

Job No.: The Wright Build

Address: River Terrace, Cromwell, New Zealand

Date: 8/22/2022

Latitude: -45.056186

Longitude: 169.162575

Elevation: 226.5 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N5	Ground Snow Load	0.9 KPa	Roof Snow Load	0.63 KPa
Earthquake Zone	2	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	3.8 m
Wind Region	NZ2	Terrain Category	2.34	Design Wind Speed	37.07 m/s
Wind Pressure	0.82 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High				

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 3.60 m $C_{p,e} = -0.9$ $p_e = -0.67$ KPa $p_{net} = -0.67$ KPa

For roof $C_{p,e}$ from 3.60 m To 7.20 m $C_{p,e} = -0.5$ $p_e = -0.37$ KPa $p_{net} = -0.37$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 9 m $C_{p,e} = 0.7$ $p_e = 0.52$ KPa $p_{net} = 0.77$ KPa

For side wall $C_{p,e}$ from 0 m To 3.60 m $C_{p,e} =$ $p_e = -0.48$ KPa $p_{net} = -0.48$ KPa

Maximum Upward pressure used in roof member Design = 0.67 KPa

Maximum Downward pressure used in roof member Design = 0.40 KPa

Maximum Wall pressure used in Design = 0.77 KPa

Maximum Racking pressure used in Design = 0.9 KPa

Design Summary

Purlin Design

Purlin Spacing = 900 mm

Purlin Span = 4800 mm

Try Purlin 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet

condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 =1 K5 =1 K8 Downward =0.97

K8 Upward =0.34 S1 Downward =12.68 S1 Upward =29.28

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	0.87 Kn-m	Capacity	3.51 Kn-m	Passing Percentage	403.45 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	2.41 Kn-m	Capacity	4.67 Kn-m	Passing Percentage	193.78 %
M _{0.9D-W_nUp}	-1.15 Kn-m	Capacity	-2.04 Kn-m	Passing Percentage	177.39 %
V _{1.35D}	0.73 Kn	Capacity	12.06 Kn	Passing Percentage	1652.05 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	2.01 Kn	Capacity	16.08 Kn	Passing Percentage	800.00 %
V _{0.9D-W_nUp}	-0.96 Kn	Capacity	-20.10 Kn	Passing Percentage	2093.75 %

Deflections

Modulus of Elasticity = 8000 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 7.17 mm Limit by AS1170.0 Table C1 Span/250 = 19.20 mm

Deflection under Dead and Service Wind = 8.36 mm Limit by AS1170.0 Table C1 Span/120 = 40.00 mm

Reactions

Maximum downward =2.01 kn Maximum upward = -0.96 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 5000 mm Internal Rafter Span = 4350 mm Try Rafter 2x300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =1.00 S1 Downward =6.81 S1 Upward =6.81

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

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M _{1.35D}	3.99 Kn-m	Capacity	11.32 Kn-m	Passing Percentage	283.71 %
M _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	11.00 Kn-m	Capacity	15.08 Kn-m	Passing Percentage	137.09 %
M _{0.9D-WnUp}	-5.26 Kn-m	Capacity	-18.86 Kn-m	Passing Percentage	358.56 %
V _{1.35D}	3.67 Kn	Capacity	28.94 Kn	Passing Percentage	788.56 %
V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	10.11 Kn	Capacity	38.6 Kn	Passing Percentage	381.80 %
V _{0.9D-WnUp}	-4.84 Kn	Capacity	-48.24 Kn	Passing Percentage	996.69 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 5.935 mm Limit by AS1170.0 Table C1 Span/250 = 18.00 mm

Deflection under Dead and Service Wind = 7.69 mm Limit by AS1170.0 Table C1 Span/120 = 37.50 mm

Reactions

Maximum downward = 10.11 kn Maximum upward = -4.84 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 14.9 f_{pj} = 12.9 Mpa for Rafter with effective thickness = 100 mm

For Parallel to grain loading

K₁₁ = 2.0 f_{cj} = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 21.67 Kn > -4.84 Kn

Rafter Design External

External Rafter Load Width = 2500 mm External Rafter Span = 4304 mm Try Rafter 300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

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K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.94

K8 Upward = 0.94 S1 Downward = 13.93 S1 Upward = 13.93

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	1.95 Kn-m	Capacity	4.72 Kn-m	Passing Percentage	242.05 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	5.38 Kn-m	Capacity	6.30 Kn-m	Passing Percentage	117.10 %
M _{0.9D-W_nUp}	-2.58 Kn-m	Capacity	-7.87 Kn-m	Passing Percentage	305.04 %
V _{1.35D}	1.82 Kn	Capacity	14.47 Kn	Passing Percentage	795.05 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	5.00 Kn	Capacity	19.30 Kn	Passing Percentage	386.00 %
V _{0.9D-W_nUp}	-2.39 Kn	Capacity	-24.12 Kn	Passing Percentage	1009.21 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 6.59 mm Limit by AS1170.0 Table C1 Span/250 = 18.00 mm

Deflection under Dead and Service Wind = 7.69 mm Limit by AS1170.0 Table C1 Span/120 = 37.50 mm

Reactions

Maximum downward = 5.00 kn Maximum upward = -2.39 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 14.9 f_{pj} = 12.9 Mpa for Rafter with effective thickness = 50 mm

For Parallel to grain loading

K₁₁ = 2.0 f_{cj} = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V = $\phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s$ (Eq 4.12) = -25.20 kn > -2.39 Kn

Single Shear Capacity under short term loads = -10.84 Kn > -2.39 Kn

Girt Design Front and Back

Girt's Spacing = 1300 mm Girt's Span = 5000 mm Try Intermediate 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.41 S1 Downward =11.27 S1 Upward =26.57

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	1.56 Kn-m	Capacity	1.64 Kn-m	Passing Percentage	105.13 %
V _{0.9D-WnUp}	1.25 Kn-m	Capacity	16.08 Kn-m	Passing Percentage	1286.40 %

Deflections

Modulus of Elasticity = 8000 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 27.77 mm Limit by AS1170.0 Table C1 Span/120 = 41.67 mm
Sag during installation = 31.74 mm

Reactions

Maximum = 1.25 kn

Girt Design Sides

Girt's Spacing = 1300 mm Girt's Span = 4500 mm Try Intermediate 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.60 S1 Downward =9.63 S1 Upward =21.54

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	1.27 Kn-m	Capacity	1.41 Kn-m	Passing Percentage	111.02 %
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V _{0.9D-WnUp}	1.13 Kn-m	Capacity	12.06 Kn-m	Passing Percentage	1067.26 %
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Deflections

Modulus of Elasticity = 8000 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 43.19 mm Limit by AS1170.0 Table C1 Span/120 = 37.50 mm
 Sag during installation = 20.82 mm

Reactions

Maximum = 1.13 kn

Middle Pole Design

Geometry

150 SED H5 (Minimum 200 dia. at Floor Level)	Dry Use	Height	3300 mm
Area	31400 mm ²	As	23550 mm ²
I _x	78500000 mm ⁴	Z _x	785000 mm ³
I _y	78500000 mm ⁴	Z _y	785000 mm ³
Lateral Restraint	3300 mm c/c		

Loads

Total Area over Pole = 22.5 m²

Dead	5.63 Kn	Live	5.63 Kn
Wind	9.00 Kn	Snow	14.18 Kn
Moment wind	Kn-m	Moment snow	2.84 Kn-m
Phi	0.8	K ₈	0.84
K ₁ snow	0.8	K ₁ Dead	0.6
K ₁ wind	1		

Material

Peeling	Steaming	Normal	Dry Use
f _b =	36.3 MPa	f _s =	2.96 MPa
f _c =	18 MPa	f _p =	7.2 MPa
f _t =	22 MPa	E =	9257 MPa

Capacities

PhiN _{Cx} Wind	378.83 Kn	PhiM _{Nx} Wind	19.10 Kn-m	PhiV _{Nx} Wind	55.77 Kn
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PhiNcx Dead	227.30 Kn	PhiMnx Dead	11.46 Kn-m	PhiVnx Dead	33.46 Kn
PhiNcx Snow	303.06 Kn	PhiMnx Snow	15.28 Kn-m	PhiVnx Snow	44.61 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.50 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.26 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 19.65 \text{ mm} < 44.00 \text{ mm}$$

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Soil Properties

Gamma 18 Kn/m³ Friction angle 30 deg Cohesion 0 Kn/m³

$$K_0 = (1 - \sin(30)) / (1 + \sin(30))$$

$$K_p = (1 + \sin(30)) / (1 - \sin(30))$$

Geometry For Middle Bay Pole

Ds = 600 mm Pile Diameter
L = 1350 mm Pile embedment length
f1 = 2850 mm Distance at which the shear force is applied
f2 = 0 mm Distance of top soil at rest pressure

Loads

Moment Wind = 8.10 Kn-m Moment Snow = Kn-m
Shear Wind = 2.84 Kn Shear Snow = 2.84 Kn

Pile Properties

Safety Factory 0.55
Hu = 5.22 Kn Ultimate Lateral Strength of the Pile, Short pile
Mu = 8.81 Kn-m Ultimate Moment Capacity of Pile

Checks

$$\text{Applied Forces/Capacities} = 0.92 < 1 \text{ OK}$$

End Pole Design

Geometry For End Bay Pole

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Ds =	600 mm	Pile Diameter
L =	1350 mm	Pile embedment length
f1 =	2850 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Total Area over Pole = 5.625 m²

Moment Wind =	4.05 Kn-m	Moment Snow =	1.42 Kn-m
Shear Wind =	1.42 Kn	Shear Snow =	1.42 Kn

Pile Properties

Safety Factory	0.55	
Hu =	5.22 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	8.81 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.46 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For End Bay Pole

Ds =	600 mm	Pile Diameter
L =	1350 mm	Pile embedment length
f1 =	2850 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	4.05 Kn-m	Moment Snow =	1.42 Kn-m
Shear Wind =	1.42 Kn	Shear Snow =	1.42 Kn

Pile Properties

Safety Factor	0.55	
$H_u =$	5.22 Kn	Ultimate Lateral Strength of the Pile, Short pile
$M_u =$	8.81 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = $0.46 < 1$ OK

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

K_s (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1350) x K_s (1.5) x $\tan(30)$ x π x Dia of Pile(0.6) x Height of Pile(1350)

Skin Friction = 14.72 Kn

Weight of Pile + Pile Skin Friction = 18.44 Kn

Uplift on one Pile = 10.01 Kn

Uplift is ok