

#### Overview

- Analysis of hydraulic flocculators
  - Ratio of maximum to average energy dissipation rate
  - Inefficiency of energy use due to nonuniformity of energy dissipation rate
  - The great transition at H<sub>e</sub>/S=5
- Flocculator Design
  - Head loss, collision potential, residence time
  - Geometry of a baffle space to obtain desired energy dissipation rate



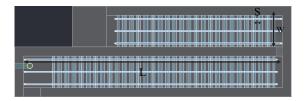
#### **Top View**



W = Width of the flocculator channel

 $S = Space \underline{between}$  baffles

L = Length of a flocculator channel



#### Side View



- H = Water depth

- The white tephin L = L charged of the flocculator channel S = S pace between baffles S + T = BB =Perpendicular center to center distance between baffles
- Exit to the sedimentation tank entrance channel Minimum water level Upper baffle Lower baffle Port from previous channel

### **Design Considerations**

- The length of the flocculator channels matches the length of the sedimentation tank
- Width of the flocculation channel?
  - Minimum? Human width
  - Material limitations (polycarbonate or concrete)
  - Vary to optimize flocculation efficiency (function of geometry
- Need to determine
  - Head loss
  - Residence time
  - Baffle spacing
  - Number of baffles

#### More Design Considerations

- Even number of channels for AguaClara design (to keep chemical dose controller near stock tanks), but this may change if flocculators get smaller
- Even or odd number of baffles depending on channel inlet and outlet conditions
- Begin with the energy source for the turbulence that creates shear that creates collisions: head loss for a baffle

#### Vena Contracta around a bend?

Sluice gate (almost closed)\*

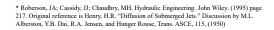
0.59

• Small hole in a tank

0.62

Exit from a pipe

No Vena Contracta



### Vena Contracta ( $\Pi_{VC}$ ) Conclusions

- Draw the most extreme streamline through the transition and determine the total change in direction
- If the change in direction for most of the fluid is 90°, then the  $\Pi_{VC}$  is approximately 0.62
- If the change in direction for most of the fluid is 180°, then the  $\Pi_{VC}$  is approximately 0.62²=0.384



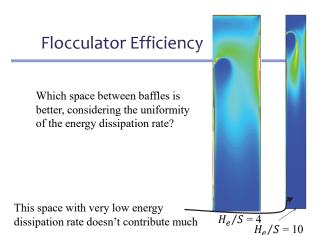
#### Head Loss coefficient for a Baffle

 $h_e = rac{V_{out}^2}{2g} \left(rac{A_{out}}{A_{in}} - 1
ight)^2$  Head loss in an expansion

 $K_e = \left(\frac{A_{out}}{A_{in}} - 1\right)^2$  e - expansion

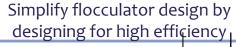
$$K_e = \left(\frac{1}{\Pi_{VCBaffle}} - 1\right)^2 = 2.56$$
 the contraction coefficient for a sharp 
$$180^{\rm o}~{\rm bend}~(0.62^2)$$

We need to measure this in one of the new AguaClara plants!

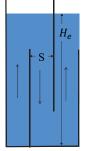


# Why a transition at $H_e/S$ of 5?

- Jets expand in width at the rate of approximately 1 unit in width per 10 units forward
- Expansion length is 10(0.6S)
- Expansion requires a distance of approximately 6S
- ullet The  $H_e/S$  transition is related to the distance required for the jet to fully expand



- Efficiency will be a function of the variability of the energy dissipation rate  $\prod_{\bar{\epsilon}}^{\epsilon_{Max}} = \frac{\varepsilon_{Max}}{\bar{\epsilon}}$
- We expect a relation of the form such that efficiency is 1 when  $\Pi_{\epsilon}^{Max}$ =1 and efficiency is less than 1 for higher values of  $\Pi_{\epsilon}^{Max}$
- We "solve" this unknown by always designing efficient flocculators with 3<H/S<6</li>

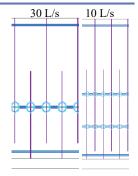


### Prior to 2015 AguaClara used designs that were far from the optimum

- A compact plant layout was possible for small flows by using a vertical flow flocculator with a high  $H_e/S$  ratio
- For small plants the width of the channel was determined by the need to construct the channel using humans (45 cm or more)
- The space between baffles was very narrow and thus  $H_e/S$  was very high (for low flow plants)
- Small plants needed longer residence time and more baffles to achieve adequate flocculation because efficiency was reduced.

### New Approach: Always efficient

- Add obstacles to have a maximum  $H_e/S$  ratio of between 3 and 6.
- Flocculation efficiency can be considered constant (and close to 1)



### Viscous collisions or inertial collisions

Prior to 2016 I had assumed that the appropriate length scale comparison was particle separation distance and Kolmogorov length scale - thus concluded inertia was important

- Particle separation distances are smaller than inner viscous length scale
  - Collisions in turbulent flocculators are dominated by viscosity (fluid shear, not turbulent eddies)\* \* Edge of knowledge

#### Collision Potential



Not yet

- The target collision potential used for the design of AguaClara plants since about 2013 has been 37,000
- The actual collision potential in operating AguaClara plants may be lower because the head loss per baffle may be lower than we assumed

### Energy use (head loss) in flocculation controls velocity gradient

- Head loss
  - High head loss results in a taller building for the water treatment plant
  - High head loss means higher velocities and that reduces settling of flocs in the flocculator
  - Some gravity flow water supplies don't have much elevation difference between source and storage tank
- Velocity gradient (G)
  - Higher  $\overline{G}$  allows lower residence time
  - Higher  $\bar{G}$  results in smaller flocs

- $h_{Floc} = \frac{\theta \bar{\varepsilon}}{a}$
- $h_e = K_e \frac{V^2}{2a}$

$$h_{Floc} = \sum K_e \frac{V^2}{2g}$$

$$\bar{G} = \sqrt{\frac{\bar{\varepsilon}}{\nu}}$$

$$\bar{\varepsilon}=\nu\bar{G}^2$$

$$h_{Floc} = \bar{G}\theta \frac{\nu G}{g}$$

## The Influence of $\bar{G}$ or $G_{Max}$

- The value of  $\bar{G}$  or  $\bar{\varepsilon}$  determines the head loss through the flocculator
- Maximum size of the flocs is controlled by
  - $\bar{G}$  or  $\bar{\varepsilon}$  (assuming shear limits attachment)
  - $G_{Max}$  or  $\varepsilon_{Max}$  (assuming floc break up controls max size)
- $\varepsilon_{Max}$  = 10 mW/kg ( $G_{Max}$  = 100 Hz) was the AguaClara standard (2011-2015)
- Summer 2015 new designs have head loss of approximately 40 cm
  - Expect smaller flocs (but still captured by plate settlers)
  - Less sedimentation of flocs in flocculator
  - Smaller flocculator
- Casey Garland has tested  $\bar{G}$  values as high as 340 Hz

## The design inputs for flocculation

 We need collisions and thus Gθ is a logical design specification

$$\bar{G}\theta = \frac{3}{2} \frac{\left(\Lambda^2 - \Lambda_0^2\right)}{k\pi d_D^2 \alpha}$$

- We need to specify energy use
  - Velocity gradient  $\bar{G}$
  - Energy dissipation rate  $\bar{\varepsilon}$
  - Total head loss h<sub>Floc</sub>
- Or t (θ)

More time helps diffusion of coagulant nanoparticles to clay surfaces

Higher G means smaller flocs and more elevation drop (head loss) through flocculator

Current approach

### Our current choice of parameter that sets energy input is head loss

• Head loss is independent of temperature

$$h_e = K_e \frac{V^2}{2g}$$

- Velocity gradient is f(temperature)
- Option 1 Start with  $(\bar{G}, \bar{G}\theta)$  and coldest temperature
  - Calculate θ

$$\theta = \frac{G\theta}{G}$$
  
 $h_{Floc} = \bar{G}\theta \frac{\nu G}{g}$ 

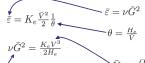
- Calculate h<sub>Floc</sub>

- $oxedsymbol{\Phi}^{ ext{ption 2}ullet}$  Start with  $(h_{ ext{Floc}},ar{G} heta)$  and coldest temperature ullet Calculate  $ar{G}$ 
  - Calculate θ

- $\bar{G}$  (and hence  $\bar{G}\theta$ ) will increase when the flocculator is operated at warmer temperatures due to decrease in viscosity

#### Design the reactor geometry to get the target velocity gradient





Kinetic energy dissipated per residence time



 $\bar{V} = \frac{Q}{WS} \text{ Rectangular geometry}$   $\bar{V} = \frac{Q}{WS} \text{ Rectangular geometry}$   $H_e \text{ is height of one expansion zone.}$   $\bar{G} = \sqrt{\frac{K_e}{2\nu H_e}} \left(\frac{Q}{WS}\right)^3$ Could be the depth of water if the only expansion is from the 180 degree bend

This is our general equation relating velocity gradient to reactor geometry

### Solve for channel width to set constraints on viable solutions

$$\nu \bar{G}^2 = \frac{K_e}{2H_e} \left(\frac{Q}{WS}\right)^3$$

 $W = \frac{Q}{S} \left( \frac{K_e}{2H_e \nu G^2} \right)^{\frac{1}{3}} \qquad \mathcal{S} = \frac{H_e}{\Pi_{HS}} \qquad H_e = \Pi_{HS} S$ 

This is the minimum channel width if we set  $\Pi_{HS} = 3$  and set the expansion height to equal water depth

As channel gets narrower the spacing between baffles

Channels narrower than this would have barely any or negative baffle overlap!



## Minimum number of expansions per depth of flocculator (given W)

$$\nu \bar{G}^2 = \frac{K_e}{2H_e} {\left( \frac{Q}{WS} \right)}^3 \qquad \Pi_{HS} = \frac{H_e}{S} \qquad S = \frac{H_{e_{Max}}}{\Pi_{HS_{Max}}} \; \; \text{Eliminate S}$$

$$H_{e_{Max}} = \left[\frac{K_e}{2\nu G^2} \left(\frac{Q\Pi_{HS_{Max}}}{W}\right)^3\right]^{\frac{1}{4}} \quad \text{Solve for maximum distance} \\ \text{between expansions, } H_e, \\ \text{using } \Pi_{HS_{Max}} = 6$$

$$N_{e_{Min}} = \frac{H_{Floc}}{H_{e_{Max}}}$$
 Round **up** to get the minimum number of expansions per depth of the flocculator



### Our Design Approach Given energy ( $h_{Floc}$ or $\overline{G}$ ) and $G\theta$

- Start big and then design the details
  - Calculate volume of flocculator
    - approach as of summer 2015)
  - Split it into channels

channels to get target  $\overline{\textbf{\textit{G}}}$ 

- Then design baffles, and obstacles to fill the
- We can use this design approach because we are assuming that we will design for high efficiency ( $3 < H_e/S < 6$ ) and thus we don't have to add extra volume to account for inefficiencies. (Don't forget this requirement!)

# Design Algorithm (as of 2016) Start with $h_{Floc}$ and G $\theta$

- Velocity gradient and flocculator volume given  $\bar{G} = \frac{gh_{Floc}}{(\bar{G}\theta)_{H}}$ head loss and collision potential
- Minimum channel width required to achieve  $W_{Min}=rac{\Pi_{HS}Q}{H_e}\Big(rac{K_e}{2H_e
  u^G^2}\Big)^rac{1}{3}$  $H_e/S > 3$  and required for constructability
- Number of channels by taking the total width and dividing by the minimum channel width (floor)
- Channel width (total width over number of channels)
- $H_{e_{Max}} = \left[ \frac{K_e}{2\nu G^2} \left( \frac{Q\Pi_{HS_{Max}}}{W} \right)^3 \right]$ Maximum distance between expansions
- 6. Minimum number of expansions per baffle space
- 7. Actual distance between expansions
- 8. Baffle spacing
- Calculate the obstacle width to obtain the same jet expansion conditions as produced by the 180 degree bend

$$S = \left(\frac{K_e}{2H_eG^2\nu}\right)^{\frac{1}{3}} \frac{Q}{W}$$

$$N_{e_{Min}} = \frac{H_{Floc}}{H}$$

#### Viscous Collision Potential per Flow Expansion (the detailed perspective)

$$G\theta = \theta \sqrt{\frac{\varepsilon}{\nu}}$$
 
$$\theta_e = \frac{H_e}{V}$$

Collision potential for one flow expansion Height of one expansion zone (in a vertical flow flocculator) Hydraulic residence time for one expansion zone These are the average velocities through the expanded flow area

Energy dissipation rate is energy loss per time

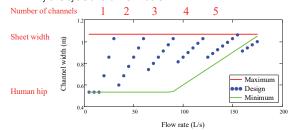
$$G\theta_e = \frac{H_e}{\bar{V}} \sqrt{\frac{K_e}{\nu} \frac{\bar{V}^2}{2} \frac{\bar{V}}{H_e}}$$

Collision potential is a function of velocity. This suggests that a flocculator would perform poorly if the flow rate were decreases. I don't know if anyone has ever demonstrated that!

$$G\theta_e = \sqrt{\frac{H_e K_e Q}{2\nu W S}}$$

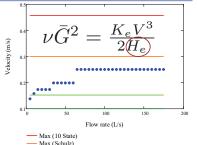
# Almost Real Designs (Flocculator exit depth of 2 m)

- What sets maximum channel width?
- What sets minimum channel width?
- Why this cycle of channel widths?

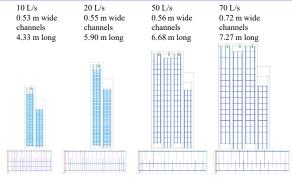


### Velocity guidelines?

- Why does V increase with flow rate?
- Why does V increase in steps?
- Why does V remain constant above 70 L/s?



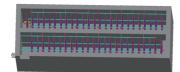
# **Design Scaling** (Design Engine version 7099)



#### More details



- The ports between channels should have the same cross sectional area as WS
- The number of chambers per canal (except in the last canal) is even - the number of baffles is
- The number of chambers in the last canal is odd - the number of baffles is even
- Why?



# Use a Pipe with orifices to make a flocculator for small flows (S=D)

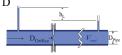
$$\nu \overline{G}^2 = \frac{K_e V^3}{2H}$$

$$v\overline{G}^2 = \frac{K_e V^3}{2H_e}$$
  $\overline{V} = \frac{4Q}{\pi D_{Pipe}^2}$  Continuity

$$H_e = \Pi_{\mathit{HS}} D_{\mathit{Pipe}}$$

Here we assume that S is like D

$$\nu \overline{G}^2 = \frac{K_e}{2\Pi_{HS} D_{Pipe}} \left( \frac{4Q}{\pi D_{Pipe}}^2 \right)^3$$



$$D_{Pipe} = \left[ \frac{K_e}{2\Pi_{HS} v \overline{G}^2} \left( \frac{4Q}{\pi} \right)^3 \right]^{\frac{1}{7}}$$

Round to nearest inner pipe diameter? Or round down to get higher velocities to prevent sedimentation?

# Estimate the orifice diameter





We need to estimate Ke!

- The head loss for these orifices spaced so closely may be less than what we calculate
  - Vena contracta may not be as severe for orifices that are close to the inner diameter of the pipe
  - Insufficient length for full expansion before next orifice

## Estimate the orifice diameter using the correct value of Ke

$$H_e = \Pi_{HS} D_{Pine}$$

$$h_e = K_e \frac{V^2}{2\sigma}$$

Need to find actual Ke given pipe diameter to develop target G

$$\begin{split} \overline{G} = \sqrt{\frac{gh_c}{\theta \nu}} & \text{Replace residence time with volume/Q} \\ \overline{G} = \sqrt{\frac{4gh_cQ}{\nu \pi H_c D_{pqe}^2}} & \longrightarrow h_c = \frac{\overline{G}^2 \nu \pi H_c D_{pqe}^2}{4gQ} \\ h_c = K_c \frac{V^2}{2g} = K_c \frac{16Q^2}{2g\pi^2 D_{pqe}^4} & \frac{\overline{G}^2 \nu \pi H_c D_{pqe}^2}{4gQ} & \longrightarrow K_c = \frac{\pi^3 D_{pqe}^7 \overline{G}^2 \nu \Pi_{HSMax}}{32Q^3} \end{split}$$

$$h_e = K_e \frac{V^2}{2g} = K_e \frac{16Q^2}{2g\pi^2 D_{ploc}^4} = \frac{G^2 v \pi H_e D_{ploc}^2}{4gQ} \longrightarrow K_e = \frac{\pi^3 D_{ploc}^7 G^2 v \Pi_{HSMet}}{32Q^3}$$

$$D_{Orifice} = \frac{D_{Pipe}}{\sqrt{\prod_{vc} \left(\sqrt{K_{e_{orifice}}} + 1\right)}}$$

# Use a Pipe with orifices to make a flocculator for small flows (H=D)

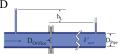
$$\nu \overline{G}^2 = \frac{K_e V^3}{2H}$$

$$v\overline{G}^2 = \frac{K_e V^3}{2H_e}$$
  $\overline{V} = \frac{4Q}{\pi D_{Pipe}^2}$  Continuity

$$H_e = D_{Pipe}$$

Here we assume that S is like D

$$\nu \overline{G}^2 = \frac{K_e}{2D_{Pine}} \left( \frac{4Q}{\pi D_{Pine}^2} \right)^3$$

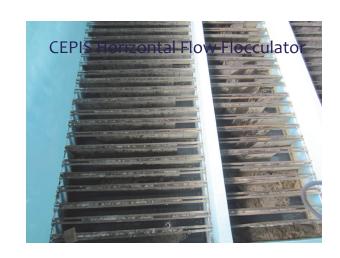


$$D_{Pipe} = \left[ \frac{K_e}{2\nu \overline{G}^2} \left( \frac{4Q}{\pi} \right)^3 \right]^{\frac{1}{7}}$$

Round to nearest inner pipe diameter? Or round down to get higher velocities to prevent sedimentation?

# An interesting design No this wasn't AguaClara...



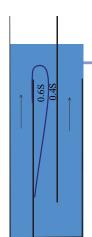


#### A few Reflections

- Floc size doesn't seem to be a significant constraint for flocculator design
- We may increase energy dissipation rate significantly as we experiment with maintaining small flocs that primary particles can attach to
- Our broad goal is to maximize performance at minimum cost. Thus cost minimization may be an important constraint for setting the target velocity gradient.
- Maintaining the flocs in suspension is another important constraint

#### **Reflection Questions**

- How does the collision potential in a flocculator change with flow rate?
- What is the ratio of  $G_{Max}$  to  $\bar{G}$  for well designed hydraulic flocculators?
- Why might mechanical flocculators break more flocs than hydraulic flocculators?



# **Reflection Questions**

- What are some alternate geometries?
- How else could you generate head loss to create collisions?



#### **Reflection Questions**

- What is the relationship between potential energy loss and the average velocity gradient in a flocculator?
- How did AguaClara get around the 45 cm limitation?
- How does the non uniformity of ε (or G) influence efficiency of energy use?



#### Conclusions

- Energy dissipation rate determines the spacing of the baffles.
- Energy is used most efficiently to create collisions when the energy dissipation rate is uniform. Therefore H/S between 3 and 6 is best.
- Collision potential is a function of geometry and a function of flow rate