

CALCULATION OF LIVE LOAD REACTION FOR PIER SUBSTRUCTURE FOR SIMPLY SUPPORTED SPANS OF A TWO LANE BRIDGE STRUCTURE

Centre line of pier w.r.t. the bearings :-

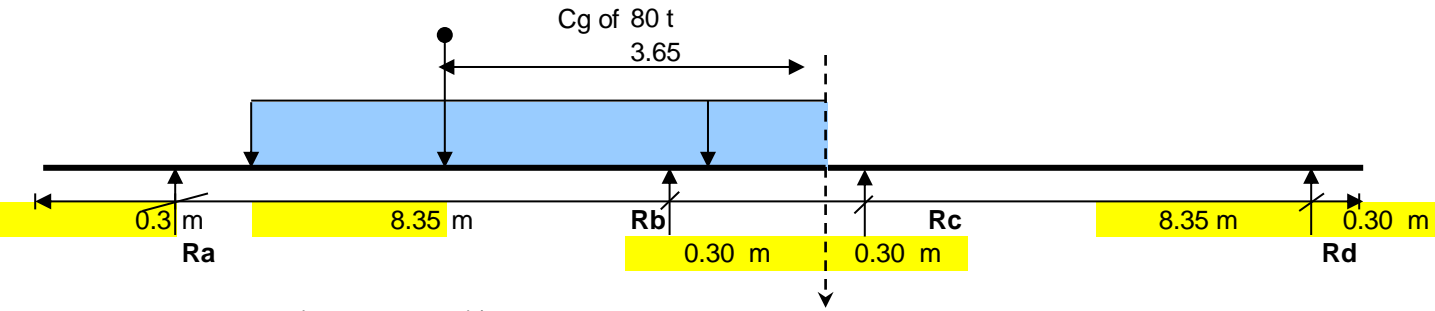
Rb = 0.3 m  
Rc = 0.3 m

Reaction has been calculated for the following cases

- 1. One lane of class 70-R(W)
- 2. One lane of class - A
- 3. Two lane of class - A
- 4. Three lane of class - A
- 5. One lane of class 70-R(W) + One lane of class - A

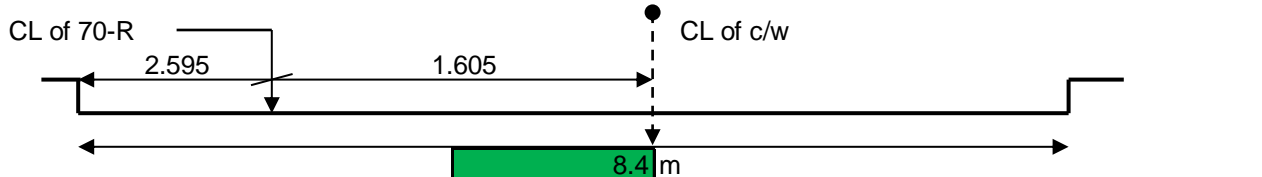
Condition A: MAXIMUM LONGITUDINAL MOMENT CASE

Case 1: One lane of class 70-R(W)



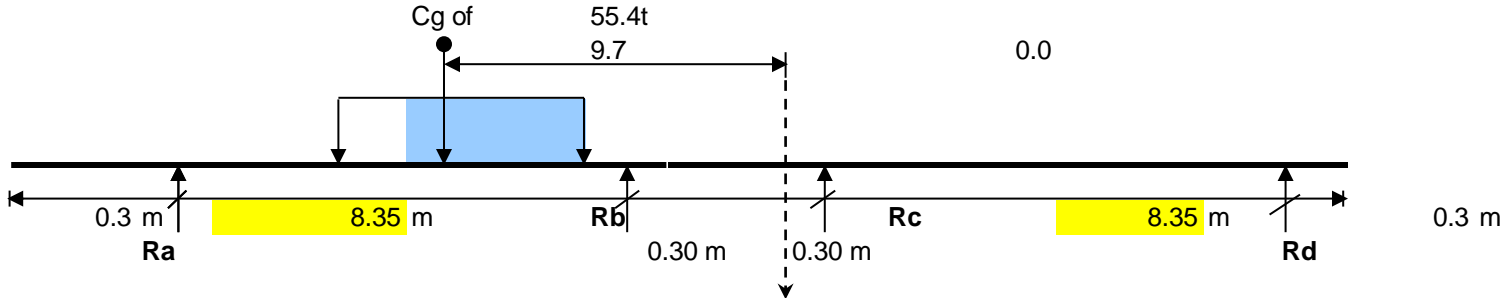
Rb =  $80 \times (8.35 - 3.65 + 0.3) / 8.35$  = 60.4 t  
Rc = 0.0 t  
Ra = 19.6 t  
Vert.Reaction = 60.4 + 0 = 60.4 t  
Braking Force, B =  $0.2 \times 80$  = 16.0 t  
Dead load reaction on the pier, Rg = 485.0 t

Value of "  $\mu$  " = 0.00  
Horizontal force due to temperature, T =  $\mu \times (Rg + Ra)$  = 0.0 t  
Design horizontal force is higher of either ( B/2+T ) or ( B-T ) = 16.0 t  
( neglecting shear rating of elastomeric bearing in the adjacent span, which is on the conservative side )



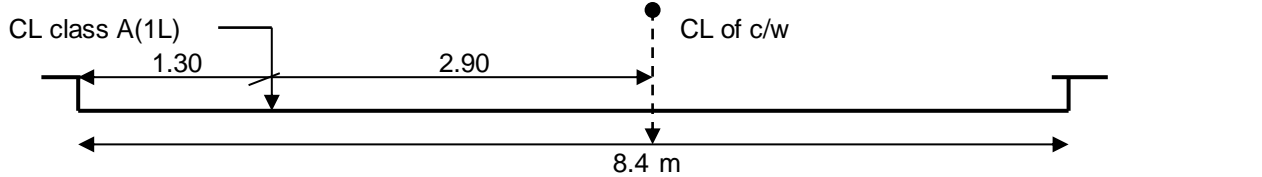
Transverse eccentricity = 2.905 m  
Transverse moment =  $2.905 \times (60.4 + 0)$  = 175.3 t.m  
Long. moment =  $60.4 \times 0.3 - 0 \times 0.3$  = 18.1 t.m  
Long. Eccentricity ( for input ) = 0.300 m

B) One lane of class-A



Rc =  $0 \times (8.35 - 0.3) / 8.35$  = 0.0 t  
Rb =  $55.4 \times (8.35 - 9.7 + 0.3/2) / 8.35$  = 7.0 t  
Ra = 48.4 t  
Vert.Reaction = 48.4 + 7 = 55.4 t  
Braking Force, B =  $0.2 \times (0 + 55.4)$  = 11.1 t  
Dead load reaction on the pier, Rg = 485.0 t

Value of "  $\mu$  " = 0.00  
Horizontal force due to temperature, T =  $\mu \times (Rg + Ra)$  = 0.0 t  
Design horizontal force is higher of either ( B/2+T ) or ( B-T ) = 11.1 t  
( neglecting shear rating of elastomeric bearing in the adjacent span, which is on the conservative side )



Transverse eccentricity = 2.90 m  
Transverse moment =  $2.9 \times 55.4$  = 160.7 t.m  
Long. moment =  $7 \times 0.3 - 0 \times 0.3$  = 2.1 t.m  
Long. Eccentricity ( for input ) = 0.038 m

Case 3 : Two lane of class-A

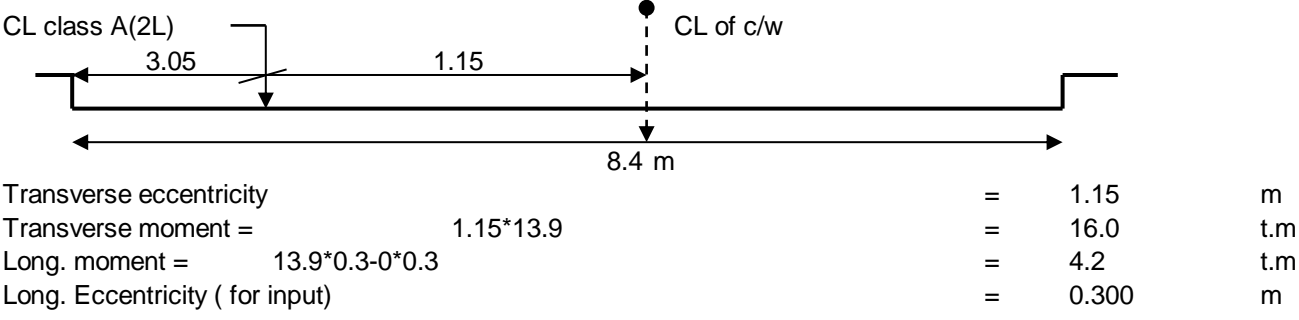
Rc =  $2 \times 0$  = 0.0 t  
Rb =  $2 \times 7$  = 13.9 t  
Ra = 96.9 t  
Vert.Reaction = 0 + 13.9 = 13.9 t  
Braking Force(For single lane only) = 11.1 t  
Dead load reaction on the pier, Rg = 485.0 t  
Value of "  $\mu$  " = 0.00  
Horizontal force due to temperature, T =  $\mu \times (Rg + Ra)$  = 0.0 t

span	load	cg
4.42	51	1.93
5.79	68	2.895
7.92	80	3.65
9.44	92	4.4
13.4	100	5.12

8.78

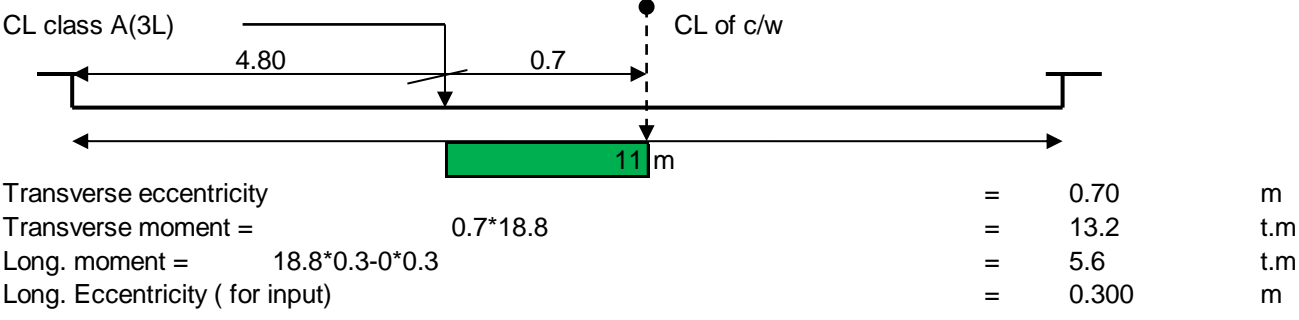
SPAN	LOAD	CG
5.5	29.6	1.73
8.5	36.4	2.99
11.5	43.2	4.33
14.5	50	5.71
24	50	5.71
8.78		

Design horizontal force is higher of either ( B/2+T ) or ( B-T ) = 11.1 t  
( neglecting shear rating of elastomeric bearing in the adjacent span, which is on the conservative side )



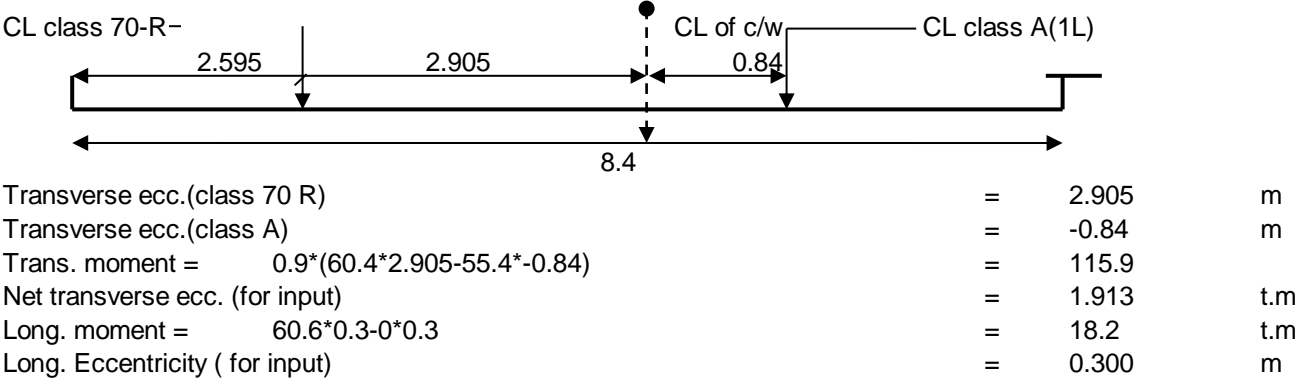
Case 4 : Three lane of class-A

Rc = 90% of 3\*0 = 0.0 t  
Rb = 90% of 3\*7 = 18.8 t  
Ra = 61.1 t  
Vert.Reaction = 0 + 18.8 = 18.8 t  
Braking Force, B = (0.2)\*55.4+0.05\*55.4 = 13.9 t  
(5% extra taken for third lane)  
Dead load reaction on the pier , Rg = 485.0 t  
Value of "  $\mu$  " = 0.00  
Horizontal force due to temperature, T =  $\mu$ \*(Rg+Ra) = 0.0 t  
Design horizontal force is higher of either ( B/2+T ) or ( B-T ) = 13.9 t  
( neglecting shear rating of elastomeric bearing in the adjacent span, which is on the conservative side )



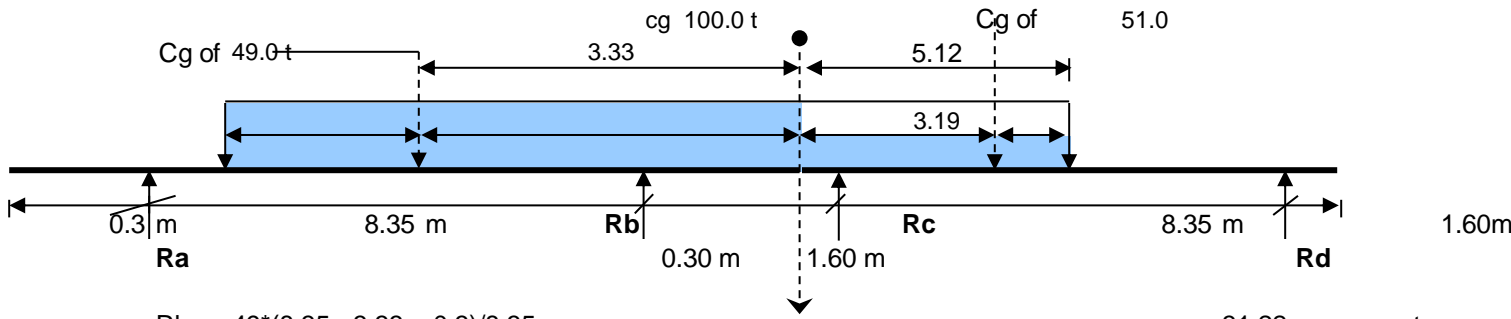
Case 5 : One lane of class-70R(W)+One lane of class-A

Rc = 90% of(0+0) = 0.0 t  
Rb = 90% of(6.97+60.36) = 60.6 t  
Ra = 61.3 t  
Braking Force = 16 + 5% of 55.4 = 18.8 t  
(5% extra taken for class A)  
Dead load reaction on the pier , Rg = 485.0 t  
Value of "  $\mu$  " = 0.00  
Horizontal force due to temperature, T =  $\mu$ \*(Rg+Ra) = 0.0 t  
Design horizontal force is higher of either ( B/2+T ) or ( B-T ) = 18.8 t  
( neglecting shear rating of elastomeric bearing in the adjacent span, which is on the conservative side )

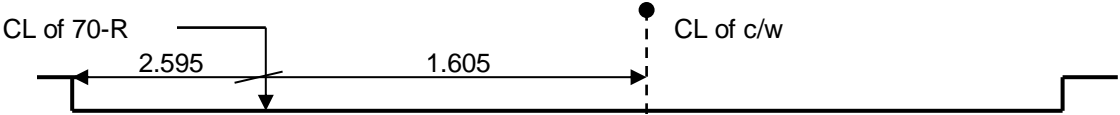


Condition B: MAXIMUM TRANSVERSE MOMENT / REACTION CASE

CASE 1: ONE LANE OF CLASS 70-R(W)



Rb = 49\*(8.35 - 3.33 + 0.3)/8.35 = 31.22 t  
Rc = 51\*(8.35-3.19+1.6)/8.35 = 21.74 t  
Ra = 27.3 t  
Vert. Reaction = 31.2 + 21.7 = 53.0 t  
Braking Force, B = 0.2\*100 = 20.0 t  
Dead load reaction on the pier , Rg = 485.0 t  
Value of "  $\mu$  " = 0.00  
Horizontal force due to temperature, T =  $\mu$ \*(Rg+Ra) = 0.0 t  
Design horizontal force is higher of either ( B/2+T ) or ( B-T ) = 20.0 t  
( neglecting shear rating of elastomeric bearing in the adjacent span, which is on the conservative side )



first span			
SPAN	LOAD	CG	
8.28	49	3.33	
5.04	58	2.18	
8.95			

second span			
SPAN	LOAD	CG	
4.4	34	3.715	
5.12	51	3.19	
11.55			

second span			
SPAN	LOAD	CG	
3	80	3.65	
4.52	92	4.4	
8.48	100	5.12	
24	100	5.12	
8.95			

first span			
SPAN	LOAD	CG	
3	17	0.87	
4.52	29	1.75	
8.48	41	2.56	
24	49	3.53	
8.95			

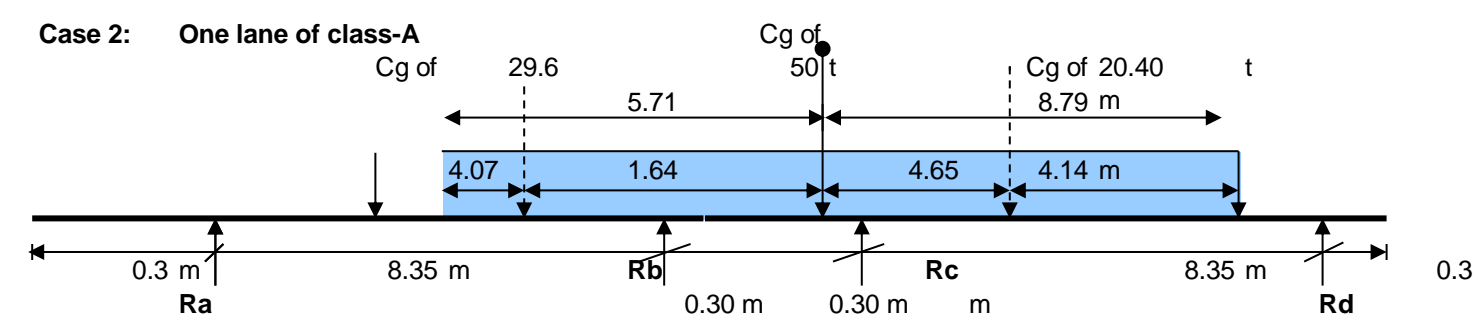


Diagram illustrating the cross-section of the bridge deck. The total width is 11.0 m. The centerline of the deck (CL of c/w) is marked. The centerline of the class A lane (CL class A(1L)) is marked. The distance from the CL class A(1L) to the CL of c/w is 4.2 m. The distance from the left edge of the deck to the CL class A(1L) is 1.30 m.

**Case 3 : Two lane of class-A**

**Case 4 : Three lane of class-A**

CL class A(3L) 4.80 0.7 CL of c/w 11 m

**Case 5 :        One lane of class-70R(W)+One lane of class-A**

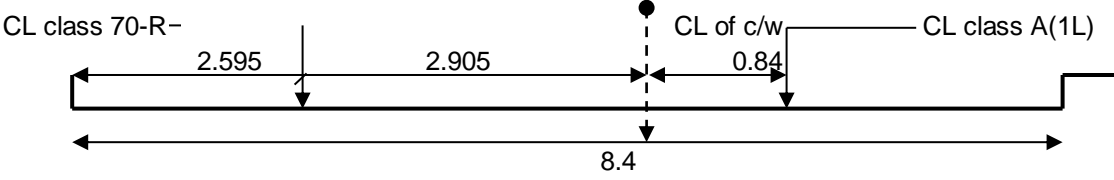
two span length	load	cg6.8 end	cg2.7 end
9	27.2	4.5	4.5
13.3	38.6	7.1	6.2
14.5	50	8.79	5.71
18.7	52.7	9.24	9.46
18.8	55.4	9.71	9.09
17.6	55.4	9.71	9.09

load	Span2load	cg 6.8	load	Span2 load	cg 6.8
27.2	13.6	1.5	55.4	27.2	4.5
38.6	20.4	4.14	52.7	27.2	4.5
50	20.4	4.14	50	20.4	4.14
52.7	27.2	4.5	38.6	20.4	4.14
55.4	27.2	4.5	27.2	13.6	1.5
			span2	8.78	

load 1	Cg 2.7 end
13.6	1.5
18.2	1.81
25.5	3.4
28.2	4.07

load 1	Cg 2.7 end
28.2	4.07
25.5	3.4
29.6	1.73
18.2	1.81

Braking Force = 16 + 5% of 55.4 = 18.8 t  
(5% extra taken for class A)  
Dead load reaction on the pier , Rg = 485.0 t  
Value of "  $\mu$  " = 0.00  
Horizontal force due to temperature, T =  $\mu \cdot (Rg+Ra)$  = 0.0 t  
Design horizontal force is higher of either ( B/2+T ) or ( B-T ) = 18.8 t  
( neglecting shear rating of elastomeric bearing in the adjacent span, which is on the conservative side )



Transverse ecc.(class 70 R) = 2.905 m  
Transverse ecc.(class A) = -0.84 m  
Trans. moment =  $0.9 \cdot (60.4 \cdot 2.9 - 0 \cdot 0.8)$  = 112.4  
Net transverse ecc. (for input) = 1.426 t.m  
Long. moment =  $50.5 \cdot 0.3 - 28.4 \cdot 0.3$  = 6.6 t.m  
Long. Eccentricity ( for input) = 0.084 m