

Job No.: Josh van der Kwaak **Address:** 152 Valley View Rd, Otaika, New Zealand **Date:** 8/9/2022
Latitude: -35.789877 **Longitude:** 174.288939 **Elevation:** 64.5 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4 m
Wind Region	NZ1	Terrain Category	2.0	Design Wind Speed	43.97 m/s
Wind Pressure	1.16 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High				

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 3.84 m $C_{p,e} = -0.9$ $p_e = -0.94$ KPa $p_{net} = -0.94$ KPa

For roof $C_{p,e}$ from 3.84 m To 7.69 m $C_{p,e} = -0.5$ $p_e = -0.52$ KPa $p_{net} = -0.52$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 12 m $C_{p,e} = 0.7$ $p_e = 0.73$ KPa $p_{net} = 1.08$ KPa

For side wall $C_{p,e}$ from 0 m To 3.84 m $C_{p,e} =$ $p_e = -0.68$ KPa $p_{net} = -0.68$ KPa

Maximum Upward pressure used in roof member Design = 0.94 KPa

Maximum Downward pressure used in roof member Design = 0.56 KPa

Maximum Wall pressure used in Design = 1.08 KPa

Maximum Racking pressure used in Design = 1.25 KPa

Design Summary

Purlin Design

Purlin Spacing = 900 mm Purlin Span = 5800 mm Try Purlin 300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet

condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.94

K8 Upward = 0.46 S1 Downward = 13.93 S1 Upward = 25.01

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	1.28 Kn-m	Capacity	4.72 Kn-m	Passing Percentage	368.75 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	3.25 Kn-m	Capacity	6.30 Kn-m	Passing Percentage	193.85 %
M _{0.9D-W_nUp}	-2.71 Kn-m	Capacity	-3.87 Kn-m	Passing Percentage	142.80 %
V _{1.35D}	0.88 Kn	Capacity	14.47 Kn	Passing Percentage	1644.32 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	2.24 Kn	Capacity	19.30 Kn	Passing Percentage	861.61 %
V _{0.9D-W_nUp}	-1.87 Kn	Capacity	-24.12 Kn	Passing Percentage	1289.84 %

Deflections

Modulus of Elasticity = 8000 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 8.84 mm Limit by AS1170.0 Table C1 Span/250 = 23.20 mm

Deflection under Dead and Service Wind = 11.49 mm Limit by AS1170.0 Table C1 Span/120 = 48.33 mm

Reactions

Maximum downward = 2.24 kn Maximum upward = -1.87 kn

Number of Blocking = 1 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 6000 mm Internal Rafter Span = 5850 mm Try Rafter 2x360x45 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 8.40 S1 Upward = 8.40

Shear Capacity of timber = 5.3 MPa Bending Capacity of timber = 48 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

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M _{1.35D}	8.66 Kn-m	Capacity	50.28 Kn-m	Passing Percentage	580.60 %
M _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	22.07 Kn-m	Capacity	67.04 Kn-m	Passing Percentage	303.76 %
M _{0.9D-WnUp}	-18.35 Kn-m	Capacity	-83.82 Kn-m	Passing Percentage	456.78 %
V _{1.35D}	5.92 Kn	Capacity	55.22 Kn	Passing Percentage	932.77 %
V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	15.09 Kn	Capacity	73.64 Kn	Passing Percentage	488.01 %
V _{0.9D-WnUp}	-12.55 Kn	Capacity	-92.04 Kn	Passing Percentage	733.39 %

Deflections

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 7.1 mm Limit by AS1170.0 Table C1 Span/250 = 24.00 mm

Deflection under Dead and Service Wind = 10.26 mm Limit by AS1170.0 Table C1 Span/120 = 50.00 mm

Reactions

Maximum downward = 15.09 kn Maximum upward = -12.55 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 12.6 f_{pj} = 22.7 Mpa for Rafter with effective thickness = 90 mm

For Parallel to grain loading

K₁₁ = 2.0 f_{cj} = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 29.11 Kn > -12.55 Kn

Rafter Design External

External Rafter Load Width = 3000 mm External Rafter Span = 5808 mm Try Rafter 360x45 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

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K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.81

K8 Upward = 0.81 S1 Downward = 17.01 S1 Upward = 17.01

Shear Capacity of timber = 5.3 MPa Bending Capacity of timber = 48 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	4.27 Kn-m	Capacity	17.70 Kn-m	Passing Percentage	414.52 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	10.88 Kn-m	Capacity	23.60 Kn-m	Passing Percentage	216.91 %
M _{0.9D-W_nUp}	-9.04 Kn-m	Capacity	-29.50 Kn-m	Passing Percentage	326.33 %
V _{1.35D}	2.94 Kn	Capacity	27.61 Kn	Passing Percentage	939.12 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	7.49 Kn	Capacity	36.82 Kn	Passing Percentage	491.59 %
V _{0.9D-W_nUp}	-6.23 Kn	Capacity	-46.02 Kn	Passing Percentage	738.68 %

Deflections

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 7.89 mm Limit by AS1170.0 Table C1 Span/250 = 24.00 mm

Deflection under Dead and Service Wind = 10.26 mm Limit by AS1170.0 Table C1 Span/120 = 50.00 mm

Reactions

Maximum downward = 7.49 kn Maximum upward = -6.23 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 12.6 f_{pj} = 22.7 Mpa for Rafter with effective thickness = 45 mm

For Parallel to grain loading

K₁₁ = 2.0 f_{cj} = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V = $\phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s$ (Eq 4.12) = -50.09 kn > -6.23 Kn

Single Shear Capacity under short term loads = -14.56 Kn > -6.23 Kn

Girt Design Front and Back

Girt's Spacing = 900 mm Girt's Span = 6000 mm Try Intermediate 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.64 S1 Downward =11.27 S1 Upward =20.58

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	2.19 Kn-m	Capacity	2.56 Kn-m	Passing Percentage	116.89 %
V _{0.9D-WnUp}	1.46 Kn-m	Capacity	16.08 Kn-m	Passing Percentage	1101.37 %

Deflections

Modulus of Elasticity = 8000 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 30.75 mm Limit by AS1170.0 Table C1 Span/120 = 50.00 mm
Sag during installation = 65.81 mm

Reactions

Maximum = 1.46 kn

Girt Design Sides

Girt's Spacing = 900 mm Girt's Span = 6000 mm Try Intermediate 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.64 S1 Downward =11.27 S1 Upward =20.58

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	2.19 Kn-m	Capacity	2.56 Kn-m	Passing Percentage	116.89 %
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V _{0.9D-WnUp}	1.46 Kn-m	Capacity	16.08 Kn-m	Passing Percentage	1101.37 %
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Deflections

Modulus of Elasticity = 8000 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 30.75 mm Limit by AS1170.0 Table C1 Span/120 = 50.00 mm

Sag during installation = 65.81 mm

Reactions

Maximum = 1.46 kn

Middle Pole Design

Geometry

200 SED H5 (Minimum 250 dia. at Floor Level)	Dry Use	Height	3700 mm
Area	49063 mm ²	As	36796.875 mm ²
I _x	191650391 mm ⁴	Z _x	1533203 mm ³
I _y	191650391 mm ⁴	Z _y	1533203 mm ³
Lateral Restraint	900 mm c/c		

Loads

Total Area over Pole = 18 m²

Dead	4.50 Kn	Live	4.50 Kn
Wind	10.08 Kn	Snow	0.00 Kn
Moment wind	Kn-m		
Phi	0.8	K ₈	1.00
K ₁ snow	0.8	K ₁ Dead	0.6
K ₁ wind	1		

Material

Peeling	Steaming	Normal	Dry Use
f _b =	36.3 MPa	f _s =	2.96 MPa
f _c =	18 MPa	f _p =	7.2 MPa
f _t =	22 MPa	E =	9257 MPa

Capacities

PhiN _{cx} Wind	706.50 Kn	PhiM _{nx} Wind	44.52 Kn-m	PhiV _{nx} Wind	87.14 Kn
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PhiNcx Dead 423.90 Kn PhiMnx Dead 26.71 Kn-m PhiVnx Dead 52.28 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.53 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.28 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 26.31 \text{ mm} < 49.33 \text{ mm}$$

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Soil Properties

Gamma 18 Kn/m³ Friction angle 30 deg Cohesion 0 Kn/m³

$$K_0 = (1 - \sin(30)) / (1 + \sin(30))$$

$$K_p = (1 + \sin(30)) / (1 - \sin(30))$$

Geometry For Middle Bay Pole

Ds = 600 mm Pile Diameter
L = 1600 mm Pile embedment length
f1 = 3000 mm Distance at which the shear force is applied
f2 = 0 mm Distance of top soil at rest pressure

Loads

Moment Wind = 22.44 Kn-m
Shear Wind = 7.48 Kn

Pile Properties

Safety Factory 0.55
Hu = 7.93 Kn Ultimate Lateral Strength of the Pile, Short pile
Mu = 14.27 Kn-m Ultimate Moment Capacity of Pile

Checks

$$\text{Applied Forces/Capacities} = 1.57 < 1 \text{ OK}$$

End Pole Design

Geometry For End Bay Pole

Ds = 600 mm Pile Diameter

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L =	1600 mm	Pile embedment length
f1 =	3000 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Total Area over Pole = 9 m²

Moment Wind =	11.22 Kn-m
Shear Wind =	3.74 Kn

Pile Properties

Safety Factory	0.55	
Hu =	7.93 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	14.27 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.79 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For End Bay Pole

Ds =	600 mm	Pile Diameter
L =	1600 mm	Pile embedment length
f1 =	3000 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	11.22 Kn-m
Shear Wind =	3.74 Kn

Pile Properties

Safety Factory	0.55
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Hu =	7.93 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	14.27 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.79 < 1 OK

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1600) x Ks(1.5) x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1600)

Skin Friction = 20.68 Kn

Weight of Pile + Pile Skin Friction = 24.11 Kn

Uplift on one Pile = 12.87 Kn

Uplift is ok