

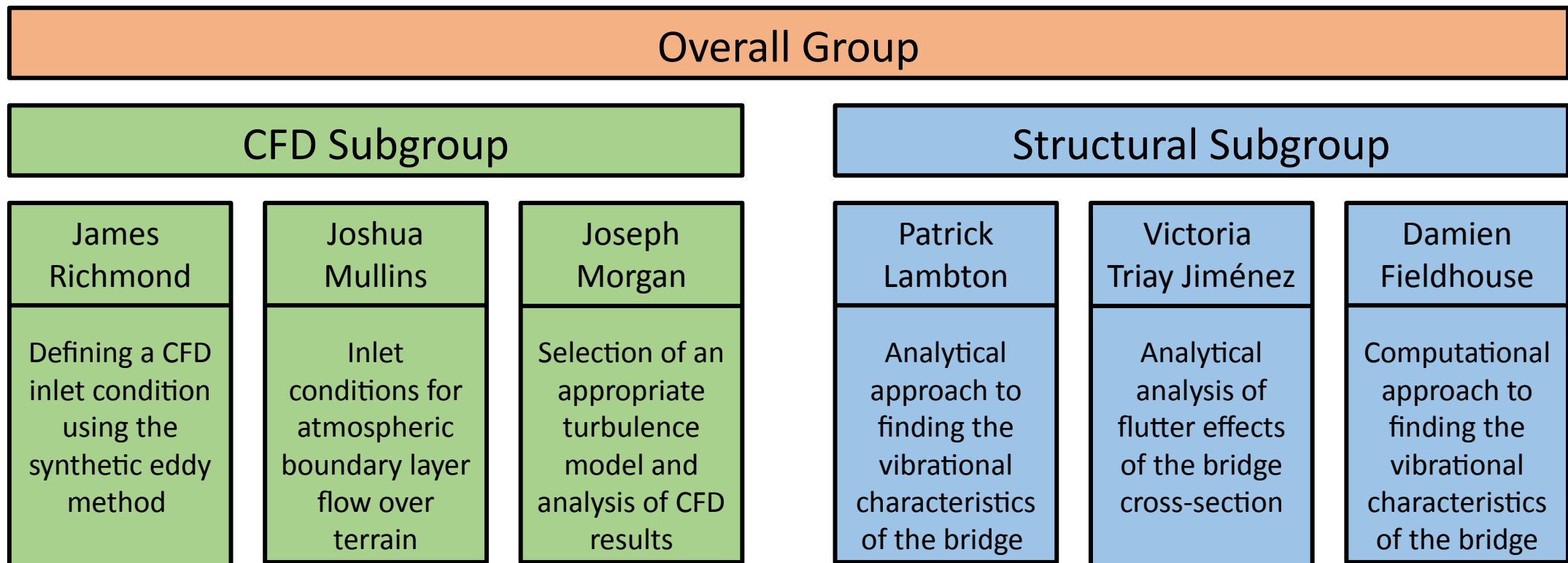
Wind Induced Vibrations on the Humber Bridge

4th Year MEng Group Project

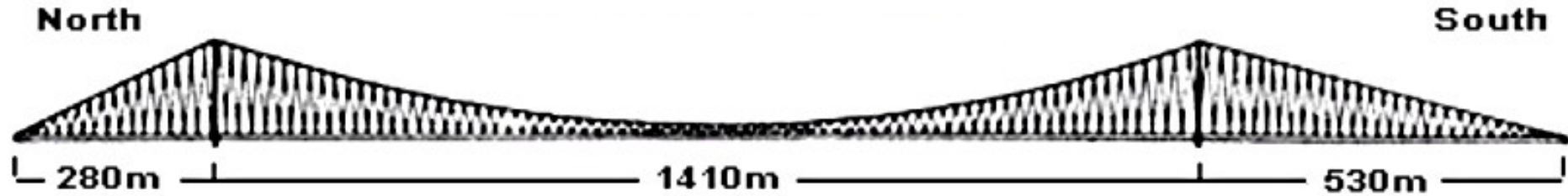
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J. Richmond, V. Triay Jiménez**

Project Overview

- Long span bridges can be susceptible to wind induced vibrations
- The Humber Bridge was used as a case study



Humber Bridge



- Steel box girder construction
- 2220m total length
- 7th longest single span bridge in the world
- Longest ever when constructed
- Length of bridge and relative low stiffness can lead to aero-elastic effects

Structural Sub-group

- Vibrational characteristics
 - Analytical Model
 - Computational model
 - Mode shapes
 - Natural frequencies
- Critical Flutter wind speed
 - Analytical Model
 - Aerodynamic Derivatives
 - Critical Wind Speed



Analytical Model

- Aimed to find the mode shapes and natural frequencies
- Inherent complexity of the structure doesn't allow conventional analysis
- Direct stiffness method can be used to simplify the problem
- Breaking the problem down into elements of known stiffness
- Each element matrix combined into global matrix
- Global Mass matrix created from equivalent masses at each point

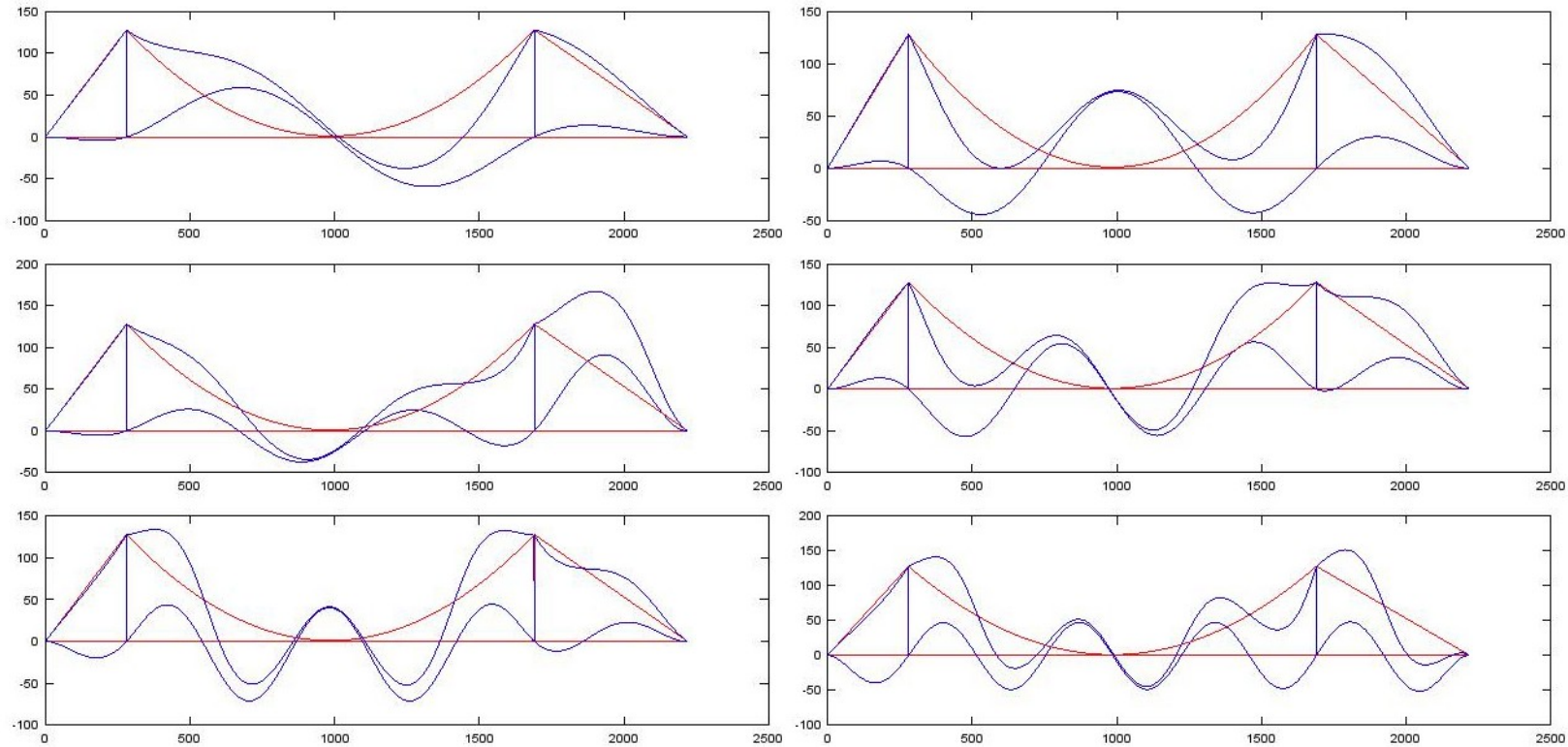
Analytical Model

- Each model took into account different degrees of freedom for each element
- First model only took into account vertical degrees of freedom in the central span
- Simple model used to facilitate learning
- First vertical model 668 degrees of freedom
- Models became progressively more complex
- Final vertical model 1957 degrees of freedom
- All calculations carried out using GNU Octave

Issues

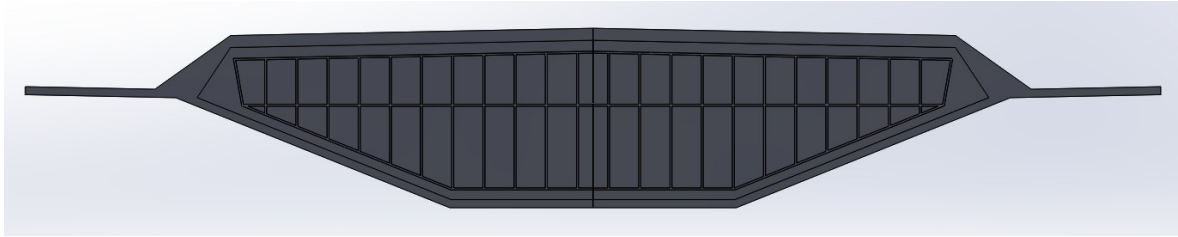
- Modelling of rotational inertia difficult
 - Non uniform rotation
 - Some rotations already accounted for by vertical deflections
- For lateral and torsional vibrations a full 3D model was necessary
 - Taking into account main cable deflections properly
 - Too complex to model in given timeframe

Results

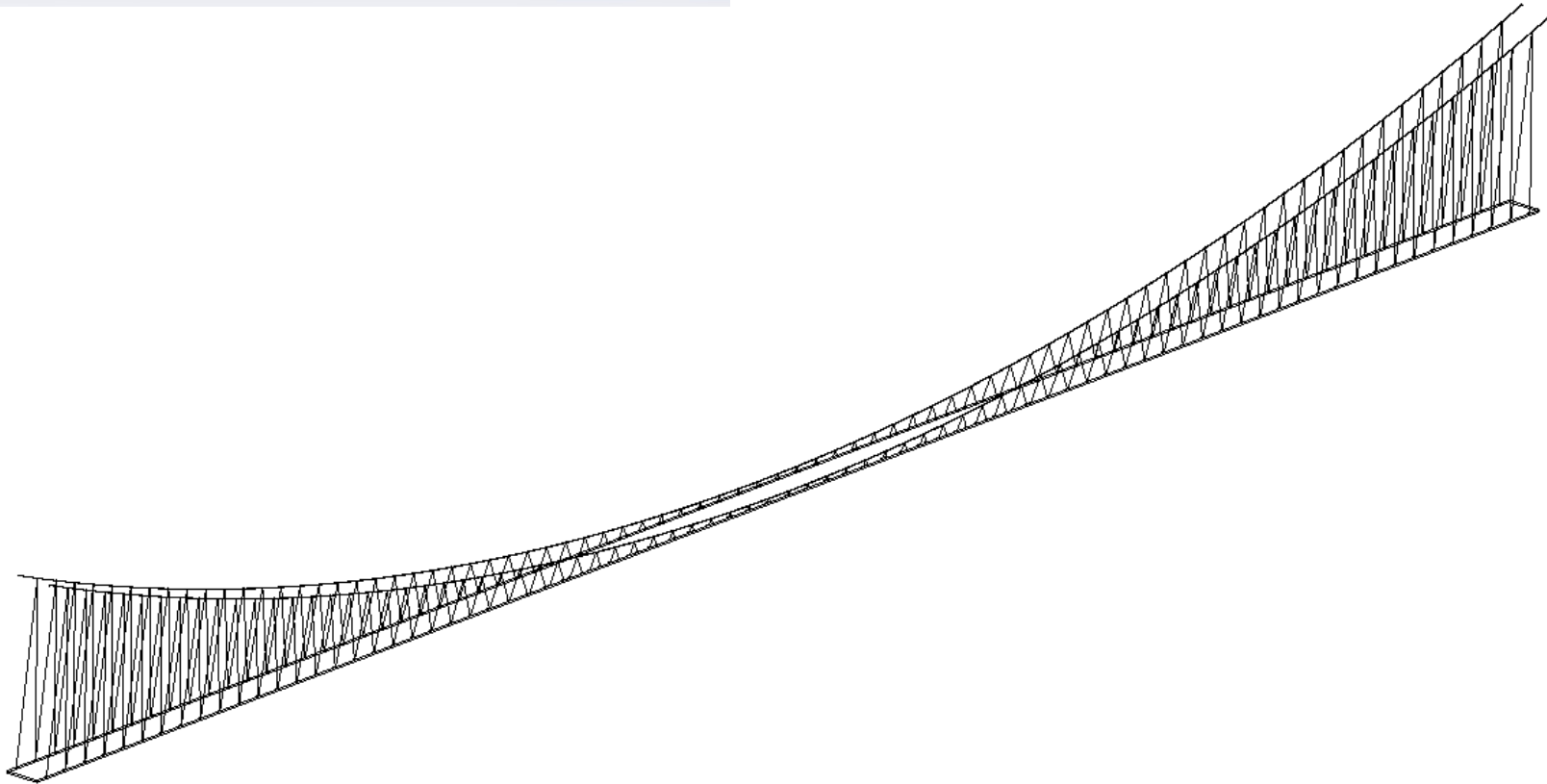


Mode number	Natural Frequency / (Hz)
1	0.125
2	0.171
3	0.204
4	0.238
5	0.282
6	0.333

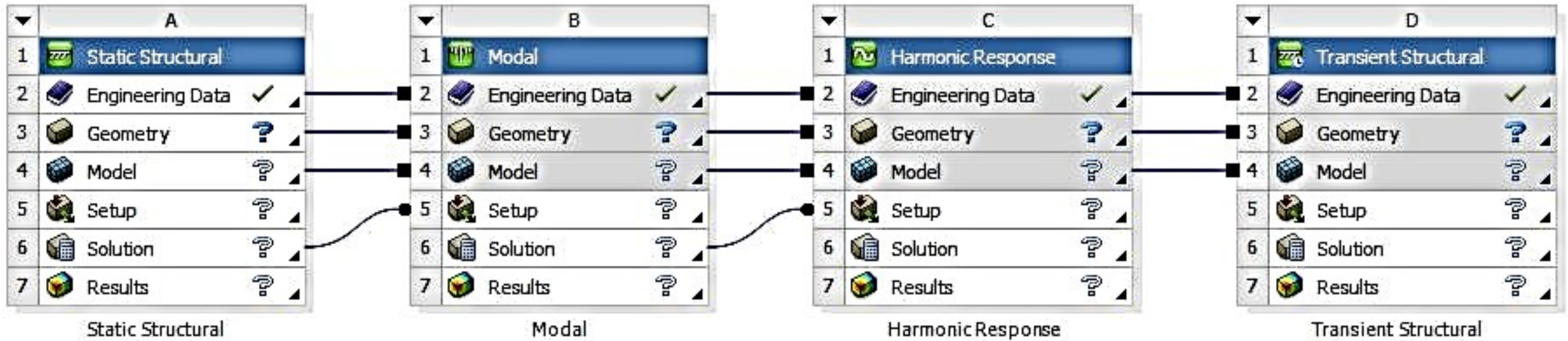
CAD Model



$$\delta_{\max} = \frac{Pl^3}{3EI}$$



Methodology



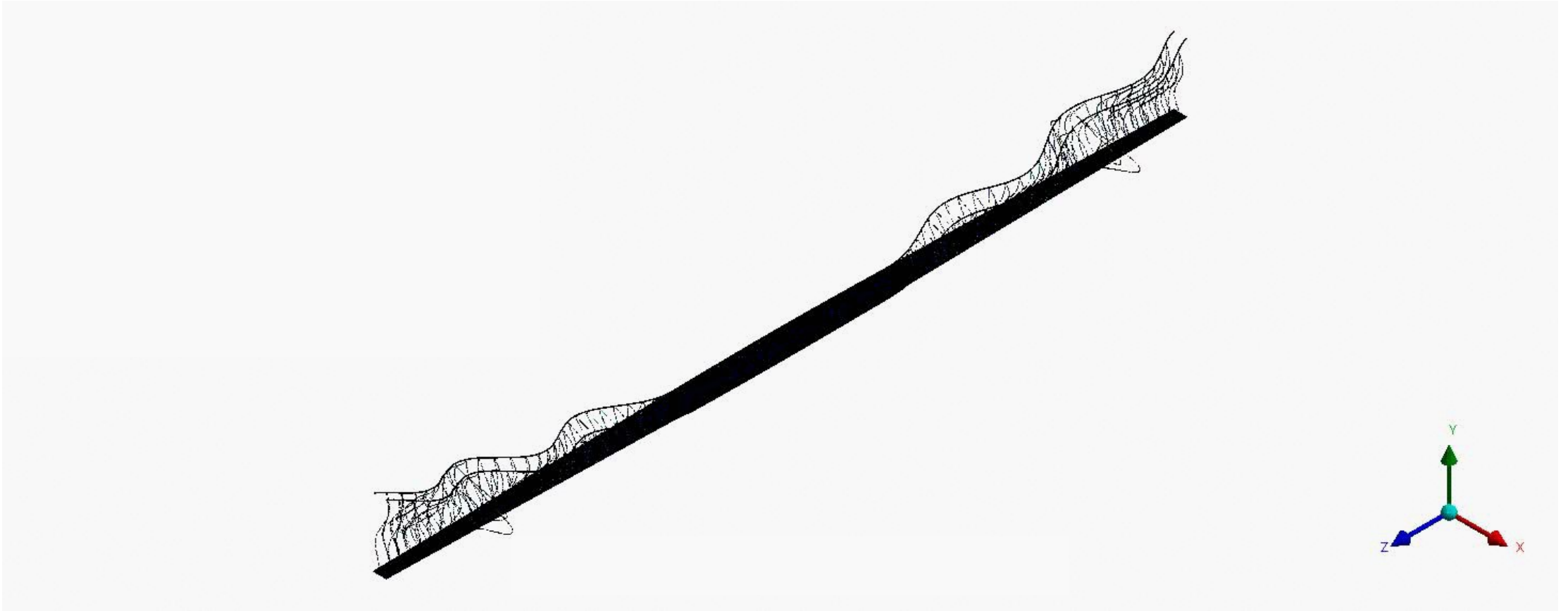
- Non-Transient
- Steady Loads

- Natural Frequencies
- Mode Shapes

- Frequency Response
- Phase Response

- Time Dependent Loading
- Dynamic Response

First Mode of Torsional Vibration

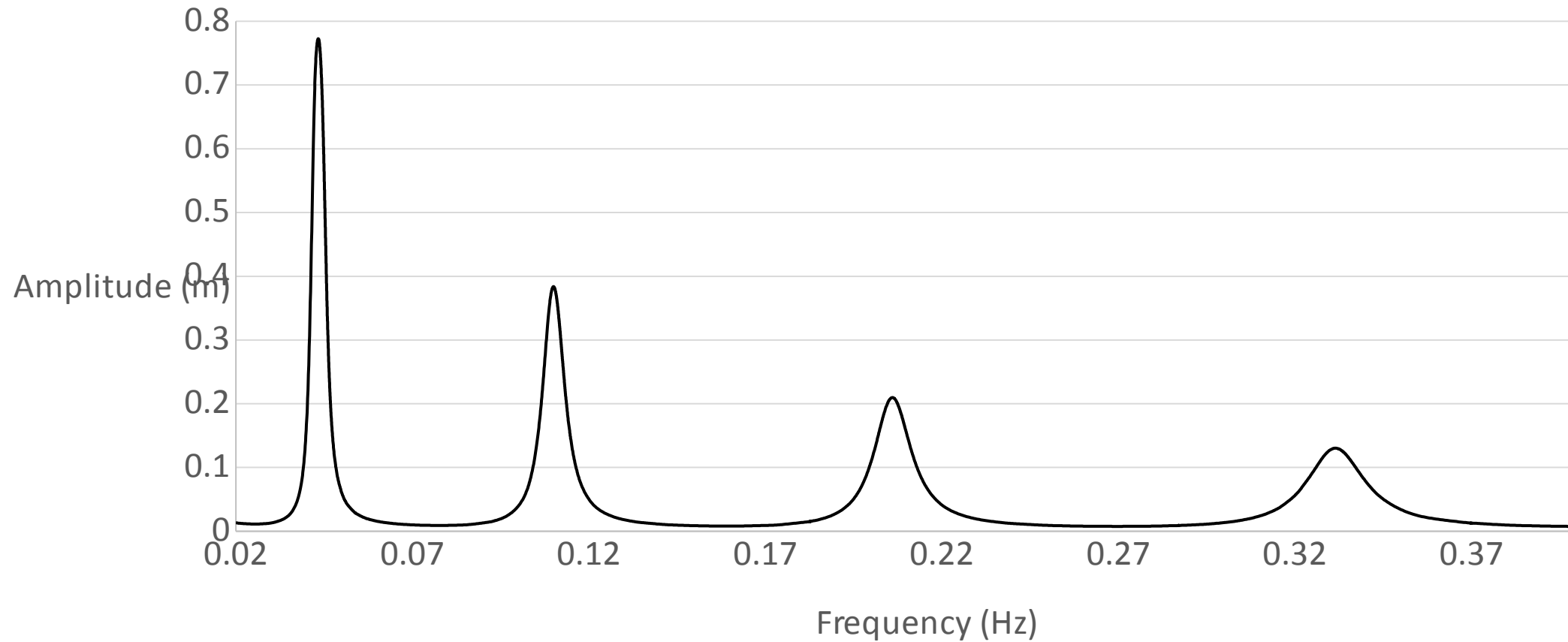


Natural Frequencies

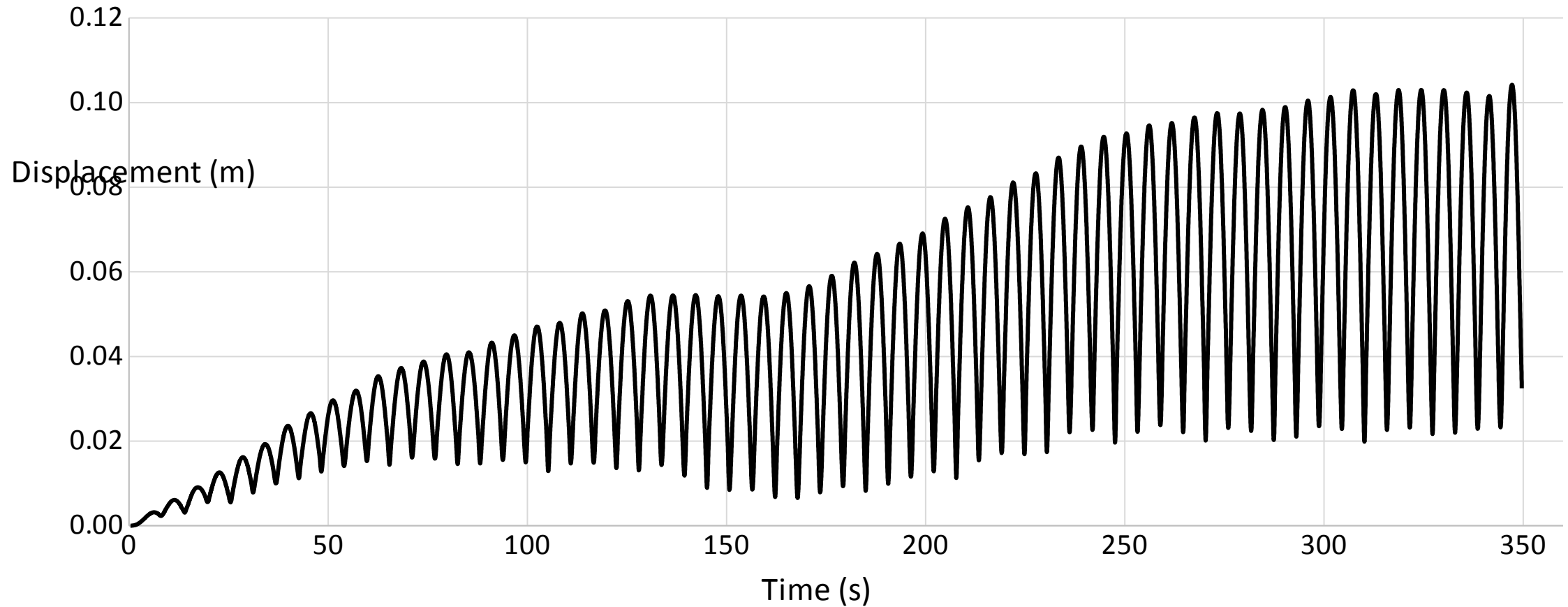
Mode	Vertical			Torsional			Lateral		
	Lambton	Fieldhouse	Brownjohn	Lambton	Fieldhouse	Brownjohn	Lambton	Fieldhouse	Brownjohn
1	0.125	0.046	0.116	0.564	0.168	0.308	0.278	0.057	0.056
2	0.171	0.088	0.149	1.127	0.276	0.479	0.287	0.160	0.141
3	0.204	0.096	0.172	1.499	0.366	0.643			
4	0.238	0.099	0.215	1.691	0.422	0.848			
5	0.282	0.149	0.240						
6	0.333	0.175	0.309						

Calculated and experimental frequencies associated with each mode (Hz)

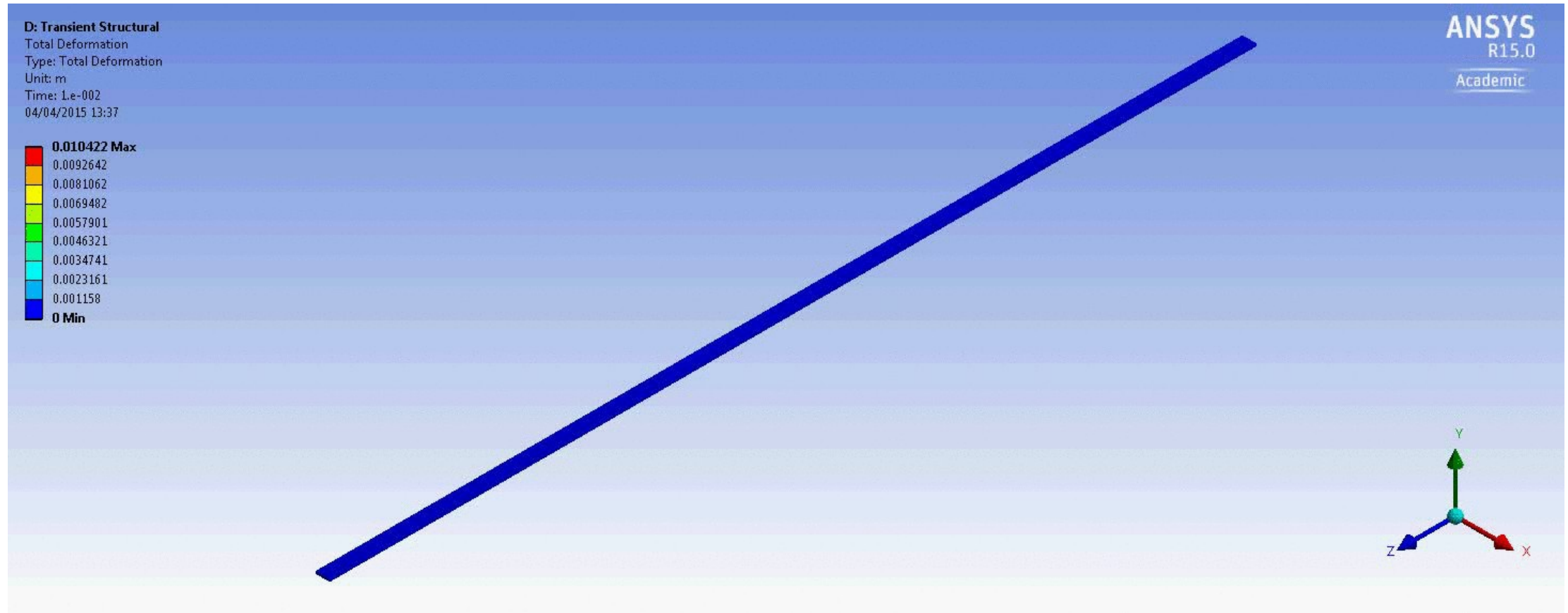
Frequency Response



Transient Response

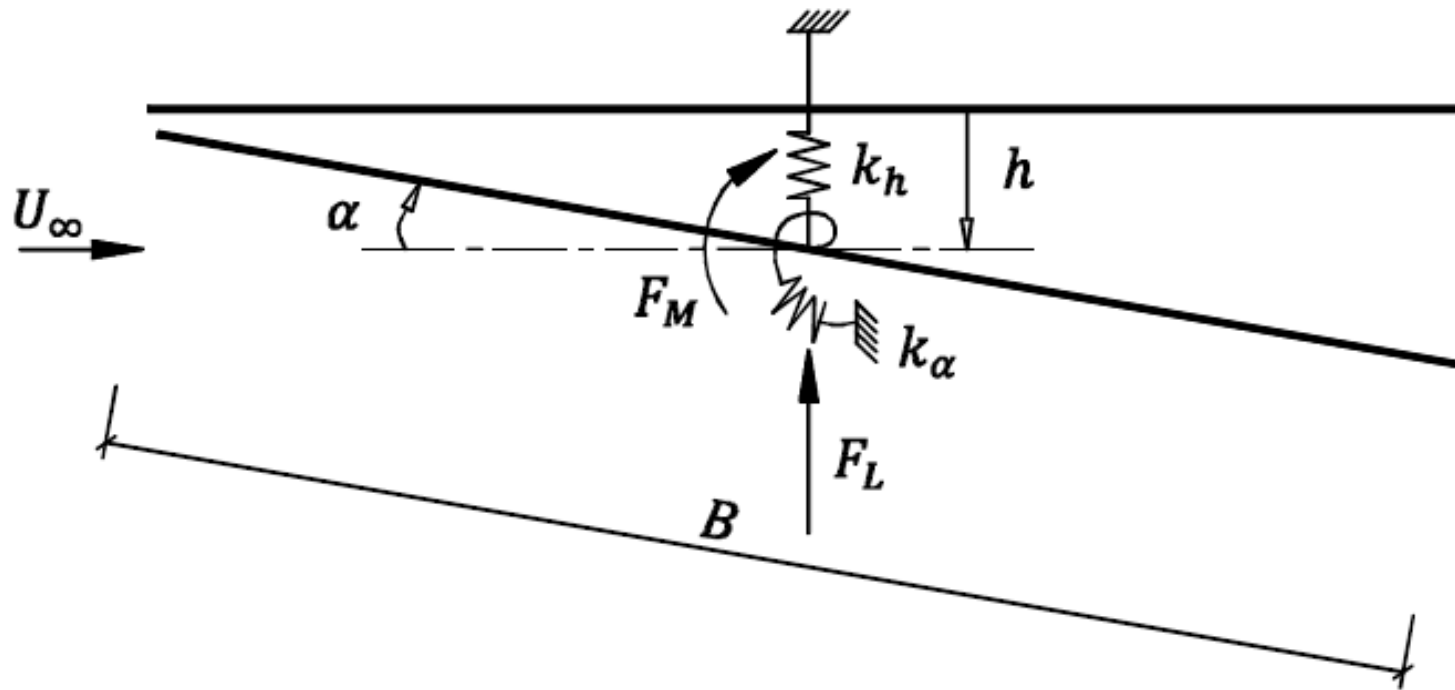


Transient Response



Flutter

- Type of aerodynamic effect on bluff bodies
- Coupled motion between vertical and torsional oscillations
- Critical condition occurs at zero damping



Analytical Approach

Equation of motion

Theodorsen theory

Scanlan theory

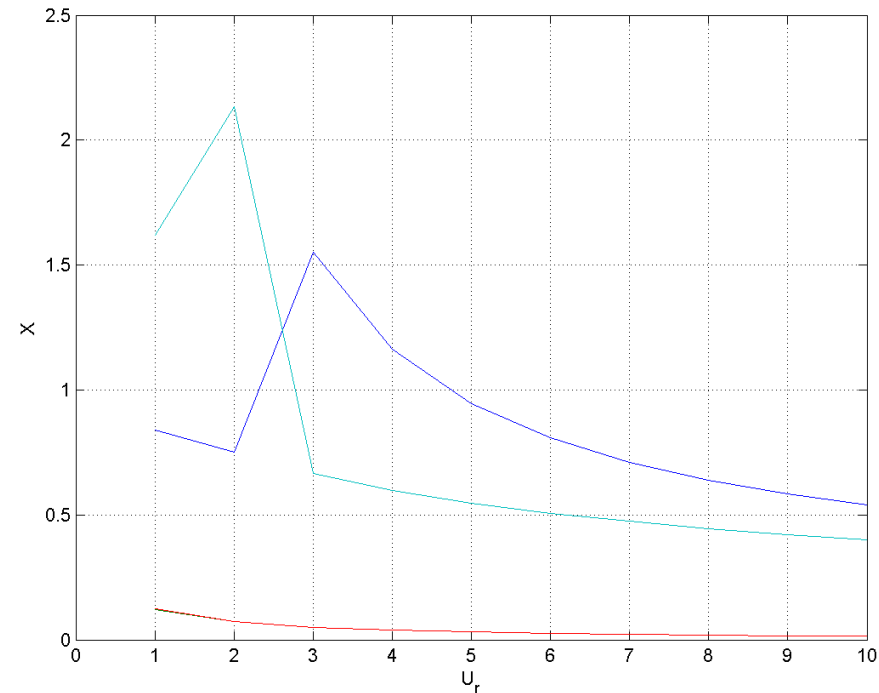
Analysis and Results

Theodorsen Circulatory Function

Aerodynamic Derivatives

Critical Wind Speed

- Aerodynamic derivatives are obtained from equating Theodorsen and Scanlan theories
- Resolving the system of equations formed with the equations of motion and Scanlan self-excited force equations
- The solution for varying reduced frequencies is plotted and the critical wind speed is found where they intersect
- Obtaining a critical wind speed of 20m/s



Structural Concluding Remarks

- Produced both an analytical and computational model of the Humber Bridge
- For each model mode shapes and natural frequencies were found
- A transient response to wind induced vibrations was obtained
- A frequency and phase response of the bridge was calculated
- A flutter critical wind speed was found by using the first natural frequencies of the computational model

CFD Aim

- To accurately model wind flow in order to derive forces
- Vortex shedding is a cause of some wind induced vibrations
- Vortices impart a force on an object which they are shed from
- Vortex shedding can be periodic - causing oscillating forces
- Aim of the subgroup was to determine the frequencies of oscillating forces

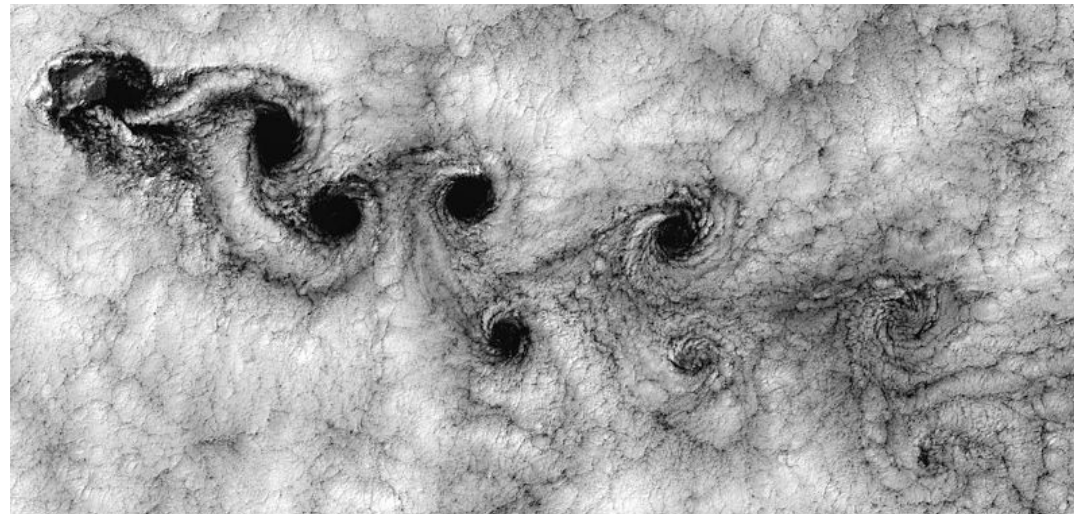


Image credit – Image by Landsat 7 of clouds off of the Chilean coast near Juan Fernandez Islands. The clouds are showing alternating vortex shedding. (1999).

Computational Fluid Dynamics

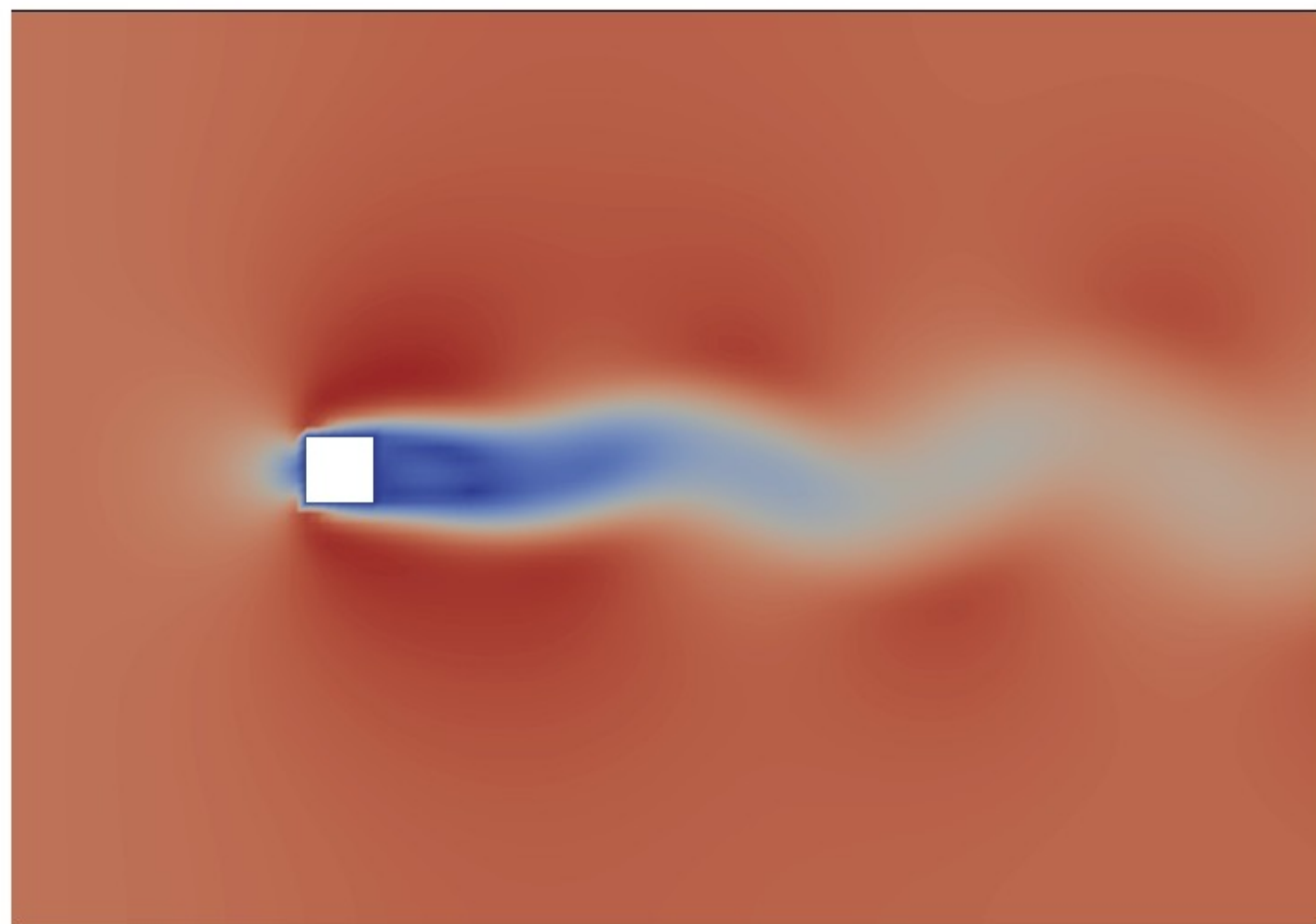
- Developed a CFD simulation of turbulent atmospheric wind flow over the Humber Bridge deck
- Used open source CFD software – OpenFOAM.

Parameters that were considered:

- Needed to capture time-dependent flow phenomena e.g. vortex shedding
- Required an inlet condition representing realistic atmospheric turbulence
- Required a mean inlet profile for realistic air flow

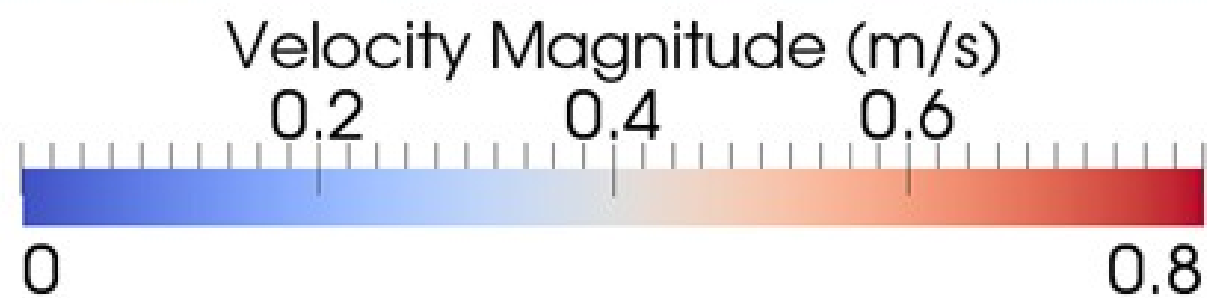
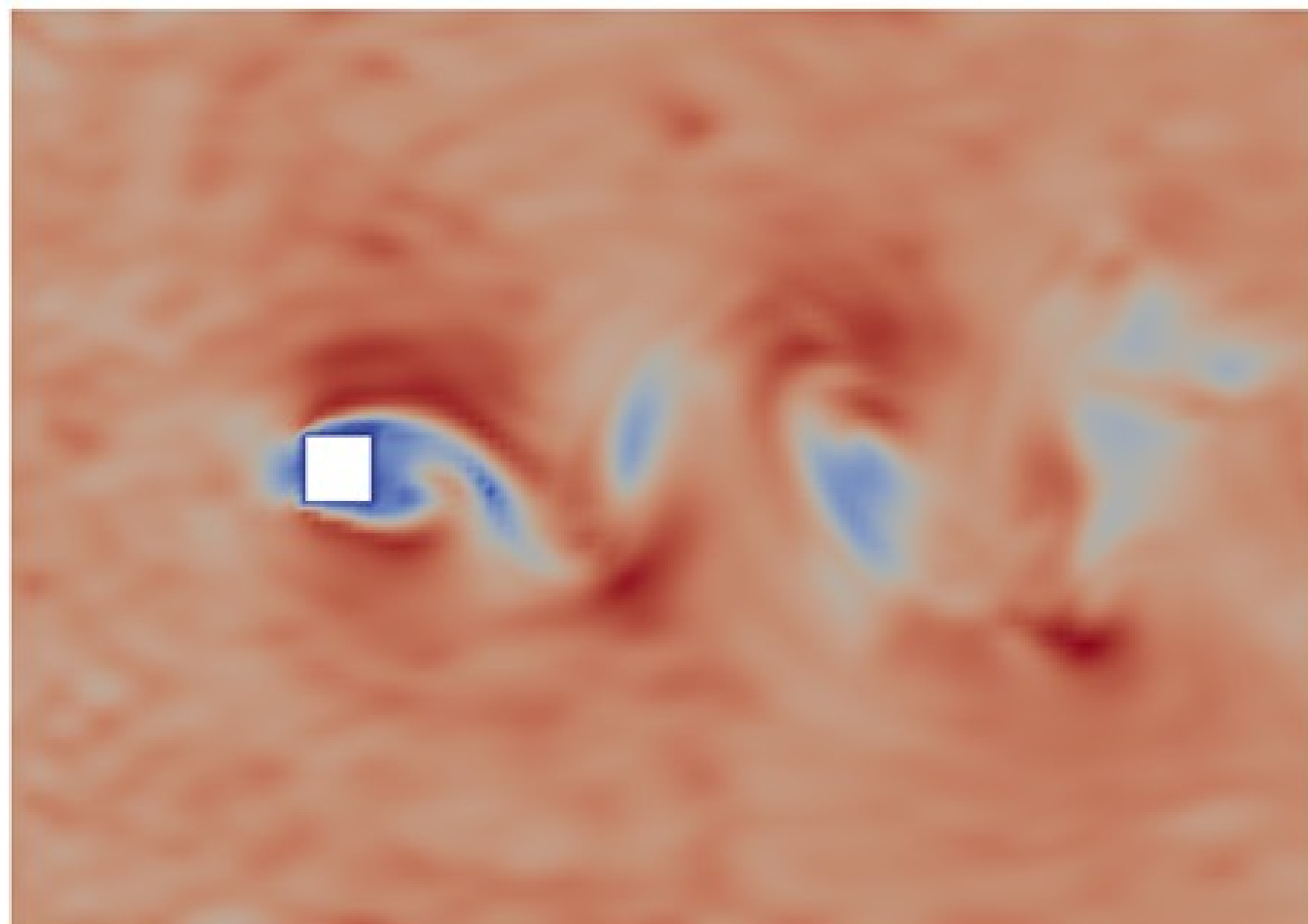
Turbulence Modelling

- Reynolds-Averaged-Navier-Stokes (RANS) models not appropriate
- Used a Delayed Detached Eddy Simulation (DDES) turbulence model
- DDES uses spatial filtering
- DDES requires a high resolution mesh
- Requires an explicitly defined turbulent inlet condition



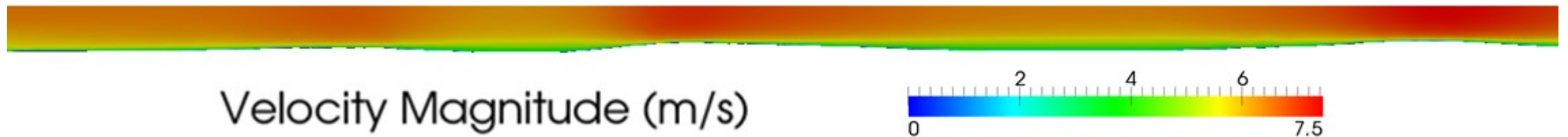
Velocity Magnitude (m/s)

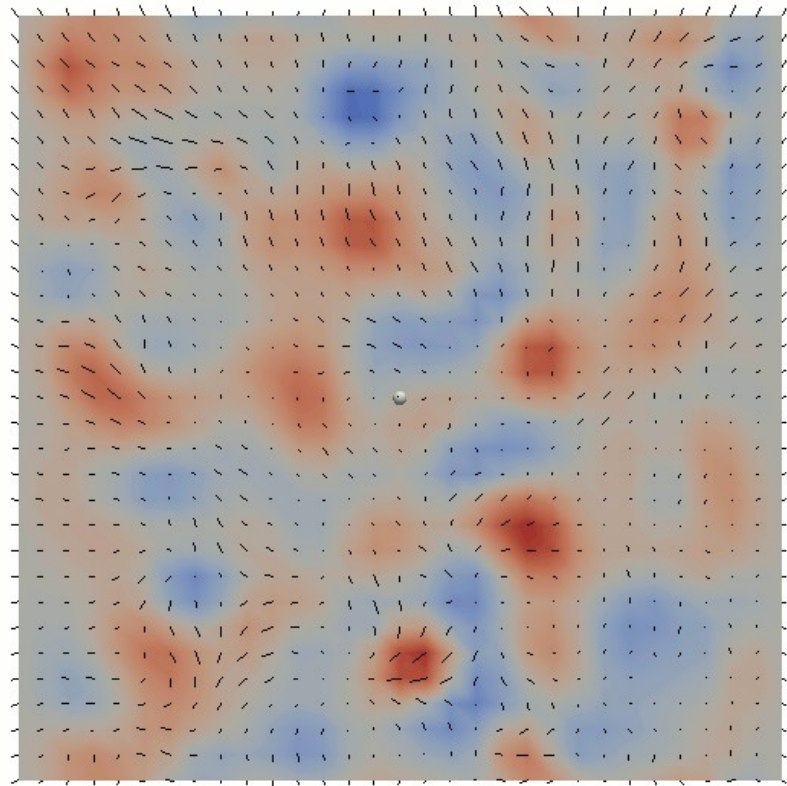




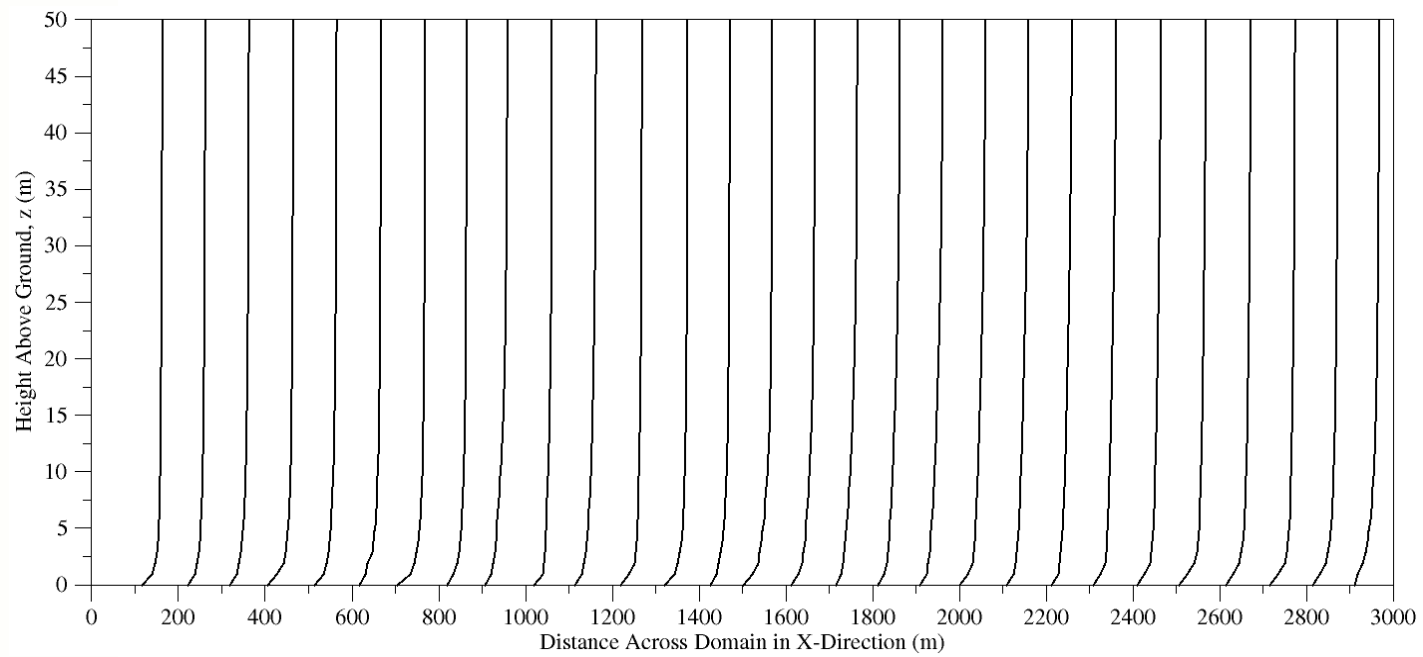
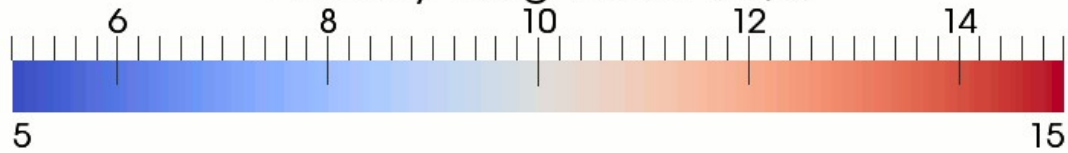
Inlet Conditions

- Required a mean inlet profile superimposed with artificial turbulence
- A turbulent inlet was created using a synthetic eddy method (SEM)
- Flow over terrain used to develop a mean inlet profile



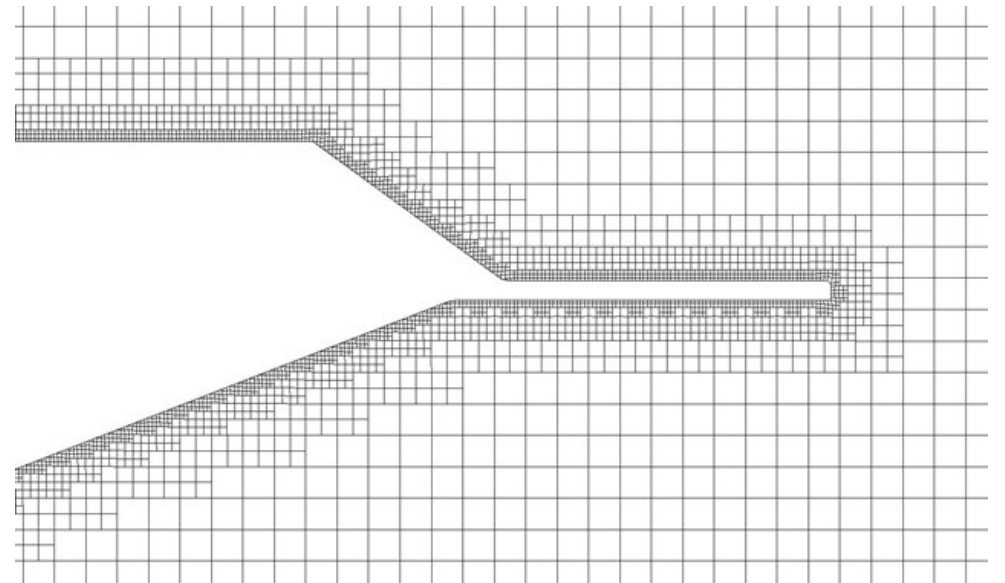
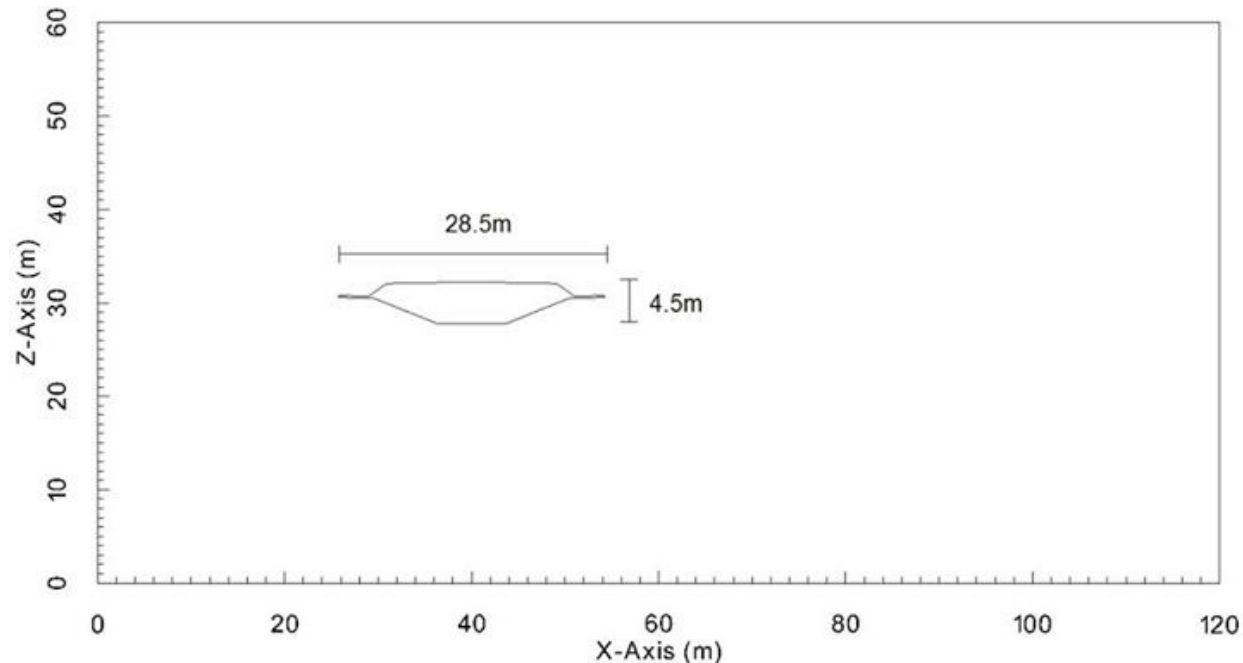


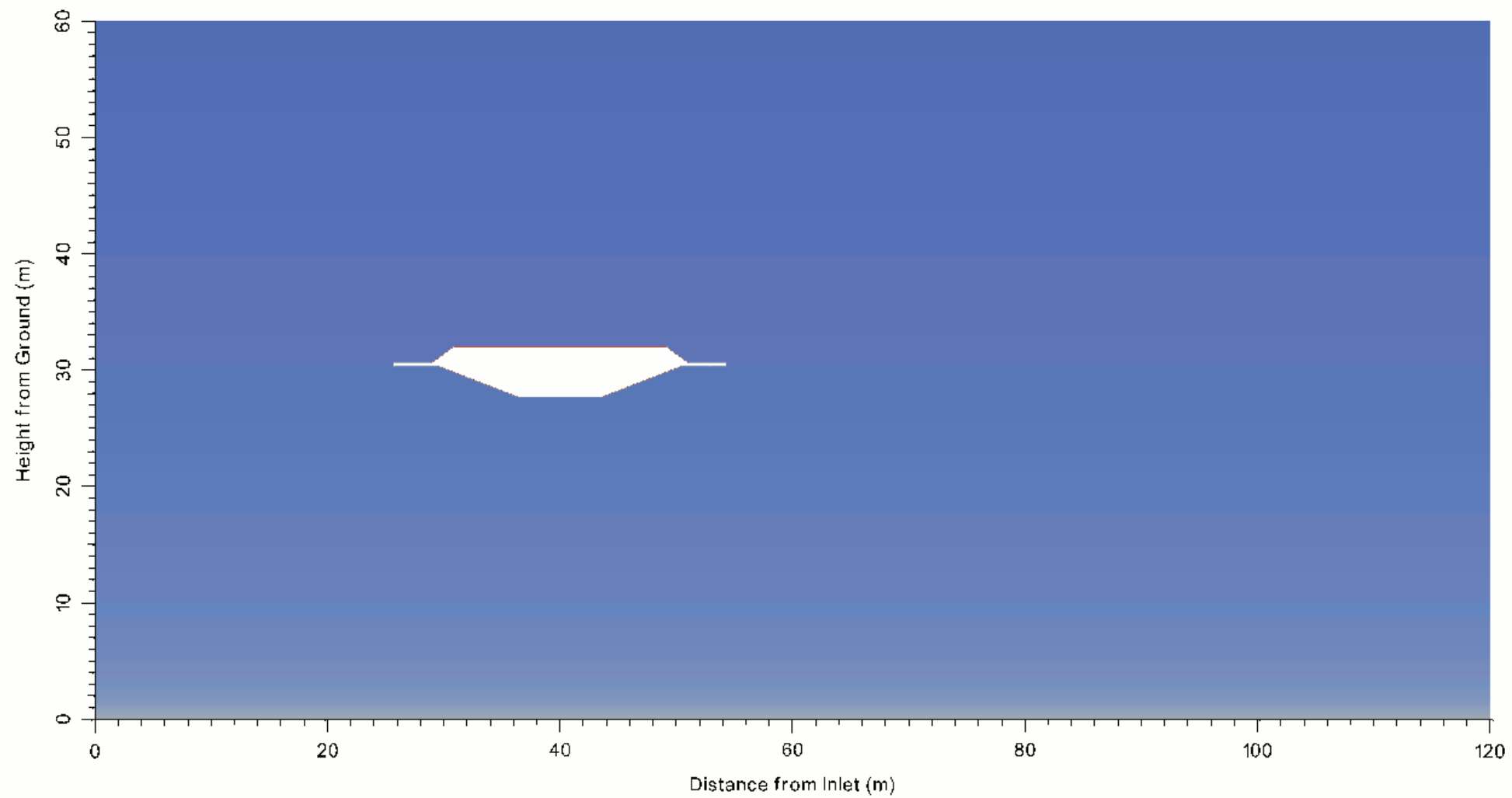
Velocity Magnitude (m/s)

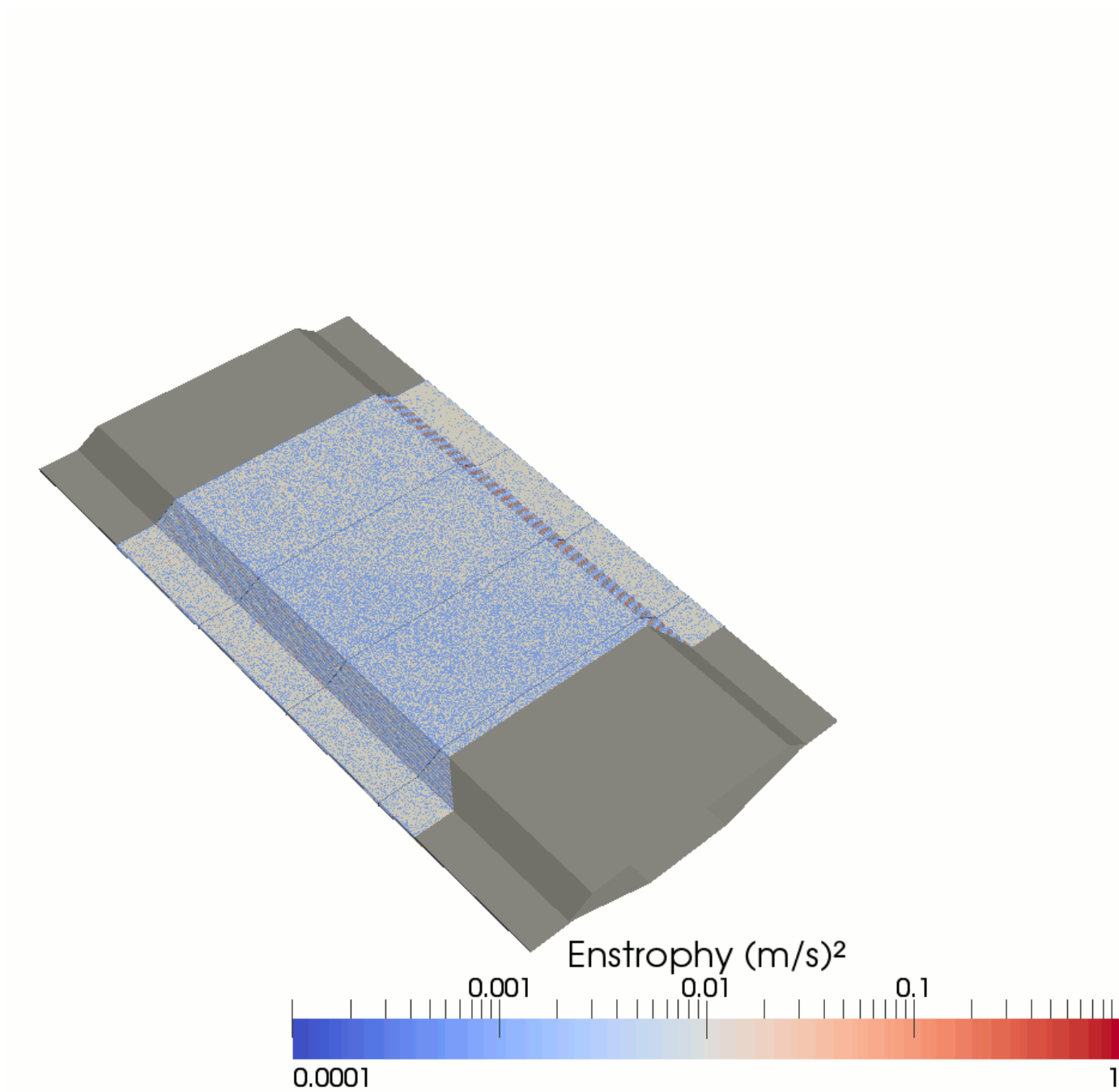


Simulation Setup

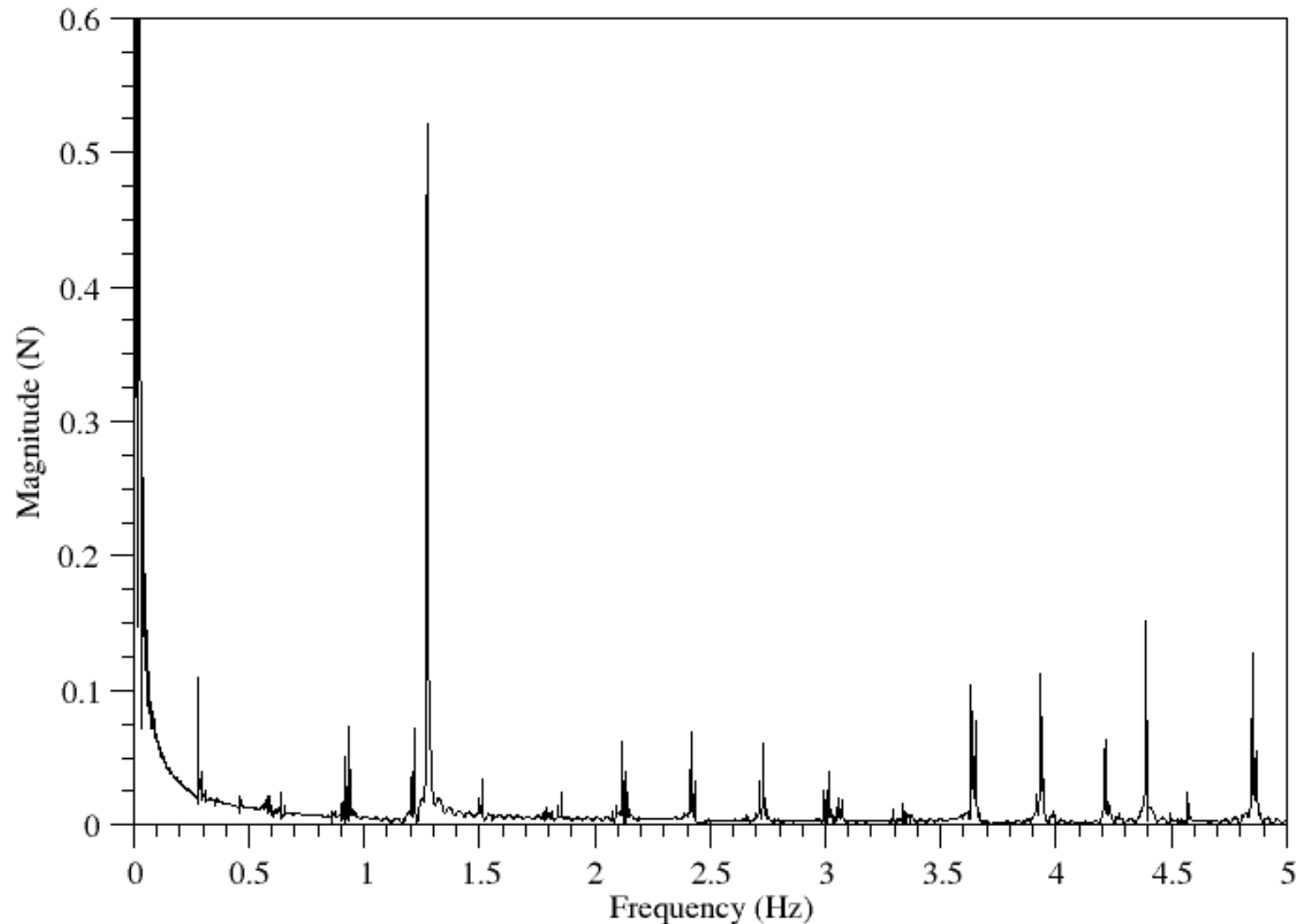
- Simulated flow over a 10m stretch of the bridge deck
- The velocity profile moved at 1m/s across the deck
- 12 minutes of air flow was simulated
- Simulation took 4 weeks to develop a wake

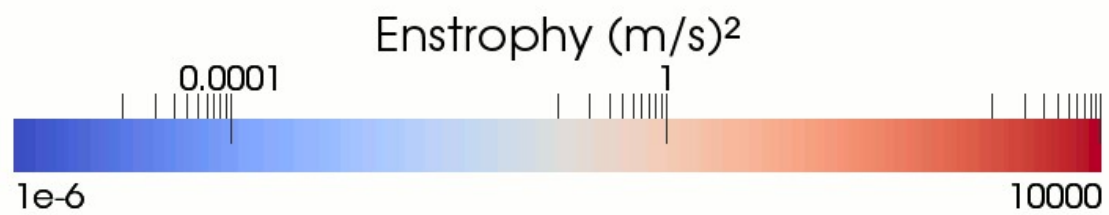
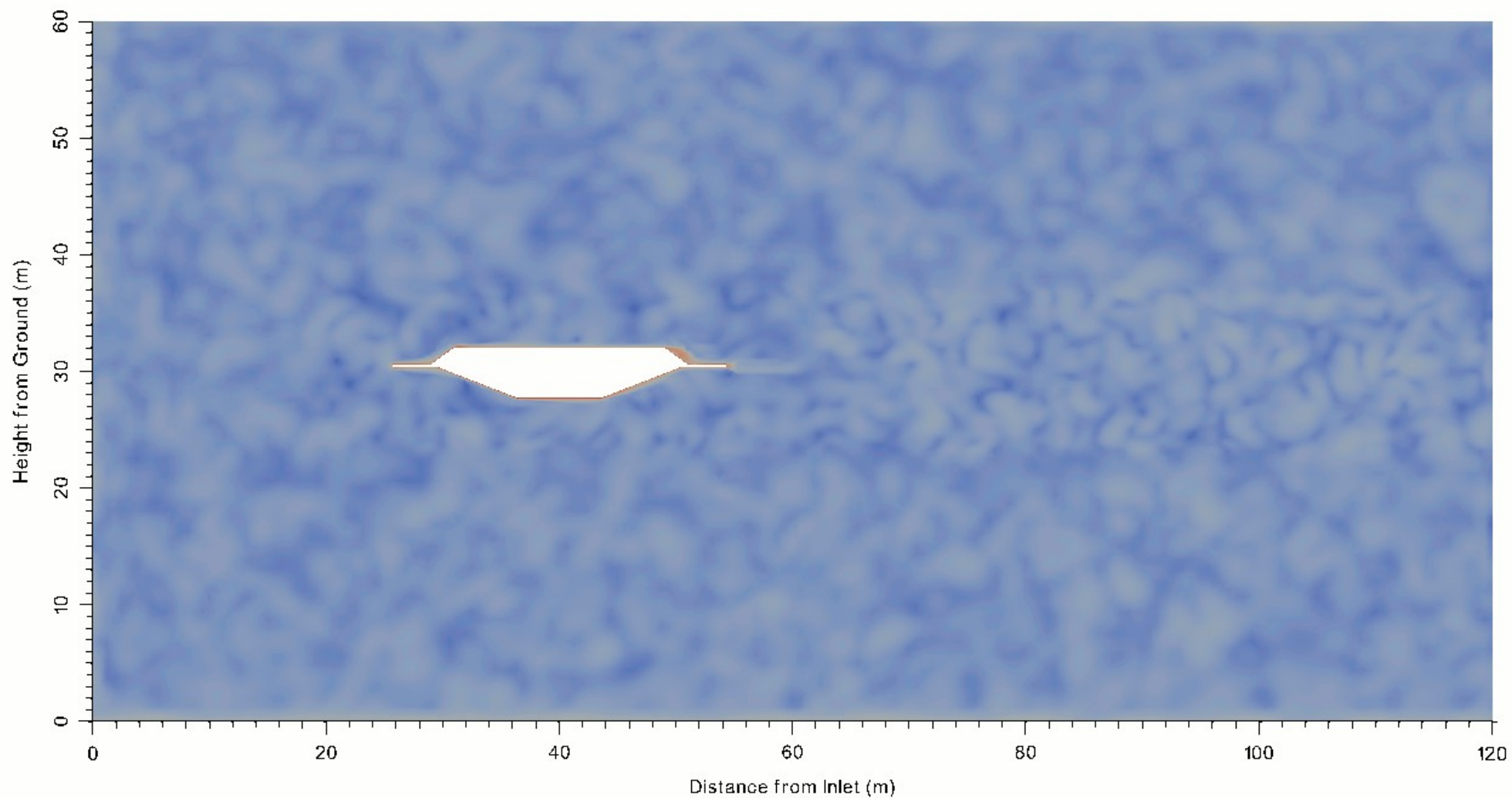






Fast Fourier Transform – Frequencies





CFD Concluding Remarks

- Produced a comprehensive model of wind flow over the Humber Bridge deck
- Included relevant parameters – atmospheric, transient flow with real turbulence
- Theoretically could be applied at other wind speeds and directions
- Compared with structural analysis group natural frequencies

Final Conclusions

- CFD simulation is capable of generating forcing frequencies and amplitudes
- Structural analysis enables the calculation of natural frequencies and dynamic response of the bridge
- The forcing and natural frequencies were different
- Resonance will not occur at this wind speed
- Further simulation of CFD testing would be prohibitively costly at higher wind speeds
- Wind tunnel testing is currently more appropriate

Any Questions?

