

Job No.: 5115014290

Address: 303 Taverners Road, Dromore, New Zealand

Date: 9/5/2022

Latitude: -43.844945

Longitude: 171.888281

Elevation: 107 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N4	Ground Snow Load	0.9 KPa	Roof Snow Load	0.63 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4.8 m
Wind Region	NZ2	Terrain Category	2.0	Design Wind Speed	38.22 m/s
Wind Pressure	0.88 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High				

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Open

For roof $C_{p,i} = -0.52$

For roof $C_{p,e}$ from 0 m To 4.50 m $C_{p,e} = -0.9$ $p_e = -0.58$ KPa $p_{net} = -1.07$ KPa

For roof $C_{p,e}$ from 4.50 m To 9 m $C_{p,e} = -0.5$ $p_e = -0.32$ KPa $p_{net} = -0.81$ KPa

For wall Windward $C_{p,i} = 0.63$ side Wall $C_{p,i} = -0.52$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 38.40 m $C_{p,e} = 0.7$ $p_e = 0.55$ KPa $p_{net} = 1.04$ KPa

For side wall $C_{p,e}$ from 0 m To 4.50 m $C_{p,e} =$ $p_e = -0.51$ KPa $p_{net} = 0.33$ KPa

Maximum Upward pressure used in roof member Design = 1.07 KPa

Maximum Downward pressure used in roof member Design = 0.65 KPa

Maximum Wall pressure used in Design = 1.04 KPa

Maximum Racking pressure used in Design = 0.94 KPa

Design Summary

Purlin Design

Purlin Spacing = 800 mm

Purlin Span = 4600 mm

Try Purlin 200x50 SG8 Dry

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Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 0.44 S1 Downward = 11.27 S1 Upward = 25.48

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	0.71 Kn-m	Capacity	2.39 Kn-m	Passing Percentage	336.62 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	2.01 Kn-m	Capacity	3.18 Kn-m	Passing Percentage	158.21 %
M _{0.9D-W_nUp}	-1.79 Kn-m	Capacity	-1.78 Kn-m	Passing Percentage	99.44 %
V _{1.35D}	0.62 Kn	Capacity	9.65 Kn	Passing Percentage	1556.45 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	1.75 Kn	Capacity	12.86 Kn	Passing Percentage	734.86 %
V _{0.9D-W_nUp}	-1.55 Kn	Capacity	-16.08 Kn	Passing Percentage	1037.42 %

Deflections

Modulus of Elasticity = 8000 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 10.49 mm Limit by AS1170.0 Table C1 Span/250 = 18.40 mm

Deflection under Dead and Service Wind = 14.43 mm Limit by AS1170.0 Table C1 Span/120 = 38.33 mm

Reactions

Maximum downward = 1.75 kn Maximum upward = -1.55 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 4800 mm Internal Rafter Span = 4350 mm Try Rafter 2x300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 6.81 S1 Upward = 6.81

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

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M1.35D	3.83 Kn-m	Capacity	11.32 Kn-m	Passing Percentage	295.56 %
M1.2D+1.5L 1.2D+Sn 1.2D+WnDn	10.79 Kn-m	Capacity	15.08 Kn-m	Passing Percentage	139.76 %
M0.9D-WnUp	-9.59 Kn-m	Capacity	-18.86 Kn-m	Passing Percentage	196.66 %
V1.35D	3.52 Kn	Capacity	28.94 Kn	Passing Percentage	822.16 %
V1.2D+1.5L 1.2D+Sn 1.2D+WnDn	9.92 Kn	Capacity	38.6 Kn	Passing Percentage	389.11 %
V0.9D-WnUp	-8.82 Kn	Capacity	-48.24 Kn	Passing Percentage	546.94 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 5.695 mm Limit by AS1170.0 Table C1 Span/250 = 18.00 mm

Deflection under Dead and Service Wind = 8.7 mm Limit by AS1170.0 Table C1 Span/120 = 37.50 mm

Reactions

Maximum downward = 9.92 kn Maximum upward = -8.82 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 14.9 fpj = 12.9 Mpa for Rafter with effective thickness = 100 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 21.67 Kn > -8.82 Kn

Rafter Design External

External Rafter Load Width = 2400 mm External Rafter Span = 4310 mm Try Rafter 300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet

condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 =1 K5 =1 K8 Downward =0.94

K8 Upward =0.94 S1 Downward =13.93 S1 Upward =13.93

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	1.88 Kn-m	Capacity	4.72 Kn-m	Passing Percentage	251.06 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	5.29 Kn-m	Capacity	6.30 Kn-m	Passing Percentage	119.09 %
M _{0.9D-W_nUp}	-4.71 Kn-m	Capacity	-7.87 Kn-m	Passing Percentage	167.09 %
V _{1.35D}	1.75 Kn	Capacity	14.47 Kn	Passing Percentage	826.86 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	4.91 Kn	Capacity	19.30 Kn	Passing Percentage	393.08 %
V _{0.9D-W_nUp}	-4.37 Kn	Capacity	-24.12 Kn	Passing Percentage	551.95 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 6.33 mm Limit by AS1170.0 Table C1 Span/250 = 18.00 mm

Deflection under Dead and Service Wind = 8.70 mm Limit by AS1170.0 Table C1 Span/120 = 37.50 mm

Reactions

Maximum downward =4.91 kn Maximum upward = -4.37 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 14.9 f_{pj} = 12.9 Mpa for Rafter with effective thickness = 50 mm

For Parallel to grain loading

K₁₁ = 2.0 f_{cj} = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

$V = \phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots \dots \dots$ (Eq 4.12) = -25.20 kn > -4.37 Kn

Single Shear Capacity under short term loads = -10.84 Kn > -4.37 Kn

Girt Design Front and Back

Girt's Spacing = 1000 mm Girt's Span = 4800 mm Try Intermediate 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.43 S1 Downward =11.27 S1 Upward =26.03

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	1.50 Kn-m	Capacity	1.70 Kn-m	Passing Percentage	113.33 %
$V_{0.9D-WnUp}$	1.25 Kn-m	Capacity	16.08 Kn-m	Passing Percentage	1286.40 %

Deflections

Modulus of Elasticity = 8000 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 21.64 mm Limit by AS1170.0 Table C1 Span/120 = 40.00 mm

Sag during installation = 26.96 mm

Reactions

Maximum = 1.25 kn

Girt Design Sides

Girt's Spacing = 1000 mm Girt's Span = 4500 mm Try Intermediate 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.45 S1 Downward =11.27 S1 Upward =25.20

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

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M _{Wind+Snow}	1.32 Kn-m	Capacity	1.81 Kn-m	Passing Percentage	137.12 %
V _{0.9D-WnUp}	1.17 Kn-m	Capacity	16.08 Kn-m	Passing Percentage	1374.36 %

Deflections

Modulus of Elasticity = 8000 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 16.72 mm Limit by AS1170.0 Table C1 Span/120 = 37.50 mm
Sag during installation = 20.82 mm

Reactions

Maximum = 1.17 kn

Middle Pole Design

Geometry

150 SED H5 (Minimum 200 dia. at Floor Level)	Dry Use	Height	4200 mm
Area	31400 mm ²	As	23550 mm ²
I _x	78500000 mm ⁴	Z _x	785000 mm ³
I _y	78500000 mm ⁴	Z _y	785000 mm ³
Lateral Restraint	4200 mm c/c		

Loads

Total Area over Pole = 21.6 m²

Dead	5.40 Kn	Live	5.40 Kn
Wind	14.04 Kn	Snow	13.61 Kn
Moment wind	Kn-m	Moment snow	3.45 Kn-m
Phi	0.8	K ₈	0.62
K ₁ snow	0.8	K ₁ Dead	0.6
K ₁ wind	1		

Material

Peeling	Steaming	Normal	Dry Use
f _b =	36.3 MPa	f _s =	2.96 MPa
f _c =	18 MPa	f _p =	7.2 MPa
f _t =	22 MPa	E =	9257 MPa

Capacities

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PhiNcx Wind	281.06 Kn	PhiMnx Wind	14.17 Kn-m	PhiVnx Wind	55.77 Kn
PhiNcx Dead	168.64 Kn	PhiMnx Dead	8.50 Kn-m	PhiVnx Dead	33.46 Kn
PhiNcx Snow	224.85 Kn	PhiMnx Snow	11.34 Kn-m	PhiVnx Snow	44.61 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 1.01 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.94 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 50.53 \text{ mm} < 56.00 \text{ mm}$$

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K ₀ =	$(1 - \sin(30)) / (1 + \sin(30))$				
K _p =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For Middle Bay Pole

D _s =	600 mm	Pile Diameter
L =	1550 mm	Pile embedment length
f ₁ =	3600 mm	Distance at which the shear force is applied
f ₂ =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	12.96 Kn-m	Moment Snow =	Kn-m
Shear Wind =	3.60 Kn	Shear Snow =	3.45 Kn

Pile Properties

Safety Factory	0.55	
H _u =	6.44 Kn	Ultimate Lateral Strength of the Pile, Short pile
M _u =	13.62 Kn-m	Ultimate Moment Capacity of Pile

Checks

$$\text{Applied Forces/Capacities} = 0.95 < 1 \text{ OK}$$

End Pole Design

Geometry For End Bay Pole

Ds = 600 mm Pile Diameter
L = 1550 mm Pile embedment length
f1 = 3600 mm Distance at which the shear force is applied
f2 = 0 mm Distance of top soil at rest pressure

Loads

Total Area over Pole = 5.4 m²

Moment Wind = 6.48 Kn-m Moment Snow = 1.72 Kn-m
Shear Wind = 1.80 Kn Shear Snow = 1.72 Kn

Pile Properties

Safety Factory 0.55
Hu = 6.44 Kn Ultimate Lateral Strength of the Pile, Short pile
Mu = 13.62 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.48 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Soil Properties

Gamma 18 Kn/m³ Friction angle 30 deg Cohesion 0 Kn/m³
 $K_0 = (1 - \sin(30)) / (1 + \sin(30))$
 $K_p = (1 + \sin(30)) / (1 - \sin(30))$

Geometry For End Bay Pole

Ds = 600 mm Pile Diameter
L = 1550 mm Pile embedment length
f1 = 3600 mm Distance at which the shear force is applied
f2 = 0 mm Distance of top soil at rest pressure

Loads

Moment Wind = 6.48 Kn-m Moment Snow = 1.72 Kn-m
Shear Wind = 1.80 Kn Shear Snow = 1.72 Kn

Pile Properties

Safety Factor	0.55	
$H_u =$	6.44 Kn	Ultimate Lateral Strength of the Pile, Short pile
$M_u =$	13.62 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.48 < 1 OK

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

K_s (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1550) x K_s (1.5) x $\tan(30)$ x π x Dia of Pile(0.6) x Height of Pile(1550)

Skin Friction = 19.40 Kn

Weight of Pile + Pile Skin Friction = 23.68 Kn

Uplift on one Pile = 18.25 Kn

Uplift is ok