

Job No.: Van den Bemd

Address: Lot 2 Old Hill Rd, Porangahau, New Zealand

Date: 8/8/2022

Latitude: -40.299367

Longitude: 176.609913

Elevation: 34 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N1	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	3	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	500 Years	Max Height	4.8 m
Wind Region	NZ2	Terrain Category	2.0	Design Wind Speed	46.47 m/s
Wind Pressure	1.3 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	Very High				

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Gable Enclosed

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 4.80 m $C_{p,e} = -0.9$ $p_e = -1.05$ KPa $p_{net} = -1.05$ KPa

For roof $C_{p,e}$ from 4.80 m To 9.60 m $C_{p,e} = -0.5$ $p_e = -0.58$ KPa $p_{net} = -0.58$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 24 m $C_{p,e} = 0.7$ $p_e = 0.82$ KPa $p_{net} = 1.21$ KPa

For side wall $C_{p,e}$ from 0 m To 4.80 m $C_{p,e} =$ $p_e = -0.76$ KPa $p_{net} = -0.76$ KPa

Maximum Upward pressure used in roof member Design = 1.05 KPa

Maximum Downward pressure used in roof member Design = 0.62 KPa

Maximum Wall pressure used in Design = 1.21 KPa

Maximum Racking pressure used in Design = 1.16 KPa

Design Summary

Purlin Design

Purlin Spacing = 900 mm

Purlin Span = 5800 mm

Try Purlin 250x50 SG8 Dry

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Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.97

K8 Upward = 0.28 S1 Downward = 12.68 S1 Upward = 32.18

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	1.28 Kn-m	Capacity	3.51 Kn-m	Passing Percentage	274.22 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	3.48 Kn-m	Capacity	4.67 Kn-m	Passing Percentage	134.20 %
M _{0.9D-W_nUp}	-3.12 Kn-m	Capacity	-1.70 Kn-m	Passing Percentage	54.49 %
V _{1.35D}	0.88 Kn	Capacity	12.06 Kn	Passing Percentage	1370.45 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	2.40 Kn	Capacity	16.08 Kn	Passing Percentage	670.00 %
V _{0.9D-W_nUp}	-2.15 Kn	Capacity	-20.10 Kn	Passing Percentage	934.88 %

Deflections

Modulus of Elasticity = 8000 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 15.28 mm Limit by AS1170.0 Table C1 Span/250 = 23.20 mm

Deflection under Dead and Service Wind = 20.62 mm Limit by AS1170.0 Table C1 Span/120 = 48.33 mm

Reactions

Maximum downward = 2.40 kn Maximum upward = -2.15 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 6000 mm Internal Rafter Span = 12850 mm Try Rafter 2x610x31.5 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.87

K8 Upward = 0.87 S1 Downward = 15.83 S1 Upward = 15.83

Shear Capacity of timber = 5.3 MPa Bending Capacity of timber = 48 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

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M1.35D	41.80 Kn-m	Capacity	87.58 Kn-m	Passing Percentage	209.52 %
M1.2D+1.5L 1.2D+Sn 1.2D+WnDn	113.93 Kn-m	Capacity	116.78 Kn-m	Passing Percentage	102.50 %
M0.9D-WnUp	-102.17 Kn-m	Capacity	-145.96 Kn-m	Passing Percentage	142.86 %
V1.35D	13.01 Kn	Capacity	65.5 Kn	Passing Percentage	503.46 %
V1.2D+1.5L 1.2D+Sn 1.2D+WnDn	35.47 Kn	Capacity	87.34 Kn	Passing Percentage	246.24 %
V0.9D-WnUp	-31.80 Kn	Capacity	-109.18 Kn	Passing Percentage	343.33 %

Deflections

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 45.96 mm Limit by AS1170.0 Table C1 Span/250 = 52.00 mm

Deflection under Dead and Service Wind = 68.94 mm Limit by AS1170.0 Table C1 Span/120 = 108.33 mm

Reactions

Maximum downward = 35.47 kn Maximum upward = -31.80 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 4

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 54.375 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 12.6 fpj = 22.7 Mpa for Rafter with effective thickness = 63 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 48.05 Kn > -31.80 Kn

Rafter Design External

External Rafter Load Width = 3000 mm External Rafter Span = 4411 mm Try Rafter 240x45 LVL8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet

condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.94

K8 Upward = 0.94 S1 Downward = 13.82 S1 Upward = 13.82

Shear Capacity of timber = 5 MPa Bending Capacity of timber = 30 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	2.46 Kn-m	Capacity	6.08 Kn-m	Passing Percentage	247.15 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	6.71 Kn-m	Capacity	8.10 Kn-m	Passing Percentage	120.72 %
M _{0.9D-W_nUp}	-6.02 Kn-m	Capacity	-10.13 Kn-m	Passing Percentage	168.27 %
V _{1.35D}	2.23 Kn	Capacity	17.37 Kn	Passing Percentage	778.92 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	6.09 Kn	Capacity	23.16 Kn	Passing Percentage	380.30 %
V _{0.9D-W_nUp}	-5.46 Kn	Capacity	-28.94 Kn	Passing Percentage	530.04 %

Deflections

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 11.39 mm Limit by AS1170.0 Table C1 Span/250 = 17.33 mm

Deflection under Dead and Service Wind = 15.37 mm Limit by AS1170.0 Table C1 Span/120 = 36.11 mm

Reactions

Maximum downward = 6.09 kn Maximum upward = -5.46 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 14.9 f_{pj} = 12.9 Mpa for Rafter with effective thickness = 45 mm

For Parallel to grain loading

K₁₁ = 2.0 f_{cj} = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

$V = \phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots\dots\dots$ (Eq 4.12) = -28.35 kn > -5.46 Kn

Single Shear Capacity under short term loads = -9.75 Kn > -5.46 Kn

Girt Design Front and Back

Girt's Spacing = 1000 mm Girt's Span = 6000 mm Try Intermediate 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =0.97

K8 Upward =0.53 S1 Downward =12.68 S1 Upward =23.15

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	2.72 Kn-m	Capacity	3.16 Kn-m	Passing Percentage	116.18 %
$V_{0.9D-WnUp}$	1.81 Kn-m	Capacity	20.10 Kn-m	Passing Percentage	1110.50 %

Deflections

Modulus of Elasticity = 8000 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 19.60 mm Limit by AS1170.0 Table C1 Span/120 = 50.00 mm

Sag during installation = 65.81 mm

Reactions

Maximum = 1.81 kn

Girt Design Sides

Girt's Spacing = 1000 mm Girt's Span = 4333 mm Try Intermediate 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.62 S1 Downward =9.63 S1 Upward =21.14

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

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M _{Wind+Snow}	1.42 Kn-m	Capacity	1.45 Kn-m	Passing Percentage	102.11 %
V _{0.9D-WnUp}	1.31 Kn-m	Capacity	12.06 Kn-m	Passing Percentage	920.61 %

Deflections

Modulus of Elasticity = 8000 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 24.69 mm Limit by AS1170.0 Table C1 Span/120 = 36.11 mm
Sag during installation = 17.91 mm

Reactions

Maximum = 1.31 kn

Middle Pole Design

Geometry

250 SED H5 (Minimum 300 dia. at Floor Level)	Dry Use	Height	3300 mm
Area	70650 mm ²	As	52987.5 mm ²
I _x	397406250 mm ⁴	Z _x	2649375 mm ³
I _y	397406250 mm ⁴	Z _y	2649375 mm ³
Lateral Restraint	1000 mm c/c		

Loads

Total Area over Pole = 39 m²

Dead	9.75 Kn	Live	9.75 Kn
Wind	24.18 Kn	Snow	0.00 Kn
Moment wind	Kn-m		
Phi	0.8	K ₈	1.00
K ₁ snow	0.8	K ₁ Dead	0.6
K ₁ wind	1		

Material

Peeling	Steaming	Normal	Dry Use
f _b =	36.3 MPa	f _s =	2.96 MPa
f _c =	18 MPa	f _p =	7.2 MPa
f _t =	22 MPa	E =	9257 MPa

Capacities

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PhiNcx Wind	1017.36 Kn	PhiMnx Wind	76.94 Kn-m	PhiVnx Wind	125.47 Kn
PhiNcx Dead	610.42 Kn	PhiMnx Dead	46.16 Kn-m	PhiVnx Dead	75.28 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.43 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.19 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 18.14 \text{ mm} < 44.00 \text{ mm}$$

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Soil Properties

Gamma 18 Kn/m³ Friction angle 30 deg Cohesion 0 Kn/m³

$$K_0 = (1 - \sin(30)) / (1 + \sin(30))$$

$$K_p = (1 + \sin(30)) / (1 - \sin(30))$$

Geometry For Middle Bay Pole

Ds = 600 mm Pile Diameter
L = 2100 mm Pile embedment length
f1 = 3600 mm Distance at which the shear force is applied
f2 = 0 mm Distance of top soil at rest pressure

Loads

Moment Wind = 29.99 Kn-m
Shear Wind = 8.33 Kn

Pile Properties

Safety Factory 0.55
Hu = 14.48 Kn Ultimate Lateral Strength of the Pile, Short pile
Mu = 31.57 Kn-m Ultimate Moment Capacity of Pile

Checks

$$\text{Applied Forces/Capacities} = 0.95 < 1 \text{ OK}$$

End Pole Design

Geometry For End Bay Pole

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Ds = 600 mm Pile Diameter
L = 1300 mm Pile embedment length
f1 = 3600 mm Distance at which the shear force is applied
f2 = 0 mm Distance of top soil at rest pressure

Loads

Total Area over Pole = 6.5 m²

Moment Wind = 7.50 Kn-m
Shear Wind = 2.08 Kn

Pile Properties

Safety Factory 0.55
Hu = 3.99 Kn Ultimate Lateral Strength of the Pile, Short pile
Mu = 8.33 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.90 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Soil Properties

Gamma 18 Kn/m³ Friction angle 30 deg Cohesion 0 Kn/m³
 $K_0 = (1 - \sin(30)) / (1 + \sin(30))$
 $K_p = (1 + \sin(30)) / (1 - \sin(30))$

Geometry For End Bay Pole

Ds = 600 mm Pile Diameter
L = 1300 mm Pile embedment length
f1 = 3600 mm Distance at which the shear force is applied
f2 = 0 mm Distance of top soil at rest pressure

Loads

Moment Wind = 7.50 Kn-m
Shear Wind = 2.08 Kn

Pile Properties

Safety Factor 0.55

$H_u = 3.99 \text{ Kn}$ Ultimate Lateral Strength of the Pile, Short pile

$M_u = 8.33 \text{ Kn-m}$ Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = $0.90 < 1$ OK

Uplift Check

Density of Concrete = 24 Kn/m^3

Density of Timber Pole = 5 Kn/m^3

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

K_s (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(2100) x K_s (1.5) x $\tan(30)$ x π x Dia of Pile(0.6) x Height of Pile(2100)

Skin Friction = 35.62 Kn

Weight of Pile + Pile Skin Friction = 39.03 Kn

Uplift on one Pile = 32.18 Kn

Uplift is ok