

Energy Methods

- Recall previous discussion of strain energy
- Work done by force acting through deformation

$$U = \int \underline{P} \cdot d\underline{s}$$
- If response is linear and direction of force is same as deformation,

$$U = \int_0^{\delta} k s ds = \frac{1}{2} k \delta^2 = \frac{1}{2} P \delta$$

- Reciprocity Theorem, or sets,

- Consider two stages of loading on simple cantilever

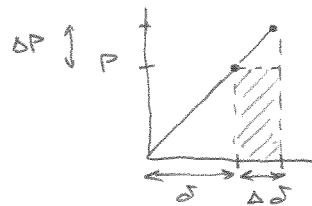
- 1) P , causing δ

$$U_1 = \frac{1}{2} P \delta$$

- 2) ΔP , causing $\Delta \delta$ more

$$U_2 = \frac{1}{2} \Delta P \Delta \delta$$

$$U_{1,2} = P \Delta \delta$$



- Now change the order

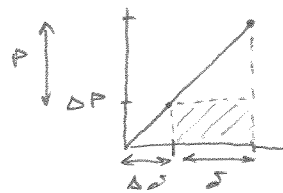
- 1) ΔP , causing $\Delta \delta$

$$U'_1 = \frac{1}{2} \Delta P \Delta \delta$$

- 2) P , causing δ more

$$U'_2 = \frac{1}{2} P \delta$$

$$U_{2,1} = \Delta P \delta$$



- Since $U = U'$, $U_1 + U_2 + U_{1,2} = U'_1 + U'_2 + U_{2,1}$
 $\Rightarrow U_{1,2} = U_{2,1} \Rightarrow P \Delta \delta = \Delta P \delta$

- Can generalize to include any number of forces

$$\Rightarrow \sum P'_k \delta''_k = \sum P''_j \delta'_j$$

\Rightarrow work done by one set of forces through displacement due to second set = work done by 2nd set on displacements due to 1st set

Castigliano's Theorem

Consider again P and ΔP

$$\Delta U_{1-2} = U_2 + U_{1,2} = \frac{1}{2} \Delta P \Delta \delta + \underbrace{P \Delta \delta}_{\Delta P \delta}$$

$$\Rightarrow \frac{\Delta U}{\Delta P} = \frac{1}{2} \Delta \delta + \delta$$

$$\lim_{\Delta P \rightarrow 0} \frac{\Delta U}{\Delta P} = \frac{1}{2} \cancel{\Delta \delta} + \delta$$

$$\Rightarrow \frac{\partial U}{\partial P} = \delta$$

Again, we generalize here to allow any number of forces be applied as part of set 1. Then

$$\frac{\partial U}{\partial P_i} = \delta_i$$

and we can find the displacement ^{δ_i} associated with the action of any force P_i by taking

$$\frac{\partial U}{\partial P_i}.$$

Can similarly show,

$$\frac{\partial U}{\partial C_i} = \Theta_i, \text{ where } C_i \text{ is any applied couple moment and } \Theta_i \text{ is the slope at the point of application}$$

Express U as func of load. In general,

$$U = \int \frac{N^2}{2AE} dx + \int \frac{M^2}{2EI} dx + \int \frac{\alpha V^2}{2AG} dx + \int \frac{T^2}{2JG} dx$$

where $N \equiv$ axial load, $M \equiv$ bending moment,
 $V \equiv$ shearing force, $T \equiv$ torque,

$$\text{and } \alpha = \frac{A}{I^2} \int \frac{\phi^2}{b^2} dA \equiv \text{form factor for shear}$$

$$= \int_L \int_A \frac{\sigma}{2E} dA dx = \int_L \frac{\sigma}{2E} \int_A dA dx = \int_L \frac{P}{A} \frac{1}{2E} A dx$$

Strain Energy Expressions for Common Applications

• Axial Loading

$$U = \int_V U_0 dV = \int_V \frac{\sigma^2}{2E} dV = \int_V \frac{P^2}{2EA^2} dV = \int_0^L \frac{P^2}{2EA} A dx = \int_0^L \frac{P^2}{2EA} dx$$

• For prismatic bar, $U = \frac{P^2 L}{2EA}$

• Torsion

$$U = \int_V U_0 dV = \int_V \frac{\tau_{xy}^2}{2G} dV = \int_V \frac{1}{2G} \left(\frac{T\rho}{J} \right)^2 dV = \int_0^L \frac{1}{2G} \frac{T^2}{J^2} \left(\int_A \rho^2 dA \right) dx$$

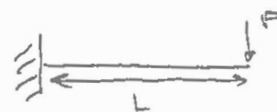
$$= \int_0^L \frac{T^2}{2GJ} dx$$

• Bending

$$U = \int_V \frac{\sigma^2}{2E} dV = \int_V \frac{1}{2E} \left(\frac{My}{I} \right)^2 dA dx = \int_0^L \frac{M^2}{2EI^2} \left(\int_A y^2 dA \right) dx = \int_0^L \frac{M^2}{2EI} dx$$

Example

Find deflection of cantilever beam using Castigliano's Method. Compare to traditional solution.



$$\delta_i = \frac{\partial U}{\partial P_i}$$

$$\Rightarrow \delta_P = \frac{\partial U}{\partial P} = \frac{\partial}{\partial P} \int \frac{M^2}{2EI} dx = \frac{1}{2EI} \int \frac{\partial}{\partial P} (M^2) dx = \frac{1}{2EI} \int \frac{\partial(M^2)}{\partial M} \frac{\partial M}{\partial P} dx$$
$$= \frac{1}{EI} \int M \frac{\partial M}{\partial P} dx \quad (\text{ignoring shear, } V)$$

• Need $M(x)$

$$\bullet \sum M_x = 0 \Rightarrow M(x) + PL - Px$$

$$\Rightarrow M(x) = Px - PL$$

$$\Rightarrow \frac{\partial M}{\partial P} = x - L$$

$$\Rightarrow \delta_P = \frac{1}{EI} \int_0^L (Px - PL)(x - L) dx$$

$$= \frac{1}{EI} \int_0^L (Px^2 - 2PLx + PL^2) dx$$

$$= \frac{1}{EI} \left[P \frac{x^3}{3} - 2PL \frac{x^2}{2} + PL^2 x \right]_0^L$$

$$= \frac{1}{EI} \left[\frac{PL^3}{3} - PL^3 + PL^3 \right] = \underline{\underline{\frac{PL^3}{3EI}}}$$

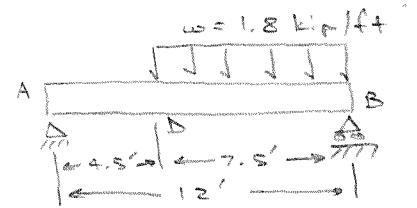


* Table D.4 gives same answer

* Note that we neglected the shear term, so neither sol'n is exact, but we will see that shear term is small

Example (S.P. 11.6, Beer 7, p. 813)

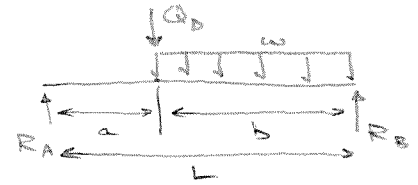
Given the beam and loading shown, find the deflection @ D using Castigliano's Method.



Sol'n

No load @ D \rightarrow apply dummy Q_D

$$y_D = \frac{\partial U}{\partial Q_D} = \int_0^L \frac{M}{EI} \frac{\partial M}{\partial Q_D} dx$$



* Need to do this separately for AD and BD since $M(x)$ changes @ D

Reactions

$$\sum M_B = 0 = Q_D b + \frac{wb^2}{2} - R_A L \Rightarrow R_A = Q_D \frac{b}{L} + \frac{wb^2}{2L}$$

$$\sum M_A = 0 = R_B L - Q_D a - wb \left(a + \frac{b}{2}\right) \Rightarrow R_B = Q_D \frac{a}{L} + \frac{wb}{L} \left(a + \frac{b}{2}\right)$$

Section AD

$$\sum M_{cut} = 0 = M(x) - R_A x \Rightarrow M(x) = R_A x$$

$$\Rightarrow M(x) = Q_D \frac{b}{L} x + \frac{wb^2}{2L} x$$



$$\frac{\partial M}{\partial Q_D} = \frac{b}{L} x$$

$$\Rightarrow \frac{1}{EI} \int M \frac{\partial M}{\partial Q_D} dx = \frac{1}{EI} \int_0^a \left(Q_D \frac{b}{L} x + \frac{wb^2}{2L} x \right) \left(\frac{b}{L} x \right) dx = \frac{1}{EI} \int_0^a \frac{wb^3}{2L^2} x^2 dx$$

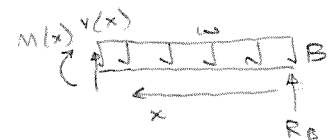
$$= \frac{wb^3}{2EIL^2} \frac{x^3}{3} \Big|_0^a = \frac{wa^3 b^3}{6EIL^2}$$

Section BD

$$\sum M_{cut} = 0 = R_B x - \frac{wx^2}{2} - M(x)$$

$$\Rightarrow M(x) = R_B x - \frac{wx^2}{2} = Q_D \frac{a}{L} x + \frac{wb}{L} \left(a + \frac{b}{2}\right) x - \frac{wx^2}{2}$$

$$\frac{\partial M}{\partial Q_D} = \frac{a}{L} x$$



$$\Rightarrow \frac{1}{EI} \int M \frac{\partial M}{\partial Q_D} dx = \frac{1}{EI} \int_0^b \left[\frac{wb}{L} \left(a + \frac{b}{2}\right) x - \frac{wx^2}{2} \right] \frac{a}{L} x dx$$

$$= \frac{1}{EI} \left[\frac{wab}{L^2} \left(a + \frac{b}{2}\right) \frac{x^3}{3} - \frac{wa}{2L} \frac{x^4}{4} \right]_0^b = \frac{1}{EI} \left[\frac{8wab^4}{24L^2} \left(a + \frac{b}{2}\right) - \frac{3wab^4}{24L} \right]$$

$$= \frac{wab^3}{24EIL} \left[\frac{8ab}{L} + \frac{4b^2}{L} + 3b \frac{L}{L} \right] = \frac{wab^3}{24EIL^2} (8ab + 4b^2 + 3bL)$$

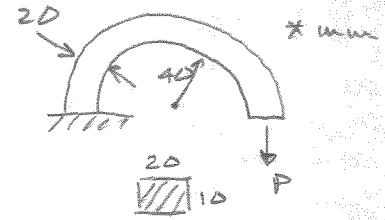
$$\Rightarrow y_D = y_{AD} + y_{BD} = \frac{wab^3}{24EIL^2} \frac{(4a^2 + 8ab + b^2)}{(4a+b)(a+b)} = \frac{wab^3}{24EIL} (4a+b) = 0.262 \text{ in}$$

* Superposition w/ strain energy

Example: Castigliano - deflection of ^{half} ring (U + F EID. +)

Given: A load of $P = 5 \text{ kN}$ is applied to curved steel bar. $E = 200 \text{ GPa}$, $G = 80 \text{ GPa}$

Find: vertical deflection of free end (considering all stresses)



Sol'n:

- FBD of section shows contributions from M , V , and N



$$\Rightarrow \delta_P = \frac{1}{AE} \int N \frac{\partial N}{\partial P} dx + \frac{1}{EI} \int M \frac{\partial M}{\partial P} dx + \frac{1}{AG} \int V \frac{\partial V}{\partial P} dx$$

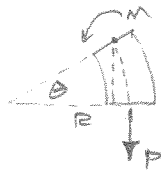
- Find expressions for N , M , & V

$$\bullet \sum F_v = 0 = V - P \sin \theta \Rightarrow V = P \sin \theta \Rightarrow \frac{\partial V}{\partial P} = \sin \theta$$

$$\bullet \sum F_n = 0 = N - P \cos \theta \Rightarrow N = P \cos \theta \Rightarrow \frac{\partial N}{\partial P} = \cos \theta$$

$$\bullet \sum M = 0 = M - P(R - R \cos \theta) \Rightarrow M = PR(1 - \cos \theta)$$

$$\Rightarrow \frac{\partial M}{\partial P} = R(1 - \cos \theta)$$



- Form factor for shear $\alpha = 6/5$ for rectangular section

- Need to integrate over length, but circular $\Rightarrow dx = R d\theta$

$$\begin{aligned} \Rightarrow \delta_P &= \frac{1}{AE} \int_0^\pi P \cos^2 \theta R d\theta + \frac{1}{EI} \int_0^\pi PR^2 (1 - \cos \theta)^2 R d\theta + \frac{6}{SAG} \int_0^\pi P \sin^2 \theta R d\theta \\ &= \frac{PR}{AE} \int_0^\pi \cos^2 \theta d\theta + \frac{PR^3}{EI} \int_0^\pi (1 - \cos \theta)^2 d\theta + \frac{6PR}{SAG} \int_0^\pi \sin^2 \theta d\theta \end{aligned}$$

$$\text{* Recall } \cos^2 \theta = \frac{1 + \cos 2\theta}{2} \text{ and } \sin^2 \theta = \frac{1 - \cos 2\theta}{2}$$

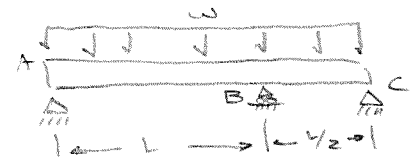
$$\Rightarrow \delta_P = \frac{\pi PR}{2AE} + \frac{3\pi PR^3}{2EI} + \frac{3\pi PR}{SAG}$$

$$= (\underbrace{0.01}_{\text{axial}} + \underbrace{2.21}_{\text{bending}} + \underbrace{0.03}_{\text{shear}}) \times 10^{-3} \text{ m} = \underline{2.25 \text{ mm}}$$

* axial & shear contribute little! \Rightarrow common to omit these when $R/c > 4$

Example (S.P. 11.7, Beer 7, p. 815)

Determine reactions @ supports



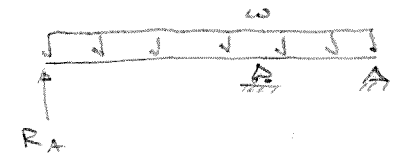
Sol'n

Indeterminate: Remove one of the supports to make determinant;
choose A to remove

Solve, then require $y_A = 0$

$$\Rightarrow y_A = \frac{1}{EI} \int M \frac{\partial M}{\partial R_A} dx$$

Find reactions in terms of R_A and w



$$\sum M_C = 0 = w \frac{3}{2} L \left(\frac{3L}{4} \right) - R_A \frac{3L}{2} - R_B \frac{L}{2}$$

$$\Rightarrow R_B = \frac{9}{4} wL - 3R_A$$

$$\sum M_B = 0 = w \frac{3L}{2} \left(\frac{3L}{4} - \frac{2L}{4} \right) + R_C \frac{L}{2} - R_A L$$

$$\Rightarrow R_C = 2 \left(R_A - \frac{3}{8} wL \right) = 2R_A - \frac{3}{4} wL$$

Segment AB

$$\sum M_{cut} = 0 = M(x) + \frac{w x^2}{2} - R_A x$$

$$\Rightarrow M(x) = R_A x - \frac{w x^2}{2} ; \quad \frac{\partial M}{\partial R_A} = x$$



$$\Rightarrow \frac{1}{EI} \int_0^L \left(R_A x - \frac{w x^2}{2} \right) x dx = \frac{1}{EI} \left(R_A \frac{x^3}{3} - w \frac{x^4}{8} \right) \Big|_0^L = \frac{1}{EI} \left(R_A \frac{L^3}{3} - w \frac{L^4}{8} \right)$$

Segment BC

$$\sum M_{cut} = 0 = -M(x) - w \frac{x^2}{2} + R_C x$$

$$\Rightarrow M(x) = R_C x - w \frac{x^2}{2} = 2R_A x - \frac{3}{4} wL x - w \frac{x^2}{2}$$



$$\frac{\partial M}{\partial R_A} = 2x$$

$$\Rightarrow \frac{1}{EI} \int_0^{L/2} \left(2R_A x - \frac{3}{4} wL x - w \frac{x^2}{2} \right) 2x dx = \frac{1}{EI} \left[4R_A \frac{x^3}{3} - \frac{3}{2} wL \frac{x^3}{3} - w \frac{x^4}{4} \right]_0^{L/2}$$

$$= \frac{1}{EI} \left(\frac{4}{3} R_A \frac{L^3}{8} - \frac{3}{2} wL \frac{L^3}{24} - \frac{w}{4} \frac{L^4}{16} \right) = \frac{1}{EI} \left(R_A \frac{L^3}{6} - \frac{5}{64} wL^4 \right)$$

$$\text{Now, } y_A = 0 = \frac{1}{EI} \left(R_A \frac{L^3}{3} - w \frac{L^4}{8} \right) + \frac{1}{EI} \left(R_A \frac{L^3}{6} - \frac{5}{64} wL^4 \right)$$

$$= \frac{R_A L^3}{2} - \frac{13}{64} wL^4 \Rightarrow R_A \frac{L^3}{2} = \frac{13}{64} wL^4 \Rightarrow R_A = \frac{13}{32} wL$$

$$\Rightarrow R_B = \frac{9}{4} wL - 3 \left(\frac{13}{32} wL \right) = \frac{33}{32} wL ; \quad R_C = \frac{wL}{16}$$