Analysis of continuous beam by force method

Input data

Span lengths - $\vec{l} = [4; 7; 3; 5] \cdot m = [4 \text{ m } 7 \text{ m } 3 \text{ m } 5 \text{ m}]$

Number of spans - $n = len(\vec{l}) = 4$

Coordinates of supports - $\vec{x} = [4 \text{ m } 11 \text{ m } 14 \text{ m } 19 \text{ m}]$

Total beam length - $L = \vec{x}_4 = 19 \,\mathrm{m}$

Load -
$$q = 10 \frac{\mathrm{kN}}{\mathrm{m}}$$

Elastic modulus of the material - $E = 30 \, \text{GPa}$

Cross section

Rectangular section with dimensions: $b = 250 \, \text{mm}$, $h = 500 \, \text{mm}$

Area - $A = b \cdot h = 125000 \, \text{mm}^2$

Moment of inertia - $I = \frac{b \cdot h^3}{12} = 2604166667 \text{ mm}^4$

Shear area - $A_Q = \frac{5}{6} \cdot b \cdot h = 104167 \text{ mm}^2$

Solution

The solution will be performed by the force method with a primary system - simply supported beam with internal supports removed and replaced by unknown forces X_i

Bending moments

- in section a due to unit force at distance x from the beginning of the beam:

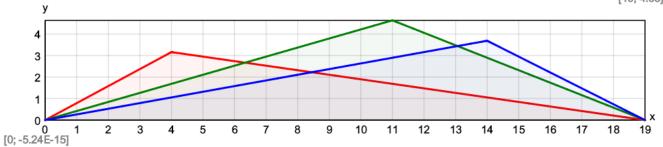
$$M_{1,max}(x) = \left(\frac{x}{L} - 1\right) \cdot x$$

$$M_{1,a}(a;x) = M_{1,max}(x) \cdot \begin{cases} if \ a < x: & \frac{a}{x} \\ else: & \frac{L-a}{L-x} \end{cases}$$

- in section a, due to unit force X_i :

$$M_1(a; i) = M_{1,a}(a; \vec{x}_i)$$





- due to external loads in primary system:

$$M_F(x) = \frac{q \cdot x}{2} \cdot (L - x)$$



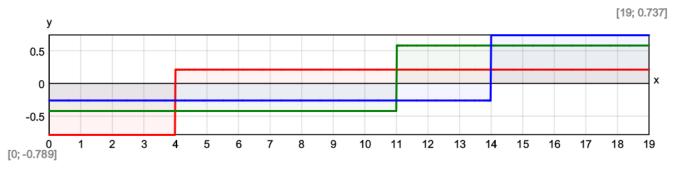
Shear forces

- in section a due to unit force at distance x from the beginning of the beam:

$$V_{1,a}(a;x) = \begin{cases} if \ a < x: & \frac{x}{L} - 1 \\ else: & \frac{x}{L} \end{cases}$$

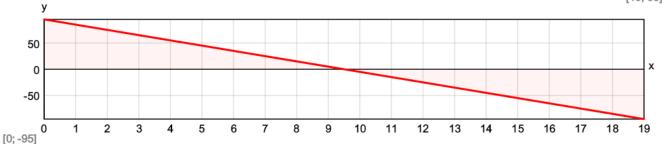
- in section a, due to unit force X_i :

$$V_1(a;i) = V_{1,a}(a;\vec{x}_i)$$



- due to external loads in primary system:

$$V_F(x) = q \cdot \left(\frac{L}{2} - x\right)$$



Number of unknowns by force method - $n_1 = n - 1 = 3$

Flexibility coefficients

$$\delta(i;j) = \int_{0 \text{ m}}^{L} \frac{M_1(x;i) \cdot M_1(x;j)}{E \cdot I} dx + \int_{0 \text{ m}}^{L} \frac{V_1(x;i) \cdot V_1(x;j)}{E \cdot A_Q} dx$$

$$\Delta_F(i) = \int_{0 \text{ m}}^L \frac{M_1(x;i) \cdot M_F(x)}{E \cdot I} dx + \int_{0 \text{ m}}^L \frac{V_1(x;i) \cdot V_F(x)}{E \cdot A_Q} dx$$

$$\delta = \operatorname{symmetric}(n_1) = \begin{bmatrix} 0 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix}, \vec{\Delta}_F = \operatorname{vector}(n_1) = \begin{bmatrix} 0 & 0 & 0 \end{bmatrix}$$

 $Repeat\{Repeat\{\delta_{i,j} = \delta(i;j); i = 1...n_1\}; j = 1...n_1\} = 0.0011 \text{ m/kN}$

$$Repeat\{\vec{\Delta}_{F.i} = \Delta_F(i); i = 1...n_1\} = -0.161 \,\mathrm{m}$$

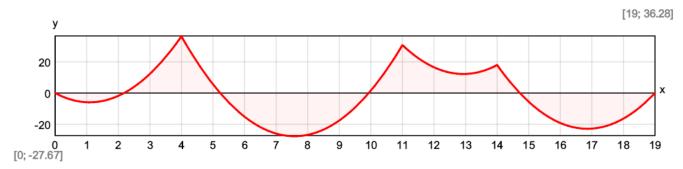
$$\delta = \begin{bmatrix} 0.000809 \, \text{m/kN} & 0.00101 \, \text{m/kN} & 0.000719 \, \text{m/kN} \\ 0.00101 \, \text{m/kN} & 0.00174 \, \text{m/kN} & 0.00133 \, \text{m/kN} \\ 0.000719 \, \text{m/kN} & 0.00133 \, \text{m/kN} & 0.00111 \, \text{m/kN} \end{bmatrix}, \vec{\Delta}_F = \begin{bmatrix} -0.135 \, \text{m} & -0.211 \, \text{m} & -0.161 \, \text{m} \end{bmatrix}$$

Calculation of the unknown forces X_i

$$\vec{X} = -\text{clsolve}(\delta; \vec{\Delta}_F) = [64.9 \,\text{kN} \, 53.47 \,\text{kN} \, 39.33 \,\text{kN}]$$

Results

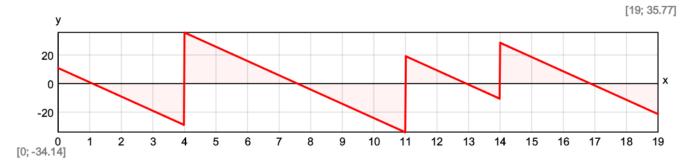
Bending moment diagram - $M(x) = M_F(x) + \sum_{i=1}^{n_1} M_1(x;i) \cdot X_i$



$$\vec{M}_{max} = [5.94 \text{ kNm } 27.69 \text{ kNm } -12.23 \text{ kNm } 22.91 \text{ kNm}]$$

$$\vec{M}_{min} = [-36.39 \,\text{kNm} - 30.79 \,\text{kNm} - 17.98 \,\text{kNm}]$$

Shear force diagram - $V(x) = V_F(x) + \sum_{i=1}^{n_1} V_1(x;i) \cdot \vec{X}_i$



$$\vec{V}_{max} = [10.9 \,\text{kN} \ 35.8 \,\text{kN} \ 19.27 \,\text{kN} \ 28.6 \,\text{kN}]$$

$$\vec{V}_{min} = [-29.1 \,\mathrm{kN} - 34.2 \,\mathrm{kN} - 10.73 \,\mathrm{kN} - 21.4 \,\mathrm{kN}]$$

Deflections

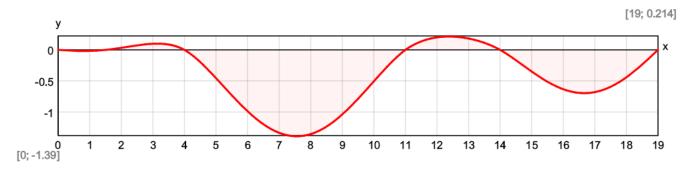
- in section a, due to unit force X_i:

$$d_1(a;i) = \int_{0 \text{ m}}^{L} \frac{M_{1,a}(x;a) \cdot M_1(x;i)}{E \cdot I} dx + \int_{0 \text{ m}}^{L} \frac{V_{1,a}(x;a) \cdot V_1(x;i)}{E \cdot A_Q} dx$$

- due to external loads in primary system:

$$d_F(a) = \int_{0 \text{ m}}^{L} \frac{M_{1,a}(x;a) \cdot M_F(x)}{E \cdot I} dx + \int_{0 \text{ m}}^{L} \frac{V_{1,a}(x;a) \cdot V_F(x)}{E \cdot A_Q} dx$$

$$d(x) = d_F(x) + \sum_{i=1}^{n_1} d_1(x; i) \cdot \vec{X}_i$$

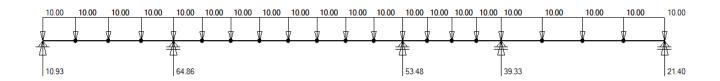


Maximum deflection - $d_{max} = \frac{1}{d(x)}; x \in [0 \text{ m}; L] = -1.39 \text{ mm}$

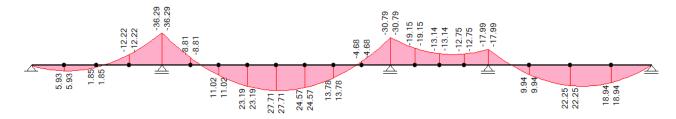
At a distance from the origin - $x_{inf} = 7.56 \, \mathrm{m}$

Comparison with Stadyps 6.0 structural analysis software

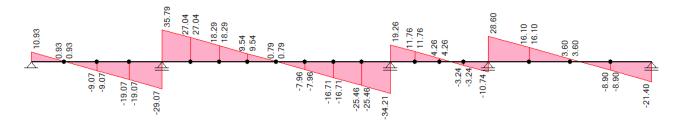
Loads and support reactions, kN



Bending moments, kNm



Shear forces, kN



Deflections, mm

