DESIGN OF ABUTMENT FOOTING

Name Of Work :- Construction of Submersible Bridge on ON KHERWARA - JAWAS - SUVERI ROAD IN KM 9/000, ACROSS RIVER SOM REDISTRIBUTION OF PRESSURE

FOR WIND AT SERVICE CONDITION

Length of footing	l _f	17.00	m			
Width of Footing	l _b	8.05	m			
Width of Abutment just above footing	ď	6.05	m			
Vertical Load	Р	1004.04	kN			
Longitudinal Moment	M _e	3661.12	kN-m			
Transverse Moment	M _b	0.00	kN-m			
Area in Tension = y x l _b	IVID	0.00	0.00 m ²	0.00.0/		
, -				0.00 %		
Maximum Pressure before Redistribution			160.06 kN/n			
Maximum Pressure After Redistribution :		160.06 kN/n	= -			
Maximum Stress at Edge of Pier		160.06 kN/n	n ²			
Distance From Face of Pier to the Edge			1.00 m			
Stress at the Edge of Pier			140.18 kN/m²			
Average Stress on Cantilevered Area			150.12 kN/m²			
Area of the Cantilever Portion			1.00 m ²			
Distance of Centroid of the Stress in		0.51 m				
Cantilever Portion						
Moment about the Face of Pier			76.72 kN-m			
CONCRETE GRADE		M-25				
FOR THIS GRADE ocbc			10 N/mm2			
m			9.33			
σst			200			
factor k			0.318			
j			0.894			
R			1.422			
Effective Depth Required			232 mm			
Adopt Total Depth			1000 mm			
Cover			50 mm			
Assume Bar Dia			16 mm			
Keeping A Cover Of 50 mm Effective Depth			942 mm			
Adopt Effective Depth			942 mm			
Steel Required Ast			456 mm ²			
Area Of One Bar			201 mm ²			

1/{2} cross section ABUTMENT FOOTING DESIGN

Spacing S Provide Bars Of Dia And Spacing Area Of Distribution Steel Dia Of Bar For Distribution Steel	16	mm 150 1884	mm) mm ¹ mm ²) mm	Adopt spacing as 150 mm		
Area Of One Bar In Distribution Rei	nforcement	314	mm²			
Using The Bars Spacing Required		167	' mm			
Provide Bars Of Dia And Spacing	16	mm 160) mm	Adopt spacing as 150 mm		
Provide Bars Of Dia And Spacing for	or					
Top Main Steel	12	mm 150) mm			
Provide Bars Of Dia And Spacing for	or					
Top Distribution Steel	12	mm 150) mm			
CHECK FOR SHEAR	(As per IRC 21-1987 Cl. 304.7)					
Critical Section is at a distance equal t	o effective depth from p	ier face 942	2 mm			
Section of Shear from end of pier		0.06	S m			
Maximum Stress at Edge of Pier		160.06	kN/m ²			
Stress at the Section for Shear Check		157.48	kN/m ²			
Average Stress on Cantilevered Area		158.77	kN/m²			
Shear Force		9.21	kN			
V=V' + M/d tanB	(B=0) Hence V =V'	(B=0) Hence V =V'				
Actual Shear Stress	, ,	0.01	N/mm ²			
Percentage Steel	100As/bd	0.14	-			
Tc		0.23	N/mm ²			
k=1						
Permissble Shear Stress = k Tc		0.23	N/mm ²			
		< Actual Shear St		Shear		
		Reinforcement sh				
Dia Of two Legged Stirrups			5 mm			
Area Of One Bar In Distribution Rei	201	mm^2				
Using The Bars Spacing Required s		3 mm				
Provide Bars Of Dia And Spacing		mm 150) mm	Adopt spacing as 150 mm		

2/{2} cross section ABUTMENT FOOTING DESIGN