#### 1 Abstract

## 2 Introduction

Short paragraph defining homogenization and how it can be used to development a better understanding of rock mechanics. Statement about gap in the literature addressed in this article, i.e., not yet applied to HF.

Discussion of the natural fractured rocks as a multiscale material.

Brief discussion of literature describing multiscale (upscaling) modelling methods. Concurrent vs hierarchical multiscale analysis. See my review article on my website and [Gracie and Belytschko(2011)Gracie, and Belytschko]. Justify that a hierarchical approach is preferable here, do to long simulated time; concurrent approach is to restrictive in terms of time step size. Also justify/explain selection of homogenization instead of an alternative method of upscaling.

Discussion of literature around modeling using homogenization.

- Continuum to continuum and DEM to Continuum. rigid versus deformable particals
- $\bullet$  Analogy to debonding of particle -; note lack of contact.
- What are the common characteristics of the models.
- What are the unique characteristics of the models.

Overview of DEM literature with a focus on HF Outline of the article

#### 3 Distinct Element Method

# 4 Homogenization Approach

#### 4.1 Stress Homogenization

$$\langle \boldsymbol{\sigma} \rangle^{\Omega} = \frac{1}{A} \int_{\Omega} \boldsymbol{\sigma} dA \tag{1}$$

$$\langle \boldsymbol{\sigma} \rangle^{\Omega} = \frac{1}{A} \left[ \int_{\Omega_r} \boldsymbol{\sigma} dA_r + \int_{\Omega_f} \boldsymbol{\sigma} dA_f \right]$$
 (2)

$$\langle \boldsymbol{\sigma} \rangle^{\Omega} = \frac{1}{A} \left[ \sum_{i=1}^{N_b} \boldsymbol{\sigma}^i A_b^i + \sum_{i=1}^{N_d} \boldsymbol{\sigma}^i A_d^i \right]$$
 (3)

$$\langle \boldsymbol{\sigma} \rangle^{\Omega} = \frac{1}{A} \left[ \sum_{i=1}^{N_b} \sum_{j=1}^{N_z} \boldsymbol{\sigma}_z^{ij} A_z^{ij} + \sum_{i=1}^{N_d} p_d^i \mathbf{I} A_d^i \right]$$
(4)

#### 4.2 Strain Homogenization

$$\boldsymbol{\epsilon} = \frac{1}{2} \left[ \nabla \mathbf{u} + (\nabla \mathbf{u})^T \right] \tag{5}$$

$$\langle \epsilon \rangle^{\Omega} = \frac{1}{A} \int_{\Omega} \epsilon dA \tag{6}$$

$$\langle \epsilon \rangle^{\Omega} = \frac{1}{A} \left[ \int_{\Omega_r} \epsilon dA_r + \int_{\Omega_f} \epsilon dA_f \right]$$
 (7)

$$\langle \boldsymbol{\epsilon} \rangle^{\Omega} = \frac{1}{2A} \left[ \int_{\Omega_r} \left[ \nabla \mathbf{u} + (\nabla \mathbf{u})^T \right] dA_r + \int_{\Omega_f} \left[ \nabla \mathbf{u} + (\nabla \mathbf{u})^T \right] dA_f \right]$$
(8)

$$\langle \boldsymbol{\epsilon} \rangle^{\Omega} = \frac{1}{2A} \left[ \oint_{\Gamma_r} \left[ \mathbf{u} \otimes \mathbf{n} + \mathbf{n} \otimes \mathbf{u} \right] d\Gamma_r + \oint_{\Gamma_f} \left[ \mathbf{u} \otimes \mathbf{n} + \mathbf{n} \otimes \mathbf{u} \right] d\Gamma_f \right]$$
(9)

$$\langle \boldsymbol{\epsilon} \rangle^{\Omega} = \frac{1}{2A} \left[ \sum_{i=1}^{N_{rb}} \left[ \mathbf{u}_{rb}^{i} \otimes \mathbf{n}_{rb}^{i} + \mathbf{n}_{rb}^{i} \otimes \mathbf{u}_{rb}^{i} \right] L_{rb}^{i} + \sum_{i=1}^{N_{db}} \left[ \mathbf{u}_{db}^{i} \otimes \mathbf{n}_{db}^{i} + \mathbf{n}_{db}^{i} \otimes \mathbf{u}_{db}^{i} \right] L_{db}^{i} \right]$$

$$\tag{10}$$

# 5 Damage Plasticty Model For Quasi-Brittle Materials

# 5.1 Governing equations of the coarse-scale (continuum) model

Consider the dynamic equilibrium of a naturally fractured rock mass  $\Omega$ . Let  $\Gamma$  denote the boundary of  $\Omega$  and let  $\Gamma$  be divided into mutually exclusive sets  $\Gamma_u$  and  $\Gamma_t$ . The body contains a set of natural fractures denoted by  $\Gamma_{cr}$  and is subjected to a body force  $\mathbf{g}$ . Let material points in the the undeformed and the deformed configuration be denoted by  $\mathbf{X}$  and  $\mathbf{x}$ , respectively. Let  $\mathbf{u}(\mathbf{X},t) = \mathbf{x}(\mathbf{X},t) - \mathbf{X}$  denote the displacement of material point  $\mathbf{x}$  at time t. Equilibrium of  $\Omega$  is governed by

$$\rho_s \ddot{\mathbf{u}} = \nabla \cdot \boldsymbol{\sigma} + \mathbf{g}, \ \forall \mathbf{x} \in \Omega, t \ge 0, \tag{11}$$

in which  $\ddot{\mathbf{u}} = \ddot{\mathbf{u}}(\mathbf{x}, t)$  denotes the second partial derivative of the displacement field and  $\rho_s$  is the density of the rock mass.

- 5.2 FEM Forumulation
- 5.3 Levenburg-Marquardt Algorithm
- 5.4 Parameter Initial and Boundary Estimates
- 6 Material Testing Example
- 7 Results
- 8 Conclusions

### References

[Gracie and Belytschko(2011)Gracie, and Belytschko] Gracie, R.; Belytschko, T. International Journal for Numerical Methods in Engineering 2011, 86, 575–597.