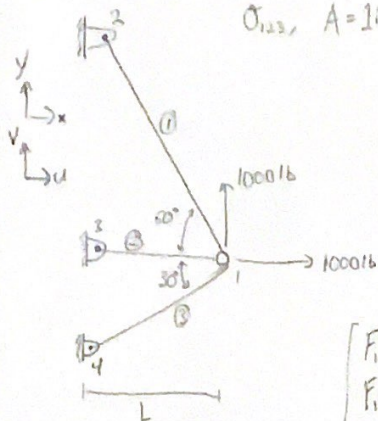


1.] Solve 3.22, $U_1, V_1, E = 10 \times 10^6 \text{ psi}$

$\sigma_{123}, A = 1 \text{ in}^2, L = 100 \text{ in}$

$$\{F\} = [K]\{d\}$$



Element	θ	C	S	C^2	S^2	CS
1	120	$-\frac{1}{2}$	$\frac{\sqrt{3}}{2}$	$\frac{1}{4}$	$\frac{3}{4}$	$-\frac{\sqrt{3}}{4}$
2	0	1	0	1	0	0
3	210	$-\frac{\sqrt{3}}{2}$	$-\frac{1}{2}$	$\frac{3}{4}$	$\frac{1}{4}$	$-\frac{\sqrt{3}}{4}$

$$\cos(60) = \frac{1}{2}$$

$$\cos(30) = \frac{\sqrt{3}}{2}$$

$$\begin{bmatrix} F_{1x} \\ F_{1y} \\ F_{2x} \\ F_{2y} \\ F_{3x} \\ F_{3y} \\ F_{4x} \\ F_{4y} \end{bmatrix} = [K] \begin{bmatrix} U_1 \\ V_1 \\ U_2 \\ V_2 \\ U_3 \\ V_3 \\ U_4 \\ V_4 \end{bmatrix}$$

$$[K] = \frac{EA}{L \cos 60}$$

$$[K^0] = \frac{EA}{L}$$

$$\begin{bmatrix} U_1 & V_1 & U_2 & V_2 \\ 1/4 & -\sqrt{3}/4 & -1/4 & -\sqrt{3}/4 \\ 3/4 & \sqrt{3}/4 & 1/4 & \sqrt{3}/4 \\ \text{sym} & & 1/4 & \sqrt{3}/4 \\ \text{sym} & & 3/4 & \sqrt{3}/4 \end{bmatrix} \begin{bmatrix} U_1 \\ V_1 \\ U_2 \\ V_2 \end{bmatrix}$$

$$\begin{bmatrix} U_1 & V_1 & U_3 & V_3 \\ 1 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 \\ \text{sym} & & 1 & 0 \\ \text{sym} & & 0 & 0 \end{bmatrix} \begin{bmatrix} U_1 \\ V_1 \\ U_3 \\ V_3 \end{bmatrix}$$

$$F_{1x} = 1000$$

$$F_{2y} = 1000$$

$$[K] = \frac{EA}{L \cos 30} \begin{bmatrix} U_1 & V_1 & U_2 & V_2 \\ 3/4 & \sqrt{3}/4 & -3/4 & -\sqrt{3}/4 \\ \sqrt{3}/4 & 1/4 & -\sqrt{3}/4 & -1/4 \\ \text{sym} & & 3/4 & \sqrt{3}/4 \\ \text{sym} & & -\sqrt{3}/4 & -1/4 \end{bmatrix} \begin{bmatrix} U_1 \\ V_1 \\ U_2 \\ V_2 \end{bmatrix}$$

$$\frac{EA}{L} \begin{bmatrix} U_1 & V_1 & U_2 & V_2 & U_3 & V_3 & U_4 & V_4 \\ 1 & 0 & -1 & 0 & 0 & 0 & 1/3 & -\sqrt{3}/3 \\ 0 & 0 & 0 & 0 & 0 & 0 & \sqrt{3}/3 & 1/3 \\ 1/3 & -\sqrt{3}/3 & -1/3 & \sqrt{3}/3 & 0 & 0 & 1 & 0 \\ -\sqrt{3}/3 & 1/3 & \sqrt{3}/3 & -1/3 & 0 & 0 & 0 & 1 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix} \begin{bmatrix} U_1 \\ V_1 \\ U_2 \\ V_2 \\ U_3 \\ V_3 \\ U_4 \\ V_4 \end{bmatrix} = \begin{bmatrix} F_{1x} \\ F_{1y} \\ F_{2x} \\ F_{2y} \\ F_{3x} \\ F_{3y} \\ F_{4x} \\ F_{4y} \end{bmatrix}$$

$$\frac{EA}{L} \begin{bmatrix} \frac{4+3\sqrt{3}}{8} & \frac{3-\sqrt{3}}{8} \\ \frac{3-\sqrt{3}}{8} & \frac{3+\sqrt{3}}{8} \end{bmatrix} \begin{bmatrix} u_1 \\ v_1 \end{bmatrix} = \begin{bmatrix} 1000 \text{ lb} \\ 1000 \text{ lb} \end{bmatrix} \quad \frac{EA}{L} \left(\frac{3-\sqrt{3}}{8} u_1 + \frac{3+\sqrt{3}}{8} v_1 \right) = 1000$$

$$\frac{3-\sqrt{3}}{8} u_1 = \frac{1000L}{EA} - \frac{3+\sqrt{3}}{8} v_1$$

$$u_1 = \left(\frac{1000L}{EA} - \frac{3+\sqrt{3}}{8} v_1 \right) \left(\frac{8}{3-\sqrt{3}} \right)$$

$$\frac{EA}{L} \left(\frac{4+3\sqrt{3}}{8} u_1 + \frac{3-\sqrt{3}}{8} v_1 \right) = 1000$$

$$\frac{4+3\sqrt{3}}{8} \left(\frac{1000L}{EA} - \frac{3+\sqrt{3}}{8} v_1 \right) \left(\frac{8}{3-\sqrt{3}} \right) = 1000$$

solve for v_1

$$v_1 = 0.01577 \text{ in}$$

$$u_1 = 0.00423 \text{ in}$$

$$\sigma_{1,2,3} = \frac{E}{L} \begin{bmatrix} -C & -S & C & S \end{bmatrix} \begin{bmatrix} u_1 \\ v_1 \\ u_2 \\ v_2 \end{bmatrix}$$

$$\sigma_1 = \frac{E}{L \cos(60)} \begin{bmatrix} \frac{1}{2} & \frac{-\sqrt{3}}{2} & \frac{-1}{2} & \frac{\sqrt{3}}{2} \end{bmatrix} \begin{bmatrix} u_1 \\ v_1 \\ u_2 \\ v_2 \end{bmatrix}$$

$$\sigma_1 = -577 \frac{\text{lb}}{\text{in}^2} \text{ Compression}$$

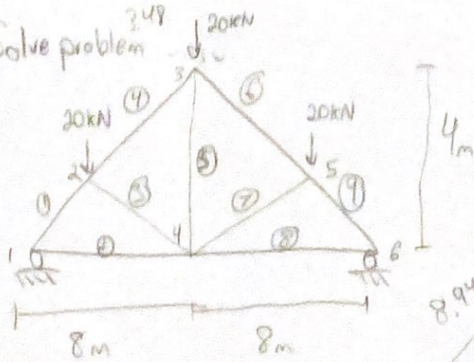
$$\sigma_2 = \frac{E}{L} \begin{bmatrix} -1 & 0 & 1 & 0 \end{bmatrix} \begin{bmatrix} u_1 \\ v_1 \\ u_2 \\ v_2 \end{bmatrix} \Rightarrow$$

$$\sigma_2 = -422 \frac{\text{lb}}{\text{in}^2} \text{ Compression}$$

$$\sigma_3 = \frac{E}{L \cos(30)} \begin{bmatrix} \frac{\sqrt{3}}{2} & \frac{1}{2} & \frac{-\sqrt{3}}{2} & \frac{-1}{2} \end{bmatrix} \begin{bmatrix} u_1 \\ v_1 \\ u_2 \\ v_2 \end{bmatrix} \Rightarrow$$

$$\sigma_3 = 999.8 \frac{\text{lb}}{\text{in}^2} \text{ Tension}$$

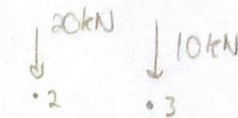
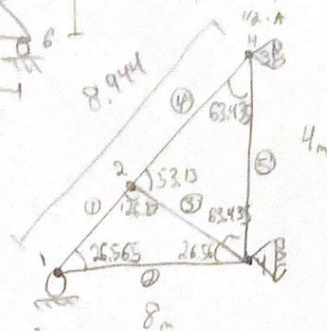
2.] Solve problem



$$A = 10 \times 10^{-4} \text{ m}^2$$

$$E = 210 \text{ GPa}$$

Symmetry Down Middle



$$L_1 = 4.47 \quad L_4 = 4.47$$

$$L_2 = 8 \quad L_5 = 4$$

$$L_3 = 4.47$$

Element	θ	C	S	E^*	S^*	CS
1	26.565	.894	.447	.8	.199	0.399
2	0	1	0	1	0	0
3	153.435	-.894	-.447	.8	.199	-0.399
4	26.565	.894	.447	.8	.199	0.399
5	90	0	1	0	1	0

$$[K^{\circ}] = \frac{EA}{4.47} \begin{bmatrix} u_1 & v_1 & u_2 & v_2 \\ 0.8 & 0.399 & -0.8 & -0.399 \\ - & 0.199 & -0.399 & -0.199 \\ \text{Sym} & & 0.8 & 0.399 \\ & & & 0.199 \end{bmatrix} \begin{matrix} u_1 \\ v_1 \\ u_2 \\ v_2 \end{matrix}$$

$$[K^{\circ}] = \frac{EA}{8} \begin{bmatrix} u_1 & v_1 & u_4 & v_4 \\ 1 & 0 & -1 & 0 \\ - & 0 & 0 & 0 \\ \text{Sym} & & 1 & 0 \\ & & & 0 \end{bmatrix} \begin{matrix} u_1 \\ v_1 \\ u_4 \\ v_4 \end{matrix}$$

$$[K^{\circ}] = \frac{EA}{4.47} \begin{bmatrix} u_2 & v_2 & u_3 & v_3 \\ 0.8 & 0.399 & -0.8 & -0.399 \\ - & 0.199 & 0.399 & -0.199 \\ \text{Sym} & & 0.8 & 0.399 \\ & & & 0.199 \end{bmatrix} \begin{matrix} u_2 \\ v_2 \\ u_3 \\ v_3 \end{matrix}$$

$$[K^{\circ}] = \frac{EA}{4.47} \begin{bmatrix} u_1 & v_1 & u_2 & v_2 \\ 0.8 & -0.399 & -0.8 & 0.399 \\ - & 0.199 & -0.399 & -0.199 \\ \text{Sym} & & 0.8 & -0.399 \\ & & & 0.199 \end{bmatrix} \begin{matrix} u_1 \\ v_1 \\ u_2 \\ v_2 \end{matrix}$$

$$[K^{\circ}] = \frac{EA}{4} \begin{bmatrix} u_4 & v_4 & u_3 & v_3 \\ 0 & 0 & 0 & 0 \\ - & 1 & 0 & -1 \\ \text{Sym} & & 0 & 0 \\ & & & 1 \end{bmatrix} \begin{matrix} u_4 \\ v_4 \\ u_3 \\ v_3 \end{matrix}$$

$$\begin{bmatrix} 0.304 & -0.179 & -0.089 & 0 & 0 \\ -0.179 & 0.586 & 0.089 & -0.089 & -0.089 \\ -0.089 & 0.089 & 0.1335 & -0.0445 & -0.089 \\ 0 & -0.089 & 0.0445 & 0.295 & -0.25 \\ 0 & -0.089 & -0.089 & -0.25 & 0.295 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \\ v_2 \\ v_3 \\ v_4 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ -20 \\ -10 \\ 0 \end{bmatrix}$$

$$EA(0.304 u_1 - 0.179 u_2 - 0.089 v_2) = 0$$

$$EA(-0.179 u_1 + 0.586 u_2 + 0.089 v_2 - 0.089 v_3 - 0.089 v_4) = 0$$

$$EA(-0.089 u_1 + 0.089 u_2 + 0.1335 v_2 - 0.0445 v_3 - 0.089 v_4) = -20$$

$$EA(-0.089 u_2 - 0.0445 v_2 + 0.295 v_3 - 0.25 v_4) = -20$$

$$EA(-0.089 u_2 - 0.089 v_2 - 0.25 v_3 + 0.25 v_4) = 0$$

$$u_1 = -1.13 \times 10^{-3} \text{ m} = u_6$$

$$u_2 = 3.93 \times 10^{-4} \text{ m} = u_5$$

$$v_2 = -4.61 \times 10^{-3} \text{ m} = v_5$$

$$v_3 = -5.09 \times 10^{-3} \text{ m}$$

$$v_4 = -5.13 \times 10^{-3} \text{ m}$$

$$\sigma_1 = \frac{E}{L} \begin{bmatrix} -c & -s & c & s \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \\ v_2 \end{bmatrix} \Rightarrow \frac{210 \times 10^9}{4.47} \left(-0.894(-1.13 \times 10^{-3}) + 0.894(3.93 \times 10^{-4}) + 0.447(-4.61 \times 10^{-3}) \right)$$

$$\sigma_1 = \sigma_2 = -31.36 \text{ MPa} \quad \text{Compression}$$

$$\sigma_2 = \frac{E}{L} \begin{bmatrix} -c & s & c & -s \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \\ v_2 \end{bmatrix} \Rightarrow \frac{210 \times 10^9}{8} \left(-1 \times -1.13 \times 10^{-3} \right)$$

$$\sigma_2 = \sigma_3 = 29.66 \text{ MPa} \quad \text{Tension}$$

$$\sigma_3 = \frac{E}{L} \begin{bmatrix} +0.894 & -0.447 & -0.894 & 0.447 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \\ v_2 \end{bmatrix} \Rightarrow \frac{210 \times 10^9}{4.47} \left(-0.447(-5.13 \times 10^{-3}) - 0.894(3.93 \times 10^{-4}) + 0.447(-4.61 \times 10^{-3}) \right)$$

$$\sigma_3 = \sigma_4 = -5.67 \text{ MPa} \quad \text{Compression}$$

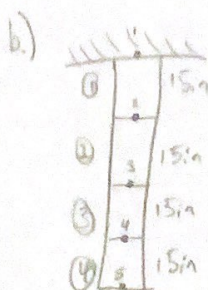
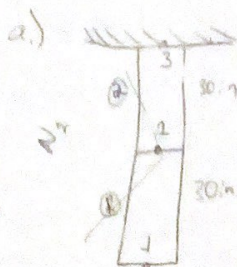
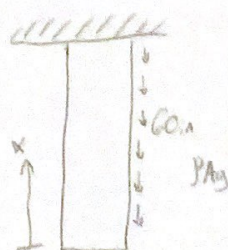
$$\sigma_4 = \frac{E}{L} \begin{bmatrix} -0.894 & -0.447 & 0.894 & 0.447 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \\ v_2 \end{bmatrix} \Rightarrow \frac{210 \times 10^9}{4.47} \left(-0.894(3.93 \times 10^{-4}) - 0.447(-4.61 \times 10^{-3}) + 0.447(-5.09 \times 10^{-3}) \right)$$

$$\sigma_4 = \sigma_5 = -32.08 \text{ MPa} \quad \text{Compression}$$

$$\sigma_5 = \frac{E}{L} \begin{bmatrix} 0 & -1 & 0 & 1 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \\ v_2 \end{bmatrix} \Rightarrow \frac{210 \times 10^9}{4} \left(-1(-5.13 \times 10^{-3}) + 1(-5.09 \times 10^{-3}) \right)$$

$$\sigma_5 = 2.1 \text{ MPa} \quad \text{Tension}$$

3.]



$$A = 2 \text{ in}^2$$

$$E = 30 \times 10^6 \text{ psi}$$

$$P = 0.283 \text{ lb/in}^3$$

$$g = 2.683 \frac{\text{in}}{\text{s}^2}$$

$$\downarrow PAQ \text{ (force/length)}$$

$$\downarrow = 0.566 \frac{\text{lb}}{\text{in}}$$

$$\downarrow = -33.96 \text{ lb}$$

$$T(x) = -0.566$$

$$\frac{d\pi}{dx} = 0 \Rightarrow \frac{\delta U}{\delta \delta} + \frac{\delta \Omega_a}{\delta \delta} + \frac{\delta \Omega_b}{\delta \delta} + \frac{\delta \Omega_c}{\delta \delta} = 0$$

Internal Force is a function of x

Determine nodal displacements

a.)

$$[K]\{\delta\} = \{f_B\} \quad 0 = \frac{\delta U}{\delta \delta} + \frac{\delta \Omega_b}{\delta \delta}$$

$$[K]\{\delta\} = \{f_B\} + \{f_a\} + \{f_b\} \Rightarrow$$

$$\Omega_b = -\int_0^L T(x) u(x) dx \Rightarrow \{f_B\} = \int_0^L \begin{Bmatrix} N_1(x) \\ N_2(x) \\ N_3(x) \end{Bmatrix} T(x) dx$$

$$\int_0^L \begin{Bmatrix} (1 - \frac{x}{L})(-0.5663) \\ (\frac{x}{L})(-0.5663) \\ (1 - \frac{x}{L})(-0.5663) \\ (\frac{x}{L})(-0.5663) \end{Bmatrix} dx$$

$$f_1^0 = -(0.5663) \left(\frac{x^2}{2} - \frac{x^3}{6L} \right) \Big|_0^L = -8.4945 \text{ lb}$$

$$f_2^0 = -(0.5663) \left(\frac{x^2}{2L} \right) \Big|_0^L = -8.4945 \text{ lb}$$

$$f_3^0 = -8.4945 \text{ lb}$$

$$f_4^0 = -8.4945 \text{ lb}$$

$$[K]\{\delta\} = \{f_B\}$$

$$K = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$K^0 = \begin{bmatrix} 2 \times 10^6 & -2 \times 10^6 \\ -2 \times 10^6 & 2 \times 10^6 \end{bmatrix}$$

$$K^0 = \begin{bmatrix} 2 \times 10^6 & -2 \times 10^6 \\ -2 \times 10^6 & 2 \times 10^6 \end{bmatrix}$$

$$\begin{bmatrix} 2 \times 10^6 & -2 \times 10^6 & 0 \\ -2 \times 10^6 & 4 \times 10^6 & -2 \times 10^6 \\ 0 & -2 \times 10^6 & 2 \times 10^6 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \\ u_3 \end{bmatrix} = \begin{bmatrix} -8.4945 \\ -16.989 \\ -8.4945 \end{bmatrix}$$

$$u_1 = -1.6989 \times 10^{-5} \text{ in}$$

$$u_2 = -1.6989 \times 10^{-5} \text{ in}$$

$$u_3 = 0$$

$$\frac{EA}{L} = 2 \times 10^6$$



$$b) \{f_B\} = \int_0^L N(x) T(x) dx$$

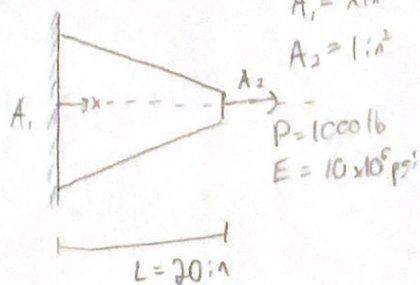
$$\int_0^L \left\{ \begin{array}{l} (1 - \frac{x}{L})(0.566) \\ (\frac{x}{L})(0.566) \\ (1 - \frac{x}{L})(0.566) \\ (\frac{x}{L})(0.566) \\ (1 - \frac{x}{L})(0.566) \\ (\frac{x}{L})(0.566) \\ (1 - \frac{x}{L})(0.566) \\ (\frac{x}{L})(0.566) \end{array} \right\} N_i T dx \Rightarrow \text{All equal values}$$

$$\{f_B\} = \begin{bmatrix} -4.245 \\ -8.49 \\ -8.49 \\ -8.49 \\ -8.49 \\ -4.245 \end{bmatrix} = \begin{bmatrix} F_1 \\ F_2 \\ F_3 \\ F_4 \\ F_5 \end{bmatrix}$$

$$\begin{bmatrix} 2 \times 10^6 & -2 \times 10^6 & 0 & 0 & 0 \\ -2 \times 10^6 & 4 \times 10^6 & -2 \times 10^6 & 0 & 0 \\ 0 & -2 \times 10^6 & 4 \times 10^6 & -2 \times 10^6 & 0 \\ 0 & 0 & -2 \times 10^6 & 4 \times 10^6 & -2 \times 10^6 \\ 0 & 0 & 0 & -2 \times 10^6 & 2 \times 10^6 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \\ u_5 \end{bmatrix} = \begin{bmatrix} -4.245 \\ -8.49 \\ -8.49 \\ -8.49 \\ -4.245 \end{bmatrix}$$

$$\begin{aligned} u_1 &= -1.6989 \times 10^{-5} \\ u_2 &= -1.593 \times 10^{-5} \\ u_3 &= -1.274 \times 10^{-5} \\ u_4 &= -7.432 \times 10^{-6} \\ u_5 &= 0 \end{aligned}$$

4.]



Point collocation method
at $x = \frac{L}{3}, \frac{2L}{3}, L$

$$A\left(\frac{L}{3}\right) = 1\frac{2}{3}$$

$$A\left(\frac{2L}{3}\right) = 1\frac{1}{3}$$

$$A(L) = 1$$

$$U(x) = C_1 x + C_2 x^2 + C_3 x^3$$

$$R\left(C, x = \frac{L}{3}\right) = 0 = AE\left[C_1 + 2C_2\left(\frac{L}{3}\right) + 3C_3\left(\frac{L}{3}\right)^2\right]$$

$$R\left(C, x = \frac{2L}{3}\right) = 0 = AE\left[C_1 + 2C_2\left(\frac{2L}{3}\right) + 3C_3\left(\frac{2L}{3}\right)^2\right]$$

$$R\left(C, x = L\right) = 0 = AE\left[C_1 + 2C_2(L) + 3C_3(L)^2\right] - 1000$$

$$AE\left(C_1 \frac{L}{3} + C_2 \frac{L^2}{9} + C_3 \frac{L^3}{27}\right) = 0$$

$$C_1 = 5.5 \times 10^{-5}$$

$$C_2 = 1 \times 10^{-8}$$

$$C_3 = 3.75 \times 10^{-8}$$

Solve $U\left(\frac{L}{3}\right), U\left(\frac{2L}{3}\right), U(L)$

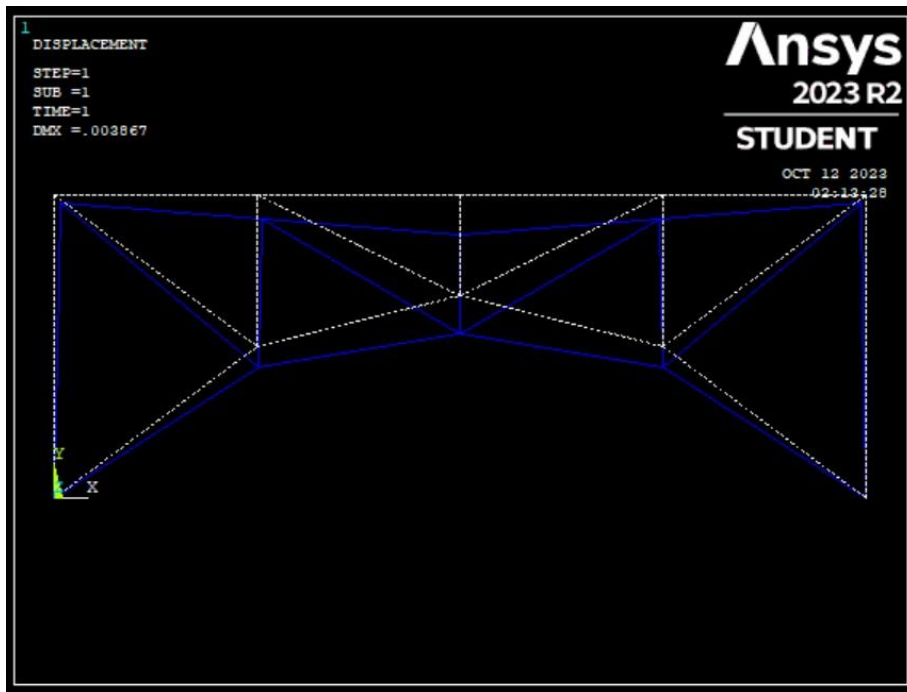
$$U(x) = (5.5 \times 10^{-5})x + (1 \times 10^{-8})x^2 + (3.75 \times 10^{-8})x^3$$

$$U\left(\frac{L}{3}\right) = 3.77 \times 10^{-4}$$

$$U\left(\frac{2L}{3}\right) = 2.22 \times 10^{-4}$$

$$U(L) = 1.4 \times 10^{-3}$$

Problem 5



PRINT ELEMENT TABLE ITEMS PER ELEMENT

***** POST1 ELEMENT TABLE LISTING *****

STAT ELEM	CURRENT AXSTRESS
1	-421.41
2	171.83
3	-623.94
4	-387.04
5	-137.47
6	-274.93
7	-318.31
8	153.69
9	-372.82
10	-274.93
11	-372.82
12	-387.04
13	153.69
14	171.83
15	-137.47
16	-421.41
17	-623.94

MINIMUM VALUES
ELEM 3
VALUE -623.94

MAXIMUM VALUES
ELEM 2
VALUE 171.83

PRINT U NODAL SOLUTION PER NODE

***** POST1 NODAL DEGREE OF FREEDOM LISTING *****

LOAD STEP= 1 SUBSTEP= 1
TIME= 1.0000 LOAD CASE= 0

THE FOLLOWING DEGREE OF FREEDOM RESULTS ARE IN THE GLOBAL COORDINATE SYSTEM

NODE	UX	UY	UZ	USUM
1	0.0000	0.0000	0.0000	0.0000
2	0.56882E-003	-0.87188E-003	0.0000	0.10410E-002
3	0.37921E-003	-0.23589E-002	0.0000	0.23892E-002
4	0.12418E-003	-0.19585E-002	0.0000	0.19624E-002
5	-0.24068E-018	-0.36473E-002	0.0000	0.36473E-002
6	-0.18191E-018	-0.38668E-002	0.0000	0.38668E-002
7	-0.12418E-003	-0.19585E-002	0.0000	0.19624E-002
8	-0.37921E-003	-0.23589E-002	0.0000	0.23892E-002
9	-0.56882E-003	-0.87188E-003	0.0000	0.10410E-002
10	0.0000	0.0000	0.0000	0.0000

MAXIMUM ABSOLUTE VALUES
NODE 2 6 0 6
VALUE 0.56882E-003 -0.38668E-002 0.0000 0.38668E-002

/FILNAM,P1-6
/title, P1-6_Truss /prep7
et, 1, link180

! Material 1
mp, ex, 1, 3.e7
mp, prxy, 1, 0.28

r, 1, 3.141592


```

n, 1, 0.0, 0.0, 0.0
n, 2, 0.0, 60.0, 0.0
n, 3, 40.0, 60.0, 0.0
n, 4, 40.0, 30.0, 0.0
n, 5, 80.0, 60.0, 0.0
n, 6, 80.0, 40.0, 0.0
n, 7, 120.0, 60.0, 0.0
n, 8, 120.0, 30.0, 0.0
n, 9, 160.0, 60.0, 0.0
n, 10, 160.0, 0.0, 0.0

```

! Set properties before creating elements

```
mat, 1 real, 1
```

```
en, 1, 1, 2
```

```
en, 2, 2, 3
```

```
d, 1, all, 0.
```

```
d, 10, all, 0.
```

```
f, 2, fy, -1000.
```

```
f, 3, fy, -1000.
```

```
f, 5, fy, -1000.
```

```
f, 7, fy, -1000.
```

```
f, 9, fy, -1000.
```

```
finish
```

```
/solu
```

```
antype, static
```

```
solve
```

```
save
```

```
finish
```

Problem 6

```
PRINT U      NODAL SOLUTION PER NODE
```

```
***** POST1 NODAL DEGREE OF FREEDOM LISTING *****
```

```
LOAD STEP=      1  SUBSTEP=      1
TIME=      1.0000      LOAD CASE=      0
```

```
THE FOLLOWING DEGREE OF FREEDOM RESULTS ARE IN THE GLOBAL COORDINATE SYSTEM
```

NODE	UX	UY	UZ	USUM
1	-0.22857E-002	0.0000	0.0000	0.22857E-002
2	0.89383E-003	-0.95535E-002	0.0000	0.95952E-002
3	0.0000	-0.98954E-002	0.0000	0.98954E-002
4	0.0000	-0.10276E-001	0.0000	0.10276E-001

```
MAXIMUM ABSOLUTE VALUES
```

NODE	1	4	0	4
VALUE	-0.22857E-002	-0.10276E-001	0.0000	0.10276E-001

PRINT S ELEMENT SOLUTION PER ELEMENT

***** POST1 ELEMENT NODAL STRESS LISTING *****

LOAD STEP= 1 SUBSTEP= 1
TIME= 1.0000 LOAD CASE= 0

THE FOLLOWING X,Y,Z VALUES ARE IN GLOBAL COORDINATES

ELEMENT=	10	LINK180				
NODE	SX	SY	SZ	SXY	SYZ	SXZ
1	-0.67082E+008	0.0000	0.0000	0.0000	0.0000	0.0000
2	-0.67082E+008	0.0000	0.0000	0.0000	0.0000	0.0000

ELEMENT=	11	LINK180				
NODE	SX	SY	SZ	SXY	SYZ	SXZ
1	0.60000E+008	0.0000	0.0000	0.0000	0.0000	0.0000
4	0.60000E+008	0.0000	0.0000	0.0000	0.0000	0.0000

ELEMENT=	12	LINK180				
NODE	SX	SY	SZ	SXY	SYZ	SXZ
2	-0.22361E+008	0.0000	0.0000	0.0000	0.0000	0.0000
4	-0.22361E+008	0.0000	0.0000	0.0000	0.0000	0.0000

ELEMENT=	13	LINK180				
NODE	SX	SY	SZ	SXY	SYZ	SXZ
2	-0.44721E+008	0.0000	0.0000	0.0000	0.0000	0.0000
3	-0.44721E+008	0.0000	0.0000	0.0000	0.0000	0.0000

ELEMENT=	14	LINK180				
NODE	SX	SY	SZ	SXY	SYZ	SXZ
3	0.20000E+008	0.0000	0.0000	0.0000	0.0000	0.0000
4	0.20000E+008	0.0000	0.0000	0.0000	0.0000	0.0000

Problem 7

PRINT ELEMENT TABLE ITEMS PER ELEMENT

***** POST1 ELEMENT TABLE LISTING *****

STAT	CURRENT
ELEM	AXSTRESS
1	-67082040.
2	49081504.
3	-22360680.
4	-44721360.
5	20000000.
6	-44721360.
7	-22360680.
8	49081504.
9	-67082040.

MINIMUM VALUES
ELEM 1
VALUE -0.67082E+008

MAXIMUM VALUES
ELEM 2
VALUE 0.49082E+008

***** POST1 NODAL DEGREE OF FREEDOM LISTING *****

LOAD STEP= 1 SUBSTEP= 1
TIME= 1.0000 LOAD CASE= 0

THE FOLLOWING DEGREE OF FREEDOM RESULTS ARE IN THE GLOBAL COORDINATE SYSTEM

NODE	UX	UY	UZ	USUM
1	0.0000	0.0000	0.0000	0.0000
2	0.25935E-002	-0.83813E-002	0.0000	0.87734E-002
3	0.15295E-002	-0.83830E-002	0.0000	0.85214E-002
4	0.18698E-002	-0.87640E-002	0.0000	0.89612E-002
5	0.80581E-003	-0.77008E-002	0.0000	0.77429E-002
6	0.37395E-002	0.13610E-002	0.0000	0.39795E-002

MAXIMUM ABSOLUTE VALUES

NODE	6	4	0	4
VALUE	0.37395E-002	-0.87640E-002	0.0000	0.89612E-002