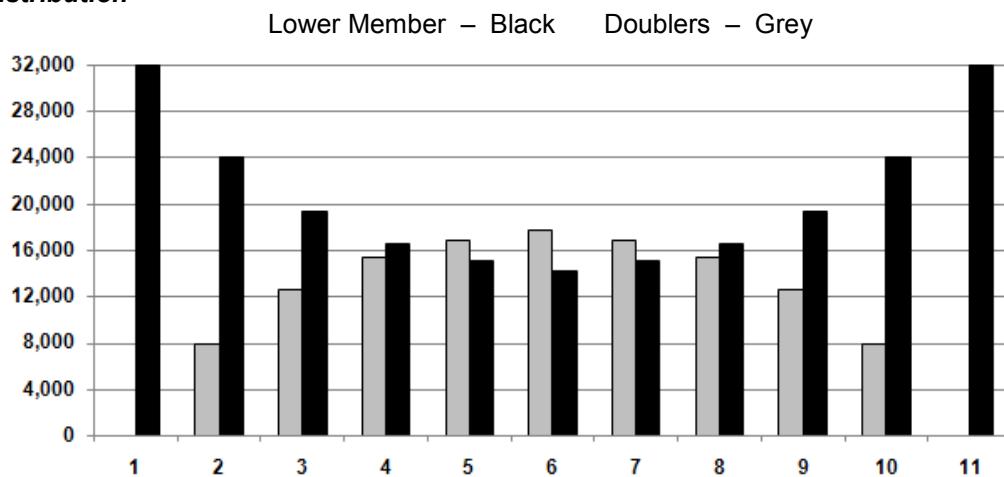
Doubler Strain	Base Strain	Ratio	K	K_{eff}
inch / inch	inch / inch			lb/in per inch
0.00313	0.00313	1.00	2.470	2.470
0.00399	0.00399	1.00	3.080	3.080
0.00448	0.00448	1.00	3.347	3.347
0.00475	0.00475	1.00	3.483	3.483
0.00490	0.00490	1.00	3.554	3.554
0.00490	0.00490	1.00	3.554	3.554
0.00475	0.00475	1.00	3.483	3.483
0.00448	0.00448	1.00	3.347	3.347
0.00399	0.00399	1.00	3.080	3.080
0.00313	0.00313	1.00	2.470	2.470

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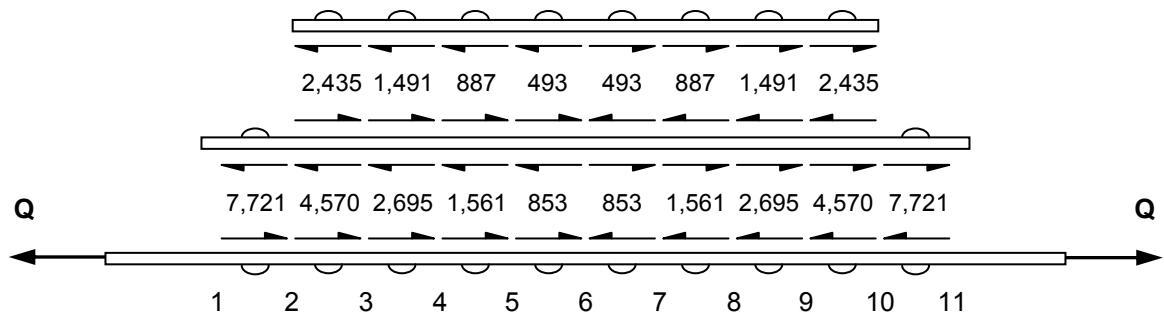
Case I - Free Body Diagram



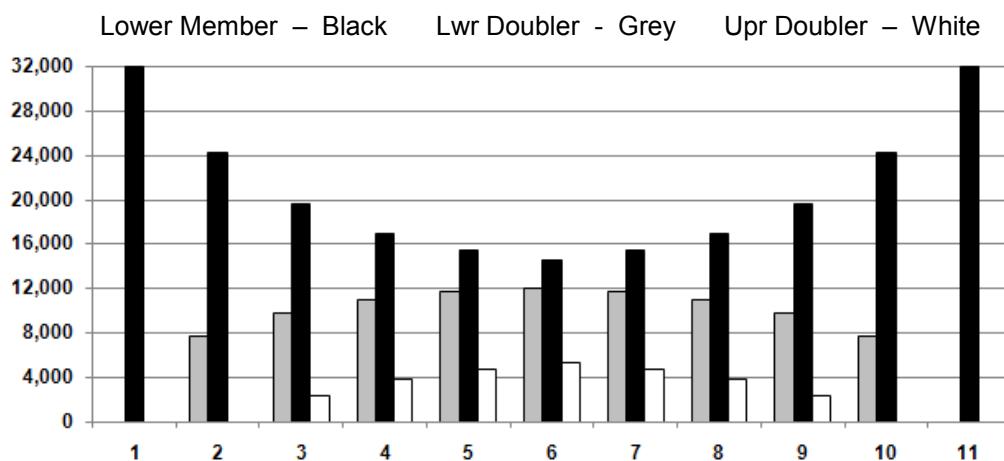
Load Distribution



Case II - Free Body Diagram

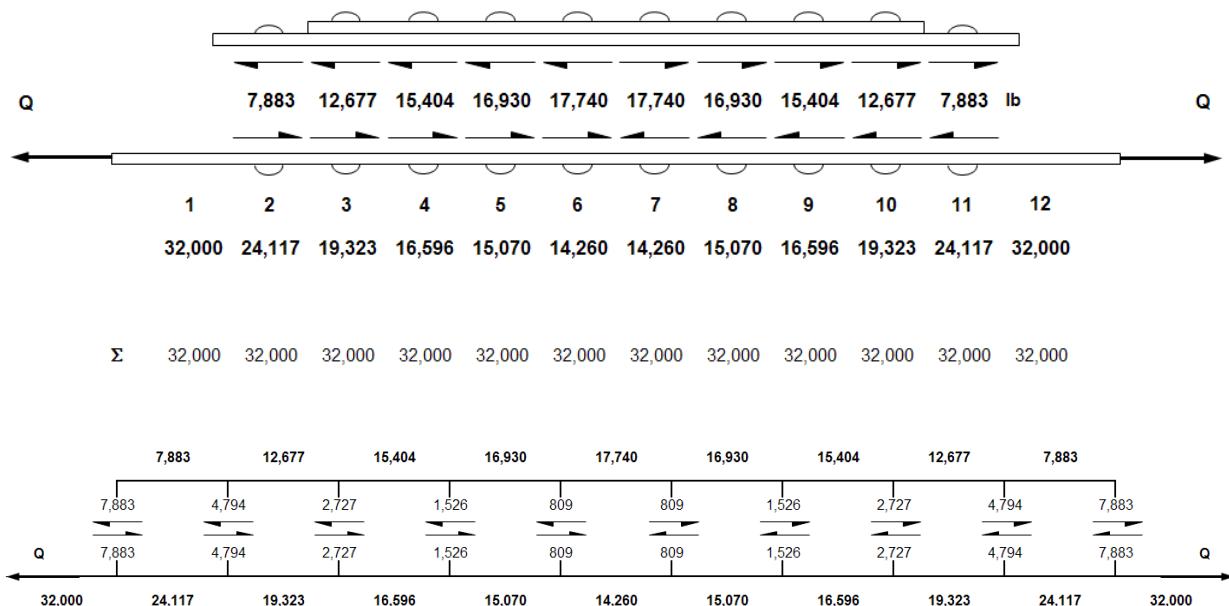


Load Distribution

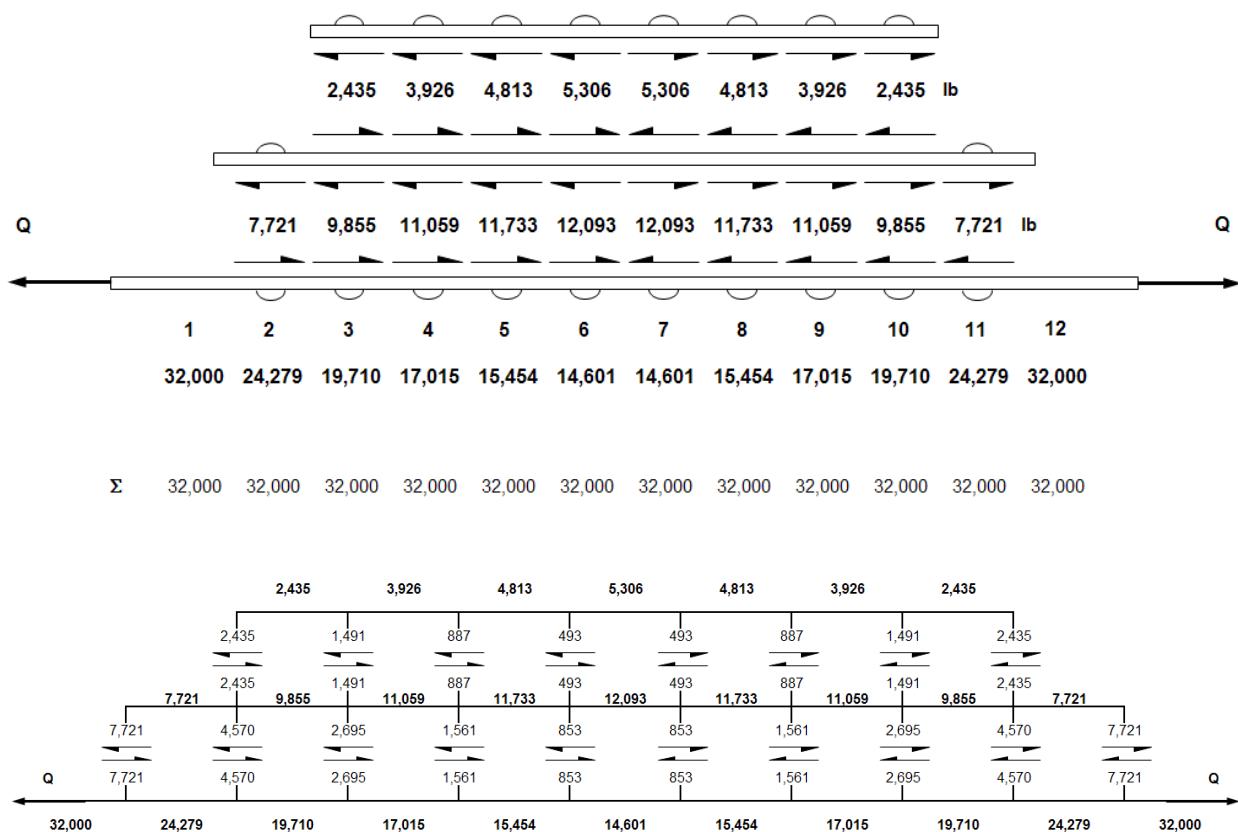


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Case I – Load Distribution



Case II – Load Distribution



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Spreadsheet

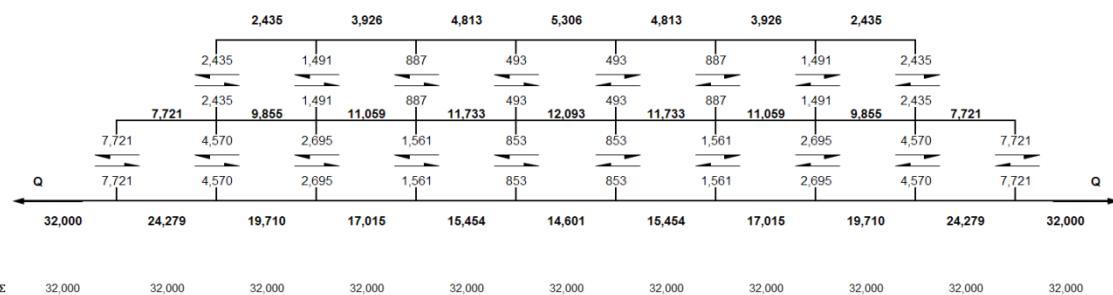
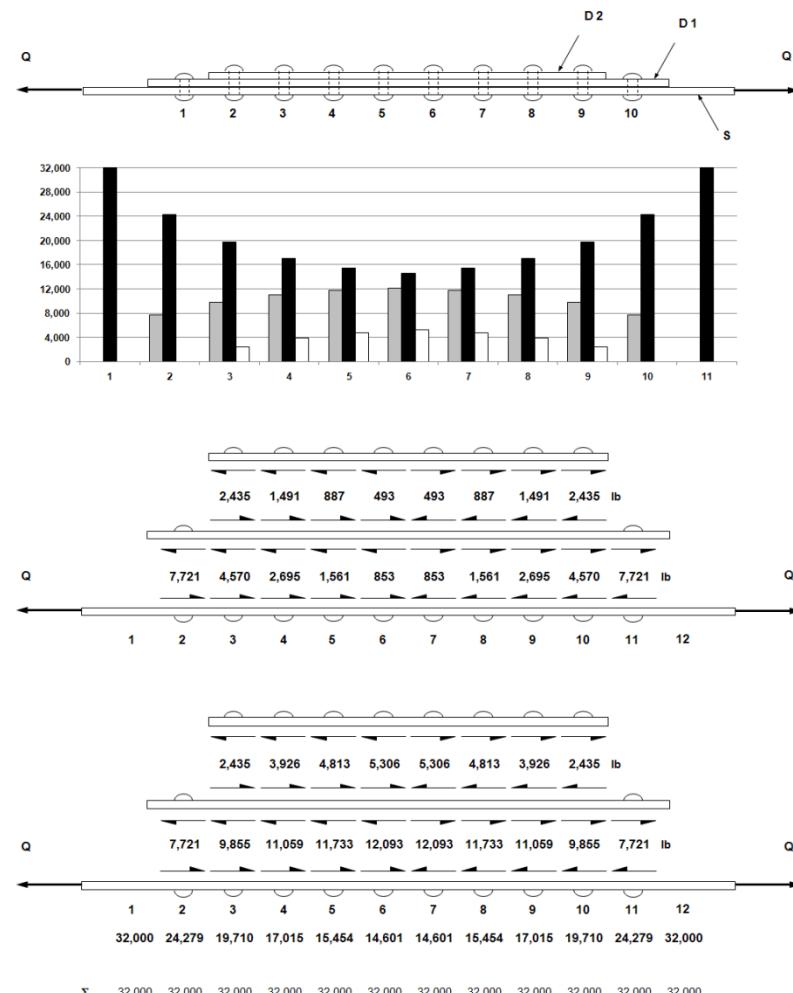
Numerical Method for Multiple Doublers

William F. McCombs

AFFDL-TR AFFDL-TR-67-184 Analytical Design Methods for Aircraft Structural Joints pages 57 thru 63

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n 10 fasteners
D 0.375 inch
Width 1.84 inch
Pitch 1.00 inch
 k_F 800,000 lb / inch
 t_d 0.200 inch
 t_s 0.200 inch
 E_d 2.90E+07 psi
 E_s 2.90E+07 psi
 A_s 0.3080 in²
 A_d 0.3080 in²
 $A_d E_d$ 8.93E+06
 $A_s E_s$ 8.93E+06
 C_1 184.878
k 800,000 lb / inch per inch
 k_s 893,200 lb / inch per inch
L 10.0 inch
M 0.20000
 \sqrt{M} 0.44721
N 3,200.0
 k_d 893,200 lb / inch per inch
Q 32,000 lb
 K_{D1n} 2.47E+06 lb/in per inch
 K_{D2n} 1.23E+06 lb/in per inch
 K_{Dn} 3.70E+06 lb/in per inch
 K_{S1n} 2.47E+06 lb/in per inch
 K_{Fn} 4.70E+05 lb/in per inch



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William F. McCombs, James C. McQueen, Jeffrey L. Perry

AFFDL-TR-67-184 Analytical Design Methods for Aircraft Structural Joints

1	2	4	5	6	7	8	9	10	11	12	13	14	15	16
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Intermed Loads q_{aL_n}	Intermed Loads Sum (10)	Applied Load $q_L + (11)$	Load in Base P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$
n	(2) \approx (16) \approx	(2) \times (3)	$\times 10^6$ lb/in	(4) \times (5)	$\Sigma(6)$	$\times 10^6$ lb/in	(7) / (8)	q_{aL_n}	$\Sigma(10)$	$q_L + (11)$	(12) - (7) $\times 10^6$ lb/in	(13) / (14)	(15) - (9)	
1	16,772	16,772	0.47	7,883	7,883	2.47	3,191	0	0	32,000	24,117	2.47	9,764	6,572
2	10,200	10,200	0.47	4,794	12,677	3.70	3,426	0	0	32,000	19,323	2.47	7,823	4,397
3	5,803	5,803	0.47	2,727	15,404	3.70	4,163	0	0	32,000	16,596	2.47	6,719	2,566
4	3,247	3,247	0.47	1,526	16,930	3.70	4,576	0	0	32,000	15,070	2.47	6,101	1,525
5	1,722	1,722	0.47	809	17,740	3.70	4,795	0	0	32,000	14,260	2.47	5,773	979
6	743	743	0.47	349	18,089	3.70	4,889	0	0	32,000	13,911	2.47	5,632	743
7	0	0	0.47	0	18,089	3.70	4,889	0	0	32,000	13,911	2.47	5,632	743
8	-743	-743	0.47	-349	17,740	3.70	4,795	0	0	32,000	14,260	2.47	5,773	979
9	-1,722	-1,722	0.47	-809	16,930	3.70	4,576	0	0	32,000	15,070	2.47	6,101	1,525
10	-3,247	-3,247	0.47	-1,526	15,404	3.70	4,163	0	0	32,000	16,596	2.47	6,719	2,566
11	-5,803	-5,803	0.47	-2,727	12,677	3.70	3,426	0	0	32,000	19,323	2.47	7,823	4,397
12	-10,200	-10,200	0.47	-4,794	7,883	2.47	3,191	0	0	32,000	24,117	2.47	9,764	6,572
	-16,772	-16,772	0.47	-7,883										

Goal Seek		
Sum Doubler	Lwr Doubler	Load in Base
7,883	7,883	24,117
12,677	10,174	19,323
15,404	11,378	16,596
16,930	12,007	15,070
17,740	12,324	14,260
18,089	12,456	13,911
18,089	12,456	13,911
17,740	12,324	14,260
16,930	12,007	15,070
15,404	11,378	16,596
12,677	10,174	19,323
7,883	7,883	24,117
0	32,000	

0 Goal Seek Set = 0 0 Changing Cell C45

1	2	4	5	6	7	8	9	10	11	12	13	14	15	16
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Intermed Loads q_{aL_n}	Intermed Loads Sum (10)	Applied Load $q_L + (11)$	Load in Base P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$
n	(2) \approx (16) \approx	(2) \times (3)	$\times 10^6$ lb/in	(4) \times (5)	$\Sigma(6)$	$\times 10^6$ lb/in	(7) / (8)	q_{aL_n}	$\Sigma(10)$	$q_L + (11)$	(12) - (7) $\times 10^6$ lb/in	(13) / (14)	(15) - (9)	
1	0	0	0.47	0	0	0	8,517	7,883	7,883	7,883	7,883	2.47	3,191	-5,325
2	5,325	5,325	0.47	2,503	2,503	1.23	2,035	4,794	12,677	10,174	4,119	2,084		
3	3,241	3,241	0.47	1,523	4,026	1.23	3,273	2,727	15,404	15,404	11,378	4,606	1,333	
4	1,908	1,908	0.47	897	4,923	1.23	4,002	1,526	16,930	16,930	12,007	2,47	4,861	859
5	1,049	1,049	0.47	493	5,416	1.23	4,403	809	17,740	17,740	12,324	2,47	4,989	586
6	463	463	0.47	217	5,633	1.23	4,580	349	18,089	18,089	12,456	2,47	5,043	463
7	0	0	0.47	0	5,633	1.23	4,580	0	18,089	18,089	12,456	2,47	5,043	463
8	-463	-463	0.47	-217	5,416	1.23	4,403	-349	17,740	17,740	12,324	2,47	4,989	586
9	-1,049	-1,049	0.47	-493	4,923	1.23	4,002	-809	16,930	16,930	12,007	2,47	4,861	859
10	-1,908	-1,908	0.47	-897	4,026	1.23	3,273	-1,526	15,404	15,404	11,378	2,47	4,606	1,333
11	-3,241	-3,241	0.47	-1,523	2,503	1.23	2,035	-2,727	12,677	12,677	10,174	2,47	4,119	2,084
12	-5,325	-5,325	0.47	-2,503	0	0	8,578	-4,794	7,883	7,883	7,883	2,47	3,191	-5,366
	-16,425	-16,425	0.47	-7,720										

Goal Seek		
Table I Column 9	Table II Column 15	
Doubler Strain	Base Strain	Ratio
0.00319	0.00319	1.00
0.00343	0.00412	0.83
0.00416	0.00461	0.90
0.00458	0.00486	0.94
0.00479	0.00499	0.96
0.00489	0.00504	0.97
0.00479	0.00499	0.96
0.00458	0.00486	0.94
0.00416	0.00461	0.90
0.00343	0.00412	0.83
0.00319	0.00319	1.00

Goal Seek		
Sum Doubler	Lwr Doubler	Load in Base
7,720	7,720	24,280
12,289	9,853	19,711
14,983	11,057	17,017
16,546	11,733	15,454
17,401	12,094	14,599
17,778	12,252	14,222
17,778	12,252	14,222
17,401	12,094	14,599
16,546	11,733	15,454
14,983	11,057	17,017
12,289	9,853	19,711
7,720	7,720	24,280
0	32,000	

0 Goal Seek Set = 0 0 Changing Cell C70

1	2	4	5	6	7	8	9	10	11	12	13	14	15	16
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Intermed Loads q_{aL_n}	Intermed Loads Sum (10)	Applied Load $q_L + (11)$	Load in Base P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$
n	(2) \approx (16) \approx	(2) \times (3)	$\times 10^6$ lb/in	(4) \times (5)	$\Sigma(6)$	$\times 10^6$ lb/in	(7) / (8)	q_{aL_n}	$\Sigma(10)$	$q_L + (11)$	(12) - (7) $\times 10^6$ lb/in	(13) / (14)	(15) - (9)	
1	4,153	4,153	0	0	0	0	4,153	7,720	7,720	7,720	7,720	2.47	3,125	-1,028
2	5,181	5,181	0.47	2,435	2,435	1.23	1,980	4,569	12,289	12,289	9,853	2.47	3,989	2,009
3	3,172	3,172	0.47	1,491	3,926	1.23	3,192	2,695	14,983	14,983	11,057	2.47	4,477	1,285
4	1,887	1,887	0.47	887	4,813	1.23	3,913	1,563	16,546	16,546	11,733	2.47	4,750	837
5	1,050	1,050	0.47	493	5,306	1.23	4,314	855	17,401	17,401	12,094	2.47	4,896	582
6	467	467	0.47	220	5,526	1.23	4,493	377	17,778	17,778	12,252	2.47	4,960	467
7	0	0	0.47	0	5,526	1.23	4,493	0	17,778	17,778	12,252	2.47	4,960	467
8	-467	-467	0.47	-220	5,306	1.23	4,314	-377	17,401	17,401	12,094	2.47	4,896	582
9	-1,050	-1,050	0.47	-493</										

GRAN Corporation

William F. McCombs, James C. McQueen, Jeffrey L. Perry

AFFDL-TR-67-184 Analytical Design Methods for Aircraft Structural Joints

Third Iteration															
1	2	4	5	6	7	8	9	10	11	12	13	14	15	16	
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Intermed Loads q_{aL_n}	Intermed Loads Sum (10)	Applied Load P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$		
n	(2) $\Delta(\delta_s - \delta_d)$	(2) $\Delta\delta_F$	(2) $\times 10^6$ lb/in	(4) $\times 5$	$\Sigma(6)$	$\times 10^6$ lb/in	(7) / (8)	q_{aL_n}	$\Sigma(10)$	$q_L + (11)$	(12) - (7)	$\times 10^6$ lb/in	(13) / (14)	(15) - (9)	
1	16.427	16.427	0.47	7.721	7.721	2.47	3.126	0	0	32,000	24,279	2.47	9.830	6,704	
2	9.723	9.723	0.47	4,570	12,290	3,080	3,990	0	0	32,000	19,710	2.47	7,980	3,990	
3	5.733	5.733	0.47	2,695	14,985	3,347	4,477	0	0	32,000	17,015	2.47	6,889	2,412	
4	3.321	3.321	0.47	1,561	16,546	3,483	4,750	0	0	32,000	15,454	2.47	6,257	1,506	
5	1.815	1.815	0.47	853	17,399	3,554	4,896	0	0	32,000	14,601	2.47	5,911	1,015	
6	800	800	0.47	376	17,775	3,584	4,959	0	0	32,000	14,225	2.47	5,759	800	
7	0	0	0.47	0	17,775	3,584	4,959	0	0	32,000	14,225	2.47	5,759	800	
8	-800	-800	0.47	-376	17,399	3,554	4,896	0	0	32,000	14,601	2.47	5,911	1,015	
9	-1.815	-1.815	0.47	853	16,546	3,483	4,750	0	0	32,000	15,454	2.47	6,257	1,506	
10	-3.321	-3.321	0.47	-1,561	14,985	3,347	4,477	0	0	32,000	17,015	2.47	6,889	2,412	
11	-5.733	-5.733	0.47	-2,695	12,290	3,080	3,990	0	0	32,000	19,710	2.47	7,980	3,990	
12	-9.723	-9.723	0.47	-4,570	7.721	2.47	3.126	0	0	32,000	24,279	2.47	9.830	6,704	
	-16.427	-16.427	0.47	-7.721											

1	2	4	5	6	7	8	9	10	11	12	13	14	15	16		
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Intermed Loads q_{aL_n}	Intermed Loads Sum (10)	Applied Load P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$			
n	(2) $\Delta(\delta_s - \delta_d)$	(2) $\Delta\delta_F$	(2) $\times 10^6$ lb/in	(4) * (5)	$\Sigma(6)$	$\times 10^6$ lb/in	(7) / (8)	q_{aL_n}	$\Sigma(10)$	$q_L + (11)$	(12) - (7)	$\times 10^6$ lb/in	(13) / (14)	(15) - (9)		
1	4,154	4,154	0	0	0	0	4,154	7.721	7,721	7,721	7,721	2.47	3,126	-1,028		
2	5,182	5,182	0.47	2,435	2,435	1.23	1,980	4,570	12,290	12,290	9,855	2.47	3,990	2,010		
3	3,172	3,172	0.47	1,491	3,926	1.23	3,192	2,695	14,985	14,985	11,059	2.47	4,477	1,285		
4	1,887	1,887	0.47	887	4,813	1.23	3,913	1,561	16,546	16,546	11,733	2.47	4,750	837		
5	1,049	1,049	0.47	493	5,306	1.23	4,314	853	17,399	17,399	12,093	2.47	4,896	582		
6	467	467	0.47	220	5,525	1.23	4,492	376	17,775	17,775	12,249	2.47	4,959	467		
7	0	0	0.47	0	5,525	1.23	4,492	0	17,775	17,775	12,249	2.47	4,959	467		
8	-467	-467	0.47	-220	5,306	1.23	4,314	-376	17,399	17,399	12,093	2.47	4,896	582		
9	-1,049	-1,049	0.47	-493	4,813	1.23	3,913	-853	16,546	16,546	11,059	2.47	4,477	1,285		
10	-1,887	-1,887	0.47	-887	3,926	1.23	3,192	-1,561	14,985	14,985	11,059	2.47	4,750	837		
11	-3,172	-3,172	0.47	-1,491	2,435	1.23	4,477	0	0	32,000	9,855	2.47	3,990	2,010		
12	-5,182	-5,182	0.47	-2,435	0	0	4,119	-4,570	7,721	7,721	7,721	2.47	3,126	-993		
	-16,427	-16,427	0.47	-7.721												

1	2	4	5	6	7	8	9	10	11	12	13	14	15	16		
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Intermed Loads q_{aL_n}	Intermed Loads Sum (10)	Applied Load P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$			
n	(2) $\Delta(\delta_s - \delta_d)$	(2) $\Delta\delta_F$	(2) $\times 10^6$ lb/in	(4) * (5)	$\Sigma(6)$	$\times 10^6$ lb/in	(7) / (8)	q_{aL_n}	$\Sigma(10)$	$q_L + (11)$	(12) - (7)	$\times 10^6$ lb/in	(13) / (14)	(15) - (9)		
1	4,154	4,154	0	0	0	0	4,154	7.721	7,721	7,721	7,721	2.47	3,126	-1,028		
2	5,182	5,182	0.47	2,435	2,435	1.23	1,980	4,570	12,290	12,290	9,855	2.47	3,990	2,010		
3	3,172	3,172	0.47	1,491	3,926	1.23	3,192	2,695	14,985	14,985	11,059	2.47	4,477	1,285		
4	1,887	1,887	0.47	887	4,813	1.23	3,913	1,561	16,546	16,546	11,733	2.47	4,750	837		
5	1,049	1,049	0.47	493	5,306	1.23	4,314	853	17,399	17,399	12,093	2.47	4,896	582		
6	467	467	0.47	220	5,525	1.23	4,492	376	17,775	17,775	12,250	2.47	4,959	467		
7	0	0	0.47	0	5,525	1.23	4,492	0	17,775	17,775	12,250	2.47	4,959	467		
8	-467	-467	0.47	-220	5,306	1.23	4,314	-376	17,399	17,399	12,093	2.47	4,896	582		
9	-1,049	-1,049	0.47	-493	4,813	1.23	3,913	-853	16,546	16,546	11,733	2.47	4,750	837		
10	-1,887	-1,887	0.47	-887	3,926	1.23	3,192	-1,561	14,985	14,985	11,059	2.47	4,477	1,285		
11	-3,172	-3,172	0.47	-1,491	2,435	1.23	4,477	0	0	32,000	9,855	2.47	3,990	2,010		
12	-5,182	-5,182	0.47	-2,435	0	0	4,222	-4,570	7,721	7,721	7,721	2.47	3,126	-1,097		
	-16,427	-16,427	0.47	-7.721												

1	2	4	5	6	7	8	9	10	11	12	13	14	15	16		
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Intermed Loads q_{aL_n}	Intermed Loads Sum (10)	Applied Load P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$			
n	(2) $\Delta(\delta_s - \delta_d)$	(2) $\Delta\delta_F$	(2) $\times 10^6$ lb/in	(4) * (5)	$\Sigma(6)$	$\times 10^6$ lb/in	(7) / (8)	q_{aL_n}	$\Sigma(10)$	$q_L + (11)$	(12) - (7)	$\times 10^6$ lb/in	(13) / (14)	(15) - (9)		
1	4,154	4,154	0	0	0	0	4,154	7.721	7,721	7,721	7,721	2.47	3,126	-1,028		
2	5,182	5,182	0.47	2,435	2,435	1.23	1,980	4,570	12,290	12,290	9,855	2.47	3,990	2,010		
3	3,172	3,172	0.47	1,491	3,926	1.23	3,192	2,695	14,985	14,985	11,059	2.47	4,477	1,285		
4	1,887	1,887	0.47	887	4,813	1.23	3,913	1,561	16,546	16,546	11,733	2.47	4,750	837		
5	1,049	1,049	0.47	493	5,306	1.23	4,314	853	17,399	17,399	12,093	2.47	4,896	582		
6	467	467	0.47	220	5,525	1.23	4,492	376	17,775	17,775	12,250	2.47	4,959	467		
7	0	0	0.47	0	5,525	1.23	4,492	0	17,775	17,775	12,250	2.47	4,959	467		
8	-467	-467	0.47	-220	5,306	1.23	4,314	-376	17,399	17,399	12,093	2.47	4,896	582		
9	-1,049	-1,049	0.47	-493	4,813	1.23	3,913	-853	16,546	16,546	11,733	2.47	4,750	837		
10	-1,887	-1,887	0.47	-887	3,926	1.23	3,192	-1,561	14,985	14,985	11,059	2.47	4,477	1,285		
11	-3,172	-3,172	0.47	-1,491	2,435	1.23	4,477	0	0	32,000	9,855	2.47	3,990	2,010		
12	-5,182	-5,182	0.47	-2,435	0	0	4,222	-4,570	7,721	7,721	7,721	2.47	3,126	-1,097		
	-16,427	-16,427	0.47	-7.721												

1	2	4	5	6	7	8	9	10	11	12	13	14	15	16		
Fastener Number	Strain Delta $\Delta(\delta_s - \delta_d)$	Fastener Strain $\Delta\delta_F$	Spring Constant k_F	Fastener Load P_F	Doubler Load P_D	Spring Constant k_D	Doubler Strain $\Delta\delta_D$	Intermed Loads q_{aL_n}	Intermed Loads Sum (10)	Applied Load P_s	Spring Constant k_s	Base Strain δ_s	Strain Delta $\delta_s - \delta_D$			
n	(2) $\Delta(\delta_s - \delta_d)$	(2) $\Delta\delta_F$	(2) $\times 10^6$ lb/in	(4) * (5)	$\Sigma(6)$	$\times 10^6$ lb/in	(7) / (8)	q_{aL_n}	$\Sigma(10)$	$q_L + (11)$	(12) - (7)	$\times 10^6$ lb/in	(13) / (14)	(15) - (9)		
1	4,154	4,154	0	0	0	0	4,154	7.721	7,721	7,721	7,721	2.47	3,126	-1,028		
2	5,182	5,182	0.47	2,435	2,435</td											

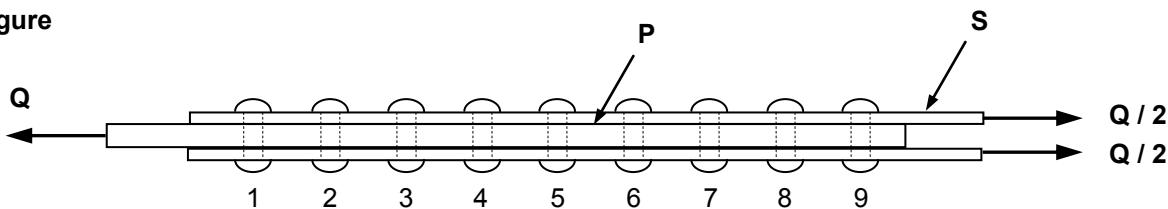
6.0 Butt Joints

Recurrence Formula

Samuel J. Rosenfeld

NACA TN-1458 *Analytical and Experimental Investigation of Bolted Joints* Appendix B, page 21

Figure



24S-T Plates $E = 10,500 \text{ ksi}$ Pitch, $p = 1.25 \text{ inch}$ $t_p = 0.375 \text{ inch}$ $t_s = 0.1875 \text{ inch}$

Alloy Steel Bolts $E_{bb} = 29,000 \text{ ksi}$ $D = 0.250 \text{ inch}$ Width, $b = 3.50 \text{ inch}$

Plate Constant

$$K_s = \frac{p}{b t_s E} = \frac{1.25 \text{ inch}}{3.50 \text{ inch} (0.1875 \text{ inch}) 10,500 \text{ ksi}} = 0.0001814$$

Bolt Constant

General Expression

NACA TN-1051 *Preliminary Investigation of the Loads Carried by Individual Bolts in Bolted Joints*

$$C = \frac{2 t_s + t_p}{3 G_b A_b} + \frac{8 t_s^3 + 16 t_s^2 t_p + 8 t_s t_p^2 + t_p^3}{192 E_{bb} I_b} + \frac{2 t_s + t_p}{t_s t_p E_{b br}} + \frac{1}{t_s E_{s br}} + \frac{2}{t_s E_{p br}}$$

Equal Plate Areas

For 24S-T plates with alloy steel bolts and equal plate areas:

$$t_s = \frac{t_p}{2}$$

$$C = \frac{8}{t_p E_{bb}} \left\{ 0.13 \left(\frac{t_p}{D} \right)^2 \left[2.12 + \left(\frac{t_p}{D} \right)^2 \right] + 1.87 \right\} \quad \text{NACA TN-1051 Equation 4, page 7}$$

$$C = \frac{8}{0.375 (29,000 \text{ ksi})} \left\{ 0.13 \left(\frac{0.375 \text{ inch}}{0.250 \text{ inch}} \right)^2 \left[2.12 + \left(\frac{0.375 \text{ inch}}{0.250 \text{ inch}} \right)^2 \right] + 1.87 \right\} = 0.002316$$

Loads in Successive Bolts – General Relationship

NACA TN-1051 Equation 1, page 6

$$R_{i+1} = \frac{C_i}{C_{i+1}} R_i + \frac{2 K_p + K_s}{C_{i+1}} R_i - \frac{2 K_p}{C_{i+1}} Q + \frac{2 K_p + K_s}{C_{i+1}} \sum_1^{i-1} R$$

For a butt strap thickness half of the main plate thickness

NACA TN-1458 Equation 7, page 7

$$R_{i+1} = R_{i-1} - \left(2 + \frac{2 K_s}{C} \right) R_i + R_{i+1}$$

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Matrix Form

$$\left[\begin{array}{ccccccccc} -1.1567 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 1 & -2.1567 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & -2.1567 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & -2.1567 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & -2.1567 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & -2.1567 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & -2.1567 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 1 & -2.1567 & 1 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 & -1.1567 \end{array} \right] = \left\{ \begin{array}{l} R_0 \\ R_1 \\ R_2 \\ R_3 \\ R_4 \\ R_5 \\ R_6 \\ R_7 \\ R_8 \end{array} \right\} = \left\{ \begin{array}{l} -0.0783 Q \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ -0.0783 Q \end{array} \right\}$$

Invert Matrix and Multiply

$$\left[\begin{array}{ccccccccc} -2.080 & -1.406 & -0.952 & -0.647 & -0.443 & -0.309 & -0.224 & -0.173 & -0.150 \\ -1.406 & -1.626 & -1.101 & -0.748 & -0.513 & -0.358 & -0.259 & -0.200 & -0.173 \\ -0.952 & -1.101 & -1.422 & -0.967 & -0.663 & -0.462 & -0.334 & -0.259 & -0.224 \\ -0.647 & -0.748 & -0.967 & -1.337 & -0.916 & -0.639 & -0.462 & -0.358 & -0.309 \\ -0.443 & -0.513 & -0.663 & -0.916 & -1.313 & -0.916 & -0.663 & -0.513 & -0.443 \\ -0.309 & -0.358 & -0.462 & -0.639 & -0.916 & -1.337 & -0.967 & -0.748 & -0.647 \\ -0.224 & -0.259 & -0.334 & -0.462 & -0.663 & -0.967 & -1.422 & -1.101 & -0.952 \\ -0.173 & -0.200 & -0.259 & -0.358 & -0.513 & -0.748 & -1.101 & -1.626 & -1.406 \\ -0.150 & -0.173 & -0.224 & -0.309 & -0.443 & -0.647 & -0.952 & -1.406 & -2.080 \end{array} \right] = \left\{ \begin{array}{l} -0.0783 Q \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ -0.0783 Q \end{array} \right\} = \left\{ \begin{array}{l} 0.1746 Q \\ 0.1237 Q \\ 0.0921 Q \\ 0.0749 Q \\ 0.0695 Q \\ 0.0749 Q \\ 0.0921 Q \\ 0.1237 Q \\ 0.1746 Q \end{array} \right\}$$

$$R_0 = 0.1746 Q$$

$$R_1 = 0.1237 Q$$

$$R_2 = 0.0921 Q$$

$$R_3 = 0.0749 Q$$

$$R_4 = 0.0695 Q$$

$$R_5 = 0.0749 Q$$

$$R_6 = 0.0921 Q$$

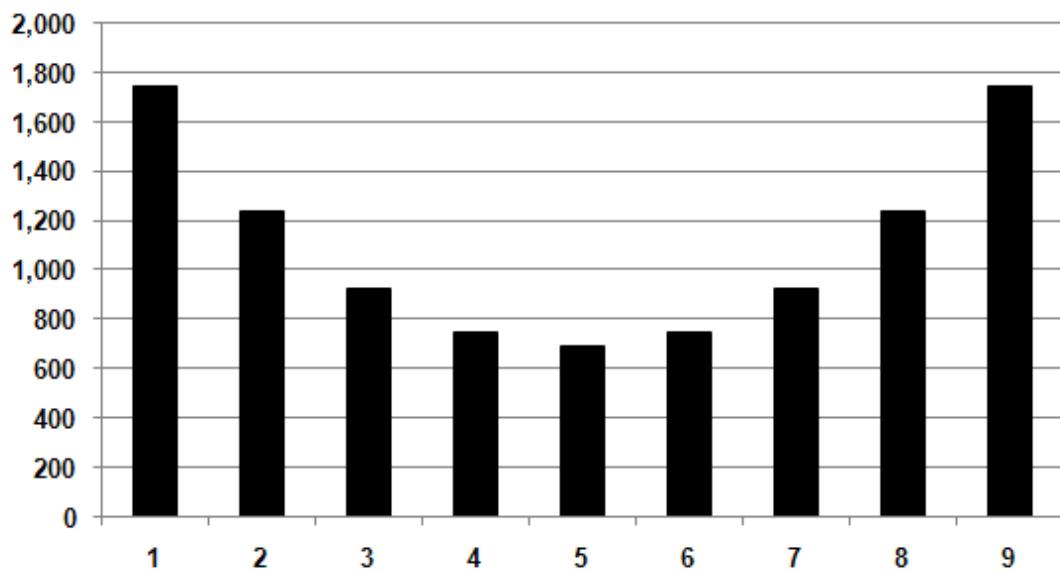
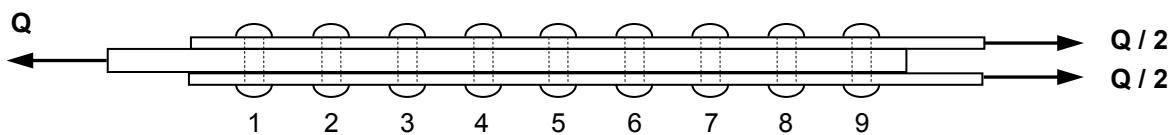
$$R_7 = 0.1237 Q$$

$$R_8 = 0.1746 Q$$

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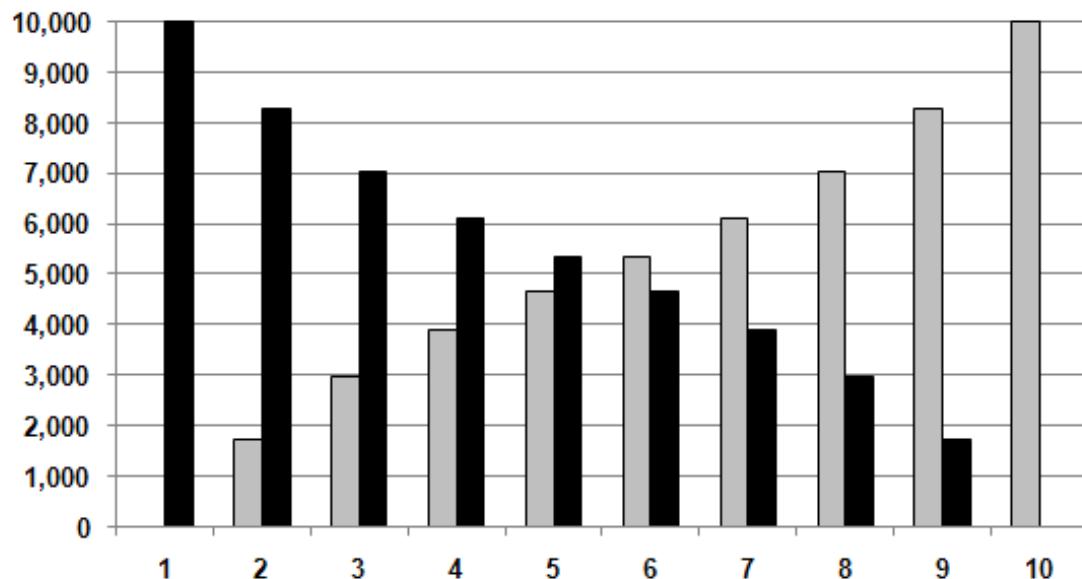
Fastener Loads

For $Q = 10,000$ lb



Load Distribution

For $Q = 10,000$ lb

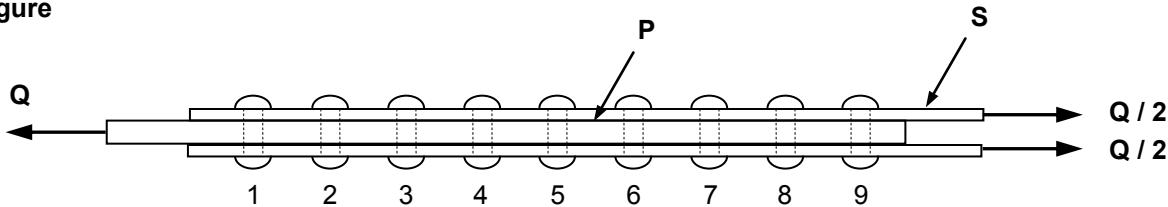


Finite Difference Equation

Samuel J. Rosenfeld

NACA TN-1458 *Analytical and Experimental Investigation of Bolted Joints* Appendix B, page 22

Figure



24S-T Plates $E = 10,500 \text{ ksi}$ Pitch, $p = 1.25 \text{ inch}$ $t_p = 0.375 \text{ inch}$ $t_s = 0.1875 \text{ inch}$
 Alloy Steel Bolts $E_{bb} = 29,000 \text{ ksi}$ $D = 0.250 \text{ inch}$ Width, $b = 3.50 \text{ inch}$

Constants

$$2 K_p = K_s = \frac{p}{b t_s E} = \frac{1.25 \text{ inch}}{3.50 \text{ inch} (0.1875 \text{ inch}) 10,500 \text{ ksi}} = 0.0001814$$

$$C = \frac{8}{t_p E_{bb}} \left\{ 0.13 \left(\frac{t_p}{D} \right)^2 \left[2.12 + \left(\frac{t_p}{D} \right)^2 \right] + 1.87 \right\}$$

$$C = \frac{8}{0.375 (29,000 \text{ ksi})} \left\{ 0.13 \left(\frac{0.375 \text{ inch}}{0.250 \text{ inch}} \right)^2 \left[2.12 + \left(\frac{0.375 \text{ inch}}{0.250 \text{ inch}} \right)^2 \right] + 1.87 \right\} \\ = 0.002316$$

$$\varphi = \frac{2 K_p + K_s}{C} = \frac{0.0001814 + 0.0001814}{0.002316} = 0.15666$$

$$\lambda = \cosh^{-1} \left(1 + \frac{\varphi}{2} \right) = \cosh^{-1} \left(1 + \frac{0.15666}{2} \right) = 0.39326$$

$$\alpha = - \left[\frac{\varphi + (2 K_p / C) (e^{-n\lambda} - 1)}{(e^{n\lambda} - e^{-n\lambda})(e^{-\lambda} - 1)} \right] Q$$

$$\alpha = - \left\{ \frac{0.15666 + (0.0001814 / 0.002316) [e^{-9(0.39326)} - 1]}{[e^{9(0.39326)} - e^{-9(0.39326)}] (e^{-0.39326} - 1)} \right\} Q = 0.007203 Q$$

$$\beta = \left[\frac{\varphi + (2 K_p / C) (e^{n\lambda} - 1)}{(e^{n\lambda} - e^{-n\lambda})(e^{\lambda} - 1)} \right] Q$$

$$\beta = \left\{ \frac{0.15666 + (0.0001814 / 0.002316) [e^{9(0.39326)} - 1]}{[e^{9(0.39326)} - e^{-9(0.39326)}] (e^{0.39326} - 1)} \right\} Q = 0.167435 Q$$

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Bolt Expressions

$$R_0 = \alpha + \beta = 0.007203 Q + 0.167435 Q = 0.1746 Q$$

$$R_1 = \alpha e^{\lambda} + \beta e^{-\lambda} = 0.007203 Q e^{0.39326} + 0.167435 Q e^{-0.39326} = 0.1237 Q$$

$$R_2 = \alpha e^{2\lambda} + \beta e^{-2\lambda} = 0.007203 Q e^{2(0.39326)} + 0.167435 Q e^{-2(0.39326)} = 0.0921 Q$$

$$R_3 = \alpha e^{3\lambda} + \beta e^{-3\lambda} = 0.007203 Q e^{3(0.39326)} + 0.167435 Q e^{-3(0.39326)} = 0.0749 Q$$

$$R_4 = \alpha e^{4\lambda} + \beta e^{-4\lambda} = 0.007203 Q e^{4(0.39326)} + 0.167435 Q e^{-4(0.39326)} = 0.0695 Q$$

$$R_5 = \alpha e^{3\lambda} + \beta e^{-3\lambda} = 0.007203 Q e^{3(0.39326)} + 0.167435 Q e^{-3(0.39326)} = 0.0749 Q$$

$$R_6 = \alpha e^{2\lambda} + \beta e^{-2\lambda} = 0.007203 Q e^{2(0.39326)} + 0.167435 Q e^{-2(0.39326)} = 0.0921 Q$$

$$R_7 = \alpha e^{\lambda} + \beta e^{-\lambda} = 0.007203 Q e^{0.39326} + 0.167435 Q e^{-0.39326} = 0.1237 Q$$

$$R_8 = \alpha + \beta = 0.007203 Q + 0.167435 Q = 0.1746 Q$$

$$R_0 = 0.1746 Q$$

$$R_1 = 0.1237 Q$$

$$R_2 = 0.0921 Q$$

$$R_3 = 0.0749 Q$$

$$R_4 = 0.0695 Q$$

$$R_5 = 0.0749 Q$$

$$R_6 = 0.0921 Q$$

$$R_7 = 0.1237 Q$$

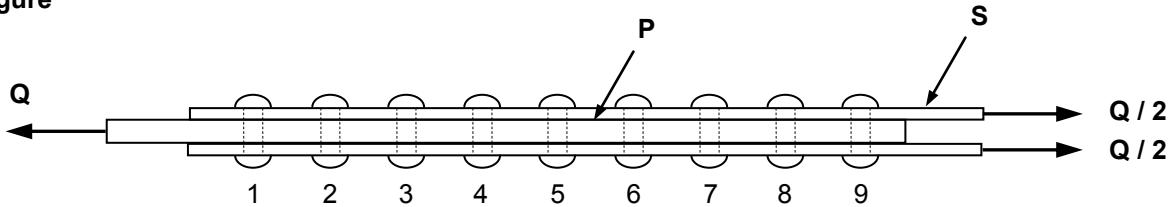
$$R_8 = 0.1746 Q$$

Shear Lag Analogy

Samuel J. Rosenfeld

NACA TN-1458 *Analytical and Experimental Investigation of Bolted Joints* Appendix B, page 24

Figure



24S-T Plates $E = 10,500 \text{ ksi}$ Pitch, $p = 1.25 \text{ inch}$ $t_p = 0.375 \text{ inch}$ $t_s = 0.1875 \text{ inch}$
 Alloy Steel Bolts $E_{bb} = 29,000 \text{ ksi}$ $D = 0.250 \text{ inch}$ Width, $b = 3.50 \text{ inch}$

Bolt Constant

General Expression

NACA TN-1458 Equation 2, page 5

NACA TN-1051 Equation A14, page 30

$$C = \frac{2 t_s + t_p}{3 G_b A_b} + \frac{8 t_s^3 + 16 t_s^2 t_p + 8 t_s t_p^2 + t_p^3}{192 E_{bb} I_b} + \frac{2 t_s + t_p}{t_s t_p E_{bb}} + \frac{1}{t_s E_{sb}} + \frac{2}{t_s E_{pb}}$$

Equal Plate Areas

NACA TN-1458 Equation 8, page 7

NACA TN-1051 Equation 4, page 7

When the joints are made of 24S-T plates with $t_s = t_p / 2$, fastened by alloy steel bolts, the expression for the bolt constant (equation 2) reduces to

$$C = \frac{8}{t_p E_{bb}} \left\{ 0.13 \left(\frac{t_p}{D} \right)^2 \left[2.12 + \left(\frac{t_p}{D} \right)^2 \right] + 1.87 \right\}$$

$$C = \frac{8}{0.375 (29,000 \text{ ksi})} \left\{ 0.13 \left(\frac{0.375 \text{ inch}}{0.250 \text{ inch}} \right)^2 \left[2.12 + \left(\frac{0.375 \text{ inch}}{0.250 \text{ inch}} \right)^2 \right] + 1.87 \right\} = 0.002316$$

Modified Shear Lag Parameter

$$A_s = t_s b$$

$$k^2 = \frac{2}{p E C A_s} = \frac{2}{1.25 \text{ inch} (10,500 \text{ ksi}) 0.002316 (0.1875 \text{ inch}) 3.50 \text{ inch}} = 0.1003$$

$$k = \sqrt{0.1003} = 0.3166$$

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Bolt Expressions

$$R_i = k p \left[\frac{\cosh k (L/2 - x)}{2 \sinh \left(\frac{k L}{2} \right)} \right] Q$$

$$R_i = \left\{ \left[\frac{k p}{2 \sinh \left(\frac{k L}{2} \right)} \right] \cosh k (L/2 - x) \right\} Q$$

$$R_i = \left\{ \left[\frac{0.3166 (1.25 \text{ inch})}{2 \sinh \left[\frac{0.3166 (11.25 \text{ inch})}{2} \right]} \right] \cosh 0.3166 (11.25 \text{ inch}/2 - x) \right\} Q$$

$$R_0 = 0.0686 \cosh \{ 0.3166 [5.625 \text{ inch} - 0.5 (1.25 \text{ inch})] \} Q = 0.1742 Q$$

$$R_1 = 0.0686 \cosh \{ 0.3166 [5.625 \text{ inch} - 1.5 (1.25 \text{ inch})] \} Q = 0.1230 Q$$

$$R_2 = 0.0686 \cosh \{ 0.3166 [5.625 \text{ inch} - 2.5 (1.25 \text{ inch})] \} Q = 0.0913 Q$$

$$R_3 = 0.0686 \cosh \{ 0.3166 [5.625 \text{ inch} - 3.5 (1.25 \text{ inch})] \} Q = 0.0741 Q$$

$$R_4 = 0.0686 \cosh \{ 0.3166 [5.625 \text{ inch} - 4.5 (1.25 \text{ inch})] \} Q = 0.0686 Q$$

$$R_5 = 0.0686 \cosh \{ 0.3166 [5.625 \text{ inch} - 3.5 (1.25 \text{ inch})] \} Q = 0.0741 Q$$

$$R_6 = 0.0686 \cosh \{ 0.3166 [5.625 \text{ inch} - 2.5 (1.25 \text{ inch})] \} Q = 0.0913 Q$$

$$R_7 = 0.0686 \cosh \{ 0.3166 [5.625 \text{ inch} - 1.5 (1.25 \text{ inch})] \} Q = 0.1230 Q$$

$$R_8 = 0.0686 \cosh \{ 0.3166 [5.625 \text{ inch} - 0.5 (1.25 \text{ inch})] \} Q = 0.1742 Q$$

$$R_0 = 0.1742 Q$$

$$R_1 = 0.1230 Q$$

$$R_2 = 0.0913 Q$$

$$R_3 = 0.0741 Q$$

$$R_4 = 0.0686 Q$$

$$R_5 = 0.0741 Q$$

$$R_6 = 0.0913 Q$$

$$R_7 = 0.1230 Q$$

$$R_8 = 0.1742 Q$$

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Comparison

NACA TN-1458 *Analytical and Experimental Investigation of Bolted Joints* Appendix B, page 30

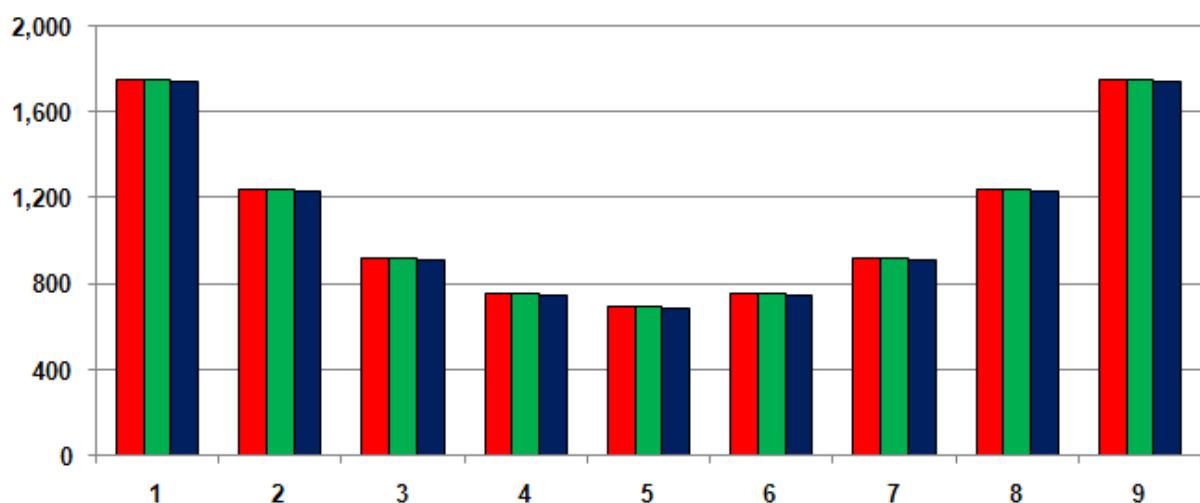
Bolt	Recurrence Formula		Finite Difference		Shear Lag Analogy	
	Fraction	R / R_{avg}	Fraction	R / R_{avg}	Fraction	R / R_{avg}
1	0.1746	1.5717	0.1746	1.5717	0.1742	1.5674
2	0.1237	1.1130	0.1237	1.1130	0.1230	1.1066
3	0.0921	0.8286	0.0921	0.8286	0.0913	0.8214
4	0.0749	0.6741	0.0749	0.6741	0.0741	0.6666
5	0.0695	0.6251	0.0695	0.6251	0.0686	0.6176
6	0.0749	0.6741	0.0749	0.6741	0.0741	0.6666
7	0.0921	0.8286	0.0921	0.8286	0.0913	0.8214
8	0.1237	1.1130	0.1237	1.1130	0.1230	1.1066
9	0.1746	1.5717	0.1746	1.5717	0.1742	1.5674
	<hr/>	<hr/>	<hr/>	<hr/>	<hr/>	<hr/>
	1.0000		1.0000		0.9935	

$$R_{avg} = Q / 9 \text{ fasteners}$$

Fastener Loads

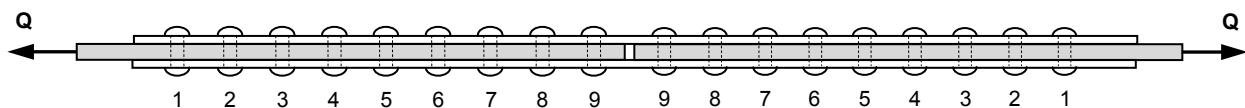
For Q = 10,000 lb

Recurrence Formula - Red **Finite Difference - Green** **Shear Lag Analogy - Blue**



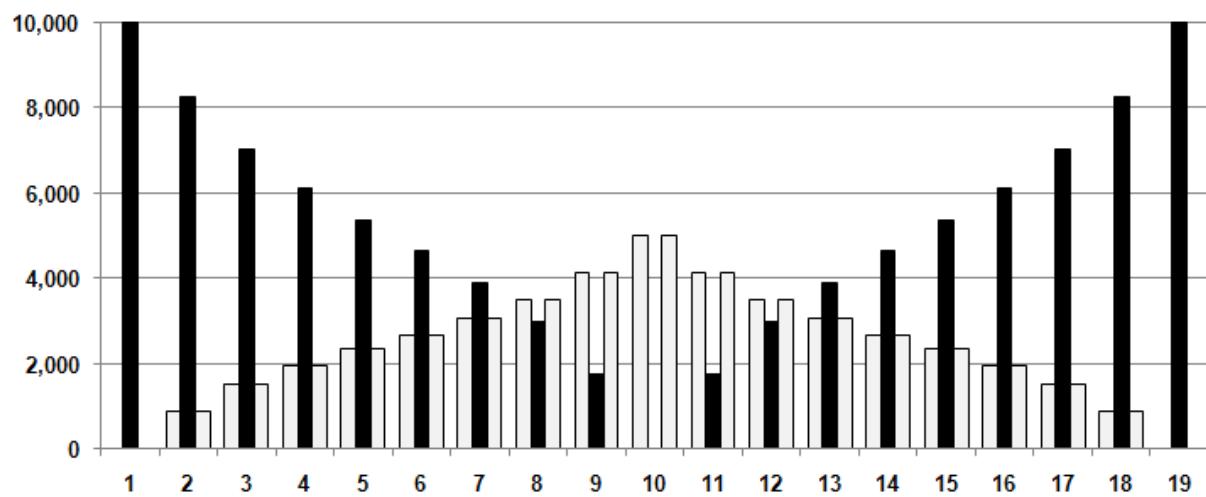
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Butt Joint

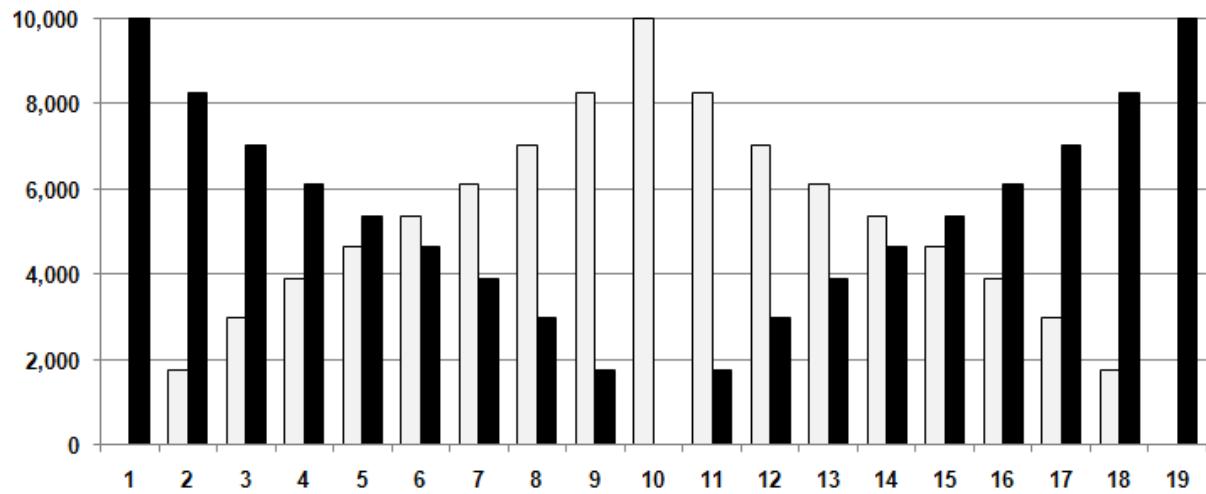


Load Distribution

Three Members



Combining Upper and Lower Straps



Note:

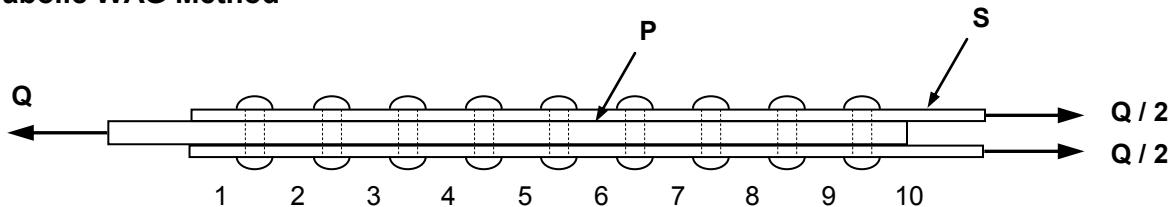
AFFDL-TR-67-184 Analytical Design Methods for Aircraft Structural Joints page 22

If desired, hyperbolic functions can be used to replace some of the exponential forms since

$$e^z - e^{-z} = 2 \sinh z \quad \text{and} \quad e^z + e^{-z} = 2 \cosh z$$

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Parabolic WAG Method



24S-T Plates $E = 10,500 \text{ ksi}$ Pitch, $p = 1.25 \text{ inch}$ $t_p = 0.375 \text{ inch}$ $t_s = 0.1875 \text{ inch}$
 Alloy Steel Bolts $E_{bb} = 29,000 \text{ ksi}$ $D = 0.250 \text{ inch}$ Width, $b = 3.50 \text{ inch}$

Average Bolt Load

$$P_{avg} = 10,000 \text{ lb} / 9 \text{ bolts} = 1,111.1 \text{ lb}$$

Assume Bolt Loads

Assume a parabolic bolt load distribution where:

$$\text{End Bolts} \quad P_1 = P_9 = P_{avg} (1 + n\%) = 1,111.1 \text{ lb} (1 + n\%)$$

$$\text{Center Bolt} \quad P_5 = P_{avg} (n\%) = 1,111.1 \text{ lb} (n\%)$$

$$\left(\frac{E_s A_s}{E_p A_p + E_s A_s} \right) = \left[\frac{10,500 \text{ ksi} (3.50 \text{ inch}) 0.1875 \text{ inch} (2)}{10,500 \text{ ksi} (3.50 \text{ inch}) 0.375 \text{ inch} + 10,500 \text{ ksi} (3.50 \text{ inch}) 0.1875 \text{ inch} (2)} \right]$$

$$\left(\frac{E_s A_s}{E_p A_p + E_s A_s} \right) = 0.50$$

Goal Seek

Solve for n by *Trial and Error*. You might use the "EA Ratio" of 50% for the first guess.

		x (inch)	P_F (lb)	P_S (lb)	P_P (lb)
P_{avg}	1,111.1 lb	0		0	10,000
		1.25	R_0	1,721	1,721
$E_s A_s$	6.50E+06	2.5	R_1	1,282	3,003
		3.75	R_2	953	3,955
$E_p A_p$	1.30E+07	5	R_3	733	4,688
		6.25	R_4	623	5,312
n	Changing	7.5	R_5	733	6,045
	54.9%	8.75	R_6	953	3,955
		10	R_7	1,282	8,279
		11.25	R_8	1,721	1,721
				10,000	0
					Goal Seek

$$\text{End Bolts} \quad P_1 = P_9 = P_{avg} (1 + n\%) = 1,111.1 \text{ lb} (1 + 0.549) = 1,721 \text{ lb}$$

$$\text{Center Bolt} \quad P_5 = P_{avg} (n\%) = 1,111.1 \text{ lb} (0.549) = 610 \text{ lb}$$

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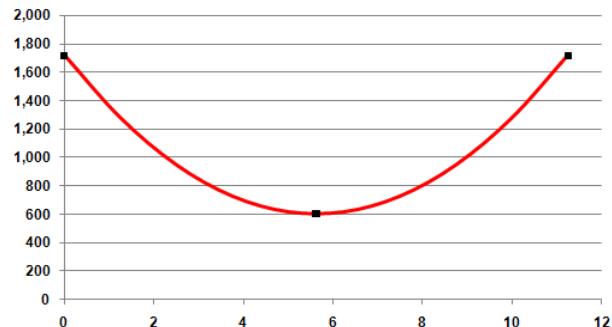
Parabolic Equation

DATA

x_1	0	inch	y_1	1,721	lb
x_2	5.625	inch	y_2	610	lb
x_3	11.250	inch	y_3	1,721	lb

3 Equations, 3 Unknowns

$$\begin{aligned}y_1 &= a x_1^2 + b x_1 + c & 0.000 & 1,721 \\y_2 &= a x_2^2 + b x_2 + c & 5.625 & 610 \\y_3 &= a x_3^2 + b x_3 + c & 11.250 & 1,721\end{aligned}$$



In Matrix Form

$$\left| \begin{array}{ccc} x_1^2 & x_1 & 1 \\ x_2^2 & x_2 & 1 \\ x_3^2 & x_3 & 1 \end{array} \right| \left| \begin{array}{c} a \\ b \\ c \end{array} \right| = \left| \begin{array}{c} y_1 \\ y_2 \\ y_3 \end{array} \right|$$

Inverse

$$\left| \begin{array}{ccc} 0 & 0 & 1 \\ 31.64063 & 5.625 & 1 \\ 126.5625 & 11.25 & 1 \end{array} \right|^{-1} = \left| \begin{array}{ccc} 0.016 & -0.032 & 0.016 \\ -0.267 & 0.356 & -0.089 \\ 1.000 & 0.000 & 0.000 \end{array} \right|$$

Therefore:

$$\left| \begin{array}{ccc} 0 & 0 & 1 \\ 31.6 & 5.6 & 1 \\ 126.6 & 11.3 & 1 \end{array} \right| \left| \begin{array}{c} a \\ b \\ c \end{array} \right| = \left| \begin{array}{c} 1,721 \\ 610 \\ 1,721 \end{array} \right|$$

Check

$$\left| \begin{array}{ccc} 0 & 0 & 1 \\ 31.64063 & 5.625 & 1 \\ 126.5625 & 11.25 & 1 \end{array} \right| \left| \begin{array}{c} 0.016 & -0.032 & 0.016 \\ -0.267 & 0.356 & -0.089 \\ 1.000 & 0.000 & 0.000 \end{array} \right| = \left| \begin{array}{ccc} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{array} \right|$$

Solution

$$\left| \begin{array}{c} a \\ b \\ c \end{array} \right| = \left| \begin{array}{ccc} 0.016 & -0.032 & 0.016 \\ -0.267 & 0.356 & -0.089 \\ 1.000 & 0.000 & 0.000 \end{array} \right| \left| \begin{array}{c} 1,721 \\ 610 \\ 1,721 \end{array} \right| = \left| \begin{array}{c} 35.1 \\ -395.1 \\ 1,721 \end{array} \right| \quad \text{Identity Matrix} = \left| \begin{array}{ccc} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{array} \right|$$

$$a = 35.1$$

$$b = -395.1$$

$$c = 1,721$$

$$y = a x^2 + b x + c$$

$$y = 35.12 x^2 + (-395.06) x + 1,721$$

Solution

Recurrence Formula

x (inch)		P_F (lb)	P_S (lb)	P_P (lb)
0		0	0	10,000
1.25	R_0	1,746	1,746	8,254
2.50	R_1	1,237	2,983	7,017
3.75	R_2	921	3,904	6,096
5.00	R_3	749	4,653	5,347
6.25	R_4	695	5,347	4,653
7.50	R_5	749	6,096	3,904
8.75	R_6	921	7,017	2,983
10.00	R_7	1,237	8,254	1,746
11.25	R_8	1,746	10,000	0
		0		

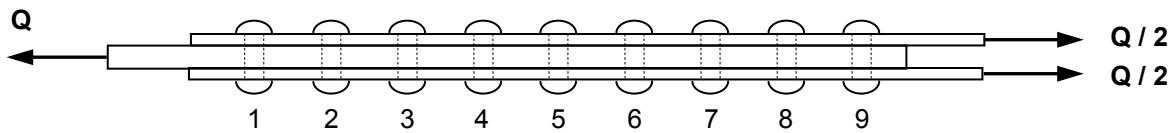
Parabolic WAG Method

P_F (lb)	P_S (lb)	P_P (lb)
0	0	10,000
1,721	1,721	8,279
1,282	3,003	6,997
953	3,955	6,045
733	4,688	5,312
623	5,312	4,688
733	6,045	3,955
953	6,997	3,003
1,282	8,279	1,721
1,721	10,000	0
0		

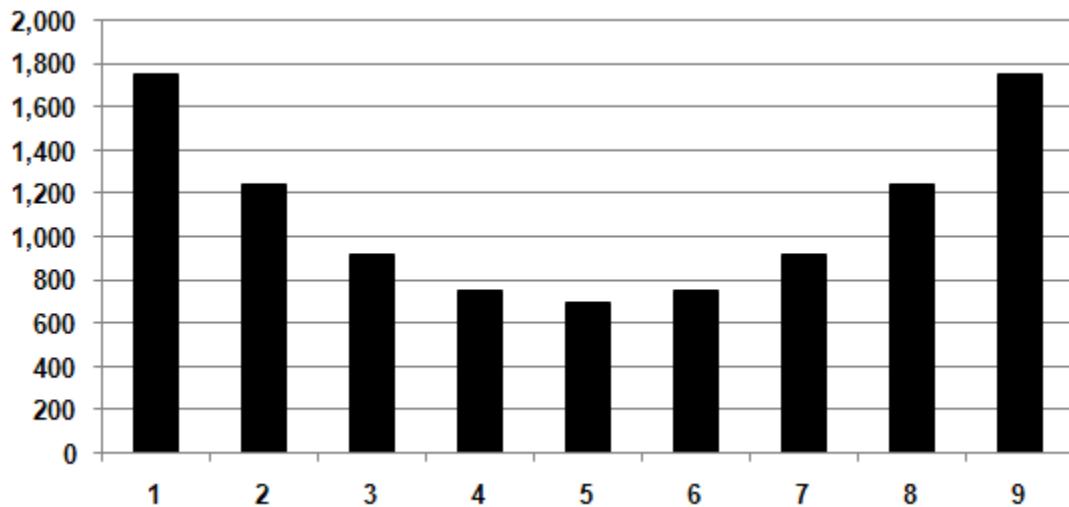
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Fastener Loads

Recurrence Formula

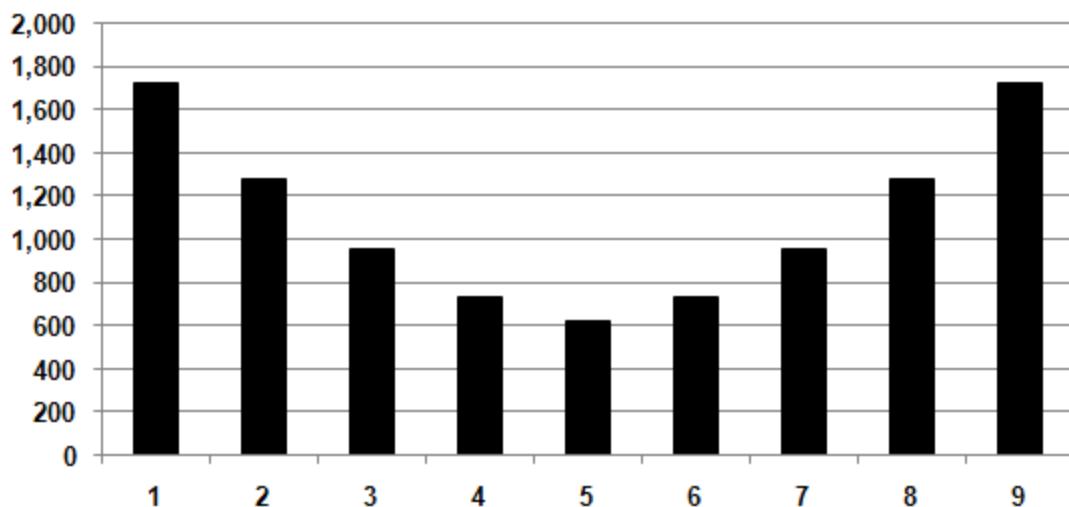


For $Q = 10,000$ lb



Parabolic WAG Method

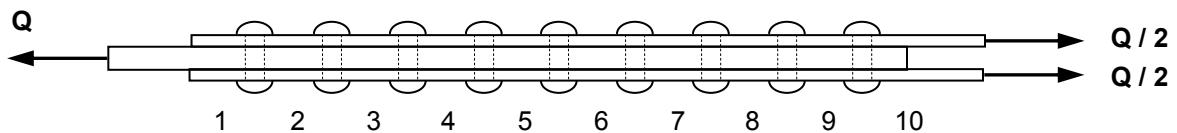
For $Q = 10,000$ lb



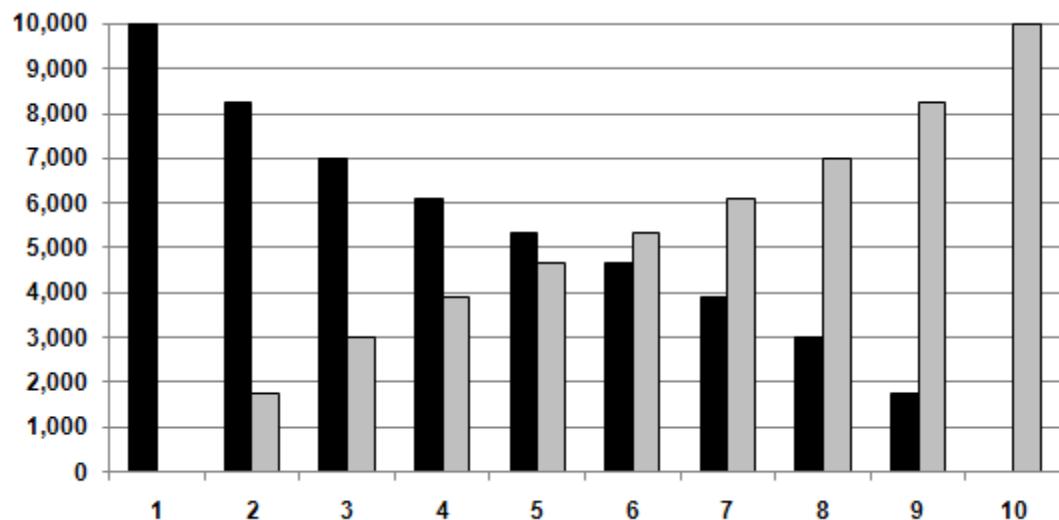
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Load Distribution

Recurrence Formula

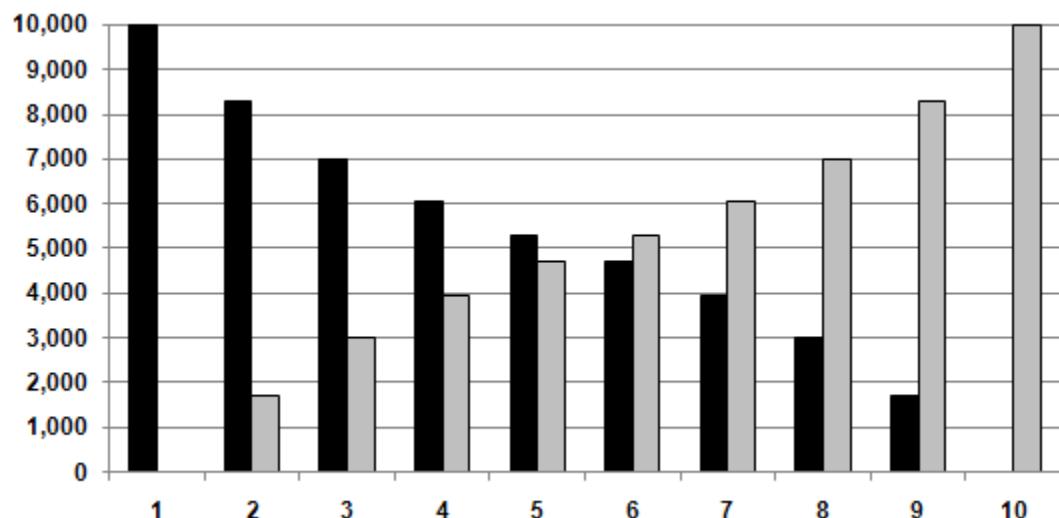


For $Q = 10,000$ lb



Parabolic WAG Method

For $Q = 10,000$ lb



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Comparison

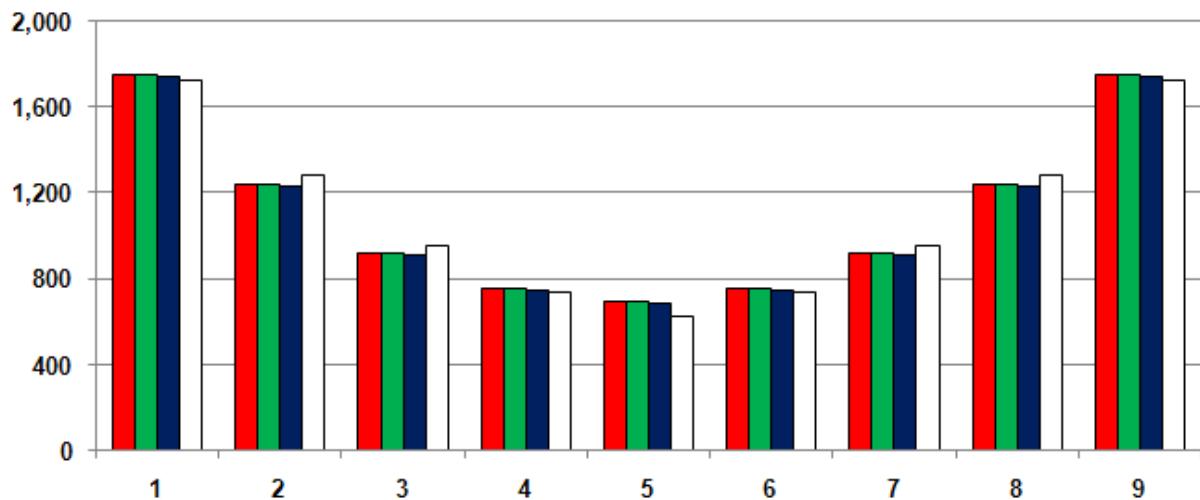
For $Q = 10,000$ lb

Parabolic WAG Method – White

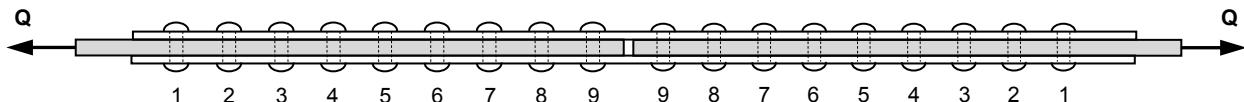
Recurrence Formula – Red

Finite Difference – Green

Shear Lag Analogy – Blue

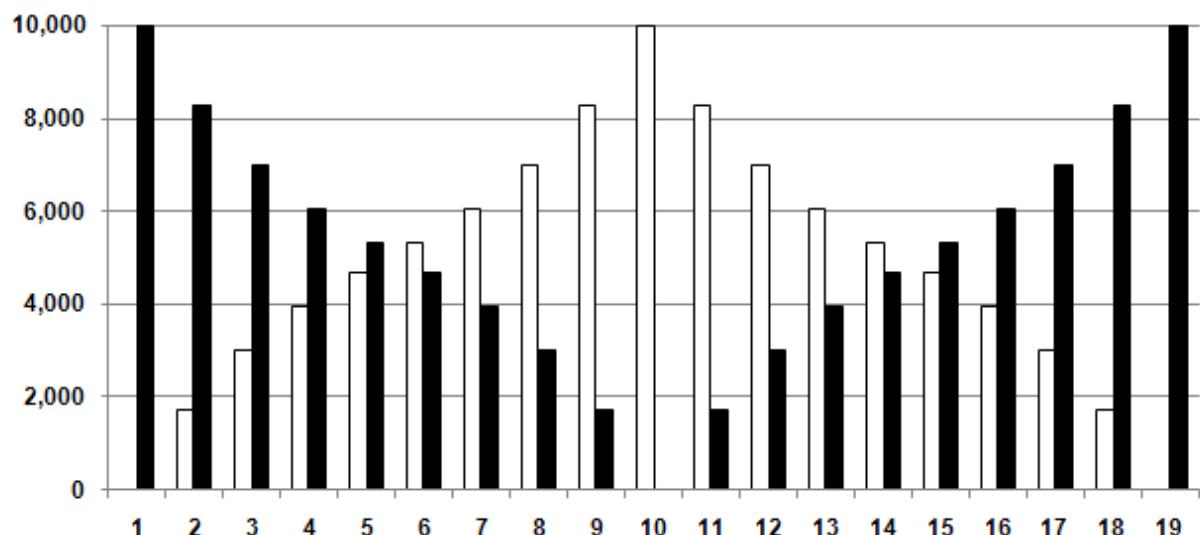


Butt Joint



Load Distribution

Combining Upper and Lower Straps



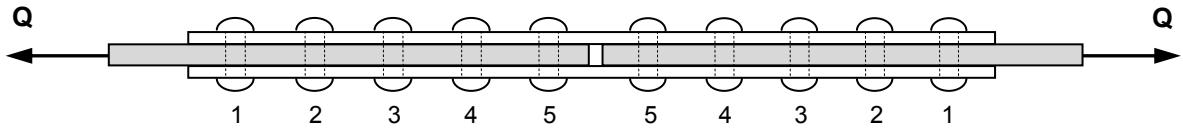
GRAN Corporation

Butt Joint

Manford B. Tate and Samuel J. Rosenfeld

NACA TN-1051 *Preliminary Investigation of the Loads Carried by Individual Bolts in Bolted Joints*

Appendix B pages 34 to 36



24S-T Plates $E = 10,500 \text{ ksi}$ Pitch, $p = 1.00 \text{ inch}$ $t_p = 0.3125 \text{ inch}$ $t_s = 0.1875 \text{ inch}$

Alloy Steel Bolts $E_{bb} = 29,000 \text{ ksi}$ $D = 0.250 \text{ inch}$ Width, $b = 2.00 \text{ inch}$

Plate Constants

$$K_s = \frac{p}{b t_s E} = \frac{1.00 \text{ inch}}{2.00 \text{ inch} (0.1875 \text{ inch}) 10,500 \text{ ksi}} = 0.000254$$

$$2 K_p = \frac{2 p}{b t_s E} = \frac{2 (1.00 \text{ inch})}{2.00 \text{ inch} (0.3125 \text{ inch}) 10,500 \text{ ksi}} = 0.000305$$

Bolt Constant

General Expression

NACA TN-1051 *Preliminary Investigation of the Loads Carried by Individual Bolts in Bolted Joints*

$$C = \frac{2 t_s + t_p}{3 G_b A_b} + \frac{8 t_s^3 + 16 t_s^2 t_p + 8 t_s t_p^2 + t_p^3}{192 E_{bb} I_b} + \frac{2 t_s + t_p}{t_s t_p E_{b br}} + \frac{1}{t_s E_{s br}} + \frac{2}{t_s E_{p br}}$$

Unequal Plate Areas

Using average thickness in the equation for 24S-T plates with alloy steel bolts and equal plate areas:

$$t_{avg} = \frac{2 t_s + t_p}{2} = \frac{2 (0.1875 \text{ inch}) + 0.3125 \text{ inch}}{2} = 0.3438 \text{ inch}$$

NACA TN-1051 Equation 4, page 7

$$C_{avg} = \frac{8}{t_{avg} E_{bb}} \left\{ 0.13 \left(\frac{t_{avg}}{D} \right)^2 \left[2.12 + \left(\frac{t_{avg}}{D} \right)^2 \right] + 1.87 \right\}$$

$$C_{avg} = \frac{8}{0.3438 (29,000 \text{ ksi})} \left\{ 0.13 \left(\frac{0.3438 \text{ inch}}{0.250 \text{ inch}} \right)^2 \left[2.12 + \left(\frac{0.3438 \text{ inch}}{0.250 \text{ inch}} \right)^2 \right] + 1.87 \right\} = 0.002292$$

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Loads in Successive Bolts – General Relationship

NACA TN-1051 Equation 1, page 6

$$R_{i+1} = \frac{C_i}{C_{i+1}} R_i + \frac{2 K_p + K_s}{C_{i+1}} R_i - \frac{2 K_p}{C_{i+1}} Q + \frac{2 K_p + K_s}{C_{i+1}} \sum_1^{i-1} R$$

Bolts are identical ...

$$C_i = C_{i+1}$$

$$R_{i+1} = R_i + \frac{2 K_p + K_s}{C} R_i - \frac{2 K_p}{C} Q + \frac{2 K_p + K_s}{C} \sum_1^{i-1} R$$

$$\frac{2 K_p + K_s}{C_{avg}} = \frac{0.000305 + 0.000254}{0.002292} = 0.2438$$

$$\frac{2 K_p}{C_{avg}} = \frac{0.000305}{0.002292} = 0.1330$$

Loads in Successive Bolts

$$R_1 = R_1$$

$$R_2 = R_1 + 0.2438 R_1 - 0.1330 Q = 1.2438 R_1 - 0.1330 Q$$

$$R_3 = R_2 + 0.2438 R_2 - 0.1330 Q + 0.2438 R_1$$

$$R_3 = 1.2438 (1.2438 R_1 - 0.1330 Q) - 0.1330 Q + 0.2438 R_1$$

$$R_3 = 1.7908 R_1 - 0.2984 Q$$

$$R_4 = R_3 + 0.2438 R_3 - 0.1130 Q + 0.2438 (R_1 + R_2)$$

$$R_4 = 1.2438 (1.7908 R_1 - 0.2984 Q) - 0.1330 Q + 0.2438 (2.2438 R_1 - 0.1330 Q)$$

$$R_4 = 2.7744 R_1 - 0.5365 Q$$

$$R_5 = R_4 + 0.2438 R_4 - 0.1130 Q + 0.2438 (R_1 + R_2 + R_3)$$

$$R_5 = 1.2438 (1.7908 R_1 - 0.2984 Q) - 0.1330 Q + 0.2438 (4.0346 R_1 - 0.4314 Q)$$

$$R_5 = 4.4346 R_1 - 0.9055 Q$$

$$Q = \sum R = 11.244 R_1 - 1.873 Q$$

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Fastener Loads

$$Q = \sum R = 11.244 R_1 - 1.873 Q$$

$$R_1 = \frac{2.873}{11.244} Q = 0.2556 Q$$

$$R_2 = 1.2438 (0.2556 Q) - 0.1330 Q = 0.1849 Q$$

$$R_3 = 1.7908 (0.2556 Q) - 0.2984 Q = 0.1593 Q$$

$$R_4 = 2.7744 (0.2556 Q) - 0.5365 Q = 0.1725 Q$$

$$R_5 = 4.4346 (0.2556 Q) - 0.9055 Q = 0.2278 Q$$

$$R_{avg} = 0.20 Q$$

$$R_1 / R_{avg} = 1.278$$

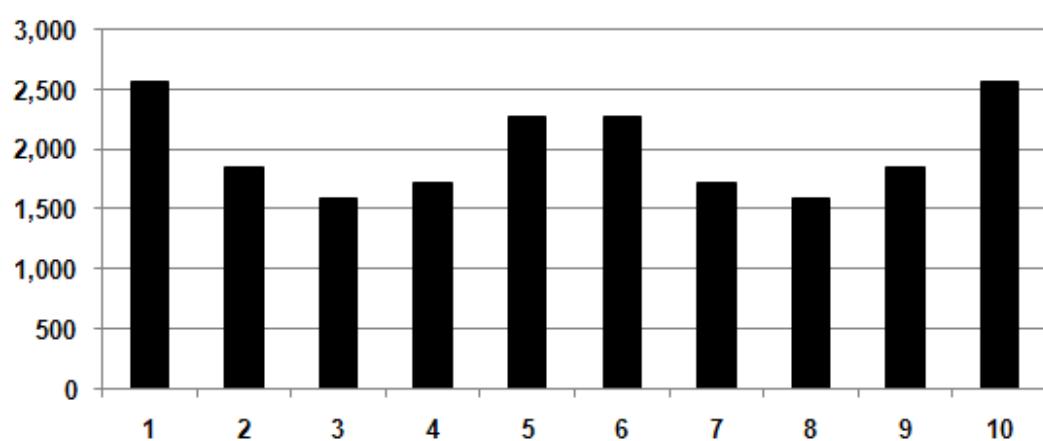
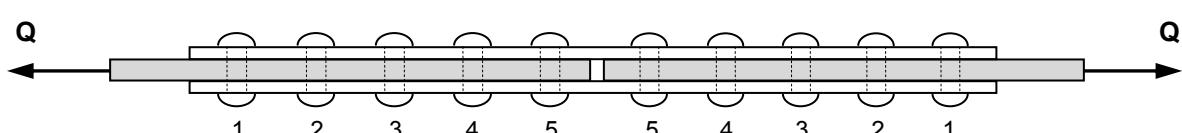
$$R_2 / R_{avg} = 0.924$$

$$R_3 / R_{avg} = 0.796$$

$$R_4 / R_{avg} = 0.863$$

$$R_5 / R_{avg} = 1.139$$

For $Q = 10,000$ lb



7.0 Fatigue

NAVWEPS 00-25-559

Department of the Navy

Tips on Fatigue

Prepared for the Bureau of Naval Weapons

Clarence R. Smith

Selected quotes below.

Lap Joints

While the lap joint is the simplest of all joints, its main problem is that, when the two sheets of material are joined, they tend to align themselves with each other. This causes the sheet to be bent at the first fastener, which is already suffering from too much load. This offset in alignment is commonly called eccentricity.

Butt Joints

Single Shear

The single shear butt joint is really two lap joints facing each other, so it has the same problem as the lap joint. One of its advantages is that the doubler can be made thicker than the material being spliced. This reduces the effects of bending, but it creates an additional problem: the rivet nearest the doubler's edge now carries most of the load just as in the case of the clevis joint ...

Double Shear

Double shear butt joints are superior to those of the single shear type. This is because the symmetry of the double shear type eliminates the bending effects found in the single shear. However, the double shear type also has the problem of load distribution between fasteners. Scarfing, or providing auxiliary doublers as for single shear joints, will improve fatigue life.

Doublers

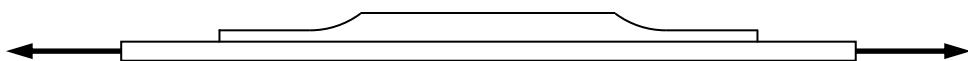
Some degree of deformation can be achieved by thinning the doubler material between the first two rows of fasteners so the second row can carry some of the load. Since doubler material must stretch in order to do this, the thickness at the first fastener should be less than half of the material being spliced.

The auxiliary doubler should be long enough to engage an extra row of rivets outside the main splice area. Here again there is a compromise between the practical and theoretical optimum thickness of auxiliary doublers.

Auxiliary thin doublers when properly used will increase the lifetime more than ten times.

Intuition should tell us that the doubler material between the first two rows of fasteners should be thinned down so it will stretch without overloading the first row of fasteners.

Stepped Doubler



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References

NAVWEPS 00-25-559 *Tips on Fatigue* Clarence R. Smith
Department of the Navy Prepared for the Bureau of Naval Weapons

ASTM International – Special Technical Publications

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- STP 91 *Manual on Fatigue Testing*
- STP 203 *Fatigue on Aircraft Structures*
- STP 237 *Symposium on Basic Mechanism of Fatigue*
- STP 274 *Symposium on Fatigue of Aircraft Structures*
- STP 284 *Symposium on Acoustical Fatigue*
- STP 338 *Symposium on Fatigue Tests of Aircraft Structures: Low-cycle, Full-scale, and Helicopters*

Note: ASTM International was originally called the American Society for Testing and Materials.

http://www.astm.org/COMMIT/filtrexx40.cgi?-P+COMMIT+E08+commitpubs_stp.frm

American Society of Mechanical Engineers – ASME

Conference (International) on Fatigue of Metals – Proceedings

ASM International (American Society for Metals)

Metals Handbook

Society for Experimental Mechanics – SEM

Handbook of Experimental Stress Edited by M. Hetenyi

Note: "The Society for Experimental Mechanics, originally called The Society for Experimental Stress Analysis, was founded in 1943 as a nonprofit scientific and educational organization ..." (Wikipedia)

Miscellaneous

- Fatigue of Metals* P.G. Forrest
- Fatigue in Aircraft Structure* A.M. Freudenthal
- The Fatigue of Metals and Structures* Gordon Grover & Jackson
- Metallic Fatigue with Particular Reference to Significance of Certain Standard Aircraft Fabrication and Finishing Process* W.J. Harris
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