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ANNEX G

(Clause 38.1)

MOMENTS OF RESISTANCE FOR RECTANGULAR AND T-SECTIONS

G-0 The moments of resistance of rectangular and T-sections based on the assumptions of 38.1 are given in this annex.

G-1 RECTANGULAR SECTIONS

G-1.1 Sections Without Compression Reinforcement

The moment of resistance of rectangular sections without compression reinforcement should be obtained as follows:

a) Determine the depth of netutral axis from the following equation:

$$\frac{x_{\rm u}}{d} = \frac{0.87 \, f_{\rm y} \, A_{\rm st}}{0.36 \, f_{\rm ck} \, b, d}$$

b) If the value of x_1/d is less than the limiting value (see Note below 38.1), calculate the moment of resistance by the following expression:

$$M_{\rm u} = 0.87 \ f_{\rm y} \ A_{\rm st} \ d \left(1 - \frac{A_{\rm st} \ f_{\rm y}}{b d \ f_{\rm ck}} \right)$$

c) If the value of x_{\parallel}/d is equal to the limiting value, the moment of resistance of the section is given by the following expression:

$$M_{\rm u,lim} = 0.36 \frac{x_{\rm u, max}}{d} \left(1 - 0.42 \frac{x_{\rm u, max}}{d} \right) b d^2 f_{\rm ck}$$

d) If x_u/d is greater than the limiting value, the section should be redesigned.

In the above equations,

x = depth of neutral axis,

d = effective depth,

 f_y = characteristic strength of reinforce-

A = area of tension reinforcement,

 f_{ck} = characteristic compressive strength of concrete,

b = width of the compression face,

M_{u,lim} = limiting moment of resistance of a section without compression reinforcement, and

x = limiting value of x_0 from 39.1.

G-1.2 Section with Compression Reinforcement

Where the ultimate moment of resistance of section

exceeds the limiting value, $M_{u, \text{lim}}$ compression reinforcement may be obtained from the following equation:

$$M_{\rm u} - M_{\rm u, lim} = f_{\rm sc} A_{\rm sc} (d-d')$$

where

 $M_{\rm u}, M_{\rm u, lim}, d$ are same as in G-1.1,

 f_{sc} = design stress in compression reinforcement corresponding to a strain of

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$$\frac{\left(x_{11, \text{ max}} - d'\right)}{x_{0, \text{ max}}}$$

where

 $x_{u, \text{max}} = \text{ the limiting value of } x_u \text{ from 38.1,}$

 A_{sc} = area of compression reinforcement, and

 d' = depth of compression reinforcement from compression face.

The total area of tension reinforcement shall be obtained from the following equation:

$$A_{\rm et} = A_{\rm et} + A_{\rm et}$$

where

 A_{a} = area of the total tensile reinforcement,

 A_{stl} = area of the tensile reinforcement for a singly reinforced section for $M_{\text{w, fim}}$, and

$$A_{\rm st2} = A_{\rm se} f_{\rm se} / 0.87 f_{\rm y}$$

G-2 FLANGED SECTION

G-2.1 For $x_u < D_p$, the moment of resistance may be calculated from the equation given in **G-1.1**.

G-2.2 The limiting value of the moment of resistance of the section may be obtained by the following equation when the ratio D_r / d does not exceed 0.2:

$$M_{\rm u} = 0.36 \frac{x_{\rm u,max}}{d} \left(1 - 0.42 \frac{x_{\rm u,max}}{d} \right) f_{\rm ck} b_{\rm w} d^2 + 0.45 f_{\rm ck} (b_{\rm f} - b_{\rm w}) D_{\rm f} \left(d - \frac{D_{\rm f}}{2} \right)$$

where

 $M_{\rm u}$, $x_{\rm u, max}$, d and $f_{\rm ck}$ are same as in G-1.1,

 b_r = breadth of the compression face/flange,

 b_{y} = breadth of the web, and

 D_r = thickness of the flange.

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G-2.2.1 When the ratio $D_{\rm f}/d$ exceeds 0.2, the moment of resistance of the section may be calculated by the following equation:

 $\label{eq:constraints} (x,y) = \left(\left(\frac{1}{2} + \frac{1}{2} \right) \left(\frac{1}{2} + \frac{1}{2} + \frac{1}{2} \right) \left(\frac{1}{2} + \frac{1}{2} +$

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$$M_{\rm u} = 0.36 \frac{x_{\rm u,max}}{d} \left(1 - 0.42 \frac{x_{\rm u,max}}{d} \right) f_{\rm ck} b_{\rm w} d^2 + 0.45 f_{\rm ck} \left(b_{\rm f} - b_{\rm w} \right) y_{\rm f} \left(d - \frac{y_{\rm f}}{2} \right)$$

where $y_f = (0.15 x_u + 0.65 D_f)$, but not greater than D_f , and the other symbols are same as in G-1.1 and G-2.2.

G-2.3 For $x_{u_1,max} > x_u > D_f$, the moment of resistance may be calculated by the equations given in G-2.2 when D_f/x_u does not exceed 0.43 and G-2.2.1 when D_f/x_u exceeds 0.43; in both cases substituting $x_{u',max}$ by x_u .