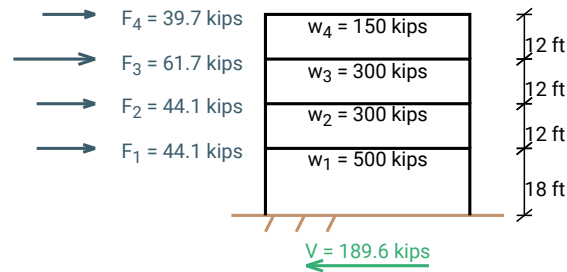




Company: Napior	Sheet:
Project: Seismic Loads	
Description: Determine the seismic loads on a project.	Engr: Charlie Misner

Seismic Loads



1 - Properties

Site Criteria

Address	= Seattle, WA	
Seismic Site Class	= D	ASCE 7-10 Chapter 20
T_L	= 6 seconds	USGS.gov
S_S	= 1.37 g	USGS.gov
S_1	= 0.50 g	USGS.gov
F_a	= 1.00	ASCE 7-10 Table 11.4-1
F_v	= 1.50	ASCE 7-10 Table 11.4-2
S_{ds}	$= \frac{2}{3} F_a S_S = \frac{2}{3} (1) (1.37g) = 0.91$ g	ASCE 7-10 11.4-3
S_{d1}	$= \frac{2}{3} F_v S_1 = \frac{2}{3} (1.5) (0.5g) = 0.53$ g	ASCE 7-10 11.4-4

Building Criteria

Building Risk Category	= II	IBC Table 1604.5
System Category	= Building Frames	ASCE 7-10 Table 12.02-1
Structural System	= Special reinforced concrete shear walls	ASCE 7-10 Table 12.02-1
R	= 6	ASCE 7-10 Table 12.02-1
C_t	= 0.02	ASCE 7-10 Table 12.8-2
x	= 0.75	ASCE 7-10 Table 12.8-2

2 - Calculate Base Shear

Determine Building Period

Building period is known.



Company: Napior

Sheet:

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$$\begin{aligned}\text{Known Period, } T &= 0.2 \text{ seconds} \\ T_a &= C_t h_n^x = (0.02)(54\text{ft})^{0.75} = 0.398 \text{ seconds} \\ C_u &= 1.4 \\ C_u T_a &= C_u T_a = (1.4)(0.398 \text{ seconds}) = 0.558 \text{ seconds} \\ T_a < T &\therefore T_a = 0.398 \text{ seconds}\end{aligned}$$

ASCE 7-10 Table 12.8-2

ASCE 7-10 Eqn. 12.8-7

ASCE 7-10 Table 12.8-1

ASCE 7-10 Section 12.8.2

ASCE 7-10 Section 12.8.2

Determine Base Shear

$$\begin{aligned}C_s &= \frac{S_{ds}}{(R/I_e)} = \frac{0.91g}{(6/1)} = 0.15 \\ &< \frac{S_{d1}}{T_a(R/I_e)} = \frac{0.53g}{0.398s(6/1)} = 0.22 \\ &> 0.44S_{ds}I_e = 0.44(0.91g)(1) > 0.01 = 0.01 \\ C_s &= 0.152 \\ V &= C_s * W = (0.152)(1250 \text{ kips}) = 189.6 \text{ kips}\end{aligned}$$

ASCE 7-10 Eqn. 12.8-2

ASCE 7-10 Eqn. 12.8-3

ASCE 7-10 Eqn. 12.8-5

ASCE 7-10 Eqn. 12.8-1

3 - Vertical Force Distribution

$$\begin{aligned}k &= 1.00 \\ C_{vx} &= \frac{w_x h_x^k}{\sum_{i=1}^n w_i h_i^k} \\ F_x &= C_{vx} * w_x\end{aligned}$$

ASCE 7-10 Section 12.8.3

ASCE 7-10 Eqn. 12.8-6

ASCE 7-10 Eqn. 12.8-6

Table 1 - Story Forces

Story	Height	Weight	$w_x h_x^k$	C_{vx}	F_x (kips)
1	18	500	9000	0.23	44.1
2	30	300	9000	0.23	44.1
3	42	300	12600	0.33	61.7
4	54	150	8100	0.21	39.7