

# Inhaltsverzeichnis

<b>1</b>	<b>Laplace equation</b>	<b>2</b>
1.1	Finite Element formulation . . . . .	2
1.2	Ansatz functions . . . . .	3
1.3	Transformation of integration domain . . . . .	3
1.4	Evaluation of the integral term . . . . .	8
1.5	Boundary Conditions . . . . .	10
1.6	Function spaces . . . . .	11
<b>2</b>	<b>Poisson Equation</b>	<b>12</b>
<b>3</b>	<b>Generalized Laplace operator</b>	<b>14</b>
<b>4</b>	<b>Diffusion Equation</b>	<b>14</b>
4.1	Analytical Solution . . . . .	15
<b>5</b>	<b>Multidomain</b>	<b>16</b>
<b>6</b>	<b>Structural mechanics</b>	<b>16</b>
6.1	Introduction of some quantities . . . . .	16
6.2	Green-Lagrange strain tensor and derivatives . . . . .	17
6.3	Material modeling . . . . .	18
6.4	Elasticity tensor . . . . .	19
6.5	Constitutive Equations . . . . .	20
6.6	Formulation in current configuration . . . . .	23
6.7	Incompressible solid mechanics . . . . .	24
6.8	Dynamic elasticity . . . . .	31
6.9	Examples with analytical solution . . . . .	31
6.10	2D uniaxial extension with constant total load . . . . .	38
<b>7</b>	<b>Quadrature</b>	<b>43</b>
7.1	Gaussian quadrature . . . . .	43
7.2	Clenshaw-Curtis quadrature . . . . .	43
<b>8</b>	<b>Propositions</b>	<b>44</b>
8.1	Divergence theorem . . . . .	44
8.2	Classical Stoke's theorem . . . . .	45
8.3	Integration on manifolds . . . . .	45

# 1 Laplace equation

For a computational domain  $\Omega \subset \mathbb{R}^d$  the Laplace equation reads

$$\Delta u = 0 \quad \text{on } \Omega. \quad (1)$$

A classical solution  $u : \Omega \rightarrow \mathbb{R}$  fulfills (1). For a unique solution also boundary conditions have to be specified, e.g.

$$\begin{aligned} \nabla u(\mathbf{x}) \cdot \mathbf{n} &= 0 & \text{on } \Gamma_N, \\ u(\mathbf{x}) &= u_0(\mathbf{x}) & \text{on } \Gamma_D, \end{aligned} \quad (2)$$

where the homogeneous Neumann-type boundary conditions for  $\mathbf{x} \in \Gamma_N$  set the flux over the boundary in normal direction  $\mathbf{n}$  to zero and the Dirichlet-type boundary conditions on  $\Gamma_D$  prescribe a value for  $u$  on the boundary.

## 1.1 Finite Element formulation

By multiplication of a testfunction  $\phi \in H_0^1(\Omega)$  and integration follows the weak formulation of (1):

$$\int_{\Omega} \Delta u \phi \, d\mathbf{x} = 0 \quad \forall \phi \in H_0^1(\Omega) \quad (3)$$

For a definition of  $H_0^1$  see section 1.6.

The Laplace operator can be written as  $\Delta u = \nabla \cdot (\nabla u)$ . Applying divergence theorem in form of (39) with  $f = \phi$  and  $\mathbf{F} = \nabla u$  yields

$$-\int_{\Omega} \nabla u \cdot \nabla \phi \, d\mathbf{x} + \int_{\partial\Omega} (\phi \nabla u) \cdot \mathbf{n} \, d\mathbf{x} = 0 \quad \forall \phi \in H_0^1(\Omega) \quad (4)$$

Because  $\phi$  is zero on the boundary, the second term vanishes:

$$-\int_{\Omega} \nabla u \cdot \nabla \phi \, d\mathbf{x} = 0 \quad \forall \phi \in H_0^1(\Omega) \quad (5)$$

Now we have to specify a finite-dimensional ansatz space to choose the solution function from. We do this by specifying a basis and take the span of it:  $V := \text{span} \{\phi_1, \dots, \phi_n\}$ .

The numerical solution is given by

$$u_h(\mathbf{x}) = \sum_{i=1}^N u_i \phi_i(\mathbf{x}). \quad (6)$$

We also take  $V$  as the space of testfunctions. Plugging this into (25) yields

$$-\sum_{i=1}^N u_i \int_{\Omega} \nabla \phi_i \cdot \nabla \phi_j \, d\mathbf{x} = 0 \quad \text{for } j = 1, \dots, N. \quad (7)$$



Abbildung 1: 2D first-order Lagrange element

The minus sign is kept for similarity with later mentioned problem equations that also have a right-hand side.

A reasonable choice of ansatz functions are functions that have limited support. We discretize the domain  $\Omega$  by Finite Elements  $\Omega_e$ ,

$$\Omega = \bigcup_{e=1}^M \Omega_e = \Omega_1 \dot{\cup} \dots \dot{\cup} \Omega_M, \quad (8)$$

and define nodes with global indices  $N(e)$  on each element  $e$ . Interpolating ansatz functions are now chosen such that they have the value 1 at only one node and the value 0 at all other nodes. The support is contained just within the elements that are adjacent to the node where the function is 1.

## 1.2 Ansatz functions

A simple choice that fulfills the requirements are first-order Lagrange functions  $L_{i,p}$ ,  $p = 1$  which are defined as follows for  $d = 1$  and depicted in fig. 1c.

$$\varphi_i : [0, 1] \rightarrow \mathbb{R}, \quad \varphi_1(x) = L_{1,1}(x) := 1 - x, \quad \varphi_2(x) = L_{2,1}(x) := x \quad (9)$$

For higher dimensions they are composed by a tensor product ansatz.

$$\varphi_i(\mathbf{x}) = \mathbf{L}_i(\mathbf{x}) := \prod_{k=1}^d L_{j,1}(x_k) \quad (10)$$

The local numbering of the ansatz functions of an element proceeds fastest in the first dimension then in the second and so on as shown in fig. 1a

## 1.3 Transformation of integration domain

The definition of the ansatz functions was in parameter space, i.e. on the unit interval  $[0, 1]^d$ . The corresponding coordinate system is  $\boldsymbol{\xi} = \{\xi_1, \dots, \xi_d\}$ . However, integration over the elements  $\Omega_e$  of the computational domain is required. The node coordinates which define the elements are given

in the global coordinate system  $\mathbf{x} = \{x_1, \dots, x_d\}$ . A mapping from  $\boldsymbol{\xi}$  to  $\mathbf{x}$  can be performed using multi-linear interpolation between the nodal coordinates  $\mathbf{x}^i$ :

$$\mathbf{x}(\boldsymbol{\xi}) = \Phi(\boldsymbol{\xi}) := \sum_i \mathbf{L}_i(\boldsymbol{\xi}) \mathbf{x}^i. \quad (11)$$

Note that again Lagrange functions of first order appear, but this is part of the parameter space to global space mapping and independent of the choosen ansatz functions. For 1D and 2D problems Eq. (11) can be written out as:

$$\begin{aligned} \text{1D: } \Phi(\xi_1) &= (1 - \xi_1) \mathbf{x}^1 + \xi_1 \mathbf{x}^2 \\ \text{2D: } \Phi(\boldsymbol{\xi}) &= (1 - \xi_1)(1 - \xi_2) \mathbf{x}^1 + \xi_1(1 - \xi_2) \mathbf{x}^2 + (1 - \xi_1)\xi_2 \mathbf{x}^3 + \xi_1\xi_2 \mathbf{x}^4. \\ \text{3D: } \Phi(\boldsymbol{\xi}) &= (1 - \xi_1)(1 - \xi_2)(1 - \xi_3) \mathbf{x}^1 + \xi_1(1 - \xi_2)(1 - \xi_3) \mathbf{x}^2 + (1 - \xi_1)\xi_2(1 - \xi_3) \mathbf{x}^3 + \xi_1\xi_2(1 - \xi_3) \mathbf{x}^4 \\ &\quad + (1 - \xi_1)(1 - \xi_2)\xi_3 \mathbf{x}^5 + \xi_1(1 - \xi_2)\xi_3 \mathbf{x}^6 + (1 - \xi_1)\xi_2\xi_3 \mathbf{x}^7 + \xi_1\xi_2\xi_3 \mathbf{x}^8 \end{aligned} \quad (12)$$

The node numbering and coordinate frames are defined by fig. 1b. The Jacobians of  $\Phi$  for the 1D and 2D case are as follows:

$$\begin{aligned} \text{1D: } J_\Phi(\xi_1) &= \Phi'(\xi_1) = \mathbf{x}^2 - \mathbf{x}^1 \\ \text{2D: } J_\Phi(\boldsymbol{\xi}) &= ((1 - \xi_2)(\mathbf{x}^2 - \mathbf{x}^1) + \xi_2(\mathbf{x}^4 - \mathbf{x}^3) \quad (1 - \xi_1)(\mathbf{x}^3 - \mathbf{x}^1) + \xi_1(\mathbf{x}^4 - \mathbf{x}^2)) \\ \text{3D: } J_\Phi(\boldsymbol{\xi}) &= (J_{\Phi,1}(\boldsymbol{\xi}) \quad J_{\Phi,2}(\boldsymbol{\xi}) \quad J_{\Phi,3}(\boldsymbol{\xi})) \\ J_{\Phi,1}(\boldsymbol{\xi}) &= (1 - \xi_2)(1 - \xi_3)(\mathbf{x}^2 - \mathbf{x}^1) + \xi_2(1 - \xi_3)(\mathbf{x}^4 - \mathbf{x}^3) + (1 - \xi_2)\xi_3(\mathbf{x}^6 - \mathbf{x}^5) + \xi_2\xi_3(\mathbf{x}^8 - \mathbf{x}^7) \\ J_{\Phi,2}(\boldsymbol{\xi}) &= (1 - \xi_1)(1 - \xi_3)(\mathbf{x}^3 - \mathbf{x}^1) + \xi_1(1 - \xi_3)(\mathbf{x}^4 - \mathbf{x}^2) + (1 - \xi_1)\xi_3(\mathbf{x}^7 - \mathbf{x}^5) + \xi_1\xi_3(\mathbf{x}^8 - \mathbf{x}^6) \\ J_{\Phi,3}(\boldsymbol{\xi}) &= (1 - \xi_1)(1 - \xi_2)(\mathbf{x}^5 - \mathbf{x}^1) + \xi_1(1 - \xi_2)(\mathbf{x}^6 - \mathbf{x}^2) + (1 - \xi_1)\xi_2(\mathbf{x}^7 - \mathbf{x}^3) + \xi_1\xi_2(\mathbf{x}^8 - \mathbf{x}^4) \end{aligned}$$

Starting from (7) we now plug in the Lagrange ansatz functions for  $\phi$ . Then the respective functions only have to be integrated over the elements where they are defined. We get

$$-\sum_{e=1}^M \sum_{i \in N(e)} u_i \int_{\Omega_e} \nabla \phi_i(\mathbf{x}) \cdot \nabla \phi_j(\mathbf{x}) \, d\mathbf{x} = 0 \quad \text{for } j = 1, \dots, N, \quad (13)$$

where the sum over  $i \in N(e)$  is over the nodes of element  $e$ . The expression  $\nabla \phi(\mathbf{x})$  means, that the gradient is with respect to  $\mathbf{x}$ , despite the function  $\phi$  being defined in parameter space, i.e.  $\nabla \phi(\mathbf{x}) = \nabla_{\mathbf{x}} \phi(\Phi^{-1}(\mathbf{x})) = \nabla_{\boldsymbol{\xi}} \phi(\boldsymbol{\xi})$ .

The integration domain,  $\Omega_e$ , is described by the mapping from parameter space,  $\Omega_e = \Phi([0, 1]^d)$ . At every point  $\mathbf{p} = \Phi(\boldsymbol{\xi})$  the gradients are with respect to orthogonal coordinates in the tangent space of the point. The tangent space coordinates for a particular point given by  $\boldsymbol{\xi}$  are introduced as  $\boldsymbol{\zeta}(\boldsymbol{\xi}) = (\zeta_1(\boldsymbol{\xi}), \dots, \zeta_d(\boldsymbol{\xi}))$ . The scaling is like in world space, i.e. the following holds:

$$\left| \frac{\partial \zeta_i}{\partial \xi_i} \right| = \left| \frac{\partial \Phi}{\partial \xi_i} \right|.$$

The integral in (13) is then

$$\int_{\Omega_e} \nabla \phi_i(\mathbf{x}) \cdot \nabla \phi_j(\mathbf{x}) \, d\mathbf{x} = \int_{\Phi([0,1]^d)} \nabla_{\boldsymbol{\zeta}(\boldsymbol{\xi}(\mathbf{x}))} \phi_i(\mathbf{x}) \cdot \nabla_{\boldsymbol{\zeta}(\boldsymbol{\xi}(\mathbf{x}))} \phi_j(\mathbf{x}) \, d\mathbf{x}. \quad (14)$$

Depending on dimension this is resolved differently.

**1D case.** With one dimension, we can choose  $\zeta_1 = \xi_1 \cdot s$ , where  $s$  is the scaling factor between the different length scales in world space ( $\zeta$ ) and parameter space ( $\xi$ ). Then with  $d\phi/d\zeta = d\phi/d\xi \cdot d\xi/d\zeta$  and  $d\xi/d\zeta = s^{-1}$  we get

$$\int_{\Omega_e} \nabla \phi_i(\mathbf{x}) \cdot \nabla \phi_j(\mathbf{x}) \, d\mathbf{x} = \int_{\Phi([0,1])} \frac{d\phi_i(\mathbf{x})}{d\xi} \frac{d\phi_j(\mathbf{x})}{d\xi} \underbrace{\left( \frac{d\xi}{d\zeta} \right)^2}_{=: s^{-1}} d\mathbf{x}.$$

The scaling factor can be computed by

$$\frac{\partial \Phi}{\partial \xi} = \frac{\partial \zeta}{\partial \xi} = s \quad \Rightarrow \quad s = \Phi'(\xi_1) = \|\mathbf{x}^2 - \mathbf{x}^1\|_2.$$

After transformation of the integration domain to parameter space this yields the following formula for the stiffness matrix:

$$\int_{\Omega_e} \nabla \phi_i(\mathbf{x}) \cdot \nabla \phi_j(\mathbf{x}) \, d\mathbf{x} = \int_{[0,1]} \frac{d\phi_i(\xi)}{d\xi} \frac{d\phi_j(\xi)}{d\xi} s^{-2} \mathcal{J}_1(\xi) \, d\xi$$

**3D case.** In 3D we use the world coordinate system as  $\boldsymbol{\zeta}$ -frame, i.e.  $\boldsymbol{\zeta}_1 = \mathbf{e}_1, \boldsymbol{\zeta}_2 = \mathbf{e}_2, \boldsymbol{\zeta}_3 = \mathbf{e}_3$ . We transform the integration domain from global to local coordinate frame using (45) and get:

$$-\sum_{e=1}^M \sum_{i \in N(e)} u_i \int_{[0,1]^d} \nabla_{\boldsymbol{\zeta}(\boldsymbol{\xi})} \phi_i(\boldsymbol{\xi}) \cdot \nabla_{\boldsymbol{\zeta}(\boldsymbol{\xi})} \phi_j(\boldsymbol{\xi}) \mathcal{J}_d(\boldsymbol{\xi}) \, d\boldsymbol{\xi} = 0 \quad \text{for } j = 1, \dots, N. \quad (15)$$

For a transformation of the gradients to parameter space we need the Jacobian  $J_\Phi$  of the coordinate mapping,  $\mathbf{x} = \Phi(\boldsymbol{\xi})$ , which consists of the entries

$$(J_\Phi)_{i,j} = \frac{dx_i}{d\xi_j}.$$

Note that the Jacobian of  $\Phi : \mathbb{R}^d \rightarrow \mathbb{R}^3$  might not be quadratic in general, but a  $3 \times d$  matrix with  $d \leq 3$ . Only in the special 3D-case it is quadratic and can therefore be inverted.

Assuming that  $\Phi$  is invertible on  $\Omega$  the inverse function theorem states

$$J_{\Phi^{-1}} = J_\Phi^{-1}.$$

Executing the chain rule on a derivative in world space,  $d\phi/dx_k$ , yields:

$$\frac{d\phi(\boldsymbol{\xi})}{dx_k} = \sum_{\ell=1}^d \frac{d\phi(\boldsymbol{\xi})}{d\xi_\ell} \frac{d\xi_\ell(\boldsymbol{\xi})}{dx_k}$$

and for the whole gradient vector:

$$\nabla_{\mathbf{x}} \phi(\boldsymbol{\xi}) = J_\Phi^{-\top}(\boldsymbol{\xi}) \nabla_{\boldsymbol{\xi}} \phi(\boldsymbol{\xi}),$$

where  $J_\Phi^{-\top} = (J_\Phi^{-1})^\top$ .

Now the expression  $\nabla_{\mathbf{x}}\phi_i \cdot \nabla_{\mathbf{x}}\phi_j$  can be computed:

$$\begin{aligned}
\nabla_{\mathbf{x}}\phi_i(\mathbf{x}) \cdot \nabla_{\mathbf{x}}\phi_j(\mathbf{x}) &= \sum_{k=1}^d \frac{d\phi_i(\boldsymbol{\xi})}{dx_k} \frac{d\phi_j(\boldsymbol{\xi})}{dx_k} = \sum_{k=1}^d \left( \sum_{\ell=1}^d \frac{d\phi_i(\boldsymbol{\xi})}{d\xi_\ell} \frac{d\xi_\ell(\boldsymbol{\xi})}{dx_k} \sum_{\ell=1}^d \frac{d\phi_j(\boldsymbol{\xi})}{d\xi_\ell} \frac{d\xi_\ell(\boldsymbol{\xi})}{dx_k} \right) \\
&= J_\Phi^{-\top}(\boldsymbol{\xi}) \nabla_{\boldsymbol{\xi}}\phi_i(\boldsymbol{\xi}) \cdot J_\Phi^{-\top}(\boldsymbol{\xi}) \nabla_{\boldsymbol{\xi}}\phi_j(\boldsymbol{\xi}) \\
&= (J_\Phi^{-\top}(\boldsymbol{\xi}) \nabla_{\boldsymbol{\xi}}\phi_i(\boldsymbol{\xi}))^\top J_\Phi^{-\top}(\boldsymbol{\xi}) \nabla_{\boldsymbol{\xi}}\phi_j(\boldsymbol{\xi}) \\
&= \nabla_{\boldsymbol{\xi}}\phi_i(\boldsymbol{\xi})^\top \underbrace{J_\Phi^{-1}(\boldsymbol{\xi}) J_\Phi^{-\top}(\boldsymbol{\xi})}_{=: T_\Phi(\boldsymbol{\xi})} \nabla_{\boldsymbol{\xi}}\phi_j(\boldsymbol{\xi}).
\end{aligned}$$

With the definition of the finite element Laplace operator transformation matrix  $T_\Phi(\boldsymbol{\xi}) := J_\Phi^{-1}(\boldsymbol{\xi}) J_\Phi^{-\top}(\boldsymbol{\xi})$  the transformation becomes:

$$\nabla_{\mathbf{x}}\phi_i(\mathbf{x}) \cdot \nabla_{\mathbf{x}}\phi_j(\mathbf{x}) = \nabla_{\boldsymbol{\xi}}\phi_i(\boldsymbol{\xi}) \cdot T_\Phi(\boldsymbol{\xi}) \nabla_{\boldsymbol{\xi}}\phi_j(\boldsymbol{\xi}).$$

**2D case.** In two dimensions we consider the 2D manifold embedded in  $\mathbb{R}^3$  with the mapping  $\Phi : [0, 1]^2 \rightarrow \Omega \subset \mathbb{R}^3$ . At a fixed point  $\mathbf{p} \in \Omega$  with  $\Phi(\boldsymbol{\xi}_p) = \mathbf{p}$  we first determine the tangent vectors  $\boldsymbol{\zeta}_1(\boldsymbol{\xi}_p), \boldsymbol{\zeta}_2(\boldsymbol{\xi}_p)$ . The first tangent vector is defined to lie on the  $\xi_1$  coordinate direction, the second vector is then constructed to be orthogonal to the first. To define the tangent vector  $\boldsymbol{\zeta}_1$  we use a curve in parameter space:

$$\gamma(t) = \begin{pmatrix} \gamma_1(t) \\ \gamma_2(t) \end{pmatrix} = \begin{pmatrix} \xi_{p1} + t \\ \xi_{p2} \end{pmatrix}.$$

Then we define

$$\begin{aligned}
\boldsymbol{\zeta}_1 &= (\Phi \circ \gamma_1)'(0) = \frac{\partial}{\partial t} \Phi(\gamma(t))|_{t=0} \\
&= \frac{\partial \Phi(\gamma(0))}{\partial \xi_1} \underbrace{\gamma_1'(t)}_{=1} + \frac{\partial \Phi(\gamma(0))}{\partial \xi_2} \underbrace{\gamma_2'(t)}_{=0} \\
&= \frac{\partial \Phi(\xi_p)}{\partial \xi_1}.
\end{aligned}$$

Similar we define the helper tangent vector  $\boldsymbol{\zeta}_h$  along the  $\xi_2$  coordinate, which is then

$$\boldsymbol{\zeta}_h = \frac{\partial \Phi(\xi_p)}{\partial \xi_2}.$$

$\boldsymbol{\zeta}_1$  and  $\boldsymbol{\zeta}_h$  are not orthogonal in general. Therefore a third tangent vector  $\boldsymbol{\zeta}_2 = \boldsymbol{\zeta}_1 \times \boldsymbol{\zeta}_h \times \boldsymbol{\zeta}_1$  will be defined.

It is the tangent vector of a curve

$$\delta(t) = \begin{pmatrix} \delta_1(t) \\ \delta_2(t) \end{pmatrix} = \begin{pmatrix} \xi_{p1} + \cos(\alpha)t \\ \xi_{p2} + \sin(\alpha)t \end{pmatrix}.$$

The angle  $\beta$  between  $\zeta_1$  and  $\zeta_h$  is given by

$$\cos(\beta)|\zeta_1||\zeta_h| = \zeta_1 \cdot \zeta_h \quad \Rightarrow \quad \beta = \arccos\left(\frac{\zeta_1 \cdot \zeta_h}{|\zeta_1||\zeta_h|}\right)$$

Then by relating angles we get

$$\frac{\pi/2}{\beta} = \frac{\alpha}{\pi/2} \quad \Rightarrow \quad \alpha = \frac{\pi^2}{4\beta}.$$

So the tangent vector becomes

$$\zeta_2 = (\Phi \circ \delta)'(0) = \frac{\partial \Phi(\xi_p)}{\partial \xi_1} \cos(\alpha) + \frac{\partial \Phi(\xi_p)}{\partial \xi_2} \sin(\alpha) = \zeta_1 \cos(\alpha) + \zeta_h \sin(\alpha). \quad (16)$$

Another approach is to use the formula

$$\zeta_2 = \zeta_1 \times \zeta_h \times \zeta_1,$$

which leads to the expression

$$\zeta_2 = \begin{pmatrix} -\frac{\partial \Phi_2}{\partial \xi_1} \left( \frac{\partial \Phi_1}{\partial \xi_1} \frac{\partial \Phi_2}{\partial \xi_2} - \frac{\partial \Phi_1}{\partial \xi_2} \frac{\partial \Phi_2}{\partial \xi_1} \right) - \frac{\partial \Phi_3}{\partial \xi_1} \left( \frac{\partial \Phi_1}{\partial \xi_1} \frac{\partial \Phi_3}{\partial \xi_2} - \frac{\partial \Phi_1}{\partial \xi_2} \frac{\partial \Phi_3}{\partial \xi_1} \right) \\ \frac{\partial \Phi_1}{\partial \xi_1} \left( \frac{\partial \Phi_1}{\partial \xi_1} \frac{\partial \Phi_2}{\partial \xi_2} - \frac{\partial \Phi_1}{\partial \xi_2} \frac{\partial \Phi_2}{\partial \xi_1} \right) - \frac{\partial \Phi_3}{\partial \xi_1} \left( \frac{\partial \Phi_2}{\partial \xi_1} \frac{\partial \Phi_3}{\partial \xi_2} - \frac{\partial \Phi_2}{\partial \xi_2} \frac{\partial \Phi_3}{\partial \xi_1} \right) \\ \frac{\partial \Phi_1}{\partial \xi_1} \left( \frac{\partial \Phi_1}{\partial \xi_1} \frac{\partial \Phi_3}{\partial \xi_2} - \frac{\partial \Phi_1}{\partial \xi_2} \frac{\partial \Phi_3}{\partial \xi_1} \right) + \frac{\partial \Phi_2}{\partial \xi_1} \left( \frac{\partial \Phi_2}{\partial \xi_1} \frac{\partial \Phi_3}{\partial \xi_2} - \frac{\partial \Phi_2}{\partial \xi_2} \frac{\partial \Phi_3}{\partial \xi_1} \right) \end{pmatrix}.$$

The mapping between parameter space and tangent space is given by  $\Psi_{\mathbf{p}}$ :

$$\Psi_{\mathbf{p}} : [0, 1]^2 \rightarrow T_{\mathbf{p}}\Omega,$$

$$\Psi_{\mathbf{p}}(\xi) = \mathbf{p} + c_1(\xi) \hat{\zeta}_1 + c_2(\xi) \hat{\zeta}_2.$$

The basis vectors  $\hat{\zeta}_i$  are the normalized tangent vectors,  $\hat{\zeta}_i = \zeta_i/|\zeta_i|$ . And the coefficients are:

$$c_1(\xi) = (\xi_1 - \xi_{p1} - (\xi_2 - \xi_{p2}) \cos(\alpha) / \sin(\alpha)) l_1$$

$$c_2(\xi) = (\xi_2 - \xi_{p2}) / \sin(\alpha) l_2,$$

with the lengths  $l_1 = |\zeta_1|, l_2 = |\zeta_2|$ .

The Jacobian of  $\Psi$  and its inverse are as follows:

$$J_{\Psi} = \begin{pmatrix} l_1 & -l_1 \cos(\alpha) / \sin(\alpha) \\ 0 & l_2 / \sin(\alpha) \end{pmatrix}, \quad J_{\Psi}^{-1} = J_{\Psi^{-1}} = \begin{pmatrix} 1/l_1 & \cos(\alpha) / l_2 \\ 0 & \sin(\alpha) / l_2 \end{pmatrix}.$$

The inverse contains the entries

$$(J_{\Psi}^{-1})_{i,j} = \frac{d\xi_i}{d\zeta_j}.$$

To compute the gradients of a function with respect to  $\zeta$ , we use the chain rule:

$$\frac{d\phi(\boldsymbol{\xi})}{d\zeta_k} = \sum_{\ell=1}^d \frac{d\phi(\boldsymbol{\xi})}{d\xi_\ell} \frac{d\xi_\ell(\boldsymbol{\xi})}{d\zeta_k},$$

so the gradient becomes

$$\nabla_{\zeta}\phi(\boldsymbol{\xi}) = J_{\Psi}^{-\top} \nabla_{\xi}\phi(\boldsymbol{\xi}),$$

where  $J_{\Psi}^{-\top} = (J_{\Psi}^{-1})^{\top}$ .

Analogous to the 3D case we define

$$T_{\Psi}(\boldsymbol{\xi}) := J_{\Psi}^{-1}(\boldsymbol{\xi}) J_{\Psi}^{-\top}(\boldsymbol{\xi}) = \begin{pmatrix} \cos(\alpha)^2/l_2^2 + 1/l_1^2 & \sin(\alpha) \cos(\alpha)/l_2^2 \\ \sin(\alpha) \cos(\alpha)/l_2^2 & \sin(\alpha)^2/l_2^2 \end{pmatrix}$$

and get:

$$\nabla_{\zeta}\phi_i(\mathbf{x}) \cdot \nabla_{\zeta}\phi_j(\mathbf{x}) = \nabla_{\xi}\phi_i(\boldsymbol{\xi}) \cdot T_{\Phi}(\boldsymbol{\xi}) \nabla_{\xi}\phi_j(\boldsymbol{\xi}).$$

## 1.4 Evaluation of the integral term

The integral in (15) defines for  $i$  and  $j$  the entries  $m_{ij}$  of the *stiffness matrix*  $M$ . The equation can be written in matrix form as

$$M \mathbf{u} = \mathbf{0},$$

where  $M$  contains the entries

$$m_{ij} = - \int_{\Omega} \nabla \phi_i \cdot \nabla \phi_j \, d\mathbf{x} = - \sum_{e=1}^M \sum_{i \in N(e)} \int_{[0,1]^d} \nabla \phi_i(\boldsymbol{\xi}) \cdot T_{\Phi}(\boldsymbol{\xi}) \nabla \phi_j(\boldsymbol{\xi}) \mathcal{J}_d(\boldsymbol{\xi}) \, d\boldsymbol{\xi}$$

and  $\mathbf{u} = (u_1, \dots, u_N)^{\top}$  is the solution vector. Given  $M$  the solution  $\mathbf{u}$  is computed by an appropriate linear system solver.

The integral for  $m_{i,j}$  depends via  $\mathcal{J}_d$  on the shape of the elements. In general, it has to be evaluated numerically. However, for special simple cases it can be computed analytically. This includes scenarios in  $d = 1, 2, 3$  dimensions where the elements are on a rectilinear cartesian grid.

If the grid is arbitrary, analytical computation for 1D is still simple. For  $d = 2, 3$  it is still possible, but involves more lengthy derivations that are usually performed using a computer algebra system such as `sympy`. In this section the 1D and 2D cases are derived, the python `sympy` code for 2D and 3D is contained in the `doc` directory for further reference.

For all 1D meshes that are embedded in a 3D domain as well as rectangular cartesian 2D and 3D meshes the terms  $T_{\Phi}(\boldsymbol{\xi})$  and  $\mathcal{J}_d(\boldsymbol{\xi})$  are constant within each element, i.e. do not depend on  $\boldsymbol{\xi}$ . In that case we can take  $\mathcal{J}_d$  out of the integral.

**1D case.** We now compute  $m_{ij}$  for  $d = 1$ . The transformation term  $\mathcal{J}_1(\xi)$  is defined as

$$\mathcal{J}_1(\xi) = \|\Phi'(\xi)\|_2.$$



Using the parametric representation of  $\Phi$  given in (12), we derive

$$\mathcal{J}_1(\xi) = \|\Phi'(\xi)\|_2 = \|\mathbf{x}^2 - \mathbf{x}^1\|_2,$$

which is the length of the element  $e$  with nodes  $\mathbf{x}^1$  and  $\mathbf{x}^2$ . We define it to be  $l_e := \|\mathbf{x}^2 - \mathbf{x}^1\|_2$ , and thus have  $\mathcal{J}_1(\xi) = l_e$ . The scaling factor is then  $s = l_e$  and the total prefactor of the integral becomes

$$\mathcal{J}_1 s^{-2} = 1/l_e.$$

**2D case.** For 2D we assume a rectangular element that lies in a  $z = \text{constant}$  plane with side lengths  $l_{1,e}$  and  $l_{2,e}$  in  $\xi_1$  and  $\xi_2$  directions. The mapping from  $\boldsymbol{\xi} = (\xi_1, \xi_2)$  to  $\mathbf{x}$  coordinate frame is given by

$$\Phi(\boldsymbol{\xi}) = \mathbf{x}^1 + \begin{pmatrix} \xi_1 l_{1,e} \\ \xi_2 l_{2,e} \end{pmatrix}. \quad (17)$$

Then we derive

$$J_\Phi(\boldsymbol{\xi}) = \begin{pmatrix} l_{1,e} & 0 \\ 0 & l_{2,e} \end{pmatrix}$$

and

$$\mathcal{J}_2 = \sqrt{\det(J_\Phi(\boldsymbol{\xi})^\top J_\Phi(\boldsymbol{\xi}))} = |l_{1,e} l_{2,e}|.$$

The tangent vectors  $\boldsymbol{\zeta}_1$  and  $\boldsymbol{\zeta}_h$  are computed to be

$$\boldsymbol{\zeta}_1 = \frac{\partial \Phi(\boldsymbol{\xi}_p)}{\partial \xi_1} = (l_{1,e}, 0)^\top, \quad \boldsymbol{\zeta}_h = \frac{\partial \Phi(\boldsymbol{\xi}_p)}{\partial \xi_2} = (0, l_{2,e})^\top, \quad l_1 = |\boldsymbol{\zeta}_1| = l_{1,e}.$$

The angle in between is

$$\beta = \arccos\left(\frac{\boldsymbol{\zeta}_1 \cdot \boldsymbol{\zeta}_h}{|\boldsymbol{\zeta}_1| |\boldsymbol{\zeta}_h|}\right) = \frac{\pi}{2}.$$

The angle in parameter space is then also  $\alpha = \pi^2/(4\beta) = \pi/2$ . Using (16) the orthogonal tangent vector  $\boldsymbol{\zeta}_2$  becomes  $\boldsymbol{\zeta}_2 = \boldsymbol{\zeta}_h$ . Then  $l_2 = |\boldsymbol{\zeta}_h| = l_{2,e}$ .

This leads to a transformation matrix

$$T_\Psi(\boldsymbol{\xi}) = \begin{pmatrix} \cos(\alpha)^2/l_2^2 + 1/l_1^2 & \sin(\alpha) \cos(\alpha)/l_2^2 \\ \sin(\alpha) \cos(\alpha)/l_2^2 & \sin(\alpha)^2/l_2^2 \end{pmatrix} = \begin{pmatrix} 1/l_{1,e}^2 & 0 \\ 0 & 1/l_{2,e}^2 \end{pmatrix}.$$

When the mesh resolution is uniform, i.e.  $l_{1,e} = l_{2,e} = l_e$ , the transformation matrix becomes a scaled identity matrix,  $T_\Psi(\boldsymbol{\xi}) = l_e^{-2} \mathbf{I}$  and the transformation can be moved out of the integral. The prefactor is then

$$\mathcal{J}_2 l_e^{-2} = 1.$$

**3D case.** Similar to the 2D case when a rectangular 3D grid with grid widths  $l_{1,e}, l_{2,e}, l_{3,e}$  is

assumed, the transformation factor becomes

$$\mathcal{J}_3 = |l_{1,e} l_{2,e} l_{3,e}|.$$

With the world mapping  $\Phi(\boldsymbol{\xi})$  and its Jacobian  $J_\Phi(\boldsymbol{\xi})$  given by

$$\Phi(\boldsymbol{\xi}) = \mathbf{x}^1 + \begin{pmatrix} \xi_1 l_{1,e} \\ \xi_2 l_{2,e} \\ \xi_3 l_{3,e} \end{pmatrix}, \quad J_\Phi(\boldsymbol{\xi}) = \begin{pmatrix} l_{1,e} & 0 & 0 \\ 0 & l_{2,e} & 0 \\ 0 & 0 & l_{3,e} \end{pmatrix},$$

the transformation matrix becomes

$$T_\Phi(\boldsymbol{\xi}) := J_\Phi^{-1}(\boldsymbol{\xi}) J_\Phi^{-\top}(\boldsymbol{\xi}) = \begin{pmatrix} l_{1,e}^{-2} & 0 & 0 \\ 0 & l_{2,e}^{-2} & 0 \\ 0 & 0 & l_{3,e}^{-2} \end{pmatrix}.$$

With uniform grid lengths,  $l_{1,e} = l_{2,e} = l_{3,e} = l_e$ , we get  $T_\Phi(\boldsymbol{\xi}) = l_e^{-2} \mathbf{I}$ . The prefactor of the integral yields

$$\mathcal{J}_3 l_e^{-2} = l_e.$$

In the presented special cases with uniform grid resolution  $\mathcal{J}_d$  did not depend on the integration domain, which allows to compute the factor separately:

$$\int_{\Omega} \nabla \phi_i \cdot T \nabla \phi_j \mathcal{J}_d d\boldsymbol{\xi} = \mathcal{J}_d l_e^{-2} \int_{\Omega} \nabla \phi_i \cdot \nabla \phi_j d\boldsymbol{\xi}.$$

Now the term  $-\int \nabla \phi_i \cdot \nabla \phi_j d\boldsymbol{\xi}$ , remains to be computed. We compute values at the nodes and visualize them using *stencil notation*. For a fixed node  $i$  we compute the respective values for adjacent nodes  $j$ . The result for  $i = j$  is underlined in the stencil, the values for adjacent nodes are placed left, right, top and bottom, in the position of the respective nodes.

We first compute element-wise stencils that state the contribution of a single element. If all elements have the same length properties, the element contributions can be summed up to get the total value at the nodes which is shown in the nodal stencils. From these stencils we can easily set up the stiffness matrix for a non-varying, equidistant mesh.

## 1.5 Boundary Conditions

The Dirichlet-type boundary condition

$$\nabla u(\mathbf{x}) \cdot \mathbf{n} = 0 \quad \text{on } \Gamma_N$$

is satisfied automatically by the Galerkin finite element formulation. Starting from the left hand side of (25) and using Divergence theorem we get:

$$-\int_{\Omega} \nabla u \cdot \nabla \phi d\mathbf{x} = -\int_{\partial\Omega} \phi (\nabla u \cdot \mathbf{n}) d\mathbf{x} + \int_{\Omega} \Delta u \phi d\mathbf{x} = 0 \quad \forall \phi \in H_0^1(\Omega)$$

dim	element contribution	node stencil
1D:	$\begin{bmatrix} -1 & 1 \end{bmatrix}$	$\begin{bmatrix} 1 & -2 & 1 \end{bmatrix}$
2D:	$\begin{bmatrix} 1/6 & 1/3 \\ -2/3 & 1/6 \end{bmatrix}$	$\frac{1}{3} \begin{bmatrix} 1 & 1 & 1 \\ 1 & -8 & 1 \\ 1 & 1 & 1 \end{bmatrix}$
3D:	<div style="display: flex; flex-direction: column; align-items: center;"> <div>center: <math>\begin{bmatrix} 0 &amp; 1/12 \\ -1/3 &amp; 0 \end{bmatrix}</math></div> <div>top: <math>\begin{bmatrix} 1/12 &amp; 1/12 \\ 0 &amp; 1/12 \end{bmatrix}</math></div> </div>	<div style="display: flex; flex-direction: column; align-items: center;"> <div>bottom: <math>\frac{1}{12} \begin{bmatrix} 1 &amp; 2 &amp; 1 \\ 2 &amp; 0 &amp; 2 \\ 1 &amp; 2 &amp; 1 \end{bmatrix}</math></div> <div>center: <math>\frac{1}{12} \begin{bmatrix} 2 &amp; 0 &amp; 2 \\ 0 &amp; -32 &amp; 0 \\ 2 &amp; 0 &amp; 2 \end{bmatrix}</math></div> <div>top: <math>\frac{1}{12} \begin{bmatrix} 1 &amp; 2 &amp; 1 \\ 2 &amp; 0 &amp; 2 \\ 1 &amp; 2 &amp; 1 \end{bmatrix}</math></div> </div>

Tabelle 1: Stencils of the Finite Element stiffness matrix of  $-\Delta u$  for an equidistant mesh with uniform resolution

Because  $\Delta u = 0$  on  $\Omega$  we get  $\nabla u \cdot \mathbf{n} = 0$  on the boundary.

Neumann boundary conditions can be easily considered at the discretized system. For each condition  $u_i = u_{0,i}$  that enforces the degree of freedom  $i$  to have the value  $u_{0,i}$  we modify the linear system of equations. In the right hand side vector we subtract from the value  $f_j$  the product of  $a_{ji}$  and the given value  $u_{0,i}$  for every  $j \neq i$ , i.e. the new value is  $\hat{f}_j = f_j - a_{ji} u_{0,i}$ . We set  $f_i = u_{0,i}$ . In the matrix we zero the row and column that contain the entry  $a_{ii}$ , i.e.  $a_{ij} = a_{ji} = 0, \forall j \neq i$  and set  $a_{ii} = 1$ . As an example, consider the system

$$\begin{pmatrix} m_{11} & m_{12} & m_{13} \\ m_{21} & m_{22} & m_{23} \\ m_{31} & m_{32} & m_{33} \end{pmatrix} \begin{pmatrix} u_1 \\ u_2 \\ u_3 \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \\ 0 \end{pmatrix}$$

with the Dirichlet boundary condition  $u_3 = u_{0,3}$ . The modified system then reads

$$\begin{pmatrix} m_{11} & m_{12} & 0 \\ m_{21} & m_{22} & 0 \\ 0 & 0 & 1 \end{pmatrix} \begin{pmatrix} u_1 \\ u_2 \\ u_3 \end{pmatrix} = \begin{pmatrix} -m_{13} u_{0,3} \\ -m_{23} u_{0,3} \\ u_{0,3} \end{pmatrix}.$$

## 1.6 Function spaces

For the weak solutions  $u$  of the problems we do not need to request  $\mathcal{C}^2(\Omega)$ , since only the first derivatives are needed and only in a weak sense. Therefore  $u \in H_0^1(\Omega)$  suffices.

The Hilbert space  $H^1(\Omega)$  is the Sobolev space  $\mathcal{W}^{1,2}(\Omega)$  which is defined using weak derivatives. The concept of weak derivatives generalizes the classical derivatives.

Let  $u, v \in \mathcal{L}_{\text{loc}}^1(\Omega)$  and  $\alpha \in \mathbb{N}_0^d$  a multi-index. Then  $v$  is called *weak derivative* of  $u$  of order  $\alpha$  iff

$$\int_{\Omega} u(\mathbf{x}) \mathcal{D}^{\alpha} \phi(\mathbf{x}) \, d\mathbf{x} = (-1)^{|\alpha|} \int_{\Omega} v(\mathbf{x}) \phi(\mathbf{x}) \, d\mathbf{x} \quad (18)$$

for all  $\phi \in \mathcal{C}_0^{\infty}(\Omega)$ . We then write  $\mathcal{D}^{\alpha} u = v$ . The derivative with the multi-index,  $\mathcal{D}^{\alpha}$  is given by

$$\mathcal{D}^{\alpha} = \frac{\partial^{|\alpha|}}{\partial_{x_1}^{\alpha_1} \dots \partial_{x_d}^{\alpha_d}} \quad (19)$$

If  $u$  is differentiable in a classical sense, the classical derivatives are also the weak derivatives.

Now we define the *Sobolev* space  $\mathcal{W}^{1,2}(\Omega)$  (1=first order weak derivatives, 2=derivatives in  $\mathcal{L}^2(\Omega)$ ) as follows:

$$\mathcal{W}^{1,2}(\Omega) := \{u \in \mathcal{L}_{\text{loc}}^1(\Omega) \mid |\alpha| \in \mathbb{N}_0^d, |\alpha| \leq 1, \mathcal{D}^{\alpha} u \text{ exists, } \mathcal{D}^{\alpha} u \in \mathcal{L}^2(\Omega)\}. \quad (20)$$

With an appropriate Sobolev norm,  $\mathcal{W}^{1,2}$  is a Banach space, i.e. complete (Cauchy series converge in it).

Together with the scalar product

$$(u, v)_{H^1} := \sum_{|\alpha| \leq 1} \int_{\Omega} \mathcal{D}^{\alpha} u(\mathbf{x}) \mathcal{D}^{\alpha} v(\mathbf{x}) \, d\mathbf{x} \quad (21)$$

we get the Hilbert space  $H^1(\Omega) := \mathcal{W}^{1,2}(\Omega)$ .

With  $H_0^1(\Omega) := \{u \in H^1(\Omega) \mid u(\mathbf{x}) = 0 \text{ for } \mathbf{x} \in \partial\Omega\}$  we denote the subspace of functions that are 0 on the boundary.

## 2 Poisson Equation

The Poisson equation is a generalization of the Laplace equation and reads

$$\Delta u = f \quad \text{on } \Omega.$$

It can be subject to the same boundary conditions as Laplace equation, i.e. Neumann-type boundary conditions

$$\nabla u(\mathbf{x}) \cdot \mathbf{n} = 0 \quad \text{on } \Gamma_N,$$

as well as Dirichlet-type boundary conditions

$$u(\mathbf{x}) = u_0(\mathbf{x}) \quad \text{on } \Gamma_D.$$

The finite element formulation proceeds similar to Chap. 1, multiplication of a testfunction  $\phi \in H_0^1(\Omega)$  and integration yields

$$\int_{\Omega} \Delta u \phi \, d\mathbf{x} = \int_{\Omega} f \phi \, d\mathbf{x}, \quad \forall \phi \in H_0^1(\Omega).$$

Applying divergence theorem we get

$$-\int_{\Omega} \nabla u \cdot \nabla \phi \, d\mathbf{x} = \int_{\Omega} f \phi \, d\mathbf{x} \quad \forall \phi \in H_0^1(\Omega). \quad (22)$$

Like the solution  $u(\mathbf{x})$  also the right hand side  $f(\mathbf{x})$  has to be spatially discretized by a linear combination of coefficients and basis functions:

$$u_h(\mathbf{x}) = \sum_{i=1}^N u_i \phi_i(\mathbf{x}),$$

$$f_h(\mathbf{x}) = \sum_{i=1}^N f_i \phi_i(\mathbf{x}).$$

By again choosing the space of testfunctions to be the same as the span of basis functions,  $V = \text{span}\{\phi_1, \dots, \phi_n\}$  we get the Galerkin formulation as

$$-\sum_{i=1}^N u_i \int_{\Omega} \nabla \phi_i \cdot \nabla \phi_j \, d\mathbf{x} = \sum_{i=1}^N f_i \int_{\Omega} \phi_i \cdot \phi_j \, d\mathbf{x} \quad \text{for } j = 1, \dots, N.$$

The domain  $\Omega$  is again decomposed into disjoint elements  $\Omega_e, e = 1, \dots, M$  and integration has only be performed over the elements where none of the basis function vanish.

The first integral term,  $\int_{\Omega} \nabla \phi_i \cdot \nabla \phi_j \, d\mathbf{x}$ , has to be computed as described in Section 1.4. How to compute the second integral term,  $\int_{\Omega} \phi_i \cdot \phi_j \, d\mathbf{x}$  is shown in the following.

Similar as before, the integration domain is transferred from element space to parameter space. For this a transformation factor  $\mathcal{J}_d$  has to be considered, which is constant for some special cases as discussed in section 1.4.

For the remaining integral,  $\int_{\Omega} \phi_i \cdot \phi_j \, d\mathbf{x}$  node stencils are provided in the following table.

dim	element contribution	node stencil
1D:	$\frac{1}{6} \begin{bmatrix} 2 & 1 \end{bmatrix}$	$\frac{1}{6} \begin{bmatrix} 1 & 4 & 1 \end{bmatrix}$
2D:	$\frac{1}{36} \begin{bmatrix} 2 & 1 \\ 4 & 2 \end{bmatrix}$	$\frac{1}{36} \begin{bmatrix} 1 & 4 & 1 \\ 4 & 16 & 4 \\ 1 & 4 & 1 \end{bmatrix}$
3D:	<div style="display: flex; flex-direction: column; align-items: center;"> <div>center: <math>\frac{1}{216} \begin{bmatrix} 4 &amp; 2 \\ 8 &amp; 4 \end{bmatrix}</math></div> <div>top: <math>\frac{1}{216} \begin{bmatrix} 2 &amp; 1 \\ 4 &amp; 2 \end{bmatrix}</math></div> </div>	<div style="display: flex; flex-direction: column; align-items: center;"> <div>bottom: <math>\frac{1}{216} \begin{bmatrix} 1 &amp; 4 &amp; 1 \\ 4 &amp; 16 &amp; 4 \\ 1 &amp; 4 &amp; 1 \end{bmatrix}</math></div> <div>center: <math>\frac{1}{216} \begin{bmatrix} 4 &amp; 16 &amp; 4 \\ 16 &amp; 64 &amp; 16 \\ 4 &amp; 16 &amp; 4 \end{bmatrix}</math></div> <div>top: <math>\frac{1}{216} \begin{bmatrix} 1 &amp; 4 &amp; 1 \\ 4 &amp; 16 &amp; 4 \\ 1 &amp; 4 &amp; 1 \end{bmatrix}</math></div> </div>

Tabelle 2: Stencils of the Finite Element right hand side for an equidistant mesh with uniform resolution

### 3 Generalized Laplace operator

The Laplace equation  $\Delta u = 0$  describes steady-state matter-/heat-/current flow where the computed quantity  $u$  designates the potential that induces the flow  $\mathbf{F}$ , which can be modelled as being directed against the potential gradient,  $\mathbf{F} = -\nabla u$ . In terms of heat transfer this is *Fick's law*. By considering the conservation law of the flowing quantity we assume  $\nabla \cdot \mathbf{F} = 0$  which leads directly to the Laplace equation.

If the medium in which the flow occurs is non-isotropic this can be modelled by adjusting the relationship between the negative potential gradient,  $-\nabla u$ , and the induced flow direction,  $F$ . By applying a linear map  $A$  onto the negative gradient vector which can be thought of being the sum of contributions in coordinate directions, each contribution vector of a coordinate direction gets scaled and projected to a new direction. The flow is then  $\mathbf{F} = -A\nabla u$  and the resulting equation is called generalized Laplace equation, reading

$$\nabla \cdot (A\nabla u) = 0.$$

The parantheses can also be neglected.  $\nabla \cdot A\nabla$  is referred to as generalized Laplace operator.

The derivation of the Finite Element formulation proceeds analogous to section 1. Multiplication with a testfunction  $\phi$  yields:

$$\int_{\Omega} \nabla \cdot A\nabla u \phi \, d\mathbf{x} = 0 \quad \forall \phi \in H_0^1(\Omega) \quad (23)$$

Applying divergence theorem (39) with  $f = \phi$  and  $\mathbf{F} = A\nabla u$  yields

$$-\int_{\Omega} A\nabla u \cdot \nabla \phi \, d\mathbf{x} + \int_{\partial\Omega} (\phi A\nabla u) \cdot \mathbf{n} \, d\mathbf{x} = 0 \quad \forall \phi \in H_0^1(\Omega). \quad (24)$$

Because  $\phi$  is zero on the boundary, the boundary integral vanishes:

$$-\int_{\Omega} A\nabla u \cdot \nabla \phi \, d\mathbf{x} = 0 \quad \forall \phi \in H_0^1(\Omega). \quad (25)$$

The discretization remains the same and leads to an integral term of

$$m_{ij} = -\int_{\Omega} A\nabla \phi_i \cdot \nabla \phi_j \, d\mathbf{x}.$$

for the stiffness matrix  $M$ . For these term the stencil notation cannot be applied in general. The stiffness matrix has to be computed using e.g. numerical quadrature.

### 4 Diffusion Equation

The diffusion equation reads

$$u_t = c \Delta u, \quad c \in \mathbb{R} \quad (26)$$

For the finite element formulation it can be seen as a Poisson equation with right hand side  $u_t$ , neglecting the constant  $c$  for now. We get from (22):

$$-\int_{\Omega} \nabla u \cdot \nabla \phi \, d\mathbf{x} = \int_{\Omega} u_t \phi \, d\mathbf{x} \quad \forall \phi \in H_0^1(\Omega). \quad (27)$$

The derivative in time,  $u_t$  can be discretized by a differential quotient as follows:

$$u_t = \frac{u^{(t+1)} - u^{(t)}}{dt},$$

where  $dt$  is the time step width. In space we formulate the quantity again with a discrete basis:

$$u_h^{(t)}(\mathbf{x}) = \sum_{i=1}^N u_i^{(t)} \phi_i(\mathbf{x}).$$

Substituting into (27) yields:

$$\begin{aligned} -\int_{\Omega} \nabla u^{(t)} \cdot \nabla \phi \, d\mathbf{x} &= \frac{1}{dt} \int_{\Omega} (u^{(t+1)} - u^{(t)}) \phi \, d\mathbf{x} \quad \forall \phi \in H_0^1(\Omega) \\ \Leftrightarrow -\sum_{i=1}^N u_i^{(t)} \int_{\Omega} \nabla \phi_i \cdot \nabla \phi_j \, d\mathbf{x} &= \frac{1}{dt} \sum_{i=1}^N (u_i^{(t+1)} - u_i^{(t)}) \int_{\Omega} \phi_i \cdot \phi_j \, d\mathbf{x} \quad \text{for } j = 1, \dots, N. \end{aligned}$$

In Matrix notation with the stiffness matrix,  $\mathbf{K}$ , and the mass matrix,  $\mathbf{M}$ , and the vectors of the degrees of freedom for time  $t$ ,  $u^t$ , we get:

$$-\mathbf{K}\mathbf{u} = \frac{1}{dt} \mathbf{M}(u^{(t+1)} - u^{(t)}).$$

Solving for  $u^{(t+1)}$  yields:

$$u^{(t+1)} = u^{(t)} - dt \mathbf{M}^{-1} \mathbf{K}\mathbf{u}.$$

## 4.1 Analytical Solution

The analytical solution to the diffusion equation (26) makes use of the fundamental solution

$$\begin{aligned} H(x, t) &= \frac{1}{\sqrt{4\pi c t}} \exp\left(-\frac{x^2}{4 c t}\right) \quad \text{for 1D} \\ H(\mathbf{x}, t) &= \frac{1}{(4\pi c t)^{n/2}} \exp\left(-\frac{\|\mathbf{x}\|_2^2}{4 c t}\right) \quad \text{for 2D, 3D} \end{aligned}$$

For  $\Omega = \mathbb{R}^d$  and initial condition

$$u(\mathbf{x}, 0) = u_0(\mathbf{x}) \quad \mathbf{x} \in \Omega$$

the solution is

$$u(\mathbf{x}, t) = \int_{\mathbb{R}^d} H(\mathbf{x} - \mathbf{y}, t) u_0(\mathbf{y}) \, d\mathbf{y}.$$

## 5 Multidomain

Currently not compiled because unpublished.

## 6 Structural mechanics

The order in this section may be a bit confusing or lacking transition texts. It should be seen more as a collection of the theory and formulas.

In this section the Einstein sum convention is used.

### 6.1 Introduction of some quantities

No quantities that are completely defined in the actual configuration are considered.  $\mathbf{F}$  maps between configurations,  $\mathbf{S}, \mathbf{C}, \boldsymbol{\varepsilon}$  are completely in the reference configuration. The definitions are briefly listed here.

- The deformation gradient  $\mathbf{F} = \partial \mathbf{x} / \partial \mathbf{X}$  gives the linear approximation to the deformed image  $\boldsymbol{\lambda}_{\mathbf{a}_0}$  (stretch vector) of a material vector  $\mathbf{a}_0$ , i.e.  $\boldsymbol{\lambda}_{\mathbf{a}_0} = \mathbf{F} \mathbf{a}$  with  $\mathbf{y} - \mathbf{x} = \mathbf{F}(\mathbf{X})(\mathbf{Y} - \mathbf{X}) + o(\mathbf{Y} - \mathbf{X})$ .
- Polar decomposition of deformation gradient  $\mathbf{F}$ :

$$\mathbf{F} = \mathbf{R} \mathbf{U}, \quad \mathbf{R}^\top \mathbf{R} = \mathbf{I}, \quad \mathbf{U} = \mathbf{U}^\top \text{ positive definite}$$

$\mathbf{R}$  rotation tensor,  $\mathbf{U}$  right stretch tensor,  $\det \mathbf{U} = \det \mathbf{F} = J > 0$ .

- Mapping properties between reference ( $d\mathbf{X}, d\mathbf{S}, dV$ ) and current configuration ( $d\mathbf{x}, d\mathbf{s}, dv$ ):

$$d\mathbf{x} = \mathbf{F}(\mathbf{X}) d\mathbf{X} \quad \text{line map,}$$

$$d\mathbf{s} = J \mathbf{F}^{-\top}(\mathbf{X}) d\mathbf{S} \quad \text{surface map (Nansons' formula),}$$

$$dv = J(\mathbf{X}) dV \quad \text{volume map.}$$

- Right Cauchy-Green tensor  $\mathbf{C} = \mathbf{F}^\top \mathbf{F}$ , symmetric, positive definite,  $\det \mathbf{C} = J^2 > 0$ .
- Left Cauchy-Green tensor  $\mathbf{b} = \mathbf{F} \mathbf{F}^\top$ , symmetric, positive definite,  $\det \mathbf{b} = J^2 > 0$ .
- Green-Lagrange strain tensor  $\boldsymbol{\varepsilon} = \mathbf{E} = \frac{1}{2}(\mathbf{C} - \mathbf{I})$ , symmetric (Both symbols are used here, because of literature). Note that the computation of  $\boldsymbol{\varepsilon}$  from a vector of displacement DOFs is nonlinear.
- 2nd Piola-Kirchhoff stress tensor  $\mathbf{S} = \frac{\partial \Psi(\mathbf{E})}{\partial \mathbf{E}} = 2 \frac{\partial \psi(\mathbf{C})}{\partial \mathbf{C}}$ , symmetric.



## 6.2 Green-Lagrange strain tensor and derivatives

In this subsection quantities of the reference configuration are denoted by left superscript  $^o$ , quantities in the current configuration at time  $t$  are denoted by left superscript  $^t$ . Derivation with respect to the reference configuration are denoted by  $^t_o$ .

The position  $(^tx_1^M, ^tx_2^M, ^tx_3^M)$  of a material point  $M$  at time  $t$  is given by

$$^tx_i^M = ^ox_i^M + ^tu_i^M,$$

where  $^ox_i$  is the position in reference configuration and  $^tu_i$  is the displacement. The deformation gradient  $\mathbf{F}$  is defined by the entries

$$F_{ij} = ^tx_{i,j} = \frac{\partial ^tx_i}{\partial ^ox_j}.$$

The subscript comma means differentiation. We have  $^tx_{i,j} = \delta_{ij} + u_{i,j}$ . The discretization of  $^tu$  reads

$$^tu_i(x) = \phi_L(x) ^tu_i^L,$$

where  $^tu_i^L$  are the coefficients, i.e. the nodal displacements. Lower case indices  $i, j, k, \dots$  refer to component indices  $\{1, 2, 3\}$ , capital indices are for dofs  $\{1, \dots, n\}$ . The symbol  $^tu_i^L$  stands for the nodal degree of freedom  $L$  of the displacement component  $i \in \{1, 2, 3\}$ .

Differentiation yields

$$^tx_{i,j} = \frac{\partial ^tx_i}{\partial ^ox_j} = \delta_{ij} + \frac{\partial ^tu_i}{\partial ^ox_j} = \delta_{ij} + \phi_{L,j}(^o\mathbf{x}) ^tu_i^L = \delta_{ij} + u_{i,j}, \quad \text{i.e.}$$

$$\mathbf{F} = \mathbf{I} + \nabla_{\mathbf{X}}\mathbf{u}.$$

We have

$$\phi_{L,j}(^o\mathbf{x}) = \frac{d\phi_L(\boldsymbol{\xi}(^o\mathbf{x}))}{d^ox_j} = \frac{\partial \phi_L}{\partial \xi_\ell} \frac{\partial \xi_\ell}{\partial ^ox_j}.$$

The Green-Lagrange strain tensor  $^t\boldsymbol{\varepsilon}$  is defined as

$$^t\boldsymbol{\varepsilon}_{kl} = \frac{1}{2} (^tx_{b,k} ^tx_{b,l} - \delta_{kl}), \quad \text{or} \quad \mathbf{E} = \frac{1}{2} (\mathbf{F}^\top \mathbf{F} - \mathbf{I})$$

From that we get

$$\begin{aligned} ^t\boldsymbol{\varepsilon}_{kl} &= \frac{1}{2} (^tx_{b,k} ^tx_{b,l} - \delta_{kl}) = \frac{1}{2} ((\delta_{bk} + u_{b,k})(\delta_{bl} + u_{b,l}) - \delta_{kl}) \\ &= \frac{1}{2} ((\delta_{bk} \delta_{bl} + u_{b,k} \delta_{bl} + \delta_{bk} u_{b,l} + u_{b,k} u_{b,l}) - \delta_{kl}) = \frac{1}{2} (\delta_{kl} + u_{l,k} + u_{k,l} + u_{b,k} u_{b,l} - \delta_{kl}) \\ &= \frac{1}{2} (u_{l,k} + u_{k,l} + u_{b,k} u_{b,l}), \quad \text{or} \end{aligned}$$

$$\mathbf{E} = \frac{1}{2} ((\nabla \mathbf{u})^\top + \nabla \mathbf{u}) + \frac{1}{2} \nabla \mathbf{u}^\top \nabla \mathbf{u}, \quad \text{where the gradient is w.r.t } \mathbf{X} \text{ (material description).}$$

Note that for virtual strain (used later) the virtual displacements are infinitesimally small and the

quadratic term can be neglected, yielding

$${}^t_o\delta\epsilon_{kl} = \frac{1}{2} (\delta u_{l,k} + \delta u_{k,l}), \quad \delta\mathbf{E} = \frac{1}{2} ((\nabla\delta\mathbf{u})^\top + \nabla\delta\mathbf{u}).$$

Next the derivative with respect to a nodal displacement  $u_i^L$  is considered. Therefore we need

$$\frac{\partial_o^t x_{i,j}}{\partial u_k^L} = \frac{\partial \phi_L^t u_i^L}{\partial u_k^L} = \phi_{L,j} \delta_{ik}.$$

We then proceed to compute

$$\frac{\partial_o^t \epsilon_{kl}}{\partial u_i^L} = \frac{1}{2} \left( \frac{\partial_o^t x_{b,k}}{\partial u_i^L} {}^t_o x_{b,l} + {}^t_o x_{b,k} \frac{\partial_o^t x_{b,l}}{\partial u_i^L} \right) = \frac{1}{2} \left( \phi_{L,k} \delta_{bi} {}^t_o x_{b,l} + {}^t_o x_{b,k} \phi_{L,l} \delta_{bi} \right) = \frac{1}{2} \left( {}^t_o x_{i,l} \phi_{L,k} + {}^t_o x_{i,k} \phi_{L,l} \right).$$

We compute the second derivative (results from [3])

$$\frac{\partial_o^t \epsilon_{kl}}{\partial u_i^L \partial u_j^M} = \dots = \frac{1}{2} (\phi_{L,k} \phi_{M,l} + \phi_{L,l} \phi_{M,k}) \delta_{ij}.$$

## 6.3 Material modeling

### 6.3.1 Principal invariants

The principal scalar invariants of the right Cauchy Green Tensor  $\mathbf{C} = \mathbf{F}^\top \mathbf{F}$  are defined as

$$I_1(\mathbf{C}) = \text{tr}(\mathbf{C}) = \mathbf{C} : \mathbf{I} = C_{ii} = \lambda_1 + \lambda_2 + \lambda_3$$

$$I_2(\mathbf{C}) = \frac{1}{2} \left( (\text{tr}(\mathbf{C}))^2 - \text{tr}(\mathbf{C}^2) \right) = \frac{1}{2} (C_{ii} C_{jj} - C_{ji} C_{ij}) = \lambda_1 \lambda_2 + \lambda_1 \lambda_3 + \lambda_2 \lambda_3$$

$$I_3(\mathbf{C}) = \det(\mathbf{C}) = \varepsilon_{ijk} C_{1i} C_{2j} C_{3k} = \lambda_1 \lambda_2 \lambda_3$$

The principal stretches  $\lambda_a, a = 1, 2, 3$  are the eigenvalues of the right Cauchy-Green tensor  $\mathbf{C}$ .  $\varepsilon_{ijk}$  is the Levi-Civita symbol

$$\varepsilon_{ijk} = \begin{cases} 1 & \text{for } (i, j, k) \in \{(1, 2, 3), (2, 3, 1), (3, 1, 2)\} \\ -1 & \text{for } (i, j, k) \in \{(3, 2, 1), (1, 3, 2), (2, 1, 3)\} \\ 0 & \text{else} \end{cases}$$

### 6.3.2 Strain energy density function

Strain energy density function  $\Psi(\mathbf{C}) = \Psi(I_1, I_2, I_3)$  for isotropic hyperelastic materials. To compute  $\mathbf{S} = 2 \frac{\partial \Psi(\mathbf{C})}{\partial \mathbf{C}}$  derivatives w.r.t. to the invariants are required. (p. 216 Holzapfel Nonlinear Solid

Mechanics [2])

$$\begin{aligned}\frac{\partial I_1}{\partial \mathbf{C}} &= \frac{\partial \text{tr} \mathbf{C}}{\partial \mathbf{C}} = \frac{\partial \mathbf{I} : \mathbf{C}}{\partial \mathbf{C}} = \mathbf{I}, & \frac{\partial I_1}{\partial C_{AB}} &= \delta_{AB} \\ \frac{\partial I_2}{\partial \mathbf{C}} &= \text{tr} \mathbf{C} \mathbf{I} - \mathbf{C}, & \frac{\partial I_2}{\partial C_{AB}} &= I_1 \delta_{AB} - C_{AB} \\ \frac{\partial I_3}{\partial \mathbf{C}} &= I_3 \mathbf{C}^{-1}, & \frac{\partial I_3}{\partial C_{AB}} &= I_3 C_{AB}^{-1}\end{aligned}$$

In total we get

$$\begin{aligned}\mathbf{S} &= \frac{\partial \Psi(\mathbf{E})}{\partial \mathbf{E}} = 2 \frac{\partial \Psi(\mathbf{C})}{\partial \mathbf{C}} = \sum_{a=1}^3 \frac{\partial \Psi}{\partial I_a} \frac{\partial I_a}{\partial \mathbf{C}} \\ &= 2 \left( \left( \frac{\partial \Psi}{\partial I_1} + I_1 \frac{\partial \Psi}{\partial I_2} \right) \mathbf{I} - \frac{\partial \Psi}{\partial I_2} \mathbf{C} + I_3 \frac{\partial \Psi}{\partial I_3} \mathbf{C}^{-1} \right)\end{aligned}$$

Useful:

$$\left( \frac{\partial \mathbf{A}^{-1}}{\partial \mathbf{A}} \right)_{ijkl} = -\frac{1}{2} (A_{ik}^{-1} A_{lj}^{-1} + A_{il}^{-1} A_{kj}^{-1})$$

## 6.4 Elasticity tensor

For the solution of the nonlinear problem the elasticity tensor  $\mathbb{C}$  has to be computed (Holzapfel p.261 [2]):

$$\begin{aligned}\mathbb{C} &= \frac{\partial \mathbf{S}}{\partial \mathbf{E}} = 2 \frac{\partial \mathbf{S}}{\partial \mathbf{C}} = 4 \frac{\partial^2 \Psi(I_1, I_2, I_3)}{\partial \mathbf{C} \partial \mathbf{C}} \\ &= \delta_1 \mathbf{I} \otimes \mathbf{I} + \delta_2 (\mathbf{I} \otimes \mathbf{C} + \mathbf{C} \otimes \mathbf{I}) + \delta_3 (\mathbf{I} \otimes \mathbf{C}^{-1} + \mathbf{C}^{-1} \otimes \mathbf{I}) \\ &\quad + \delta_4 \mathbf{C} \otimes \mathbf{C} + \delta_5 (\mathbf{C} \otimes \mathbf{C}^{-1} + \mathbf{C}^{-1} \otimes \mathbf{C}) \\ &\quad + \delta_6 \mathbf{C}^{-1} \otimes \mathbf{C}^{-1} + \delta_7 \mathbf{C}^{-1} \odot \mathbf{C}^{-1} + \delta_8 \mathbb{I}\end{aligned}$$

The coefficients  $\delta_1, \dots, \delta_8$  are defined by

$$\begin{aligned}\delta_1 &= 4 \left( \frac{\partial^2 \Psi}{\partial I_1 \partial I_1} + 2 I_1 \frac{\partial^2 \Psi}{\partial I_1 \partial I_2} + \frac{\partial \Psi}{\partial I_2} + I_1^2 \frac{\partial^2 \Psi}{\partial I_2 \partial I_2} \right), \\ \delta_2 &= -4 \left( \frac{\partial^2 \Psi}{\partial I_1 \partial I_2} + I_1 \frac{\partial^2 \Psi}{\partial I_2 \partial I_2} \right), \\ \delta_3 &= 4 \left( I_3 \frac{\partial^2 \Psi}{\partial I_1 \partial I_3} + I_1 I_3 \frac{\partial^2 \Psi}{\partial I_2 \partial I_3} \right), & \delta_4 &= 4 \frac{\partial^2 \Psi}{\partial I_2 \partial I_2}, \\ \delta_5 &= -4 I_3 \frac{\partial^2 \Psi}{\partial I_2 \partial I_3}, & \delta_6 &= 4 \left( I_3 \frac{\partial \Psi}{\partial I_3} + I_3^2 \frac{\partial^2 \Psi}{\partial I_3 \partial I_3} \right), \\ \delta_7 &= -4 I_3 \frac{\partial \Psi}{\partial I_3}, & \delta_8 &= -4 \frac{\partial \Psi}{\partial I_2}.\end{aligned}$$

The elasticity tensor  $\mathbb{C}$  has the following symmetries:

$$\mathbb{C}_{ijrs} = \mathbb{C}_{jirs} = \mathbb{C}_{ijsr} = \mathbb{C}_{jisr} \quad \text{minor symmetries}$$

$$\mathbb{C}_{ijrs} = \mathbb{C}_{rsij} \quad \text{major symmetries}$$

The minor symmetries are due to the symmetry of the 2nd Piola Kirchhoff tensor,  $\mathbf{S}$  and the right Cauchy-Green tensor,  $\mathbf{C}$ , because

$$\mathbb{C}_{ijrs} = \frac{\partial S_{ij}}{\partial C_{rs}}.$$

The major symmetries are because of the existence of the strain density function because

$$\mathbb{C}_{ijrs} = \frac{\partial^2 \Psi(\mathbf{C})}{\partial C_{ij} \partial C_{rs}}.$$

The 21 unique entries are:

$$\mathbb{C}_{ijrs} \text{ with } (i, j, r, s) =$$

$$(1, 1, 1, 1), (1, 2, 1, 1), (1, 3, 1, 1), (2, 2, 1, 1), (2, 3, 1, 1), (3, 3, 1, 1),$$

$$(1, 2, 1, 2), (1, 3, 1, 2), (2, 2, 1, 2), (2, 3, 1, 2), (3, 3, 1, 2),$$

$$(1, 3, 1, 3), (2, 2, 1, 3), (2, 3, 1, 3), (3, 3, 1, 3),$$

$$(2, 2, 2, 2), (2, 3, 2, 2), (3, 3, 2, 2),$$

$$(2, 3, 2, 3), (3, 3, 2, 3),$$

$$(3, 3, 3, 3)$$

## 6.5 Constitutive Equations

### 6.5.1 Incompressible Mooney-Rivlin

The *Mooney-Rivlin* model is used which for incompressible material defines the strain energy function as

$$\Psi_{\text{Mooney-Rivlin}}(I_1, I_2) = c_1(I_1 - 3) + c_2(I_2 - 3),$$

where  $I_1, I_2$  are the strain invariants and  $c_1, c_2$  are material parameters.

For incompressible materials the strain energy function is defined as (p.247)

$$\Psi = \Psi_{\text{Mooney-Rivlin}}(I_1, I_2) - \frac{1}{2} p (I_3 - 1), \text{ or}$$

$$\Psi = \Psi_{\text{Mooney-Rivlin}}(I_1, I_2) - \frac{1}{2} p (I_3 - 1)^2$$

and the stress as

$$\begin{aligned}\mathbf{S} &= 2 \frac{\partial}{\partial \mathbf{C}} \Psi_{\text{Mooney-Rivlin}}(I_1, I_2) - \frac{\partial(p(I_3 - 1))}{\partial \mathbf{C}} = -p \mathbf{C}^{-1} + 2 \left( \frac{\partial \Psi}{\partial I_1} + I_1 \frac{\partial \Psi}{\partial I_2} \right) \mathbf{I} - 2 \frac{\partial \Psi}{\partial I_2} \mathbf{C} \\ &= -p \mathbf{C}^{-1} + 2 (c_1 + \text{tr}(\mathbf{C}) c_2) \mathbf{I} - 2 c_2 \mathbf{C}\end{aligned}$$

The Cauchy stress:

$$\begin{aligned}\boldsymbol{\sigma} &= -p \mathbf{I} + 2 \left( \frac{\partial \Psi}{\partial I_1} + I_1 \frac{\partial \Psi}{\partial I_2} \right) \mathbf{b} - 2 \frac{\partial \Psi}{\partial I_2} \mathbf{b}^2 \\ &= -p \mathbf{I} + 2(c_1 + I_1 c_2) \mathbf{b} - 2 c_2 \mathbf{b}^2 \\ \boldsymbol{\sigma} &= -p \mathbf{I} + 2 c_1 \mathbf{b} - 2 c_2 \mathbf{b}^{-1} \quad \text{with Left Cauchy-Green tensor } \mathbf{b} = \mathbf{F} \mathbf{F}^\top \quad (\text{p.238 Holzapfel}) \\ \mathbf{b}^2 &= \mathbf{b} \mathbf{b} = \mathbf{F} \mathbf{F}^\top \mathbf{F} \mathbf{F}^\top, \quad \mathbf{b}_{ij}^2 = \mathbf{b}_{ik} \mathbf{b}_{kj} = \mathbf{F}_{il} \mathbf{F}_{lk}^\top \mathbf{F}_{km} \mathbf{F}_{mj}^\top = \mathbf{F}_{il} \mathbf{F}_{kl} \mathbf{F}_{km} \mathbf{F}_{jm}\end{aligned}$$

(Holzapfel p.224 [2])

The parameter  $p$  is here an indeterminate Lagrange multiplier that is identified as the hydrostatic pressure. However it has to be computed from the equilibrium equations and boundary conditions.

### 6.5.2 Compressible Mooney-Rivlin

$$\Psi(J, I_1, I_2) = c(J - 1)^2 - d \ln J + c_1(I_1 - 3) + c_2(I_2 - 3), \quad d = 2(c_1 + 2c_2)$$

(Holzapfel p.247 [2])

The stress tensor follows by derivation

$$\begin{aligned}\mathbf{S} &= 2 \frac{\partial \Psi}{\partial \mathbf{C}} = 2(c_1 + c_2 I_1) \mathbf{I} - 2 c_2 \mathbf{C} + (2 c J (J - 1) - d) \mathbf{C}^{-1} \\ &= 2(c_1 + c_2 \text{tr} \mathbf{C}) \mathbf{I} - 2 c_2 \mathbf{C} + \left( 2 c \det(\mathbf{F}) (\det(\mathbf{F}) - 1) - d \right) \mathbf{C}^{-1}\end{aligned}$$

Alternative model:

$$\Psi = \frac{1}{2} \kappa (J - 1)^2 + c_1(\bar{I}_1 - 3) + c_2(\bar{I}_2 - 3)$$

( [3] Sussmann, Bathe p.15)

### 6.5.3 Transversely isotropic formulation

This formulation assumes a ground matrix and fibres with direction  $\mathbf{a}_0$  in reference configuration. Two pseudo-invariants  $I_4(\mathbf{C}, \mathbf{a}_0)$ ,  $I_5(\mathbf{C}, \mathbf{a}_0)$  are introduced:

$$I_4(\mathbf{C}, \mathbf{a}_0) = \mathbf{a}_0 \cdot \mathbf{C} \mathbf{a}_0,$$

$$I_5(\mathbf{C}, \mathbf{a}_0) = \mathbf{a}_0 \cdot \mathbf{C}^2 \mathbf{a}_0.$$

The strain energy function is then stated as

$$\Psi = \Psi(I_1(\mathbf{C}), I_2(\mathbf{C}), I_3(\mathbf{C}), I_4(\mathbf{C}, \mathbf{a}_0), I_5(\mathbf{C}, \mathbf{a}_0))$$

For the formulation of the stress, derivatives of the pseudo-invariants w.r.t  $\mathbf{C}$  are needed:

$$\begin{aligned} \frac{\partial I_4}{\partial \mathbf{C}} &= \mathbf{a}_0 \otimes \mathbf{a}_0, & \frac{\partial I_4}{\partial C_{ij}} &= a_{0,i} a_{0,j}, \\ \frac{\partial I_5}{\partial \mathbf{C}} &= \mathbf{a}_0 \otimes \mathbf{C} \mathbf{a}_0 + \mathbf{a}_0 \mathbf{C} \otimes \mathbf{a}_0, & \frac{\partial I_5}{\partial C_{ij}} &= a_{0,i} C_{jk} a_{0,k} + a_{0,j} C_{ik} a_{0,k}. \end{aligned}$$

#### 6.5.4 Reduced Quantities

For compressible materials reduced quantities are used. The deformation gradient and right Cauchy-Green tensor are multiplicatively decomposed into a volume-changing (volumetric, dilational) and volume-preserving (isochoric, distortional, deviatoric) part as follows:

$$\mathbf{F} = (J^{1/3} \mathbf{I}) \bar{\mathbf{F}},$$

$$\mathbf{C} = (J^{2/3} \mathbf{I}) \bar{\mathbf{C}}.$$

The volume preserving measures are given by

$$\bar{\mathbf{F}} = J^{-1/3} \mathbf{F} \quad \dots \quad \text{modified deformation gradient}$$

$$\bar{\mathbf{C}} = J^{-2/3} \mathbf{C} \quad \dots \quad \text{modified right Cauchy-Green tensor}$$

The volume-changing deformations are associated with  $J^{1/3} \mathbf{I}$  and  $J^{2/3} \mathbf{I}$ .

Accordingly, reduced invariants are (p.233)

$$\bar{I}_1 = \text{tr}(\bar{\mathbf{C}}) = J^{-2/3} I_1,$$

$$\bar{I}_2 = \frac{1}{2} ((\text{tr}(\bar{\mathbf{C}}))^2 - \text{tr}(\bar{\mathbf{C}}^2)) = J^{-4/3} I_2,$$

$$\bar{I}_3 = \det(\bar{\mathbf{C}}) = 1.$$

The strain-energy function is now given in a decoupled representation as

$$\Psi(\mathbf{C}) = \Psi_{\text{vol}}(J) + \Psi_{\text{iso}}(\bar{\mathbf{C}}) = \Psi_{\text{vol}}(J) + \Psi_{\text{iso}}(\bar{I}_1, \bar{I}_2),$$

$$\Psi_{\text{vol}}(J) = \kappa G(J)$$

with penalty functions  $G(J)$  with conditions

$$G(J) = 0 \quad \Leftrightarrow \quad J = 1$$

and  $G(J)$  strictly convex

Possible functions in literature are:

$$G_1(J) = \frac{1}{2} (J(\mathbf{u}) - 1)^2,$$

$$G_2(J) = \frac{1}{2} (J(\mathbf{u}) - 1).$$

This leads to volumetric and deviatoric/isochoric contribution of stresses

$$\begin{aligned} \mathbf{S} &= 2 \frac{\partial \Psi(\mathbf{C})}{\partial \mathbf{C}} = \mathbf{S}_{\text{vol}} + \mathbf{S}_{\text{iso}}, \\ \mathbf{S}_{\text{vol}} &= 2 \frac{\partial \Psi_{\text{vol}}(J)}{\partial \mathbf{C}} = J p \mathbf{C}^{-1} \\ \mathbf{S}_{\text{iso}} &= 2 \frac{\partial \Psi_{\text{iso}}(\bar{\mathbf{C}})}{\partial \bar{\mathbf{C}}} = J^{-2/3} \mathbb{P} : \bar{\mathbf{S}} \quad (\text{see Holzapfel p.229 [2] for definition of } \mathbb{P}) \\ p &= \frac{d\Psi_{\text{vol}}(J)}{dJ}, \quad \bar{\mathbf{S}} = 2 \frac{\partial \Psi_{\text{iso}}(\bar{\mathbf{C}})}{\partial \bar{\mathbf{C}}} \end{aligned} \tag{28}$$

## 6.6 Formulation in current configuration

The following spatial quantities are defined in the current configuration.

$$\begin{array}{ll} \mathbf{x} & \dots \quad \text{position} \\ \mathbf{v} = \dot{\mathbf{x}} & \dots \quad \text{velocity} \\ \rho & \dots \quad \text{density} \\ \boldsymbol{\sigma} & \dots \quad \text{Cauchy stress} \end{array}$$

The connection from the symmetric Cauchy stress tensor  $\boldsymbol{\sigma}$  to the 2nd Piola-Kirchhoff stress tensor  $\mathbf{S}$  is given by

$$\begin{aligned} \boldsymbol{\sigma} &= J^{-1} \mathbf{F} \mathbf{S} \mathbf{F}^\top \\ \mathbf{S} &= J \mathbf{F}^{-1} \boldsymbol{\sigma} \mathbf{F}^{-\top} \\ \boldsymbol{\sigma} &= \frac{1}{J} \mathbf{P} \mathbf{F}^\top \Rightarrow \mathbf{P} = J \boldsymbol{\sigma} \mathbf{F}^{-\top} \\ \mathbf{S} &= \mathbf{F}^{-1} \mathbf{P} \Rightarrow \mathbf{P} = \mathbf{F} \mathbf{S} \end{aligned}$$

(Holzapfel [2] p. 135) From the *conservation of mass* we get the spatial equation

$$\dot{\rho}(\mathbf{x}, t) + \rho(\mathbf{x}, t) \nabla \cdot \mathbf{v}(\mathbf{x}, t) = 0$$

which for constant density,  $\rho$ , reduces to

$$\nabla \cdot \mathbf{v}(\mathbf{x}, t) = 0.$$

(Holzapfel [2] p. 145) From the *balance of linear momentum* the equations of motion in spatial description can be derived.

$$\text{div } \boldsymbol{\sigma} + \mathbf{b} = \rho \dot{\mathbf{v}} \tag{29}$$

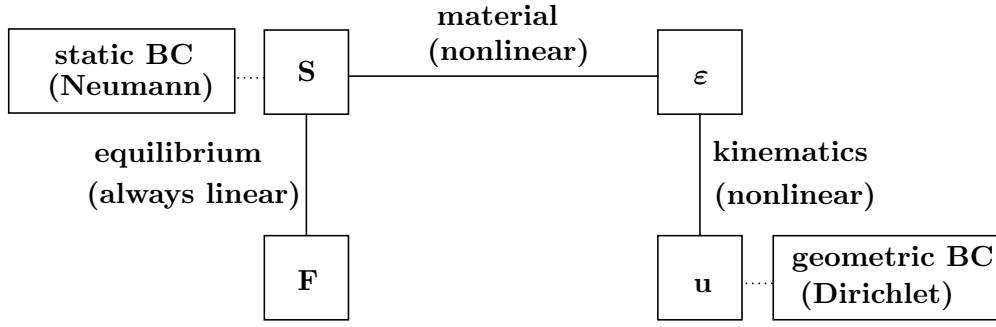


Abbildung 2: Tonti diagram

The *balance of angular momentum* enforces the Cauchy stress tensor,  $\boldsymbol{\sigma}$  to be symmetric (Holzapfel [2] p. 147).

$$\boldsymbol{\sigma} = \boldsymbol{\sigma}^\top$$

Equation (29) can be restated in terms of the displacement  $\mathbf{u}$  as

$$\rho \ddot{\mathbf{u}} - \nabla_{\mathbf{x}} \cdot \boldsymbol{\sigma} = 0,$$

$$\nabla_{\mathbf{x}} \cdot \dot{\mathbf{u}} = 0.$$

For the quasi-static case,  $\dot{\mathbf{u}} = 0$ , this reduces to

$$\nabla_{\mathbf{x}} \cdot \boldsymbol{\sigma} = 0.$$

Substituting the Cauchy stress  $\boldsymbol{\sigma}$  for the material configuration stress,  $\mathbf{S}$ , yields

$$\nabla_{\mathbf{x}} \cdot (J^{-1} \mathbf{F} \mathbf{S} \mathbf{F}^\top) = 0$$

## 6.7 Incompressible solid mechanics

A material is incompressible if one of the following necessary and sufficient conditions are satisfied (Holzapfel [2] p.103):

$$J = 1, \quad dv = \text{const}, \quad \dot{J} = 0, \quad \mathbf{F}^{-\top} : \dot{\mathbf{F}} = 0, \quad \text{div}(\mathbf{v}) = 0$$

A nonlinear case occurs if the constitutive relation between  $\mathbf{S}$  and  $\boldsymbol{\varepsilon}$  is nonlinear or if the relationship between displacement  $\mathbf{u}$  and strain  $\boldsymbol{\varepsilon}$  is nonlinear, as depicted in the Tonti diagram, fig. 2.

The case of 3D finite elasticity the relation between the displacement vector  $\hat{\mathbf{u}}$  and the strain  $\mathbf{E} = (\mathbf{C} - \mathbf{I})/2$  is nonlinear because of  $\mathbf{C} = \mathbf{F}^\top \mathbf{F}$ . In addition the material model is often nonlinear, because of the invariants.

For incompressible and nearly incompressible analyses it is necessary to use a mixed formulation to avoid convergence problems (locking). [4], [1].

Sec. 6.7.2 describes the penalty method, Sec. 6.7.4 formulates the classical mixed formulation that is used for compressible / nearly incompressible analyses.



### 6.7.1 Material description

Starting point is the equation of motion in the material description

$$\nabla_{\mathbf{X}} \cdot \mathbf{P} + \mathbf{B} = \rho_0 \ddot{\mathbf{u}},$$

with body forces in reference configuration,  $\mathbf{B} = J \mathbf{b}$ , and the first Piola-Kirchhoff stress tensor,  $\mathbf{P} = J \boldsymbol{\sigma}^\top \mathbf{F}^{-\top}$ , prescribed surface traction  $\bar{T}$ , displacement field  $u$  in the reference configuration. The principle of virtual work is (Holzapfel [2], p.385)

$$\underbrace{\int_{\Omega} \mathbf{P} : \nabla_{\mathbf{X}} \delta \mathbf{u} \, dV}_{=\delta W_{\text{int}}} = \underbrace{\int_{\Omega} (\mathbf{B} - \rho_0 \ddot{\mathbf{u}}) \cdot \delta \mathbf{u} \, dV + \int_{\partial\Omega} \bar{\mathbf{T}} \cdot \delta \mathbf{u} \, dS}_{=\delta W_{\text{ext}}}$$

The internal virtual work  $\delta W_{\text{int}}$  can be expressed in terms of Green-Lagrange strain,  $\mathbf{E}$ , and 2nd Piola-Kirchhoff stress,  $\mathbf{S}$ ,

$$\delta W_{\text{int}} = \int_{\Omega} \mathbf{P} : \nabla_{\mathbf{X}} \delta \mathbf{u} \, dV = \int_{\Omega} \mathbf{S} : \delta \mathbf{E} \, dV.$$

For the static case,  $\ddot{\mathbf{u}} = \mathbf{0}$ , the virtual work can be expressed by a potential,

$$\begin{aligned} \Pi_{\text{int}}(\mathbf{u}) &= \int_{\Omega} \Psi(\mathbf{F}(\mathbf{u})) \, dV, \\ \Pi_{\text{ext}}(\mathbf{u}) &= - \int_{\Omega} \mathbf{B} \mathbf{u} \, dV - \int_{\partial\Omega} \bar{T} \mathbf{u} \, dS. \end{aligned} \tag{30}$$

This is useful in order to describe the material behaviour by a scalar strain energy density function,  $\Psi$ .

By taking the directional derivative,

$$D_{\delta \mathbf{u}} \Pi(\mathbf{u}) = \frac{d}{d\varepsilon} \Pi(\mathbf{u} + \varepsilon \delta \mathbf{u}) \Big|_{\varepsilon=0},$$

we get back to the virtual work terms.

$$D_{\delta \mathbf{u}} \Pi_{\text{int}} = \delta W_{\text{int}}, \quad D_{\delta \mathbf{u}} \Pi_{\text{ext}} = -\delta W_{\text{ext}}.$$

### 6.7.2 Penalty method

For incompressible materials we use the decoupled representation of the strain energy function,

$$\Psi(\mathbf{C}) = \Psi_{\text{vol}}(J(\mathbf{u})) + \Psi_{\text{iso}}(\bar{\mathbf{C}}(\mathbf{u})), \quad \text{with } \Psi_{\text{vol}}(J) = \kappa G(J), \quad G(J) := \frac{1}{2} (J - 1)^2$$

The derivative yields

$$\begin{aligned}
\delta W_{\text{int}} &= D_{\delta \mathbf{u}} \Pi_{\text{int}} = \int_{\Omega} \left( \frac{\partial \Psi_{\text{vol}}(J(\mathbf{u}))}{\partial \mathbf{C}} + \frac{\partial \Psi_{\text{iso}}(\bar{\mathbf{C}}(\mathbf{u}))}{\partial \mathbf{C}} \right) : D_{\delta \mathbf{u}} \mathbf{C}(\mathbf{u}) \, dV \\
&= \int_{\Omega} \left( \underbrace{J \frac{d\Psi_{\text{vol}}}{dJ}}_{=p} \mathbf{C}^{-1} + 2 \underbrace{\frac{\partial \Psi_{\text{iso}}}{\partial \mathbf{C}}}_{=\mathbf{S}_{\text{iso}}} \right) : \delta \mathbf{E} \, dV = \int_{\Omega} (\mathbf{S}_{\text{vol}} + \mathbf{S}_{\text{iso}}) : \delta \mathbf{E} \, dV
\end{aligned}$$

In comparison with (28) we identify the split of the 2nd Piola-Kirchhoff stress tensor,  $\mathbf{S} = \mathbf{S}_{\text{vol}} + \mathbf{S}_{\text{iso}}$ . The pressure  $p$  is artificial and does not represent the physical volumetric pressure.

$$p = \frac{d\Psi_{\text{vol}}}{dJ} = \kappa \frac{dG(J)}{dJ} = \kappa(J(\mathbf{u}) - 1).$$

The penalty parameter  $\kappa$  is a numerical parameter, for  $\kappa \rightarrow \infty$  the problem gets incompressible, but ill-conditioned.

The variation of the Green-Lagrange strain tensor,  $\delta \mathbf{E}$  is given by (p.375)

$$\begin{aligned}
\delta \mathbf{E} &= \text{sym}(\mathbf{F}^{\top} \nabla \delta \mathbf{u}) = \frac{1}{2} ((\mathbf{F}^{\top} \nabla \delta \mathbf{u})^{\top} + \mathbf{F}^{\top} \nabla \delta \mathbf{u}), \\
\delta E_{AB} &= \frac{1}{2} \left( F_{aB} \frac{\partial \delta u_a}{\partial X_A} + F_{aA} \frac{\partial \delta u_a}{\partial X_B} \right)
\end{aligned} \tag{31}$$

The governing equations are given by the principle of virtual work,

$$\delta W_{\text{int}}(\mathbf{u}) - \delta W_{\text{ext}} = 0. \tag{32}$$

The extern virtual energy is assumed to not depend on the displacements.

(32) has to hold for arbitrary virtual displacements,  $\delta \mathbf{u}$ . The system of equations is thus

$$\underbrace{\int_{\Omega} \mathbf{S} : \delta \mathbf{E}(\delta \mathbf{u}) \, dV}_{=\delta W_{\text{int}}} - \underbrace{\int_{\Omega} (\mathbf{B} - \rho_0 \ddot{\mathbf{u}}) \cdot \delta \mathbf{u} \, dV + \int_{\partial \Omega} \bar{\mathbf{T}} \cdot \delta \mathbf{u} \, ds}_{=\delta W_{\text{ext}}} = 0, \quad \forall \delta \mathbf{u} \tag{33}$$

With the usual discretization,

$$u_a = u_a^L \phi_L, \quad \delta u_a = \delta u_a^L \phi_L, \quad \hat{\mathbf{u}} = (u_a^L)_{a=1,2,3}, \quad \delta \hat{\mathbf{u}} = (\delta u_a^L)_{a=1,2,3},$$

we get from (31) and (33) in index form

$$\begin{aligned}
&\int_{\Omega} \frac{1}{2} S_{AB} \left( F_{aB} \frac{\partial \delta u_a}{\partial X_A} + F_{aA} \frac{\partial \delta u_a}{\partial X_B} \right) \, dV - \int_{\Omega} (B_a - \rho_0 \ddot{u}_a) \delta u_a \, dV + \int_{\partial \Omega} \bar{T}_a \delta u_a \, ds = 0 \\
\Leftrightarrow &\int_{\Omega} \frac{1}{2} S_{AB}(\hat{\mathbf{u}}) \delta u_a^L \left( F_{aB}(\hat{\mathbf{u}}) \frac{\partial \phi_L}{\partial X_A} + F_{aA}(\hat{\mathbf{u}}) \frac{\partial \phi_L}{\partial X_B} \right) \, dV - \text{rhs} = 0
\end{aligned} \tag{34}$$

where the constant rhs, which is the discretized  $\delta W_{\text{ext}}$ , is given by

$$\text{rhs} = \int_{\Omega} (B_a - \rho_0 \ddot{u}_a) \delta u_a^L \phi_L \, dV - \int_{\partial \Omega} \bar{T}_a \delta u_a^L \phi_L \, ds.$$

This defines a nonlinear function,  $\mathbf{R}(\hat{\mathbf{u}}) \in \mathbb{R}^{3N}$ , with one value for each  $\delta u_a^L, a = 1, 2, 3, L = 1, \dots, N$  (I.e. entry  $(a, L)$  of the vector  $\mathbf{R}$  contains formula (34) with  $\delta_a^{\bar{L}} = 1$  for  $\bar{a} = a, \bar{L} = L$  and  $\delta_a^{\bar{L}} = 0$  otherwise). The aim is to find  $\hat{\mathbf{u}}$  such that  $\mathbf{R}(\hat{\mathbf{u}}) = 0$ .

Within the Newton-Raphson scheme the equation is linearized around iterative values  $\hat{\mathbf{u}}^{(k)}$ .

$$\mathbf{R}(\hat{\mathbf{u}}^{(k+1)}) = \mathbf{R}\Big|_{\hat{\mathbf{u}}=\hat{\mathbf{u}}^{(k)}} + D_{\Delta\hat{\mathbf{u}}^{(k)}}\delta W_{\text{int}}(\hat{\mathbf{u}}, \delta\hat{\mathbf{u}}) + \mathcal{O}((\Delta\hat{\mathbf{u}}^{(k)})^2).$$

The iterative procedure starts with an initial value  $\hat{\mathbf{u}}^{(0)} = \hat{\mathbf{u}}_0$  and repeatedly computes

$$\hat{\mathbf{u}}^{(k+1)} = \hat{\mathbf{u}}^{(k)} + \Delta\hat{\mathbf{u}}^{(k)}.$$

The increment  $\Delta\hat{\mathbf{u}}^{(k)}$  is computed by assuming  $\mathbf{R}(\hat{\mathbf{u}}^{(k+1)}) = 0$  and solving

$$D_{\Delta\hat{\mathbf{u}}^{(k)}}\delta W_{\text{int}}(\hat{\mathbf{u}}, \delta\hat{\mathbf{u}}) = -\mathbf{R}(\hat{\mathbf{u}}^{(k)}). \quad (35)$$

This is a linear system that has to be solved. It is repeated until the residual norm  $\|\mathbf{R}(\hat{\mathbf{u}}^{(k)})\|$  gets below a threshold.

The left-hand side term of (35) is derived from (32) (Holzapfel p.396),

$$D_{\Delta\hat{\mathbf{u}}^{(k)}}\delta W_{\text{int}}(\hat{\mathbf{u}}, \delta\hat{\mathbf{u}}) = \int_{\Omega} \frac{\partial \delta u_a}{\partial X_B} \underbrace{(\delta_{ab} S_{BD} + F_{aA} F_{bC} C_{ABCD})}_{=\tilde{k}_{abBD}} \frac{\partial \Delta u_b}{\partial X_D} dV \quad (36)$$

with

$$\begin{aligned} B, D = 1, \dots, 3 & \quad \dots \quad \text{indices over spatial dimensions} \\ a, b = 1, & \quad \dots \quad \text{indices over degrees of freedom} \\ \mathbf{S}, & \quad \dots \quad \text{current 2nd Piola-Kirchhoff stress tensor} \\ \mathbb{C} = \frac{\partial \mathbf{S}(\mathbf{E})}{\partial \mathbf{E}}, & \quad \dots \quad \text{elasticity tensor} \\ \tilde{k}_{abBD}, & \quad \dots \quad \text{integrand to be used for stiffness matrix.} \end{aligned}$$

Note that the expression is a scalar value in total with the inner parantheses term being a forth-order tensor. The expression is bilinear in  $\delta u$  and  $\Delta u$ . The virtual displacements,  $\delta \mathbf{u}$  and increments,  $\Delta \mathbf{u}$  are discretized as usual, leading to the stiffness matrix  $\mathbf{k}$ .

Herein,  $\partial \delta u_a / \partial X_B$  is a virtual displacement in component  $a = 1, 2, 3$ , derivative  $B$ . In discretized form it is  $\delta \hat{u}_{a,B}^L$  with dof index  $(L, B)$  and component number  $a$ . Eq. (36) has to hold for all  $\delta \hat{u}_{a,B}^L$ , these are the equations or rows in the stiffness matrix. The columns of the stiffness matrix correspond to the unknown increments,  $\Delta u_d$ , discretized as

$$\frac{\partial \Delta u_b}{\partial X_D} = \Delta \hat{u}_b^M \phi_{b,D}^M.$$

The coefficients,  $\Delta \hat{u}_b^M$ , are also the same for the real increments, not the derivatives, i.e.

$$\Delta u_b = \Delta \hat{u}_b^M \phi_b^M.$$

The entries of the stiffness matrix,  $k_{ij}$  are thus

$$k_{ij} = \int_{\Omega} \frac{\partial \phi_a^L}{\partial X_B} \tilde{k}_{abBD} \frac{\partial \phi_b^M}{\partial X_D} dV \quad (\text{sum over } B \text{ and } D)$$

with appropriate mappings  $i \mapsto (a, L)$  and  $j \mapsto (b, M)$ .

The stress,  $\mathbf{S}$  is computed by

$$\mathbf{S} = \mathbf{S}_{\text{vol}} + \mathbf{S}_{\text{iso}} = J p \mathbf{C}^{-1} + 2 \frac{\partial \Psi_{\text{iso}}}{\partial \mathbf{C}}$$

The elasticity tensor

$$\mathbb{C} = \mathbb{C}_{\text{vol}} + \mathbb{C}_{\text{iso}} = 2 \frac{\partial \mathbf{S}_{\text{vol}}}{\partial \mathbf{C}} + 2 \frac{\partial \mathbf{S}_{\text{iso}}}{\partial \mathbf{C}}$$

According to Holzapfel [2] the required quantities are as follows:

$$J = \det(\mathbf{F}), \quad \mathbf{C} = \mathbf{F}^\top \mathbf{F}$$

$$\bar{\mathbf{C}} = J^{-2/3} \mathbf{C} \quad (\text{p.228})$$

$$\bar{\gamma}_1 = 2 \left( \frac{\partial \Psi_{\text{iso}}(\bar{I}_1, \bar{I}_2)}{\partial \bar{I}_1} + \bar{I}_1 \frac{\partial \Psi_{\text{iso}}(\bar{I}_1, \bar{I}_2)}{\partial \bar{I}_2} \right) \quad (\text{p.234})$$

$$\bar{\gamma}_2 = -2 \frac{\partial \Psi_{\text{iso}}(\bar{I}_1, \bar{I}_2)}{\partial \bar{I}_2} \quad (\text{p.234})$$

$$\bar{\mathbf{S}} = 2 \frac{\partial \Psi_{\text{iso}}(\bar{I}_1, \bar{I}_2)}{\partial \bar{\mathbf{C}}} = \bar{\gamma}_1 \mathbf{I} + \bar{\gamma}_2 \bar{\mathbf{C}} \quad (\text{p.234})$$

$$(\mathbb{I})_{abcd} = \delta_{ac} \delta_{bd} \quad (\text{p.23})$$

$$\mathbb{P} = \mathbb{I} - \frac{1}{3} \mathbf{C}^{-1} \otimes \mathbf{C} \quad (\text{p.229})$$

$$\mathbf{S}_{\text{iso}} = J^{-2/3} \mathbb{P} : \bar{\mathbf{S}} \quad (\text{p.234})$$

$$p = \frac{d\Psi_{\text{vol}}(J)}{dJ} \quad (\text{p.230})$$

$$\mathbf{S}_{\text{vol}} = J p \mathbf{C}^{-1} \quad (\text{p.230, p.245})$$

$$\mathbf{S} = \mathbf{S}_{\text{iso}} + \mathbf{S}_{\text{vol}} \quad (\text{p.233})$$

$$\tilde{p} = p + J \frac{dp}{dJ} \quad (\text{p.255})$$

$$(\mathbf{C}^{-1} \odot \mathbf{C}^{-1})_{abcd} = \frac{1}{2} (C_{ac}^{-1} C_{bd}^{-1} + C_{ad}^{-1} C_{bc}^{-1}) \quad (\text{p.254})$$

$$\mathbb{C}_{\text{vol}} = J \tilde{p} \mathbf{C}^{-1} \otimes \mathbf{C}^{-1} - 2 J p \mathbf{C}^{-1} \odot \mathbf{C}^{-1} \quad (\text{p.254})$$

$$\bar{\delta}_1 = 4 \left( \frac{\partial^2 \Psi_{\text{iso}}}{\partial \bar{I}_1 \partial \bar{I}_1} + 2 \bar{I}_1 \frac{\partial^2 \Psi_{\text{iso}}}{\partial \bar{I}_1 \partial \bar{I}_2} + \frac{\partial^2 \Psi_{\text{iso}}}{\partial \bar{I}_2^2} + \bar{I}_1^2 \frac{\partial^2 \Psi_{\text{iso}}}{\partial \bar{I}_2 \partial \bar{I}_2} \right) \quad (\text{p.262})$$

$$\bar{\delta}_2 = -4 \left( \frac{\partial^2 \Psi_{\text{iso}}}{\partial \bar{I}_1 \partial \bar{I}_2} + \bar{I}_1 \frac{\partial^2 \Psi_{\text{iso}}}{\partial \bar{I}_2 \partial \bar{I}_2} \right) \quad (\text{p.262})$$

$$\bar{\delta}_3 = 4 \frac{\partial^2 \Psi_{\text{iso}}}{\partial \bar{I}_2 \partial \bar{I}_2} \quad (\text{p.262})$$

$$\bar{\delta}_4 = -4 \frac{\partial \Psi_{\text{iso}}}{\partial \bar{I}_2} \quad (\text{p.262})$$

$$\bar{\mathbb{C}} = J^{-4/3} (\bar{\delta}_1 \mathbf{I} \otimes \mathbf{I} + \bar{\delta}_2 (\mathbf{I} \otimes \bar{\mathbf{C}} + \bar{\mathbf{C}} \otimes \mathbf{I}) + \bar{\delta}_3 \bar{\mathbf{C}} \otimes \bar{\mathbf{C}} + \bar{\delta}_4 \mathbb{I}) \quad (\text{p.262})$$

$$\tilde{\mathbb{P}} = \mathbf{C}^{-1} \odot \mathbf{C}^{-1} - \frac{1}{3} \mathbf{C}^{-1} \otimes \mathbf{C}^{-1} \quad (\text{p.255})$$

$$\mathbb{C}_{\text{iso}} = \mathbb{P} : \bar{\mathbb{C}} : \mathbb{P}^\top + \frac{2}{3} J^{-2/3} \bar{\mathbf{S}} : \mathbf{C} \tilde{\mathbb{P}} - \frac{2}{3} (\mathbf{C}^{-1} \otimes \mathbf{S}_{\text{iso}} + \mathbf{S}_{\text{iso}} \otimes \mathbf{C}^{-1}) \quad (\text{p.255})$$

$$\mathbb{C} = \mathbb{C}_{\text{vol}} + \mathbb{C}_{\text{iso}} \quad (\text{p.254})$$

### 6.7.3 Handling of Dirichlet boundary conditions

The Newton algorithm to solve the nonlinear equation  $f(\mathbf{x}) = \mathbf{b}$  for  $\mathbf{x}$  with respect to Dirichlet boundary conditions  $\mathbf{x}_i = \mathbf{x}_{0i}$  for  $i \in \mathcal{I}$  can be summarized as follows:

```

set initial  $\mathbf{x}_0$ 
 $\mathbf{x}_i := \mathbf{x}_{0i}$  (initial values satisfy boundary conditions)
 $\mathbf{f} := f(\mathbf{x}_i)$ 
while  $\|\mathbf{b} - \mathbf{f}\| \geq \text{tol}$ 
  apply_BC_to_f
   $\mathbb{C}_i := J_f(\mathbf{x}_i)$ 
  apply_BC_to_ $\mathbb{C}_i$ 
  solve  $\mathbb{C}_i \Delta \mathbf{x}_i = \mathbf{b} - \mathbf{f}$  for  $\Delta \mathbf{x}_i$ 
   $\mathbf{x}_{i+1} := \mathbf{x}_i + \Delta \mathbf{x}_i$ 
   $\mathbf{f} := f(\mathbf{x}_{i+1})$ 

```

The operation `apply_BC_to_f` sets values  $\mathbf{f}_i = \mathbf{b}_i$  for all  $i \in \mathcal{I}$ . The operation `apply_BC_to_ $\mathbb{C}_i$`  sets rows and columns for which BC are specified to the identity matrix, i.e. set

$$\mathbb{C}_{ii} = 1 \quad \forall i \in \mathcal{I}$$

$$\mathbb{C}_{ij} = \mathbb{C}_{ji} = 0, \quad \forall j \neq i \forall i \in \mathcal{I}.$$

By doing so the system of linear equation  $\mathbb{C}_i \Delta \mathbf{x}_i = \mathbf{b} - \mathbf{f}$  contains the equations  $\Delta x_i = b_i - f_i = 0$  for  $i \in \mathcal{I}$ . Therefore the variables  $\mathbf{x}_i$  don't change their value in the loop and keep the initially assigned Dirichlet BC value.

### 6.7.4 Mixed formulation

Now a Lagrange multiplier  $p$  is added to the functional, that enforces the incompressibility constraint,  $J = 1$ . The functional analogous to (30) is expressed as

$$\Pi_L(\mathbf{u}, p) = \Pi_{\text{int}}(\mathbf{u}, p) + \Pi_{\text{ext}}(\mathbf{u}) \quad \text{with}$$

$$\Pi_{\text{int}}(\mathbf{u}, p) = \int_{\Omega} \left( p(J(\mathbf{u}) - 1) + \Psi_{\text{iso}}(\bar{\mathbf{C}}(\mathbf{u})) \right) dV.$$

The Lagrange multiplier,  $p$ , is the real hydrostatic pressure in this case. The field variable  $p$  needs to be discretized in addition to the displacement field  $\mathbf{u}$ .

The principle of virtual work in this case has the form

$$D_{\delta \mathbf{u}} \Pi_L = \delta W_{\text{int}} - \delta W_{\text{ext}} = 0$$

$$\Leftrightarrow D_{\delta \mathbf{u}} \Pi_{\text{int}}(\mathbf{u}, p) - \delta W_{\text{ext}} = 0$$

$$\Leftrightarrow \int_{\Omega} \left( J(\mathbf{u}) p \mathbf{C}^{-1} + 2 \frac{\partial \Psi_{\text{iso}}(\bar{\mathbf{C}}(\mathbf{u}))}{\partial \mathbf{C}} \right) : \delta \mathbf{E}(\mathbf{u}) dV - \delta W_{\text{ext}} = 0,$$

$$D_{\delta p} \Pi_L = \int_{\Omega} (J(\mathbf{u}) - 1) \delta p dV = 0$$

Linearization yields

$$\begin{aligned}
D_{\delta \mathbf{u}, \Delta \mathbf{u}}^2 \Pi_L(\mathbf{u}, p) &= \int_{\Omega} (\nabla \delta \mathbf{u} : \nabla \Delta \mathbf{u} \mathbf{S} + \mathbf{F}^\top \nabla \delta \mathbf{u} : (\mathbb{C}_{\text{vol}} + \mathbb{C}_{\text{iso}}) : \mathbf{F}^\top \nabla \Delta \mathbf{u}) \, dV \\
D_{\delta p, \Delta \mathbf{u}}^2 \Pi_L(\mathbf{u}, p) &= D_{\Delta \mathbf{u}, \delta p}^2 \Pi_L(\mathbf{u}, p) = \int_{\Omega} J(\mathbf{u}) \nabla \cdot \Delta \mathbf{u} \, \delta p \, dV \\
D_{\delta p, \Delta p}^2 \Pi_L(\mathbf{u}, p) &= 0
\end{aligned} \tag{37}$$

Again Holzapfel [2]  $\mathbb{C}_{\text{vol}}$  p.254,  $\mathbb{C}_{\text{iso}}$  p.255,

Note, that the term for  $D_{\delta \mathbf{u}, \Delta \mathbf{u}}^2 \Pi_L(\mathbf{u}, p)$  is the same as in (36) . (37) reads in index notation:

$$\begin{aligned}
D_{\delta p, \Delta \mathbf{u}}^2 \Pi_L(\mathbf{u}, p) &= \int_{\Omega} \frac{\partial \delta u_a}{\partial X_B} (\delta_{ab} S_{AB} + F_{aA} F_{bC} C_{ABCD}) \frac{\partial \Delta u_b}{\partial X_D} \, dV \\
D_{\delta p, \Delta \mathbf{u}}^2 \Pi_L(\mathbf{u}, p) &= \int_{\Omega} J \, \delta p_A \frac{\partial \Delta u_{aB}}{\partial X_a} \, dV \\
D_{\delta p, \Delta p}^2 \Pi_L(\mathbf{u}, p) &= 0
\end{aligned}$$

## 6.8 Dynamic elasticity

Governing equation

$$\mathbf{M} \ddot{\mathbf{u}} + \mathbf{C} \dot{\mathbf{u}} + \mathbf{K} \mathbf{u} - \mathbf{f} = \mathbf{0}$$

with

$$\begin{aligned}
\mathbf{M}_{ij} &= \int_{\Omega} \mathbf{N}_i^\top \rho \mathbf{N}_j \, dV : \text{ mass matrix, density } \rho \\
\mathbf{C}_{ij} &= \int_{\Omega} \mathbf{N}_i^\top \boldsymbol{\mu} \mathbf{N}_j \, dV : \text{ damping matrix, viscosity parameters } \boldsymbol{\mu}
\end{aligned}$$

(Zienkiewicz I p.470)

Solve with *leapfrog* integration.

## 6.9 Examples with analytical solution

### 6.9.1 Uniaxial extension with constant total load

The domain in reference and current configuration is depicted in fig. 3) and defined as follows, symmetry between  $y$  and  $z$ -direction is assumed.

$$\begin{aligned}
\Omega_{\text{ref}} &= [0, L_x] \times [0, L_z]^2 \\
\Omega_{\text{current}} &= [0, \lambda L_x] \times [0, L_z / \sqrt{\lambda}]^2,
\end{aligned}$$

A load  $t(\mathbf{x}) = t_{\text{max}} / l_z^2$  is defined on the right surface area. In reference configuration it is

$$T(\mathbf{x}) = \frac{t_{\text{max}}}{L_z^2},$$

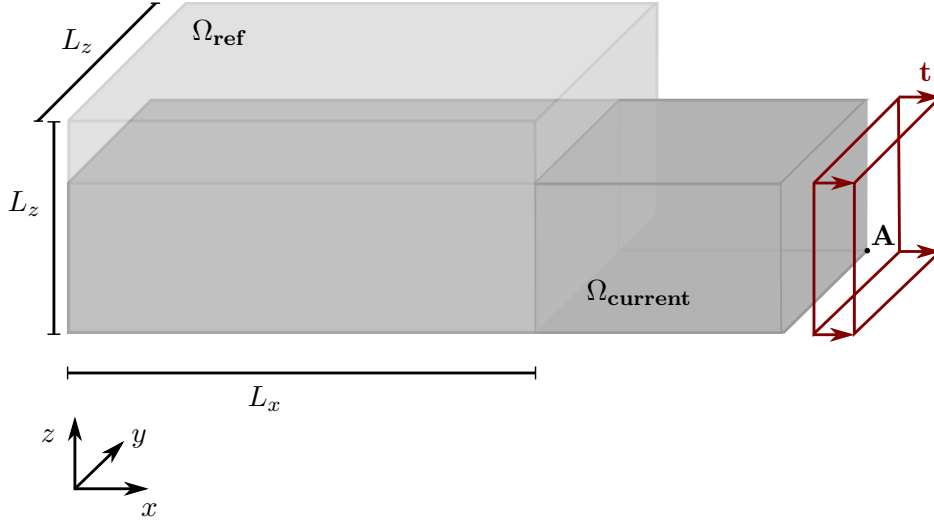


Abbildung 3: Uniaxial extension example

in current configuration

$$\begin{aligned}
 t(\mathbf{x}) &= \frac{t_{\max}}{l_z^2} = \frac{t_{\max}}{\mathbf{L}_z \cdot \mathbf{C} \mathbf{L}_z} \quad \text{with } \mathbf{L}_z = (0, 0, L_z), \quad l_z^2 = \frac{L_z^2}{\lambda} \\
 &= t_{\max} \frac{\lambda}{L_z^2}
 \end{aligned}$$

Because the normal vector is the same,  $\mathbf{n} = \mathbf{N}$ , we have

$$T(\mathbf{x}) dS = \mathbf{t} dS, \quad T(\mathbf{x}) = \mathbf{t}(\mathbf{x}) \frac{dS}{dS} = \mathbf{t}(\mathbf{x}) \frac{l_z^2}{L_z^2} = t(\mathbf{x}) \frac{1}{\lambda} = \frac{t_{\max}}{L_z^2}$$

The geometric quantities can be computed to

$$\mathbf{x} = \begin{pmatrix} \lambda X_1 \\ X_2/\sqrt{\lambda} \\ X_3/\sqrt{\lambda} \end{pmatrix}, \quad \mathbf{F} = \begin{bmatrix} \lambda & 0 & 0 \\ 0 & 1/\sqrt{\lambda} & 0 \\ 0 & 0 & 1/\sqrt{\lambda} \end{bmatrix}, \quad \mathbf{C} = \begin{bmatrix} \lambda^2 & 0 & 0 \\ 0 & 1/\lambda & 0 \\ 0 & 0 & 1/\lambda \end{bmatrix},$$

$$\text{tr}(\mathbf{C}) = \lambda^2 + 2/\sqrt{\lambda}, \quad \text{tr}(\mathbf{C}^2) = \lambda^4 + 2/\lambda^2$$

$$I_1 = \text{tr}(\mathbf{C}) = \lambda^2 + 2/\lambda$$

$$I_2 = \frac{1}{2} (\text{tr}(\mathbf{C})^2 - \text{tr}(\mathbf{C}^2))$$

$$= \frac{1}{2} \left( \lambda^4 + \frac{4\lambda^2}{\sqrt{\lambda}} + \frac{4}{\lambda} - \lambda^4 - \frac{2}{\lambda^2} \right) = 2\lambda^{3/2} + 2\lambda^{-1} - \lambda^{-2}$$

$$I_3 = J = \det \mathbf{F} = 1, \quad \bar{\mathbf{C}} = \mathbf{C}$$

$$\mathbf{E} = \frac{1}{2} \begin{bmatrix} \lambda^2 - 1 & 0 & 0 \\ 0 & 1/\lambda - 1 & 0 \\ 0 & 0 & 1/\lambda - 1 \end{bmatrix}, \quad \mathbf{C}^{-1} = \begin{bmatrix} 1/\lambda^2 & 0 & 0 \\ 0 & \lambda & 0 \\ 0 & 0 & \lambda \end{bmatrix}$$



### Solution using penalty method

For the incompressible Mooney-Rivlin material with  $\Psi(I_1, I_2) = c_1 (I_1 - 3) + c_2 (I_2 - 3)$ ,  $I_3 = 1$  we get the fictitious PK2 stress tensor as,

$$\bar{\mathbf{S}} = 2(c_1 + c_2 I_1) \mathbf{I} - 2c_2 \bar{\mathbf{C}} = \text{diag} \left( 2c_1 + 4c_2/\lambda, \quad 2c_1 - 2c_2/\lambda, \quad 2c_1 - 2c_2/\lambda \right)$$

The displacements at node A are given by

$$\hat{\mathbf{u}}^A = (\lambda L_x - L_x, \frac{L_z}{\sqrt{\lambda}} - L_z, 0) = ((\lambda - 1) L_x, (\frac{1}{\sqrt{\lambda}} - 1) L_z, 0).$$

Computation of the fictitious stress tensor involves coefficients,  $\bar{\gamma}_1, \bar{\gamma}_2$ :

$$\begin{aligned} \bar{\gamma}_1 &= 2 \left( \frac{\partial \Psi_{\text{iso}}(\bar{I}_1, \bar{I}_2)}{\partial \bar{I}_1} + \bar{I}_1 \frac{\partial \Psi_{\text{iso}}(\bar{I}_1, \bar{I}_2)}{\partial \bar{I}_2} \right) \\ \bar{\gamma}_2 &= -2 \frac{\partial \Psi_{\text{iso}}(\bar{I}_1, \bar{I}_2)}{\partial \bar{I}_2} \end{aligned}$$

With  $\Psi(\bar{I}_1, \bar{I}_2) = c_1 (\bar{I}_1 - 3) + c_2 (\bar{I}_2 - 3)$  we get

$$\frac{\partial \Psi_{\text{iso}}(\bar{I}_1, \bar{I}_2)}{\partial \bar{I}_1} = c_1, \quad \frac{\partial \Psi_{\text{iso}}(\bar{I}_1, \bar{I}_2)}{\partial \bar{I}_2} = c_2$$

Now assume  $c_2 = 0$ .

$$\mathbf{C} : \bar{\mathbf{S}} = 2c_1 \lambda^2 + 4c_1 \lambda^{-1}$$

$$\begin{aligned} \mathbf{S}_{\text{iso}} &= J^{-2/3} \mathbb{P} : \bar{\mathbf{S}} = \bar{\mathbf{S}} - \frac{1}{3} (\mathbf{C}^{-1} \otimes \mathbf{C}) : \bar{\mathbf{S}} \\ &= \bar{\mathbf{S}} - \frac{1}{3} \begin{bmatrix} 1/\lambda^2 \cdot (2c_1 \lambda^2 + 4c_1 \lambda^{-1}) & 0 & 0 \\ 0 & \lambda \cdot (2c_1 \lambda^2 + 4c_1 \lambda^{-1}) & 0 \\ 0 & 0 & \lambda \cdot (2c_1 \lambda^2 + 4c_1 \lambda^{-1}) \end{bmatrix} \\ &= \begin{bmatrix} 2c_1 & 0 & 0 \\ 0 & 2c_1 & 0 \\ 0 & 0 & 2c_1 \end{bmatrix} - \frac{1}{3} \begin{bmatrix} 2c_1 + 4c_1 \lambda^{-3} & 0 & 0 \\ 0 & 2c_1 \lambda^3 + 4c_1 & 0 \\ 0 & 0 & 2c_1 \lambda^3 + 4c_1 \end{bmatrix} \\ &= \begin{bmatrix} \frac{4}{3} c_1 (1 - \lambda^{-3}) & 0 & 0 \\ 0 & \frac{2}{3} c_1 (1 - \lambda^3) & 0 \\ 0 & 0 & \frac{2}{3} c_1 (1 - \lambda^3) \end{bmatrix} \\ \mathbf{S} &= \mathbf{S}_{\text{iso}} + \mathbf{S}_{\text{vol}} = \mathbf{S}_{\text{iso}} + Jp \mathbf{C}^{-1} = \begin{bmatrix} \frac{4}{3} c_1 (1 - \lambda^{-3}) + p \lambda^{-2} & 0 & 0 \\ 0 & \frac{2}{3} c_1 (1 - \lambda^3) + p \lambda & 0 \\ 0 & 0 & \frac{2}{3} c_1 (1 - \lambda^3) + p \lambda \end{bmatrix} \end{aligned}$$

The artificial pressure,  $p$  is different from the real hydrostatic pressure. It is given by

$$p = \frac{d\Psi_{\text{vol}}(J)}{dJ} = \frac{d}{dJ} \left( \frac{\kappa}{2} (J - 1)^2 \right) = \kappa (J - 1)$$

An incompressible solution has  $J \approx 1$  but because  $\kappa \rightarrow \infty$  it has a  $p \neq 0$ . To satisfy the force equilibrium we get

**y-direction:**

$$S_{22} = 0 \Rightarrow \frac{2}{3} c_1 (1 - \lambda^3) + p \lambda = 0 \Rightarrow p = \frac{2}{3} c_1 (\lambda^3 - 1) \lambda^{-1}$$

**x-direction:**

$$-\sigma_{11} l_z^2 + t_{\max} \frac{\lambda}{L_z^2} l_z^2 = 0, \Rightarrow \sigma_{11} = t_{\max} \frac{\lambda}{L_z^2}$$

$$\mathbf{S} = J \mathbf{F}^{-1} \boldsymbol{\sigma} \mathbf{F}^{-\top}, \quad S_{11} = \lambda^{-1} \sigma_{11} \lambda^{-1}$$

$$\Rightarrow S_{11} \stackrel{!}{=} \frac{t_{\max}}{L_z^2 \lambda}$$

$$\Leftrightarrow \frac{4}{3} c_1 (1 - \lambda^{-3}) + \left( \frac{2}{3} c_1 (\lambda^3 - 1) \lambda^{-1} \right) \lambda^{-2} \stackrel{!}{=} \frac{t_{\max}}{L_z^2 \lambda}$$

$$\Leftrightarrow \frac{4}{3} c_1 (1 - \lambda^{-3}) + \frac{2}{3} c_1 (1 - \lambda^{-3}) = \frac{t_{\max}}{L_z^2 \lambda}$$

$$\Leftrightarrow 2 c_1 (1 - \lambda^{-3}) = \frac{t_{\max}}{L_z^2 \lambda} \Leftrightarrow 2 L_z^2 c_1 (\lambda - \lambda^{-2}) = t_{\max}$$

$$\Leftrightarrow 2 c_1 L_z^2 (\lambda^3 - 1) - t_{\max} \lambda^2 = 0$$

```
>>> solve(2./3*c0*lz*lz*(lambdaValue**3 - 1) - tmax*lambdaValue**2,
lambdaValue, simplify=True, positive=True)[2]
0.5*tmax**2/(c0*lz**2*(4.0*c0**3*lz**6 + tmax**3 + (-tmax**6 + (4.0*
c0**3*lz**6 + tmax**3)**2)**0.5)**(1/3)) + 0.5*tmax/(c0*lz**2) + 0.5
*(4.0*c0**3*lz**6 + tmax**3 + (-tmax**6 + (4.0*c0**3*lz**6 + tmax**3
)**2)**0.5)**(1/3)/(c0*lz**2)
```

### Direct solution using pressure from equilibrium

Analytical approach (p.224). Assuming Mooney-Rivlin with  $c_2 = 0$  (Neo-Hookean),

$$\begin{aligned} \mathbf{S} &= -p \mathbf{C}^{-1} + 2 \left( \frac{\partial \Psi}{\partial I_1} + I_1 \frac{\partial \Psi}{\partial I_2} \right) \mathbf{I} - 2 \frac{\partial \Psi}{\partial I_2} \mathbf{C} \\ &= -p \mathbf{C}^{-1} + 2 c_1 \mathbf{I} = \begin{bmatrix} -p/\lambda^2 + 2 c_1 & 0 & 0 \\ 0 & -p \lambda + 2 c_1 & 0 \\ 0 & 0 & -p \lambda + 2 c_1 \end{bmatrix} \end{aligned}$$

From equilibrium we get

**y-direction:**

$$S_{22} = 0 \quad \Rightarrow \quad -p\lambda + 2c_1 = 0 \quad \Rightarrow \quad p = \frac{2c_1}{\lambda}$$

**x-direction:**

$$-\sigma_{11} l_z^2 + t_{\max} \frac{\lambda}{L_z^2} l_z^2 = 0, \quad \Rightarrow \quad \sigma_{11} = t_{\max} \frac{\lambda}{L_z^2}$$

$$\mathbf{S} = J \mathbf{F}^{-1} \boldsymbol{\sigma} \mathbf{F}^{-\top}, \quad S_{11} = \lambda^{-1} \sigma_{11} \lambda^{-1}$$

$$\Rightarrow \quad S_{11} \stackrel{!}{=} \frac{t_{\max}}{L_z^2 \lambda}$$

$$\Rightarrow \quad -p\lambda^{-2} + 2c_1 = -2c_1\lambda^{-3} + 2c_1 = \frac{t_{\max}}{L_z^2 \lambda}$$

$$\Rightarrow \quad 2c_1\lambda^3 - \frac{t_{\max}}{L_z^2} \lambda^2 - 2c_1 = 0 \quad \Rightarrow \quad \lambda = \text{nonlinear}(t_{\max})$$

```
>>> solve(2*c1*l**3 - t*l**2 - 2*c1, l, simplify=True, positive=True)[2]
-t**2/(6*c1*(-108*c1**3 - t**3 + sqrt(-t**6 + (108*c1**3 + t**3)**2)
)**(1/3)) + t/(6*c1) - (-108*c1**3 - t**3 + sqrt(-t**6 + (108*c1**3
+ t**3)**2))**(1/3)/(6*c1)
```

Internal energy:

$$\delta W_{\text{int}} = \int_{\Omega} \frac{1}{2} S_{AB}(\hat{\mathbf{u}}) \delta u_a^L \left( F_{aB}(\hat{\mathbf{u}}) \frac{\partial \phi_L}{\partial X_A} + F_{aA}(\hat{\mathbf{u}}) \frac{\partial \phi_L}{\partial X_B} \right) dV$$

Now we assume a virtual displacement at node  $\mathcal{A}$ ,  $\delta u_a(\mathbf{X}) = \delta u_a^{\mathcal{A}} \phi^{\mathcal{A}}(\mathbf{X})$ .

$$\delta W_{\text{int}} = \int_{\Omega} \frac{1}{2} S_{AB} \delta u_a^{\mathcal{A}} \left( F_{aB} \frac{\partial \phi^{\mathcal{A}}}{\partial X_A} + F_{aA} \frac{\partial \phi^{\mathcal{A}}}{\partial X_B} \right) dV,$$

The ansatz function  $\phi^{\mathcal{A}}$  and their derivatives are:

$$\phi^{\mathcal{A}}(\mathbf{X}) = \frac{X_1}{L_x} \frac{X_2}{L_z} \left( 1 - \frac{X_3}{L_z} \right),$$

$$\frac{\partial \phi^{\mathcal{A}}}{\partial X_1} = \frac{1}{L_x} \frac{X_2}{L_z} \left( 1 - \frac{X_3}{L_z} \right),$$

$$\frac{\partial \phi^{\mathcal{A}}}{\partial X_2} = \frac{1}{L_z} \frac{X_1}{L_x} \left( 1 - \frac{X_3}{L_z} \right),$$

$$\frac{\partial \phi^{\mathcal{A}}}{\partial X_3} = -\frac{X_1}{L_x} \frac{X_2}{L_z} \frac{1}{L_z},$$

For  $\delta \mathbf{u}(\mathbf{X}) = (\phi^{\mathcal{A}}(\mathbf{X}), 0, 0)^{\top}$ , i.e.  $\delta u_1^{\mathcal{A}} = 1, \delta u_2^{\mathcal{A}} = \delta u_3^{\mathcal{A}} = 0$  we compute the virtual internal

energy:

$$\begin{aligned}
\delta W_{\text{int}} &= \int_{\Omega} \frac{1}{2} S_{AB} \left( F_{1B} \frac{\partial \phi^A}{\partial X_A} + F_{1A} \frac{\partial \phi^A}{\partial X_B} \right) dV = \int_{\Omega} \frac{1}{2} S_{11} \left( F_{11} \frac{\partial \phi^A}{\partial X_1} + F_{11} \frac{\partial \phi^A}{\partial X_1} \right) dV \\
&= S_{11} \int_{\Omega} \lambda \frac{1}{L_x} \frac{X_2}{L_z} \left( 1 - \frac{X_3}{L_z} \right) dV = S_{11} \lambda \frac{L_x}{L_x} \frac{1}{2} \frac{L_z^2}{L_z} \int_0^{L_z} \left( 1 - \frac{X_3}{L_z} \right) dX_3 \\
&= S_{11} \lambda \frac{1}{2} L_z \left( L_z - \frac{1}{2} \frac{L_z^2}{L_z} \right) \\
&= \frac{1}{4} L_z^2 S_{11} \lambda = \frac{1}{4} L_z^2 F_{11} S_{11}
\end{aligned}$$

Virtual external energy

$$\begin{aligned}
\delta W_{\text{ext}} &= \int_{\Omega} (\mathbf{B} - \rho_0 \ddot{\mathbf{u}}) \cdot \delta \mathbf{u} dV + \int_{\partial\Omega} \bar{\mathbf{T}} \cdot \delta \mathbf{u} dS \\
&= \int_{\partial\Omega} (T(\mathbf{X}), 0, 0)^{\top} \cdot (\delta u_1(\mathbf{X}), \delta u_2(\mathbf{X}), \delta u_3(\mathbf{X}))^{\top} dS \\
&= \int_0^{L_z} \int_0^{L_z} \frac{t_{\max}}{L_z^2} \phi^A(L_x, X_2, X_3) dX_2 dX_3 \\
&= \frac{t_{\max}}{L_z^2} \int_0^{L_z} \int_0^{L_z} \frac{L_x}{L_x} \frac{X_2}{L_z} \left( 1 - \frac{X_3}{L_z} \right) dX_2 dX_3 \\
&= \frac{t_{\max}}{L_z^2} \frac{1}{2} \frac{L_z^2}{L_z} \left( L_z - \frac{1}{2} \frac{L_z^2}{L_z} \right) = \frac{1}{4} L_z^2 t_{\max} \frac{1}{L_z^2} = \frac{1}{4} t_{\max} = \frac{1}{4} L_z^2 T(\mathbf{x})
\end{aligned}$$

From principle of virtual work we should get the correct stress.

$$\begin{aligned}
\delta W_{\text{int}} - \delta W_{\text{ext}} &= 0 \quad \Leftrightarrow \quad \frac{1}{4} L_z^2 S_{11} \lambda - \frac{1}{4} t_{\max} = 0 \\
&\Leftrightarrow \quad \frac{1}{4} (L_z^2 S_{11} \lambda - t_{\max}) = 0 \\
&\Leftrightarrow \quad L_z^2 S_{11} \lambda = t_{\max} \quad \Leftrightarrow \quad S_{11} = \frac{t_{\max}}{\lambda L_z^2} = \frac{1}{\lambda^2} t(\mathbf{X}) = \frac{1}{\lambda} T(\mathbf{X}) \quad \checkmark
\end{aligned}$$

Note: we do not get any further information from the principle of virtual work further than the equilibrium conditions. This is because the stress was preserved as variable and not replaced by the constitutive equations, that relate them to strain and stretch  $\lambda$ .

Assume Mooney-Rivlin with  $c_2 \neq 0$ ,

$$\begin{aligned}
\mathbf{S} &= -p\mathbf{C}^{-1} + 2\left(\frac{\partial\Psi}{\partial I_1} + I_1\frac{\partial\Psi}{\partial I_2}\right)\mathbf{I} - 2\frac{\partial\Psi}{\partial I_2}\mathbf{C} \\
&= -p\mathbf{C}^{-1} + (2c_1 + \text{tr}\mathbf{C}c_2)\mathbf{I} - 2c_2\mathbf{C} \\
&= \text{diag}\left(-p\lambda^{-2} + 2c_1 + 2c_2\lambda^2 + 4c_2\lambda^{-1} - 2c_2\lambda^2, \right. \\
&\quad \left.-p\lambda + 2c_1 + 2c_2\lambda^2 + 4c_2\lambda^{-1} - 2c_2\lambda^{-1}, \right. \\
&\quad \left.-p\lambda + 2c_1 + 2c_2\lambda^2 + 4c_2\lambda^{-1} - 2c_2\lambda^{-1}\right) \\
&= \text{diag}\left(-p\lambda^{-2} + 2c_1 + 4c_2\lambda^{-1}, \right. \\
&\quad \left.-p\lambda + 2c_1 + 2c_2\lambda^2 + 2c_2\lambda^{-1}, \right. \\
&\quad \left.-p\lambda + 2c_1 + 2c_2\lambda^2 + 2c_2\lambda^{-1}\right)
\end{aligned}$$

### 6.9.2 Uniaxial extension with constant load per surface area

This is the same example as the previous except now with load constant per surface area, but total load varying,  $t(\mathbf{X}) = \hat{t} = t_{\max}/L_z^2$ ,  $T = \hat{t}/\lambda$ . For Neo-Hookean we have the equilibrium conditions,

**$y$ -direction:**

$$S_{22} = 0 \quad \Rightarrow \quad -p\lambda + 2c_1 = 0 \quad \Rightarrow \quad p = \frac{2c_1}{\lambda}$$

**$x$ -direction:**

$$\sigma_{11} = \hat{t} \quad \Leftrightarrow \quad \mathbf{S} = J\mathbf{F}^{-1}\boldsymbol{\sigma}\mathbf{F}^{-\top}, \quad S_{11} = \lambda^{-1}\sigma_{11}\lambda^{-1}$$

$$\Rightarrow \quad S_{11} = \hat{t}\lambda^{-2} \quad \Rightarrow \quad -p\lambda^{-2} + 2c_1 = -2c_1\lambda^{-3} + 2c_1 = \hat{t}\lambda^{-2}$$

$$\Rightarrow \quad 2c_1\lambda^3 - \hat{t}\lambda - 2c_1 = 0$$

```
>>> solve(2*c1*1**(3) -t*1**2 -2*c1, 1, simplify=True, positive=True)[0].evalf()
0.30285343213869*(1.81712059283214*that + (18.0*c0**1.5 + 2.44948974278318*(5
```

Internal virtual energy:

$$\delta W_{\text{int}} = \frac{1}{4} L_z^2 F_{11} S_{11}$$

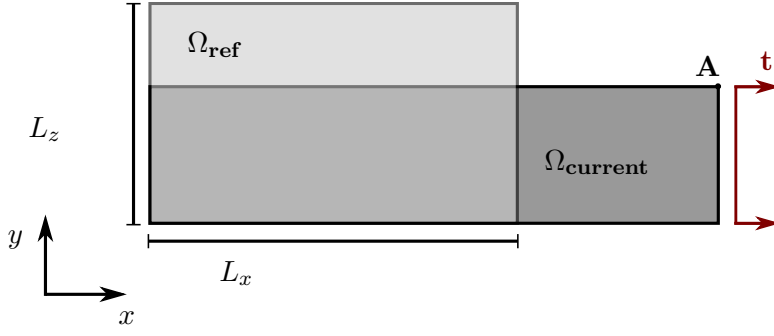


Abbildung 4: Uniaxial extension example

External virtual energy:

$$\begin{aligned}
 \delta W_{\text{ext}} &= \int_{\Omega} (\mathbf{B} - \rho_0 \ddot{\mathbf{u}}) \cdot \delta \mathbf{u} \, dV + \int_{\partial\Omega} \bar{\mathbf{T}} \cdot \delta \mathbf{u} \, dS \\
 &= \int_{\partial\Omega} (T(\mathbf{X}), 0, 0)^\top \cdot \delta \mathbf{u} \, dS = \int_0^{L_z} \int_0^{L_z} \frac{\hat{t}}{\lambda} \phi^A(L_x, X_2, X_3) \, dX_2 \, dX_3 \\
 &= \frac{\hat{t}}{\lambda} \int_0^{L_z} \int_0^{L_z} \frac{L_x}{L_x} \frac{X_2}{L_z} \left(1 - \frac{X_3}{L_z}\right) \, dX_2 \, dX_3 \\
 &= \frac{\hat{t}}{\lambda} \frac{1}{2} \frac{L_z^2}{L_z} \left(L_z - \frac{1}{2} \frac{L_z^2}{L_z}\right) = \frac{1}{4} L_z^2 \frac{\hat{t}}{\lambda}
 \end{aligned}$$

We get

$$\delta W_{\text{int}} - \delta W_{\text{ext}} = \frac{1}{4} L_z^2 \left( F_{11} S_{11} - \frac{\hat{t}}{\lambda} \right) \Leftrightarrow S_{11} = \hat{t} \lambda^{-2} \quad \checkmark$$

## 6.10 2D uniaxial extension with constant total load

The domain in reference and current configuration is depicted in fig. 4) and defined as follows,

$$\Omega_{\text{ref}} = [0, L_x] \times [0, L_y]$$

$$\Omega_{\text{current}} = [0, \lambda L_x] \times [0, L_y \lambda^{-1}],$$

A load  $t(\mathbf{x}) = t_{\text{max}}/l_y$  is defined on the right border. In reference configuration it is

$$T(\mathbf{x}) = \frac{t_{\text{max}}}{L_y},$$

in current configuration

$$t(\mathbf{x}) = \frac{t_{\text{max}}}{l_y} = t_{\text{max}} \frac{\lambda}{L_y}.$$

Because the normal vector is the same,  $\mathbf{n} = \mathbf{N}$ , we have

$$T(\mathbf{x}) dS = \mathbf{t} ds, \quad T(\mathbf{x}) = \mathbf{t}(\mathbf{x}) \frac{ds}{dS} = \mathbf{t}(\mathbf{x}) \frac{l_y}{L_y} = t(\mathbf{x}) \frac{1}{\lambda}, \quad \text{i.e.} \quad T(\mathbf{x}) = t(\mathbf{x}) \lambda^{-1}$$

The geometric quantities can be computed to

$$\mathbf{x} = \begin{pmatrix} \lambda X_1 \\ \lambda^{-1} X_2 \end{pmatrix}, \quad \mathbf{F} = \begin{bmatrix} \lambda & 0 \\ 0 & \lambda^{-1} \end{bmatrix}, \quad \mathbf{C} = \begin{bmatrix} \lambda^2 & 0 \\ 0 & \lambda^{-2} \end{bmatrix},$$

$$\text{tr}(\mathbf{C}) = \lambda^2 + \lambda^{-2}, \quad \text{tr}(\mathbf{C}^2) = \lambda^4 + \lambda^{-4}$$

$$I_1 = \text{tr}(\mathbf{C}) = \lambda^2 + \lambda^{-2}$$

$$I_2 = \frac{1}{2} (\text{tr}(\mathbf{C})^2 - \text{tr}(\mathbf{C}^2)) = \frac{1}{2} (\lambda^4 + 2 + \lambda^{-4} - \lambda^4 - \lambda^{-4}) = 1$$

$$I_3 = J = \det \mathbf{F} = 1, \quad \bar{\mathbf{C}} = \mathbf{C}$$

$$\mathbf{E} = \frac{1}{2} \begin{bmatrix} \lambda^2 - 1 & 0 \\ 0 & \lambda^{-2} - 1 \end{bmatrix}, \quad \mathbf{C}^{-1} = \begin{bmatrix} \lambda^{-2} & 0 \\ 0 & \lambda^2 \end{bmatrix}$$

### Solution using penalty method

For the incompressible Mooney-Rivlin material with  $\Psi(I_1, I_2) = c_1 (I_1 - 3) + c_2 (I_2 - 3)$ ,  $I_3 = 1$  we get the fictitious PK2 stress tensor as, The displacements at node A are given by

$$\hat{\mathbf{u}}^A = (\lambda L_x - L_x, L_y \lambda^{-1} - L_y) = ((\lambda - 1) L_x, (\lambda^{-1} - 1) L_y).$$

Assuming Neo-Hookean material law,  $\Psi(I_1) = c_1 (I_1 - 2)$  the fictitious PK2 stress tensor yields

$$\bar{\mathbf{S}} = 2 c_1 \mathbf{I}.$$

and the stress tensor

$$\mathbf{C} : \bar{\mathbf{S}} = 2 c_1 (\lambda^2 + \lambda^{-2})$$

$$\begin{aligned} \mathbf{S}_{\text{iso}} &= J^{-1} \mathbb{P} : \bar{\mathbf{S}} = \bar{\mathbf{S}} - \frac{1}{2} (\mathbf{C}^{-1} \otimes \mathbf{C}) : \bar{\mathbf{S}} \\ &= \bar{\mathbf{S}} - \frac{1}{2} \begin{bmatrix} \lambda^{-2} \cdot (2 c_1 (\lambda^2 + \lambda^{-2})) & 0 \\ 0 & \lambda^2 \cdot (2 c_1 (\lambda^2 + \lambda^{-2})) \end{bmatrix} \\ &= \begin{bmatrix} 2 c_1 & 0 \\ 0 & 2 c_1 \end{bmatrix} - \frac{1}{2} \begin{bmatrix} 2 c_1 (1 + \lambda^{-4}) & 0 \\ 0 & 2 c_1 (\lambda^4 + 1) \end{bmatrix} \\ &= \begin{bmatrix} c_1 (1 - \lambda^{-4}) & 0 \\ 0 & c_1 (1 - \lambda^4) \end{bmatrix} \end{aligned}$$

$$\mathbf{S} = \mathbf{S}_{\text{iso}} + \mathbf{S}_{\text{vol}} = \mathbf{S}_{\text{iso}} + J p \mathbf{C}^{-1} = \begin{bmatrix} c_1 (1 - \lambda^{-4}) + p \lambda^{-2} & 0 \\ 0 & c_1 (1 - \lambda^4) + p \lambda^2 \end{bmatrix}$$

The artificial pressure,  $p$  is different from the real hydrostatic pressure. It is given by

$$p = \frac{d\Psi_{\text{vol}}(J)}{dJ} = \frac{d}{dJ} \left( \frac{\kappa}{2} (J-1)^2 \right) = \kappa (J-1)$$

An incompressible solution has  $J \approx 1$  but because  $\kappa \rightarrow \infty$  it has a  $p \neq 0$ . To satisfy the force equilibrium we get

**y-direction:**

$$S_{22} = 0 \quad \Rightarrow \quad c_1 (1 - \lambda^4) + p \lambda^2 = 0 \quad \Rightarrow \quad p = (c_1 (\lambda^4 - 1)) \lambda^{-2} = c_1 (\lambda^2 - \lambda^{-2})$$

**x-direction:**

$$-\sigma_{11} l_y + t_{\text{max}} \frac{\lambda}{L_y} l_y = 0, \quad \Rightarrow \quad \sigma_{11} = t_{\text{max}} \frac{\lambda}{L_y}$$

$$\mathbf{S} = J \mathbf{F}^{-1} \boldsymbol{\sigma} \mathbf{F}^{-\top}, \quad S_{11} = \lambda^{-1} \sigma_{11} \lambda^{-1}$$

$$\Rightarrow \quad S_{11} \stackrel{!}{=} \frac{t_{\text{max}}}{L_y \lambda}$$

$$\Leftrightarrow \quad c_1 (1 - \lambda^{-4}) + (c_1 (\lambda^2 - \lambda^{-2})) \lambda^{-2} \stackrel{!}{=} \frac{t_{\text{max}}}{L_y \lambda}$$

$$\Leftrightarrow \quad c_1 (1 - \lambda^{-4}) + c_1 (1 - \lambda^{-4}) = \frac{t_{\text{max}}}{L_y \lambda}$$

$$\Leftrightarrow \quad 2 c_1 (1 - \lambda^{-4}) = \frac{t_{\text{max}}}{L_y \lambda} \quad \Leftrightarrow \quad 2 L_y c_1 (\lambda - \lambda^{-3}) = t_{\text{max}}$$

$$\Leftrightarrow \quad 2 c_1 L_y (\lambda^4 - 1) - t_{\text{max}} \lambda^3 = 0$$

```
>>> solve(2*c0*ly*(lambdaValue**4 - 1) - tmax*lambdaValue**3,
lambdaValue, simplify=True, positive=True)[2]
(tmax/8 + sqrt(-384*3**(2/3)*c0**(8/3)*ly**(8/3)/(-9*tmax**2
+ sqrt(3)*sqrt(4096*c0**4*ly**4 + 27*tmax**4))**(1/3)
+ 24*3**(1/3)*c0**(4/3)*ly**(4/3)*(-9*tmax**2 + sqrt(3)*
sqrt(4096*c0**4*ly**4 + 27*tmax**4))**(1/3) + 9*tmax**2)/24
- sqrt(384*3**(2/3)*c0**(8/3)*ly**(8/3)/(-9*tmax**2 +
sqrt(3)*sqrt(4096*c0**4*ly**4 + 27*tmax**4))**(1/3)
- 24*3**(1/3)*c0**(4/3)*ly**(4/3)*(-9*tmax**2 + sqrt(3)*
sqrt(4096*c0**4*ly**4 + 27*tmax**4))**(1/3)
+ 18*sqrt(3)*tmax**3/sqrt(-128*3**(2/3)*c0**(8/3)*ly**(8/3)
/(-9*tmax**2 + sqrt(3)*sqrt(4096*c0**4*ly**4 + 27*tmax**4))
**(1/3) + 8*3**(1/3)*c0**(4/3)*ly**(4/3)*(-9*tmax**2
+ sqrt(3)*sqrt(4096*c0**4*ly**4 + 27*tmax**4))**(1/3)
+ 3*tmax**2) + 18*tmax**2)/24)/(c0*ly)
```

From the penalty parameter  $p = c_1 (\lambda^2 - \lambda^{-2})$  we can compute the amount by which the system violates incompressibility:

$$p = \kappa (J - 1) \quad \Rightarrow \quad J = 1 + \frac{p}{\kappa}$$

**Direct solution using pressure from equilibrium**



Analytical approach (p.224). Assuming Mooney-Rivlin with  $c_2 = 0$  (Neo-Hookean),

$$\begin{aligned}\mathbf{S} &= -p\mathbf{C}^{-1} + 2\left(\frac{\partial\Psi}{\partial I_1} + I_1\frac{\partial\Psi}{\partial I_2}\right)\mathbf{I} - 2\frac{\partial\Psi}{\partial I_2}\mathbf{C} \\ &= -p\mathbf{C}^{-1} + 2c_1\mathbf{I} = \begin{bmatrix} -p\lambda^{-2} + 2c_1 & 0 \\ 0 & -p\lambda^2 + 2c_1 \end{bmatrix}\end{aligned}$$

From equilibrium we get

**y-direction:**

$$S_{22} = 0 \Rightarrow -p\lambda^2 + 2c_1 = 0 \Rightarrow p = \frac{2c_1}{\lambda^2}$$

**x-direction:**

$$-\sigma_{11}l_y + t_{\max}\frac{\lambda}{L_y}l_y = 0, \Rightarrow \sigma_{11} = t_{\max}\frac{\lambda}{L_y}$$

$$\mathbf{S} = J\mathbf{F}^{-1}\boldsymbol{\sigma}\mathbf{F}^{-\top}, \quad S_{11} = \lambda^{-1}\sigma_{11}\lambda^{-1}$$

$$\Rightarrow S_{11} \stackrel{!}{=} \frac{t_{\max}}{L_y\lambda}$$

$$\Rightarrow -p\lambda^{-2} + 2c_1 = -2c_1\lambda^{-4} + 2c_1 = \frac{t_{\max}}{L_y\lambda}$$

$$\Rightarrow 2c_1\lambda^4 - \frac{t_{\max}}{L_y}\lambda^3 - 2c_1 = 0 \Rightarrow \lambda = \text{nonlinear}(t_{\max})$$

```
>>> solve(2.*c0*ly*(lambdaValue**4. - 1.) - tmax*lambdaValue**3.,
lambdaValue, simplify=True, positive=True)[3]
```

```
(0.125*tmax + 0.04166666666666667*sqrt(34.6139896873778*
c0**1.3333333333333333*ly**1.3333333333333333*
(-9.0*tmax**2 + 1.73205080756888*(4096.0*
c0**4*ly**4 + 27.0*tmax**4)**0.5)**0.3333333333333333
- 798.752188051931*c0**2.6666666666666667*ly**2.6666666666666667
*(-9.0*tmax**2 + 1.73205080756888*(4096.0*c0**4*ly**4 + 27.0*tmax**4)**0.5)**
```

Internal energy:

$$\delta W_{\text{int}} = \int_{\Omega} \frac{1}{2} S_{AB}(\hat{\mathbf{u}}) \delta u_a^L \left( F_{aB}(\hat{\mathbf{u}}) \frac{\partial \phi_L}{\partial X_A} + F_{aA}(\hat{\mathbf{u}}) \frac{\partial \phi_L}{\partial X_B} \right) dV$$

Now we assume a virtual displacement at node  $\mathcal{A}$ ,  $\delta u_a(\mathbf{X}) = \delta u_a^{\mathcal{A}} \phi^{\mathcal{A}}(\mathbf{X})$ .

$$\delta W_{\text{int}} = \int_{\Omega} \frac{1}{2} S_{AB} \delta u_a^{\mathcal{A}} \left( F_{aB} \frac{\partial \phi^{\mathcal{A}}}{\partial X_A} + F_{aA} \frac{\partial \phi^{\mathcal{A}}}{\partial X_B} \right) dV,$$

The ansatz function  $\phi^{\mathcal{A}}$  and their derivatives are given by:

$$\phi^{\mathcal{A}}(\mathbf{X}) = \frac{X_1}{L_x} \frac{X_2}{L_y}, \quad \frac{\partial \phi^{\mathcal{A}}}{\partial X_1} = \frac{1}{L_x} \frac{X_2}{L_y}, \quad \frac{\partial \phi^{\mathcal{A}}}{\partial X_2} = \frac{1}{L_y} \frac{X_1}{L_x}$$

For  $\delta \mathbf{u}(\mathbf{X}) = (\phi^A(\mathbf{X}), 0)^\top$ , i.e.  $\delta u_1^A = 1, \delta u_2^A = 0$  we compute the virtual internal energy:

$$\begin{aligned}\delta W_{\text{int}} &= \int_{\Omega} \frac{1}{2} S_{AB} \left( F_{1B} \frac{\partial \phi^A}{\partial X_A} + F_{1A} \frac{\partial \phi^A}{\partial X_B} \right) dV = \int_{\Omega} \frac{1}{2} S_{11} \left( F_{11} \frac{\partial \phi^A}{\partial X_1} + F_{11} \frac{\partial \phi^A}{\partial X_1} \right) dV \\ &= S_{11} \int_{\Omega} \lambda \frac{1}{L_x} \frac{X_2}{L_y} dV = S_{11} \lambda \frac{L_x}{L_x} \frac{1}{2} \frac{L_y^2}{L_y} \\ &= \frac{1}{2} L_y S_{11} \lambda = \frac{1}{2} L_y F_{11} S_{11}\end{aligned}$$

Virtual external energy

$$\begin{aligned}\delta W_{\text{ext}} &= \int_{\Omega} (\mathbf{B} - \rho_0 \ddot{\mathbf{u}}) \cdot \delta \mathbf{u} dV + \int_{\partial\Omega} \bar{\mathbf{T}} \cdot \delta \mathbf{u} dS \\ &= \int_{\partial\Omega} (T(\mathbf{X}), 0)^\top \cdot (\delta u_1(\mathbf{X}), \delta u_2(\mathbf{X}))^\top dS \\ &= \int_0^{L_y} \frac{t_{\max}}{L_y} \phi^A(L_x, X_2) dX_2 \\ &= \frac{t_{\max}}{L_y} \int_0^{L_y} \frac{L_x}{L_x} \frac{X_2}{L_y} dX_2 \\ &= \frac{t_{\max}}{L_y} \frac{1}{2} \frac{L_y^2}{L_y} = \frac{1}{2} t_{\max} = \frac{1}{2} L_y T(\mathbf{x})\end{aligned}$$

From principle of virtual work we should get the correct stress.

$$\begin{aligned}\delta W_{\text{int}} - \delta W_{\text{ext}} &= 0 \quad \Leftrightarrow \quad \frac{1}{4} L_y^2 S_{11} \lambda - \frac{1}{4} t_{\max} = 0 \\ \Leftrightarrow \quad \frac{1}{2} (L_y S_{11} \lambda - t_{\max}) &= 0 \\ \Leftrightarrow \quad L_y S_{11} \lambda &= t_{\max} \quad \Leftrightarrow \quad S_{11} = \frac{t_{\max}}{\lambda L_y} = \frac{1}{\lambda^2} t(\mathbf{X}) = \frac{1}{\lambda} T(\mathbf{X}) \quad \checkmark\end{aligned}$$

Note: we do not get any further information from the principle of virtual work further than the equilibrium conditions. This is because the stress was preserved as variable and not replaced by the constitutive equations, that relate them to strain and stretch  $\lambda$ .

## 7 Quadrature

The aim is to compute a good approximation for the integral

$$I := \int_0^1 f(\xi) \, d\xi$$

with a low number of function evaluations  $f(\xi)$ .

### 7.1 Gaussian quadrature

The Gaussian quadrature rule approximates the integral by

$$I \approx \sum_{i=1}^n f(\xi_i) w_i$$

with appropriate *Gauss points*  $\xi_i$  and weights  $w_i, i = 1, \dots, n$ . The sampling points and weights are chosen such that the rule approximates polynomials of degree  $p_{\text{exact}} = 2n - 1$  exactly. Some values are listed below.

$n$	$\xi_i$	$w_i$	$p_{\text{exact}}$
1	$\frac{1}{2}$	1	1
2	$\pm \frac{1 + \sqrt{3}}{2\sqrt{3}}$	$\frac{1}{2}$	3
3	$\frac{1}{2}$	$\frac{4}{9}$	5
	$\pm \frac{\sqrt{3} + \sqrt{5}}{2\sqrt{5}}$	$\frac{5}{18}$	

Tabelle 3: Gauss points and weights

Note: Literature on Gauss quadrature often describes the case of an integral  $\int_{-1}^1 f(x) \, dx$ . The transformation reads:

$$\int_0^1 f(\xi) \, d\xi = \frac{1}{2} \int_{-1}^1 f(1/2 + x/2) \, dx.$$

### 7.2 Clenshaw-Curtis quadrature

Clenshaw-Curtis quadrature approximates the function by a number of Chebyshev polynomials for which the exact integral is known. The function  $f$  is evaluated at the  $n + 1$  roots of the Chebyshev polynomial  $T_{n+1}$ , which are:

$$\xi_i = \cos(i\pi/n), \quad i = 0, \dots, n, \quad n \text{ even.}$$

The function can be written as Chebyshev series

$$f(\xi) = \frac{a_0}{2} T_0(\xi) + \sum_{i=1}^{\infty} a_i T_i(x)$$

where the coefficients result from discrete cosine transform. The approximated integral is computed by

$$I \approx a_0 + \sum_{i=1}^{n/2-1} \frac{2 a_{2i}}{1 - (2i)^2} + \frac{a_n}{1 - n^2}.$$

The coefficients are

$$a_{2i} = \frac{2}{n} \left( \frac{f(1) + f(-1)}{2} + f(0) (-1)^i + \sum_{i=1}^{n/2-1} (f(\xi_i) + f(-\xi_i)) \cos(k/2 \xi_i) \right)$$

This quadrature rule approximates polynomials with degree  $p_{\text{exact}} = m - 1$  exactly when using  $m$  sampling points. However for some non-polynomial functions its accuracy may be better than the respective Gauss quadrature.

## 8 Propositions

In this section some propositions are collected such that they can be referenced when needed.

### 8.1 Divergence theorem

*Also called Gauss's theorem.* Let  $U \subset \mathbb{R}^d$  be a compact set with a piecewise smooth boundary  $\partial U$ ,  $\mathbf{F} : U \rightarrow \mathbb{R}^d$  a continuously differentiable vector field. Then:

$$\int_U \nabla \cdot \mathbf{F}(\mathbf{x}) \, d\mathbf{x} = \int_{\partial U} \mathbf{F}(\mathbf{x}) \cdot \mathbf{n} \, d\mathbf{x}. \quad (38)$$

For  $d = 2$  one gets *Stoke's theorem*.

#### 8.1.1 Corollary

Replacing  $\mathbf{F}$  of (38) by  $f \mathbf{F}$  yields the following proposition:

For a differentiable function  $f : U \rightarrow \mathbb{R}$  and a vector field  $\mathbf{F} : U \rightarrow \mathbb{R}^d$  it holds:

$$\int_U f(\nabla \cdot \mathbf{F}) \, d\mathbf{x} = \int_{\partial U} (f \mathbf{F}) \cdot \mathbf{n} \, d\mathbf{x} - \int_U \mathbf{F} \cdot \nabla f \, d\mathbf{x} \quad (39)$$

Now set  $\mathbf{F} \equiv (1, 0, \dots), (0, 1, \dots), \dots$  to get the following vector-valued identity:

For a differentiable function  $f : U \rightarrow \mathbb{R}$  it holds:

$$\int_U \nabla f(\mathbf{x}) \, d\mathbf{x} = \int_{\partial U} f(\mathbf{x}) \mathbf{n} \, d\mathbf{x} \quad (40)$$

## 8.2 Classical Stoke's theorem

Let  $U \subset \mathbb{R}^3$  be an open set,  $V$  a 2-manifold in  $U$  with boundary  $\partial V$  and  $\mathbf{F} : U \rightarrow \mathbb{R}^3$  a continuously differentiable vector field. Then:

$$\oint_{\partial V} \mathbf{F} \cdot d\mathbf{s} = \int_V (\nabla \times \mathbf{F}) \cdot \mathbf{n} d\mathbf{x}, \quad (41)$$

where  $\mathbf{n}$  is the normal on the surface  $V$ .

## 8.3 Integration on manifolds

In the following it is outlined how to integrate on 1D and 2D domains that are embedded in  $\mathbb{R}^d$ . The formalism of manifolds is omitted for simplicity.

### 8.3.1 1D curve integrals

Let  $U \subset \mathbb{R}$  be an open set (the parameter space) and  $\Phi : U \rightarrow \mathbb{R}^d$  a smooth mapping that defines a curve  $\Omega = \Phi(U)$  embedded in  $\mathbb{R}^d$ . An integrable function  $g : \Omega \rightarrow \mathbb{R}$  can then be integrated as follows:

$$\int_{\Phi(U)} g(\mathbf{x}) d\mathbf{x} = \int_U g(\Phi(\xi)) \|\Phi'(\xi)\|_2 d\xi \quad (42)$$

### 8.3.2 2D surface integrals

Let  $U \subset \mathbb{R}^2$  be an open set (parameter space),  $\Phi : U \rightarrow \Phi(U) =: \Omega \subset \mathbb{R}^3$  a diffeomorphism,  $\Phi$  maps parameters  $\boldsymbol{\xi} = (\xi_1, \xi_2) \in U$  to points in world space  $\mathbf{x} \in \Omega$ . The inverse map  $\Phi^{-1} : \Omega \subset \mathbb{R}^3 \rightarrow \mathbb{R}^2$  assigns coordinates  $(\xi_1, \xi_2)$  to each point  $\mathbf{x} \in \Omega$ . We name  $\Phi^{-1}(\mathbf{x}) = (x(\mathbf{x}), y(\mathbf{x}))$  in the following formula. The integration of a 2-dimensional function  $g : \Omega \rightarrow \mathbb{R}$  is performed as follows.

$$\begin{aligned} \int_{\Phi(U)} g(\mathbf{x}) d\mathbf{x} &= \int_U g(\Phi(\boldsymbol{\xi})) \sqrt{\det(J_\Phi(\boldsymbol{\xi})^\top J_\Phi(\boldsymbol{\xi}))} d\boldsymbol{\xi} \\ &= \int_U g(\Phi(\boldsymbol{\xi})) \sqrt{\det \begin{pmatrix} \frac{d\Phi}{d\xi_1} \cdot \frac{d\Phi}{d\xi_1} & \frac{d\Phi}{d\xi_1} \cdot \frac{d\Phi}{d\xi_2} \\ \frac{d\Phi}{d\xi_2} \cdot \frac{d\Phi}{d\xi_1} & \frac{d\Phi}{d\xi_2} \cdot \frac{d\Phi}{d\xi_2} \end{pmatrix}} d\boldsymbol{\xi} \\ &= \int_U g(\Phi(\boldsymbol{\xi})) \sqrt{\left\| \frac{d\Phi}{d\xi_1} \right\|_2^2 \left\| \frac{d\Phi}{d\xi_2} \right\|_2^2 - \left( \frac{d\Phi}{d\xi_1} \cdot \frac{d\Phi}{d\xi_2} \right)^2} d\boldsymbol{\xi} \end{aligned} \quad (43)$$

### 8.3.3 Substitution on domains with same dimensionality

*Integration by substitution, German “Transformationssatz”, also change of variables rule.* Let  $U \subset \mathbb{R}^d$  be an open set,  $\Phi : U \rightarrow \Phi(U) \subset \mathbb{R}^d$  a diffeomorphism ( $\Phi$  bijective and continuously differentiable, inverse map  $\Phi^{-1}$  also continuously differentiable).

Then  $g : \Phi(U) \rightarrow \mathbb{R}$  is integrable on  $\Phi(U)$  if and only if the function  $\boldsymbol{\xi} \mapsto g(\Phi(\boldsymbol{\xi})) |\det(J_\Phi(\boldsymbol{\xi}))|$  is integrable on  $U$ . It holds:

$$\int_{\Phi(U)} g(\mathbf{x}) \, d\mathbf{x} = \int_U g(\Phi(\boldsymbol{\xi})) |\det(J_\Phi(\boldsymbol{\xi}))| \, d\boldsymbol{\xi}, \quad (44)$$

where  $J_\Phi$  is the Jacobian of  $\Phi$ .

### 8.3.4 Summary

The transformation rules eqs. (42) to (44) can be summarized in a unified form as follows.

Let  $U \subset \mathbb{R}^d, d \in \{1, 2, 3\}$  be an open set (parameter space),  $\Phi : U \rightarrow \Phi(U) =: \Omega \subset \mathbb{R}^d$  a diffeomorphism that maps parameters  $\boldsymbol{\xi} \in U$  to points in world space  $\mathbf{x} \in \Omega$ . A function defined in parameter space,  $f : U \rightarrow \mathbb{R}$ , can then be integrated as follows in world space.

$$\int_{\Phi(U)} f(\Phi^{-1}(\mathbf{x})) \, d\mathbf{x} = \int_U f(\boldsymbol{\xi}) \mathcal{J}_d(\boldsymbol{\xi}) \, d\boldsymbol{\xi}, \quad (45)$$

where the definition of  $\mathcal{J}_d(\boldsymbol{\xi})$  depends on the dimension  $d$  as follows:

$$\begin{aligned} \mathcal{J}_1(\boldsymbol{\xi}) &= \|\Phi'(\boldsymbol{\xi})\|_2 && \text{for } d = 1, \boldsymbol{\xi} = \xi \in U \subset \mathbb{R}, \\ \mathcal{J}_2(\boldsymbol{\xi}) &= \sqrt{\det(J_\Phi(\boldsymbol{\xi})^\top J_\Phi(\boldsymbol{\xi}))} && \text{for } d = 2, \boldsymbol{\xi} \in U \subset \mathbb{R}^2, \phi^{-1}(\mathbf{x}) =: (x(\mathbf{x}), y(\mathbf{x})), \\ \mathcal{J}_3(\boldsymbol{\xi}) &= |\det(J_\Phi(\boldsymbol{\xi}))| && \text{for } d = 3, \boldsymbol{\xi} \in U \subset \mathbb{R}^3. \end{aligned}$$

## Literatur

- [1] K.-J. Bathe. Finite element procedures. Klaus-Jurgen Bathe, 2006.
- [2] A. G. Holzapfel. Nonlinear solid mechanics. 2000.
- [3] T. Sussman and K.-J. Bathe. A finite element formulation for nonlinear incompressible elastic and inelastic analysis. Computers & Structures, 26(1):357 – 409, 1987.
- [4] O. C. Zienkiewicz, R. L. Taylor, O. C. Zienkiewicz, and R. L. Taylor. The finite element method, volume 3. McGraw-hill London, 1977.