

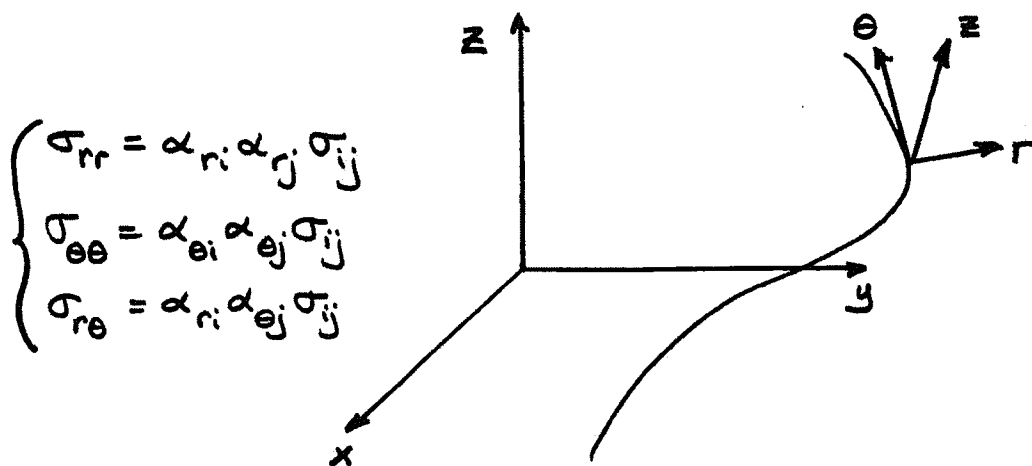
Plane Elasticity in Cylindrical Coordinates

Airy's Stress Function:

$$\sigma_{rr} = \frac{1}{r} \frac{\partial \phi}{\partial r} + \frac{1}{r^2} \frac{\partial^2 \phi}{\partial \theta^2} + \gamma$$

$$\sigma_{\theta\theta} = \frac{\partial^2 \phi}{\partial r^2} + \gamma$$

$$\sigma_{r\theta} = \frac{1}{r^2} \frac{\partial \phi}{\partial \theta} - \frac{1}{r} \frac{\partial^2 \phi}{\partial r \partial \theta}$$



$$\begin{cases} \sigma_{rr} = \alpha_{ri} \alpha_{rj} \sigma_{ij} \\ \sigma_{\theta\theta} = \alpha_{\theta i} \alpha_{\theta j} \sigma_{ij} \\ \sigma_{r\theta} = \alpha_{ri} \alpha_{\theta j} \sigma_{ij} \end{cases}$$

Laplacian Operator:

$$\nabla^2 () = \frac{\partial^2}{\partial r^2} + \frac{1}{r} \frac{\partial}{\partial r} + \frac{1}{r^2} \frac{\partial^2}{\partial \theta^2}$$

The biharmonic equation (no body forces) is:

$$\nabla^4 \phi = \left[\frac{\partial^2}{\partial r^2} + \frac{1}{r} \frac{\partial}{\partial r} + \frac{1}{r^2} \frac{\partial^2}{\partial \theta^2} \right] \left[\frac{\partial^2 \phi}{\partial r^2} + \frac{1}{r} \frac{\partial \phi}{\partial r} + \frac{1}{r^2} \frac{\partial^2 \phi}{\partial \theta^2} \right] = 0$$

Now we will look at a special class of problems termed "axisymmetric."

In cylindrical coordinates we often encounter problems that depend only on r and not on θ . In such cases the previous equations can be simplified.

Axisymmetric Problems

$$\begin{cases} \sigma_{rr} = \frac{1}{r} \frac{d\phi}{dr} + \nu \\ \sigma_{\theta\theta} = \frac{d^2 \phi}{dr^2} + \nu \\ \sigma_{r\theta} = 0 \end{cases}$$

and the Laplacian operator becomes,

$$\nabla^2(\) = \frac{d^2}{dr^2} + \frac{1}{r} \frac{d}{dr}$$

thus, the biharmonic equation is,

$$\begin{aligned} \nabla^4 \phi &= \left[\frac{d^2}{dr^2} + \frac{1}{r} \frac{d}{dr} \right] \left[\frac{d^2 \phi}{dr^2} + \frac{1}{r} \frac{d\phi}{dr} \right] = 0 \\ &= \frac{d}{dr} \left[r \frac{d}{dr} \left(\frac{1}{r} \frac{d}{dr} \left(r \frac{d\phi}{dr} \right) \right) \right] = 0 \end{aligned}$$

This leaves the equation in a form that is readily integratable to get ϕ .

$$\int \frac{d}{dr} \left[r \frac{d}{dr} \left\{ \frac{1}{r} \frac{d}{dr} \left(r \frac{d\phi}{dr} \right) \right\} \right] = \int 0$$

$$\rightarrow r \frac{d}{dr} \left\{ \frac{1}{r} \frac{d}{dr} \left(r \frac{d\phi}{dr} \right) \right\} = C_1$$

integrate again

$$\frac{1}{r} \frac{d}{dr} \left(r \frac{d\phi}{dr} \right) = C_1 \ln r + C_2$$

$$\therefore \frac{d}{dr} \left(r \frac{d\phi}{dr} \right) = c_1 r \ln r + c_2 r$$

(integrate)

$$r \frac{d\phi}{dr} = c_1 \frac{r^2}{2} (\ln r - 1) + c_2 \frac{r^2}{2} + c_3$$

$$\therefore \frac{d\phi}{dr} = c_1 \frac{r}{2} (\ln r - 1) + c_2 \frac{r}{2} + \frac{c_3}{r}$$

$$= \frac{c_1}{2} r \ln r - \frac{c_1}{2} r + \frac{c_2}{2} r + \frac{c_3}{r}$$

(integrate)

$$\phi(r) = \frac{c_1}{2} \left\{ \frac{r^2}{2} (\ln r - 1) \right\} - \frac{c_1 r^2}{4} + \frac{c_2 r^2}{4} + c_3 \ln r + c_4$$

$$= \frac{c_1}{4} r^2 \ln r - \frac{c_1 r^2}{4} - \frac{c_1 r^2}{4} + \frac{c_2 r^2}{4} + c_3 \ln r + c_4$$

$$\therefore \phi(r) = \bar{c}_1 r^2 \ln r + \bar{c}_2 r^2 + c_3 \ln r + c_4$$

where, $\bar{c}_1 = \frac{c_1}{4}$

$$\bar{c}_2 = \frac{c_2}{4} - \frac{c_1}{2}$$

dropping the overbars we have,

$$\boxed{\phi(r) = c_1 r^2 \ln r + c_2 r^2 + c_3 \ln r + c_4}$$

The stress can be obtained as,

$$\left\{ \begin{array}{l} \sigma_{rr} = \frac{1}{r} \frac{d^2 \phi}{dr^2} \\ \quad = c_1 (1 + 2 \ln r) + 2c_2 + \frac{c_3}{r^2} \\ \sigma_{\theta\theta} = \frac{d^2 \phi}{dr^2} = \\ \quad = c_1 (3 + 2 \ln r) + 2c_2 - \frac{c_3}{r^2} \\ \sigma_{r\theta} = 0 \end{array} \right.$$

The strain-stress relations are,

$$\left\{ \begin{array}{l} \epsilon_{rr} = \frac{1}{E} [\sigma_{rr} - \nu \sigma_{\theta\theta}] \\ \epsilon_{\theta\theta} = \frac{1}{E} [\sigma_{\theta\theta} - \nu \sigma_{rr}] \\ \epsilon_{zz} = \frac{-\nu}{E} [\sigma_{rr} + \sigma_{\theta\theta}] \\ \epsilon_{r\theta} = \frac{1}{2G} \sigma_{r\theta} \\ \epsilon_{rz} = \epsilon_{\theta z} = 0 \end{array} \right.$$

plane stress

$$\text{plane strain} \left\{ \begin{array}{l} \epsilon_{rr} = \frac{1}{E} [\sigma_{rr} - \nu(\sigma_{\theta\theta} + \sigma_{zz})] \\ \epsilon_{\theta\theta} = \frac{1}{E} [\sigma_{\theta\theta} - \nu(\sigma_{rr} + \sigma_{zz})] \\ \epsilon_{zz} = 0 = \frac{1}{E} [\sigma_{zz} - \nu(\sigma_{rr} + \sigma_{\theta\theta})] \\ \epsilon_{r\theta} = \frac{1}{2G} \sigma_{r\theta} \\ \epsilon_{rz} = \epsilon_{\theta z} = 0 \end{array} \right.$$

The strain-displacement relations in cylindrical coordinates are now,

$$\left\{ \begin{array}{l} \epsilon_{rr} = \frac{\partial u_r}{\partial r} \\ \epsilon_{\theta\theta} = \frac{u_r}{r} + \frac{1}{r} \frac{\partial u_\theta}{\partial \theta} \\ \epsilon_{r\theta} = 0 \end{array} \right.$$

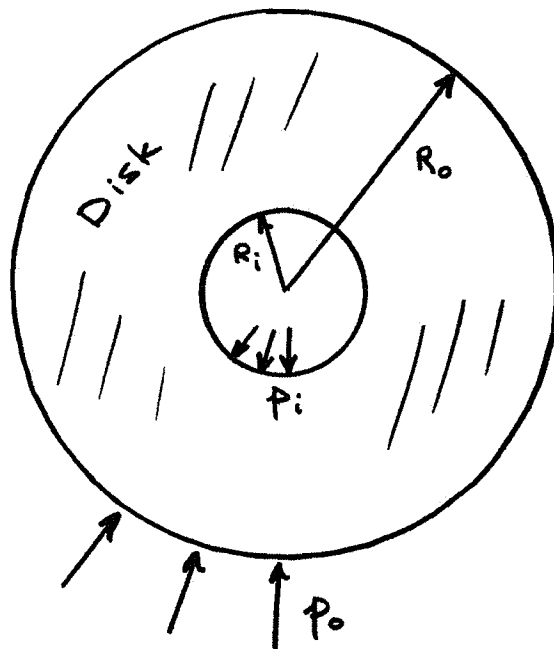
Integrating these equations gives the following displacements,

$$u_r = \frac{1}{E} \left\{ C_1 r [(1-\nu)(2 \ln r - 1) - 2\nu] + 2C_2(1-\nu)r - C_3 \left(\frac{1+\nu}{r} \right) + C_4 \sin \theta + C_5 \cos \theta \right\}$$

$$u_\theta = \frac{1}{E} \left\{ 4C_1 r \theta + C_4 \cos \theta - C_5 \sin \theta + C_6 r \right\}$$

Now for an example:

Thick-Walled Cylinder or Disk



boundary conditions:

$$\sigma_{rr}(R_o) = -p_o$$

$$\sigma_{rr}(R_i) = -p_i$$

From the first boundary condition,

$$-p_o = C_1 (1 + 2 \ln R_o) + 2C_2 + \frac{C_3}{R_o^2} \quad (1)$$

and from the second,

$$-p_i = C_1 (1 + 2 \ln R_i) + 2C_2 + \frac{C_3}{R_i^2} \quad (2)$$

However, we need another equation as

there are 3 unknown constants. This

comes from knowledge of the displacement

u_θ .

Obviously, u_θ is periodic. That is,

$$u_\theta(0) = u_\theta(2\pi) = u_\theta(4\pi) \dots \text{etc} \dots \quad (3)$$

Therefore, examining u_θ :

$$u_\theta = \frac{1}{E} \left\{ 4C_1 r \theta + C_4 \cos \theta - C_5 \sin \theta + C_6 r \right\} \quad (4)$$

we see that for u_θ to be single-valued

we must have,

$$C_1 \equiv 0 \quad (5)$$

Thus, we have 2 equations and 2 unknowns,

$$\begin{bmatrix} 2 & 1/R_i^2 \\ 2 & 1/R_o^2 \end{bmatrix} \begin{Bmatrix} C_2 \\ C_3 \end{Bmatrix} = - \begin{Bmatrix} P_i \\ P_o \end{Bmatrix} \quad (6)$$

Therefore,

$$C_2 = \frac{P_i R_i^2 - P_o R_o^2}{2(R_o^2 - R_i^2)} \quad (7)$$

$$C_3 = \frac{R_i^2 R_o^2 (P_o - P_i)}{R_o^2 - R_i^2} \quad (8)$$

and the resulting stresses are,

$$\sigma_{rr} = \frac{P_i R_i^2 - P_o R_o^2}{R_o^2 - R_i^2} + \frac{R_i^2 R_o^2 (P_o - P_i)}{r^2 (R_o^2 - R_i^2)} \quad (9)$$

$$\sigma_{\theta\theta} = \frac{p_i R_i^2 - p_o R_o^2}{R_o^2 - R_i^2} - \frac{R_i^2 R_o^2 (p_o - p_i)}{r^2 (R_o^2 - R_i^2)} \quad (10)$$

Normalizing by R_o^2 :

$$\sigma_{rr} = \frac{p_o \left[\left(\frac{R_i}{r} \right)^2 - 1 \right] + p_i \left[\left(\frac{R_i}{R_o} \right)^2 - \left(\frac{R_i}{r} \right)^2 \right]}{1 - \left(\frac{R_i}{R_o} \right)^2} \quad (11)$$

$$\sigma_{\theta\theta} = \frac{-p_o \left[1 + \left(\frac{R_i}{r} \right)^2 \right] + p_i \left[\left(\frac{R_i}{R_o} \right)^2 + \left(\frac{R_i}{r} \right)^2 \right]}{1 - \left(\frac{R_i}{R_o} \right)^2} \quad (12)$$

For the plane strain case we also have,

$$\begin{aligned} \sigma_{zz} &= 2(\sigma_{rr} + \sigma_{\theta\theta}) \\ &= \frac{2\nu \left[p_i \left(\frac{R_i}{R_o} \right)^2 - p_o \right]}{1 - \left(\frac{R_i}{R_o} \right)^2} \end{aligned} \quad (13)$$

or for plane stress,

$$\begin{aligned}\epsilon_{zz} &= \frac{-\nu}{E} (\sigma_{rr} + \sigma_{\theta\theta}) \\ &= \frac{\frac{2\nu}{E} \left\{ p_o - p_i \left(\frac{R_i}{R_o} \right)^2 \right\}}{1 - \left(\frac{R_i}{R_o} \right)^2} \quad (14)\end{aligned}$$

Now let's look at a more complicated case.

→ Thick-Walled cylinder with closed ends ←

Since we are dealing with a linear elastic theory we can superimpose solutions to other problems in order to find the solution to this problem.

We already have the solution to

the problem of an open-ended cylinder.

Closing the ends of the cylinder will generate a uniformly distributed stress σ_{zz} which maintains overall equilibrium.

$$\text{Solution} = \left\{ \begin{array}{c} \text{thick-walled} \\ \text{plane} \\ \text{strain} \\ \text{solution} \end{array} \right\} + \left\{ \begin{array}{c} \sigma_{zz} \text{ uniformly} \\ \text{distributed} \end{array} \right\}$$

$$\begin{aligned} F_z^* &= \text{force in the } z\text{-direction} \\ &= p_o \pi R_o^2 - p_i \pi R_i^2 \end{aligned}$$

$$\therefore \sigma_{zz}^* = \frac{F_z}{\pi (R_o^2 - R_i^2)} = \frac{p_i \left(\frac{R_i}{R_o} \right)^2 - p_o}{1 - \left(\frac{R_i}{R_o} \right)^2} \quad (15)$$

Thus, the total solution will be

$$\sigma_{zz} = \text{equation (13)} + \sigma_{zz}^* (\text{equ. 15})$$

by superposition