



Company: Napior, LLC

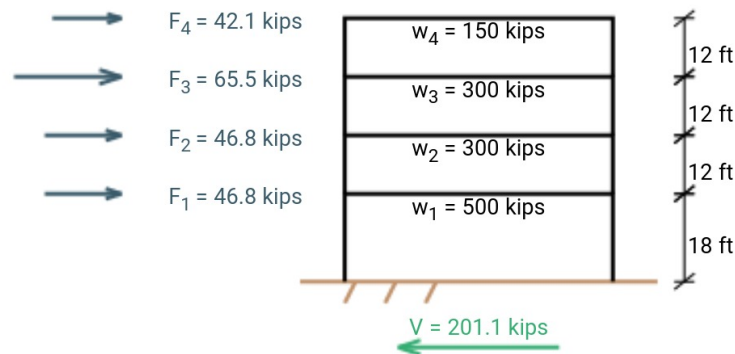
Project: Seismic Loads

Description: Determine the seismic loads.

Sheet:

Engr: Charlie Misner

Seismic Loads



1 - Properties

Site Criteria

Address = Seattle, WA

Seismic Site Class = D

T_L = 6 seconds

S_S = 1.45 g

S₁ = 0.49 g

F_a = 1.00

F_v = 1.51

S_{ds} = $\frac{2}{3}F_aS_s = \frac{2}{3}(1)(1.448g) = 0.97 g$

S_{d1} = $\frac{2}{3}F_vS_1 = \frac{2}{3}(1.511)(0.489g) = 0.49 g$

ASCE 7-10 Chapter 20

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ASCE 7-10 Table 11.4-1

ASCE 7-10 Table 11.4-2

ASCE 7-10 11.4-3

ASCE 7-10 11.4-4

Building Criteria

Building Risk Category = II

System Category = Building Frames

Structural System = Special reinforced concrete shear walls

R = 6

C_t = 0.02

x = 0.75

IBC Table 1604.5

ASCE 7-10 Table 12.02-1

ASCE 7-10 Table 12.02-1

ASCE 7-10 Table 12.02-1

ASCE 7-10 Table 12.8-2

ASCE 7-10 Table 12.8-2

2 - Calculate Base Shear

Determine Building Period

Building period is known.

Known Period, T = 0.5 seconds

ASCE 7-10 Table 12.8-2



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$$\begin{aligned}T_a &= C_t h_n^x = (0.02)(54\text{ft})^{0.75} = 0.398 \text{ seconds} && \text{ASCE 7-10 Eqn. 12.8-7} \\C_u &= 1.4 && \text{ASCE 7-10 Table 12.8-1} \\C_u T_a &= C_u T_a = (1.4)(0.398 \text{ seconds}) = 0.558 \text{ seconds} && \text{ASCE 7-10 Section 12.8.2} \\T_a > T > C_u T_a &\therefore T_a = T = 0.5 \text{ seconds} && \text{ASCE 7-10 Section 12.8.2}\end{aligned}$$

Determine Base Shear

$$\begin{aligned}C_s &= \frac{S_{ds}}{(R/I_e)} = \frac{0.965 g}{(6/1)} = 0.16 && \text{ASCE 7-10 Eqn. 12.8-2} \\&< \frac{S_{d1}}{T_a(R/I_e)} = \frac{0.493 g}{0.398 s (6/1)} = 0.16 && \text{ASCE 7-10 Eqn. 12.8-3} \\&> 0.44 S_{ds} I_e = 0.44(0.965 g)(1) > 0.01 = 0.01 && \text{ASCE 7-10 Eqn. 12.8-5} \\C_s &= 0.161 \\V &= C_s * W = (0.161)(1250 \text{ kips}) = 201.1 \text{ kips} && \text{ASCE 7-10 Eqn. 12.8-1}\end{aligned}$$

3 - Vertical Force Distribution

$$\begin{aligned}k &= 1.00 && \text{ASCE 7-10 Section 12.8.3} \\C_{vx} &= \frac{w_x h_x^k}{\sum_{i=1}^n w_i h_i^k} && \text{ASCE 7-10 Eqn. 12.8-6} \\F_x &= C_{vx} * W_x && \text{ASCE 7-10 Eqn. 12.8-6}\end{aligned}$$

Table 1 - Story Forces

Story	Height	Weight	$w_x h_x^k$	C_{vx}	F_x (kips)
1	18	500	9000	0.23	46.8
2	30	300	9000	0.23	46.8
3	42	300	12600	0.33	65.5
4	54	150	8100	0.21	42.1