

**Job No.:** Shed 1 107  
PARANUI VALLEY ROAD  
WHANGAREI NORTHLAND

**Address:** Shed 1 107 PARANUI VALLEY ROAD  
WHANGAREI NORTHLAND

**Date:** 8/12/2022

**Latitude:** -35.696118

**Longitude:** 174.335703

**Elevation:** 31 m

### General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	5.6 m
Wind Region	NZ1	Terrain Category	2.7	Design Wind Speed	38.63 m/s
Wind Pressure	0.9 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High				

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

### Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof  $C_{p,i} = -0.3$

For roof  $C_{p,e}$  from 0 m To 2.60 m  $C_{p,e} = -0.96$   $p_e = -0.78$  KPa  $p_{net} = -0.78$  KPa

For roof  $C_{p,e}$  from 2.60 m To 5.20 m  $C_{p,e} = -0.87$   $p_e = -0.70$  KPa  $p_{net} = -0.70$  KPa

For wall Windward  $C_{p,i} = -0.3$  side Wall  $C_{p,i} = -0.3$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 18 m  $C_{p,e} = 0.7$   $p_e = 0.56$  KPa  $p_{net} = 0.83$  KPa

For side wall  $C_{p,e}$  from 0 m To 5.20 m  $C_{p,e} =$   $p_e = -0.52$  KPa  $p_{net} = -0.52$  KPa

Maximum Upward pressure used in roof member Design = 0.78 KPa

Maximum Downward pressure used in roof member Design = 0.43 KPa

Maximum Wall pressure used in Design = 0.83 KPa

Maximum Racking pressure used in Design = 0.96 KPa

### Design Summary

#### Purlin Design

Purlin Spacing = 900 mm

Purlin Span = 4300 mm

Try Purlin 200x50 SG8 Dry

Pole Shed App Ver 01 2022 by RnH Consulting Engineers

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K1 Medium term = 0.8    K1 Long term = 0.6    K4 =1    K5 =1    K8 Downward =1.00

K8 Upward =0.47    S1 Downward =11.27    S1 Upward =24.64

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

M <sub>1.35D</sub>	0.7 Kn-m	Capacity	2.39 Kn-m	Passing Percentage	<b>341.43 %</b>
M <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	1.81 Kn-m	Capacity	3.18 Kn-m	Passing Percentage	<b>175.69 %</b>
M <sub>0.9D-W<sub>n</sub>Up</sub>	-1.15 Kn-m	Capacity	-1.88 Kn-m	Passing Percentage	<b>163.48 %</b>
V <sub>1.35D</sub>	0.65 Kn	Capacity	9.65 Kn	Passing Percentage	<b>1484.62 %</b>
V <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	1.41 Kn	Capacity	12.86 Kn	Passing Percentage	<b>912.06 %</b>
V <sub>0.9D-W<sub>n</sub>Up</sub>	-1.07 Kn	Capacity	-16.08 Kn	Passing Percentage	<b>1502.80 %</b>

**Deflections**

Modulus of Elasticity = 8000 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 9.01 mm      Limit by AS1170.0 Table C1 Span/250 = 17.20 mm

Deflection under Dead and Service Wind = 10.74 mm      Limit by AS1170.0 Table C1 Span/120 = 35.83 mm

**Reactions**

Maximum downward =1.41 kn    Maximum upward = -1.07 kn

Number of Blocking = 0    if 0 then no blocking required, if 1 then one midspan blocking required

**Rafter Design Internal**

Internal Rafter Load Width = 4500 mm      Internal Rafter Span = 8850 mm      Try Rafter 2x360x45 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K1 Medium term = 0.8    K1 Long term = 0.6    K4 =1    K5 =1    K8 Downward =1.00

K8 Upward =1.00    S1 Downward =8.40    S1 Upward =8.40

Shear Capacity of timber =5.3 MPa    Bending Capacity of timber =48 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

Pole Shed App Ver 01 2022 by RnH Consulting Engineers

M <sub>1.35D</sub>	14.87 Kn-m	Capacity	50.28 Kn-m	Passing Percentage	<b>338.13 %</b>
M <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	32.16 Kn-m	Capacity	67.04 Kn-m	Passing Percentage	<b>208.46 %</b>
M <sub>0.9D-WnUp</sub>	-24.45 Kn-m	Capacity	-83.82 Kn-m	Passing Percentage	<b>342.82 %</b>
V <sub>1.35D</sub>	6.72 Kn	Capacity	55.22 Kn	Passing Percentage	<b>821.73 %</b>
V <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	14.54 Kn	Capacity	73.64 Kn	Passing Percentage	<b>506.46 %</b>
V <sub>0.9D-WnUp</sub>	-11.05 Kn	Capacity	-92.04 Kn	Passing Percentage	<b>832.94 %</b>

**Deflections**

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 26.965 mm      Limit by AS1170.0 Table C1 Span/250 = 36.00 mm

Deflection under Dead and Service Wind = 35.705 mm      Limit by AS1170.0 Table C1 Span/120 = 75.00 mm

**Reactions**

Maximum downward = 14.54 kn    Maximum upward = -11.05 kn

**Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K<sub>11</sub> = 12.6 f<sub>pj</sub> = 22.7 Mpa for Rafter with effective thickness = 90 mm

For Parallel to grain loading

K<sub>11</sub> = 2.0 f<sub>cj</sub> = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 29.11 Kn > -11.05 Kn

**Rafter Design External**

External Rafter Load Width = 2250 mm      External Rafter Span = 4318 mm      Try Rafter 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

Pole Shed App Ver 01 2022 by RnH Consulting Engineers

K1 Short term = 1    K1 Medium term = 0.8    K1 Long term = 0.6    K4 = 1    K5 = 1    K8 Downward = 0.97

K8 Upward = 0.97    S1 Downward = 12.68    S1 Upward = 12.68

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

M <sub>1.35D</sub>	1.77 Kn-m	Capacity	3.51 Kn-m	Passing Percentage	<b>198.31 %</b>
M <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	3.83 Kn-m	Capacity	4.67 Kn-m	Passing Percentage	<b>121.93 %</b>
M <sub>0.9D-W<sub>n</sub>Up</sub>	-2.91 Kn-m	Capacity	-5.84 Kn-m	Passing Percentage	<b>200.69 %</b>
V <sub>1.35D</sub>	1.64 Kn	Capacity	12.06 Kn	Passing Percentage	<b>735.37 %</b>
V <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	3.55 Kn	Capacity	16.08 Kn	Passing Percentage	<b>452.96 %</b>
V <sub>0.9D-W<sub>n</sub>Up</sub>	-2.70 Kn	Capacity	-20.10 Kn	Passing Percentage	<b>744.44 %</b>

**Deflections**

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 10.25 mm    Limit by AS1170.0 Table C1 Span/250 = 18.00 mm

Deflection under Dead and Service Wind = 12.22 mm    Limit by AS1170.0 Table C1 Span/120 = 37.50 mm

**Reactions**

Maximum downward = 3.55 kn    Maximum upward = -2.70 kn

**Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K<sub>11</sub> = 14.9 f<sub>pj</sub> = 12.9 Mpa for Rafter with effective thickness = 50 mm

For Parallel to grain loading

K<sub>11</sub> = 2.0 f<sub>cj</sub> = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V =  $\phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s$  ..... (Eq 4.12) = -19.95 kn > -2.70 Kn

Single Shear Capacity under short term loads = -10.84 Kn > -2.70 Kn

### **Girt Design Front and Back**

Girt's Spacing = 1300 mm      Girt's Span = 4500 mm      Try Intermediate 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =1.00

K8 Upward =0.60    S1 Downward =9.63    S1 Upward =21.54

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

#### **Capacity Checks**

M <sub>Wind+Snow</sub>	1.37 Kn-m	Capacity	1.41 Kn-m	Passing Percentage	<b>102.92 %</b>
V <sub>0.9D-WnUp</sub>	1.21 Kn-m	Capacity	12.06 Kn-m	Passing Percentage	<b>996.69 %</b>

#### **Deflections**

Modulus of Elasticity = 8000 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 25.61 mm    Limit by AS1170.0 Table C1 Span/120 = 37.50 mm

Sag during installation = 20.82 mm

#### **Reactions**

Maximum = 1.21 kn

### **Girt Design Sides**

Girt's Spacing = 1300 mm      Girt's Span = 4500 mm      Try Intermediate 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =1.00

K8 Upward =0.60    S1 Downward =9.63    S1 Upward =21.54

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

#### **Capacity Checks**

M <sub>Wind+Snow</sub>	1.37 Kn-m	Capacity	1.41 Kn-m	Passing Percentage	<b>102.92 %</b>
------------------------	-----------	----------	-----------	--------------------	-----------------

V <sub>0.9D-WnUp</sub>	1.21 Kn-m	Capacity	12.06 Kn-m	Passing Percentage	<b>996.69 %</b>
------------------------	-----------	----------	------------	--------------------	-----------------

### Deflections

Modulus of Elasticity = 8000 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 25.61 mm      Limit by AS1170.0 Table C1 Span/120 = 37.50 mm

Sag during installation = 20.82 mm

### Reactions

Maximum = 1.21 kn

### Middle Pole Design

#### Geometry

200 SED H5 (Minimum 250 dia. at Floor Level)	Dry Use	Height	4950 mm
Area	49063 mm <sup>2</sup>	As	36796.875 mm <sup>2</sup>
I <sub>x</sub>	191650391 mm <sup>4</sup>	Z <sub>x</sub>	1533203 mm <sup>3</sup>
I <sub>y</sub>	191650391 mm <sup>4</sup>	Z <sub>y</sub>	1533203 mm <sup>3</sup>
Lateral Restraint	4950 mm c/c		

#### Loads

Total Area over Pole = 20.25 m<sup>2</sup>

Dead	5.06 Kn	Live	5.06 Kn
Wind	8.71 Kn	Snow	0.00 Kn
Moment wind	Kn-m		
Phi	0.8	K <sub>8</sub>	0.68
K <sub>1</sub> snow	0.8	K <sub>1</sub> Dead	0.6
K <sub>1</sub> wind	1		

#### Material

Peeling	Steaming	Normal	Dry Use
f <sub>b</sub> =	36.3 MPa	f <sub>s</sub> =	2.96 MPa
f <sub>c</sub> =	18 MPa	f <sub>p</sub> =	7.2 MPa
f <sub>t</sub> =	22 MPa	E =	9257 MPa

#### Capacities

PhiN <sub>cx</sub> Wind	480.29 Kn	PhiM <sub>nx</sub> Wind	30.27 Kn-m	PhiV <sub>nx</sub> Wind	87.14 Kn
-------------------------	-----------	-------------------------	------------	-------------------------	----------

PhiNcx Dead      288.17 Kn      PhiMnx Dead      18.16 Kn-m      PhiVnx Dead      52.28 Kn

#### Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.88 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.74 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 55.63 \text{ mm} < 66.00 \text{ mm}$$

### Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

#### Soil Properties

Gamma    18 Kn/m<sup>3</sup>      Friction angle    30 deg    Cohesion    0 Kn/m<sup>3</sup>

$$K_0 = (1 - \sin(30)) / (1 + \sin(30))$$

$$K_p = (1 + \sin(30)) / (1 - \sin(30))$$

#### Geometry For Middle Bay Pole

Ds =      600 mm      Pile Diameter  
L =      1950 mm      Pile embedment length  
f1 =      4200 mm      Distance at which the shear force is applied  
f2 =      0 mm      Distance of top soil at rest pressure

#### Loads

Moment Wind =      25.34 Kn-m  
Shear Wind =      6.03 Kn

#### Pile Properties

Safety Factory      0.55  
Hu =      10.74 Kn      Ultimate Lateral Strength of the Pile, Short pile  
Mu =      26.68 Kn-m      Ultimate Moment Capacity of Pile

#### Checks

$$\text{Applied Forces/Capacities} = 0.95 < 1 \text{ OK}$$

### End Pole Design

#### Geometry For End Bay Pole

Ds =      600 mm      Pile Diameter

Pole Shed App Ver 01 2022 by RnH Consulting Engineers

L =	1400 mm	Pile embedment length
f1 =	4200 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

**Loads**

Total Area over Pole = 5.0625 m<sup>2</sup>

Moment Wind =	8.45 Kn-m
Shear Wind =	2.01 Kn

**Pile Properties**

Safety Factor	0.55	
Hu =	4.37 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	10.57 Kn-m	Ultimate Moment Capacity of Pile

**Checks**

Applied Forces/Capacities = 0.80 < 1 OK

**Drained Lateral Strength of End pile in cohesionless soils Free Head short pile**

**Soil Properties**

Gamma	18 Kn/m <sup>3</sup>	Friction angle	30 deg	Cohesion	0 Kn/m <sup>3</sup>
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

**Geometry For End Bay Pole**

Ds =	600 mm	Pile Diameter
L =	1400 mm	Pile embedment length
f1 =	4200 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

**Loads**

Moment Wind =	8.45 Kn-m
Shear Wind =	2.01 Kn

**Pile Properties**

0.55



Safety Factor

$H_u =$	4.37 Kn	Ultimate Lateral Strength of the Pile, Short pile
$M_u =$	10.57 Kn-m	Ultimate Moment Capacity of Pile

#### Checks

Applied Forces/Capacities = 0.80 < 1 OK

#### Uplift Check

Density of Concrete = 24 Kn/m<sup>3</sup>

Density of Timber Pole = 5 Kn/m<sup>3</sup>

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

$K_s$  (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1950) x  $K_s$ (1.5) x  $\tan(30)$  x  $\pi$  x Dia of Pile(0.6) x Height of Pile(1950)

Skin Friction = 30.71 Kn

Weight of Pile + Pile Skin Friction = 34.90 Kn

Uplift on one Pile = 11.24 Kn

Uplift is ok