

Job No.: Mark Gapp

Address: 277 Kaimatarau Rd, Rongatea, New Zealand

Date: 8/15/2022

Latitude: -40.304647

Longitude: 175.39053

Elevation: 20 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N1	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	3	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	3 m
Wind Region	NZ2	Terrain Category	2.0	Design Wind Speed	38.22 m/s
Wind Pressure	0.88 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High				

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 2.70 m $C_{p,e} = -0.9$ $p_e = -0.71$ KPa $p_{net} = -0.71$ KPa

For roof $C_{p,e}$ from 2.70 m To 5.40 m $C_{p,e} = -0.5$ $p_e = -0.39$ KPa $p_{net} = -0.39$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 8 m $C_{p,e} = 0.7$ $p_e = 0.55$ KPa $p_{net} = 0.81$ KPa

For side wall $C_{p,e}$ from 0 m To 2.70 m $C_{p,e} =$ $p_e = -0.51$ KPa $p_{net} = -0.51$ KPa

Maximum Upward pressure used in roof member Design = 0.71 KPa

Maximum Downward pressure used in roof member Design = 0.42 KPa

Maximum Wall pressure used in Design = 0.81 KPa

Maximum Racking pressure used in Design = 0.94 KPa

Design Summary

Purlin Design

Purlin Spacing = 900 mm

Purlin Span = 4300 mm

Try Purlin 200x50 SG8 Dry

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Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 0.47 S1 Downward = 11.27 S1 Upward = 24.64

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	0.7 Kn-m	Capacity	2.39 Kn-m	Passing Percentage	341.43 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	1.81 Kn-m	Capacity	3.18 Kn-m	Passing Percentage	175.69 %
M _{0.9D-W_nUp}	-1.01 Kn-m	Capacity	-1.88 Kn-m	Passing Percentage	186.14 %
V _{1.35D}	0.65 Kn	Capacity	9.65 Kn	Passing Percentage	1484.62 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	1.39 Kn	Capacity	12.86 Kn	Passing Percentage	925.18 %
V _{0.9D-W_nUp}	-0.94 Kn	Capacity	-16.08 Kn	Passing Percentage	1710.64 %

Deflections

Modulus of Elasticity = 8000 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 9.01 mm Limit by AS1170.0 Table C1 Span/250 = 17.20 mm

Deflection under Dead and Service Wind = 10.67 mm Limit by AS1170.0 Table C1 Span/120 = 35.83 mm

Reactions

Maximum downward = 1.39 kn Maximum upward = -0.94 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 4500 mm Internal Rafter Span = 3850 mm Try Rafter 2x300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 6.81 S1 Upward = 6.81

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

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M1.35D	2.81 Kn-m	Capacity	11.32 Kn-m	Passing Percentage	402.85 %
M1.2D+1.5L 1.2D+Sn 1.2D+WnDn	6.00 Kn-m	Capacity	15.08 Kn-m	Passing Percentage	251.33 %
M0.9D-WnUp	-4.04 Kn-m	Capacity	-18.86 Kn-m	Passing Percentage	466.83 %
V1.35D	2.92 Kn	Capacity	28.94 Kn	Passing Percentage	991.10 %
V1.2D+1.5L 1.2D+Sn 1.2D+WnDn	6.24 Kn	Capacity	38.6 Kn	Passing Percentage	618.59 %
V0.9D-WnUp	-4.20 Kn	Capacity	-48.24 Kn	Passing Percentage	1148.57 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 3.335 mm Limit by AS1170.0 Table C1 Span/250 = 16.00 mm

Deflection under Dead and Service Wind = 4.385 mm Limit by AS1170.0 Table C1 Span/120 = 33.33 mm

Reactions

Maximum downward = 6.24 kn Maximum upward = -4.20 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 14.9 fpj = 12.9 Mpa for Rafter with effective thickness = 100 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 21.67 Kn > -4.20 Kn

Rafter Design External

External Rafter Load Width = 2250 mm External Rafter Span = 3811 mm Try Rafter 300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet

condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 =1 K5 =1 K8 Downward =0.94

K8 Upward =0.94 S1 Downward =13.93 S1 Upward =13.93

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	1.38 Kn-m	Capacity	4.72 Kn-m	Passing Percentage	342.03 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	2.94 Kn-m	Capacity	6.30 Kn-m	Passing Percentage	214.29 %
M _{0.9D-W_nUp}	-1.98 Kn-m	Capacity	-7.87 Kn-m	Passing Percentage	397.47 %
V _{1.35D}	1.45 Kn	Capacity	14.47 Kn	Passing Percentage	997.93 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	3.09 Kn	Capacity	19.30 Kn	Passing Percentage	624.60 %
V _{0.9D-W_nUp}	-2.08 Kn	Capacity	-24.12 Kn	Passing Percentage	1159.62 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 3.70 mm Limit by AS1170.0 Table C1 Span/250 = 16.00 mm

Deflection under Dead and Service Wind = 4.38 mm Limit by AS1170.0 Table C1 Span/120 = 33.33 mm

Reactions

Maximum downward =3.09 kn Maximum upward = -2.08 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 14.9 f_{pj} = 12.9 Mpa for Rafter with effective thickness = 50 mm

For Parallel to grain loading

K₁₁ = 2.0 f_{cj} = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

$V = \phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots\dots\dots$ (Eq 4.12) = -25.20 kn > -2.08 Kn

Single Shear Capacity under short term loads = -10.84 Kn > -2.08 Kn

Girt Design Front and Back

Girt's Spacing = 1300 mm Girt's Span = 4500 mm Try Intermediate 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.60 S1 Downward =9.63 S1 Upward =21.54

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	1.33 Kn-m	Capacity	1.41 Kn-m	Passing Percentage	106.02 %
$V_{0.9D-WnUp}$	1.18 Kn-m	Capacity	12.06 Kn-m	Passing Percentage	1022.03 %

Deflections

Modulus of Elasticity = 8000 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 24.99 mm Limit by AS1170.0 Table C1 Span/120 = 37.50 mm

Sag during installation = 20.82 mm

Reactions

Maximum = 1.18 kn

Girt Design Sides

Girt's Spacing = 1300 mm Girt's Span = 4000 mm Try Intermediate 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.65 S1 Downward =9.63 S1 Upward =20.31

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

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M _{Wind+Snow}	1.05 Kn-m	Capacity	1.54 Kn-m	Passing Percentage	146.67 %
V _{0.9D-WnUp}	1.05 Kn-m	Capacity	12.06 Kn-m	Passing Percentage	1148.57 %

Deflections

Modulus of Elasticity = 8000 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 15.60 mm Limit by AS1170.0 Table C1 Span/120 = 33.33 mm
Sag during installation = 13.00 mm

Reactions

Maximum = 1.05 kn

Middle Pole Design

Geometry

175 SED H5)	Dry Use	Height	2400 mm
Area	24041 mm ²	As	18030.46875 mm ²
I _x	46015259 mm ⁴	Z _x	525889 mm ³
I _y	46015259 mm ⁴	Z _y	525889 mm ³
Lateral Restraint	2400 mm c/c		

Loads

Total Area over Pole = 18 m²

Dead	4.50 Kn	Live	4.50 Kn
Wind	7.56 Kn	Snow	0.00 Kn
Moment wind	Kn-m		
Phi	0.8	K ₈	0.94
K ₁ snow	0.8	K ₁ Dead	0.6
K ₁ wind	1		

Material

Peeling	Steaming	Normal	Dry Use
f _b =	36.3 MPa	f _s =	2.96 MPa
f _c =	18 MPa	f _p =	7.2 MPa
f _t =	22 MPa	E =	9257 MPa

Capacities

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PhiNcx Wind	326.85 Kn	PhiMnx Wind	14.42 Kn-m	PhiVnx Wind	42.70 Kn
PhiNcx Dead	196.11 Kn	PhiMnx Dead	8.65 Kn-m	PhiVnx Dead	25.62 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.38 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.16 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 11.27 \text{ mm} < 32.00 \text{ mm}$$

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Soil Properties

Gamma 18 Kn/m³ Friction angle 30 deg Cohesion 0 Kn/m³

$$K_0 = (1 - \sin(30)) / (1 + \sin(30))$$

$$K_p = (1 + \sin(30)) / (1 - \sin(30))$$

Geometry For Middle Bay Pole

Ds = 600 mm Pile Diameter
L = 1300 mm Pile embedment length
f1 = 2250 mm Distance at which the shear force is applied
f2 = 0 mm Distance of top soil at rest pressure

Loads

Moment Wind = 4.75 Kn-m
Shear Wind = 2.11 Kn

Pile Properties

Safety Factory 0.55
Hu = 5.51 Kn Ultimate Lateral Strength of the Pile, Short pile
Mu = 7.51 Kn-m Ultimate Moment Capacity of Pile

Checks

$$\text{Applied Forces/Capacities} = 0.63 < 1 \text{ OK}$$

End Pole Design

Geometry For End Bay Pole

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Ds = 600 mm Pile Diameter
L = 1300 mm Pile embedment length
f1 = 2250 mm Distance at which the shear force is applied
f2 = 0 mm Distance of top soil at rest pressure

Loads

Total Area over Pole = 4.5 m²

Moment Wind = 2.37 Kn-m
Shear Wind = 1.05 Kn

Pile Properties

Safety Factory 0.55
Hu = 5.51 Kn Ultimate Lateral Strength of the Pile, Short pile
Mu = 7.51 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.32 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Soil Properties

Gamma 18 Kn/m³ Friction angle 30 deg Cohesion 0 Kn/m³
 $K_0 = (1 - \sin(30)) / (1 + \sin(30))$
 $K_p = (1 + \sin(30)) / (1 - \sin(30))$

Geometry For End Bay Pole

Ds = 600 mm Pile Diameter
L = 1300 mm Pile embedment length
f1 = 2250 mm Distance at which the shear force is applied
f2 = 0 mm Distance of top soil at rest pressure

Loads

Moment Wind = 2.37 Kn-m
Shear Wind = 1.05 Kn

Pile Properties

Safety Factor 0.55

$H_u = 5.51 \text{ Kn}$ Ultimate Lateral Strength of the Pile, Short pile

$M_u = 7.51 \text{ Kn-m}$ Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = $0.32 < 1$ OK

Uplift Check

Density of Concrete = 24 Kn/m^3

Density of Timber Pole = 5 Kn/m^3

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

K_s (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1300) x K_s (1.5) x $\tan(30)$ x π x Dia of Pile(0.6) x Height of Pile(1300)

Skin Friction = 13.65 Kn

Weight of Pile + Pile Skin Friction = 17.68 Kn

Uplift on one Pile = 8.73 Kn

Uplift is ok