

# **BUILDING SUPPORT STRUCTURES**

## **a visual study with computers**

**COMPUTER-BASED ANALYSIS  
AND DESIGN  
of BUILDING STRUCTURES**

**@Seismicisolation**

**This presentation addresses the use of structure software in architectural education besides being concerned with the**

- **the position of structures in architecture**
- **a clarification of structural engineering and architecture which often is confusing to the students**

# GENERAL CONCEPTS OF STRUCTURE

The necessity of Structure: ORDER, structure is a necessary part of life

The purpose of building structure:

support structure

ordering system

form giver

art

Building vs. Structure vs. Architecture:

structure is necessary for buildings but not for architecture:

without structure there is no building, but architecture as an idea does not require structure

Expression of structure:

hidden vs. exposed

innovative vs. standard construction

Building structure type and use

Global vs. local scale of structure: DETAIL

**STRUCTURE** as *support (local and global scale)*: structure holds the building up so it does not collapse or deform excessively; it makes the building and spaces within the building possible. Structure gives support to the material and therefore is necessary. It is directly related to the structural engineer who is responsible for **safety**, to him the building is a body that is alive, its bones and muscles are activated by external and internal forces. As it reacts, it deforms and suggests the pain it must endure at points of stress concentration. In other words engineers visualize buildings in an animated state moving back and forth as can be convincingly expressed by computers through **virtual modeling**. It is this world of engineering which fascinated the early modernists in architecture.

**STRUCTURE** as *form giver*: it defines the spatial configuration and reflects other meanings and is part of esthetics

**STRUCTURE** as *ordering system*, it functions as a spatial and dimensional organizer besides identifying assembly or construction systems: **dimensional coordination**

Most students have completed at least two prior courses in structures: *introduction to engineering analysis*, and *introduction to structural design* including concrete, steel and wood. Because of this background only static loading is considered and small displacement of elements so that the magnitude of the member forces only depends on relative stiffness values, in other words the original geometry is used and not the deformed geometry. Even if the **linear analysis** does not simulate the actual behavior, it does give for ordinary conditions member forces good enough for making design decisions, at least for preliminary design purposes, besides helping to develop an understanding for the behavior of the building structure. *Indeterminate structures often are made determinate so that member forces are independent of stiffness that is independent of certain modeling assumptions, and manual calculations can be performed to check the computer output approximately. The actual displacement, which depends on the absolute stiffness, is usually only checked visually.*

Since the free academic version of SAP2000 is limited to **100 nodes**, generally only planar structures are investigated by the students. The computer program is introduced by proceeding from simple to more complex structures.

**A. General Introduction**

**B. Case Study**

The goal of the use of computers in structure course is to develop an understanding for  
the building structure as a system that **supports** and as a pattern that **orders** space and makes space possible.

The treatment of structures is broadened and enriched by integrating the traditionally separate fields of:

- construction
- structural analysis
- structural design
- structural systems
- materials
- geometrical modeling
- visual communication

The students have to **synthesize the knowledge** acquired in various courses and have to set up a **mathematical model** of the building support structure, rather than solving a given isolated analysis or design problem, as is usually done in education; they have to deal with the physical reality of the entire building rather than only an isolated part.

The primary structural engineering software used in this context is **SAP2000**, which is widely used in practice and in numerous universities internationally. The program has been developed by **COMPUTERS AND STRUCTURES, INC** ([www.csiberkeley.com/SAP2000.htm](http://www.csiberkeley.com/SAP2000.htm)), Berkeley, CA.

Refer also to:

Beijing Civil King Software Technology Co., Ltd., Beijing, Chushu LI, PHD, S.E., Chief Executive, Tel:86-10-8838 3866-101, Mobile:13601318851, Fax:86-10-88381056, Email: [csli@chinabuilding.com.cn](mailto:csli@chinabuilding.com.cn), Web: [www.bjcks.com](http://www.bjcks.com)

**Qualified universities are eligible for free software for Education and Research.**

Since the free academic version of SAP2000 is limited to **100 nodes**, generally only planar structures are investigated by the students. The computer program is introduced by proceeding from simple to more complex structures.

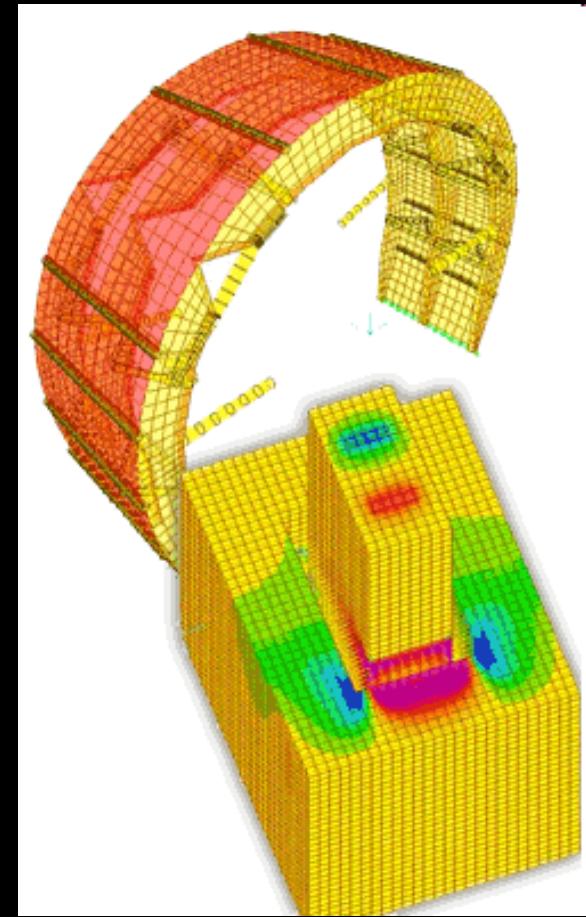
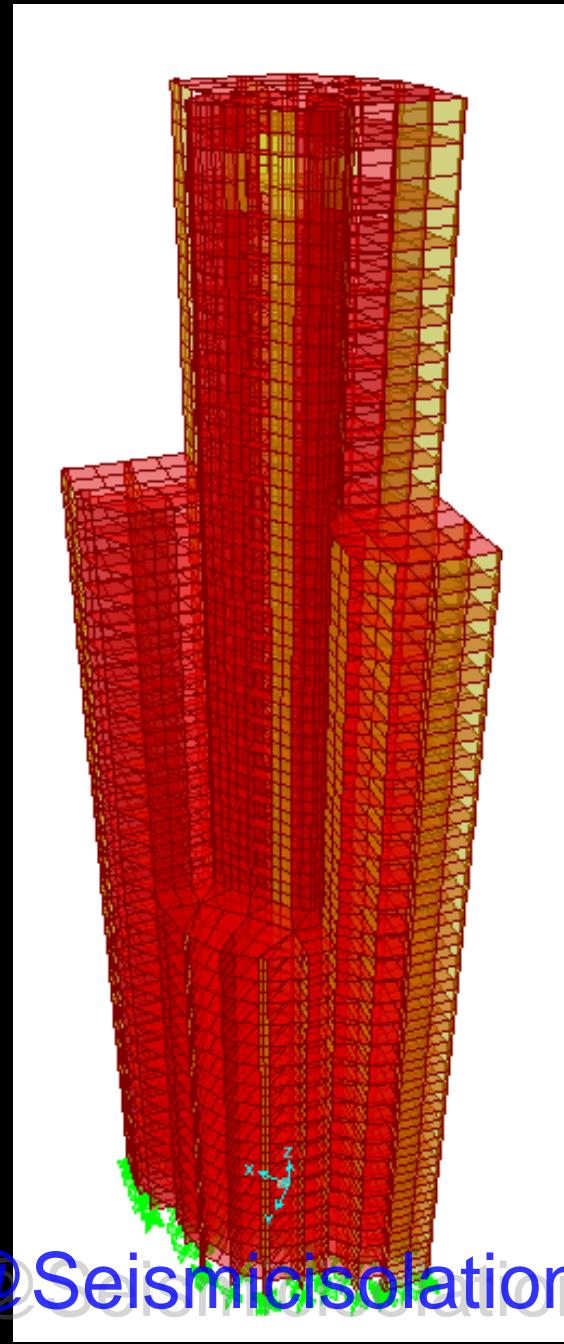
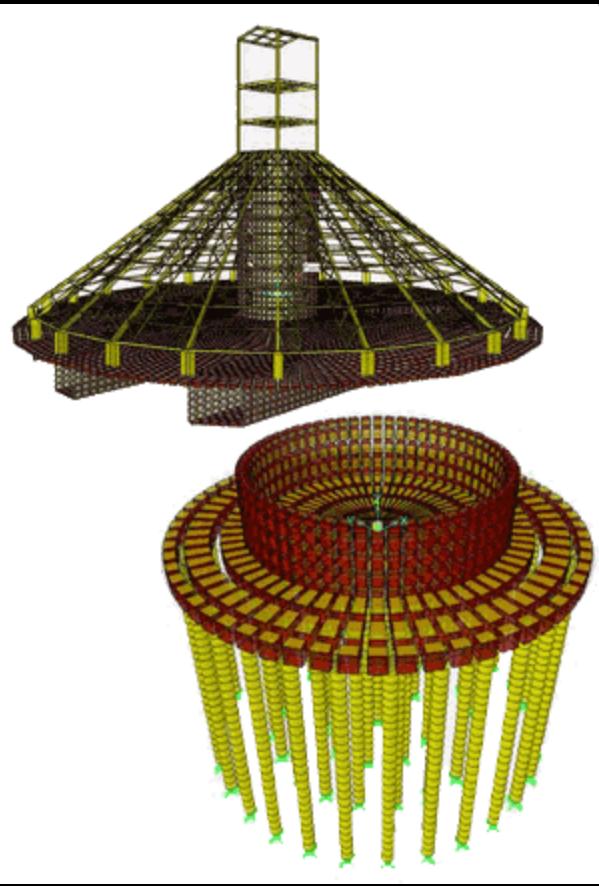
The program is fully integrated within Microsoft Windows and allows modeling of nearly all types of structures. The Windows based easy-to-use graphical interface permits the quick modeling of structures with templates and then to edit them via the graphical interface.

# **SAP2000**

**V11 – Non Linear**

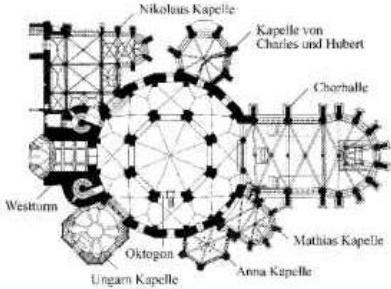
**A Comprehensive Software for the  
Finite Element Modeling, Static,  
Dynamic and Non-Linear Analysis and  
Design of Structures**

**@Seismicisolation**



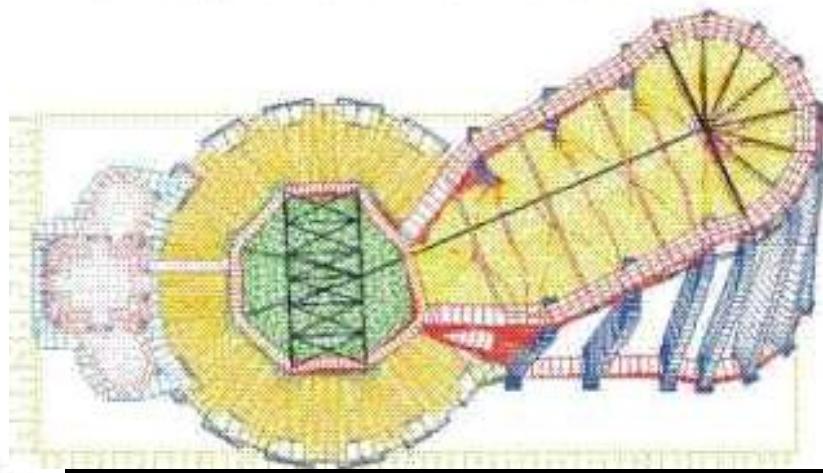
@Seismicisolation

## Finite-element model



@Seismicisolation

## 1. Eigenmode: $f = 1,13 \text{ Hz}$



Seismic Vulnerability Assessment  
of the Historic Aachen *Cathedral*,  
Germany

## 2. Eigenmode: $f = 2,13 \text{ Hz}$



To develop an understanding for the behavior of the **actual building** under load action, so it can be designed, engineers have created a **mathematical model** that is an abstract mathematical construction of an **idealized structure** that simulates the real building structure.

A typical mathematical model is defined by the principal components, that decomposes the building structure into its major parts or

**BUILDING BLOCKS** of:

***elements*** (type, shape, material, other characteristics) and ***nodes***  
***boundary conditions:***

***support joints***

***active member joints***

***material***

***action: external forces*** (e.g. nodal loads and displacements,  
member loads, temperature changes)

In other words, the structure consists of an assembly of members and joints at their ends or their corners, as well as the external supports of the global or overall structure.

## INTRODUCTION

Most computer-based analyses of structures are based on **finite element methods**, where numerical methods approximate the equations for a continuous system. Here, the name *finite* suggests the distinction from *differential* elements in calculus. The finite element analysis was developed during the 1950s in the aerospace industry and advanced rapidly further during the 1960s with the growth of computer power as engineers turned to numerical methods using **matrix mathematics** rather than **differential calculus**. In other words, engineers began to replace classical techniques to solve unknowns by the **flexibility method** (i.e. **force method**) from static equilibrium of forces, or by the **stiffness method** (i.e. **displacement method**) from compatibility of displacements, with matrix analysis.

As the name suggests, in the finite element method **the continuous structure is decomposed into a number of one-, two-, or three-dimensional elements with artificial joints and subsequently reassembled**. The finite elements can represent a piece or part of a structure. For example, in a **truss** each member represents a natural element of the finite element model. Also, in a **frame** each shallow **beam** or **column** can be represented simply as bar-type elements, but they can be further subdivided by a number of elements for stress analysis that is to study in more detail the stress flow. Many structures such as **skeletons** already represent a clearly identified assemblage of discrete members and therefore can be idealized with **one-dimensional** or **bar-type elements** (e.g. beams, columns, arches, struts, and ties).

On the other hand, for **surface structures** and complex three-dimensional structures, **the continuum must be divided into a temporary mesh or gridwork of finite pieces of polygonal elements** which can have various shapes. For instance, typical slabs, deep beams, walls, and shells can be divided into rectangular grids of planar elements. Examples of finite element models in the following figure are Joan Miro's 30-ft high outdoor sculpture in Chicago designed by SOM, or the shell dome according to Muecke Software GmbH.

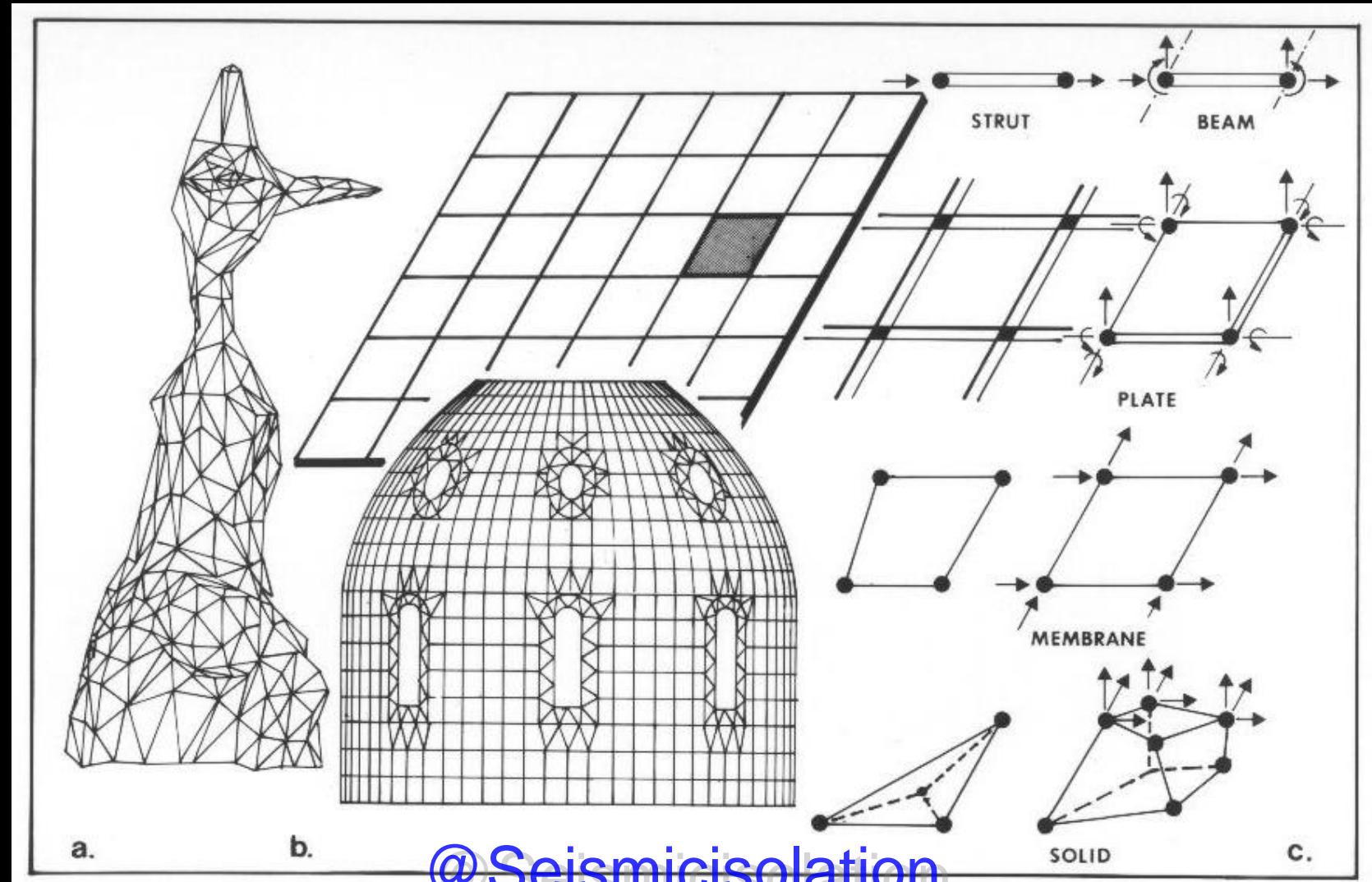
The computer program uses the ***finite element*** analysis as the mathematical model, however the students do not have to be familiar with this numerical modeling method. It surely is helpful to **visualize that the model is divided into a large number of finite elements**. The elements are considered **connected to nodes** typically located at the ends or corners of members.

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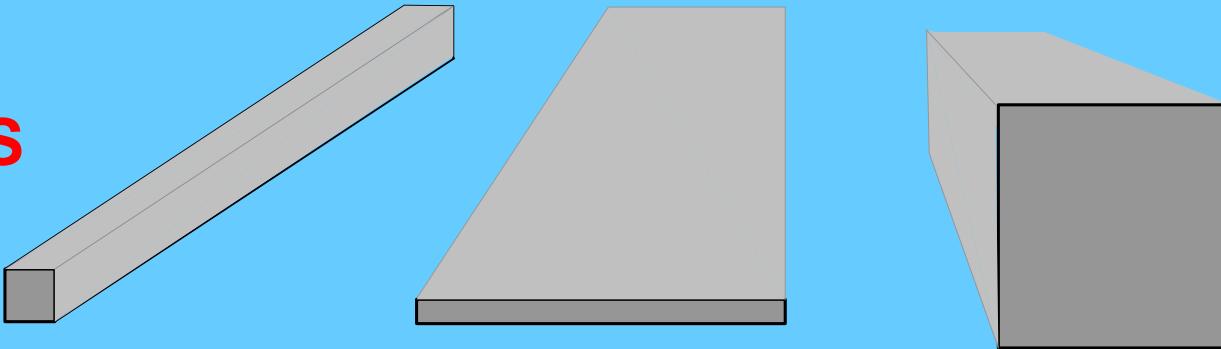
It is not necessary for the students to be able to set up the equations although assumptions and limitations of the method should be discussed. But it must be emphasized that finite element computer programs do not only represent a powerful method of engineering analysis, they also represent a **tool for learning**. The student **must understand the physical reality of the building structure in every detail to set up the mathematical model he puts into the computer**. He develops a feeling and control over the support structure by zooming from the **global scale** of the overall building behavior to the **local scale** of stress and detail.

# FINITE ELEMENTS



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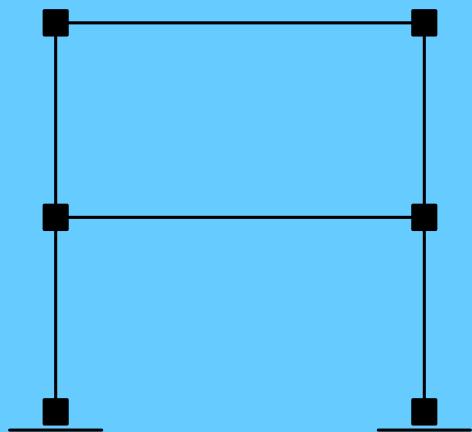
# Member ELEMENTS



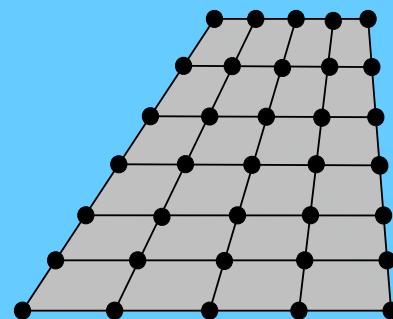
1D LINE MEMBER

2D PLANAR MEMBER

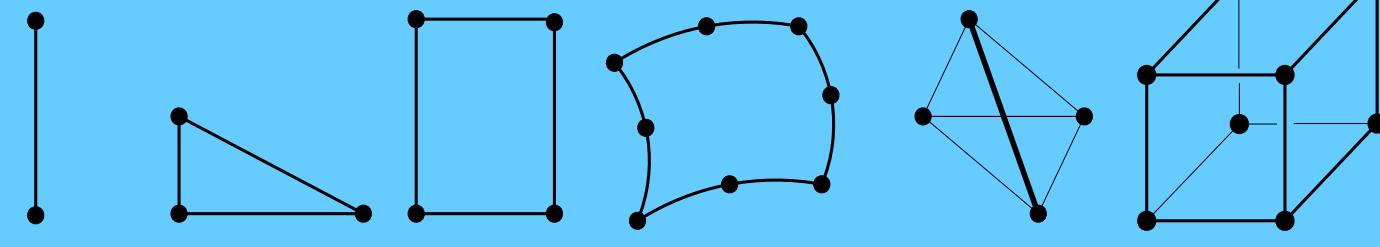
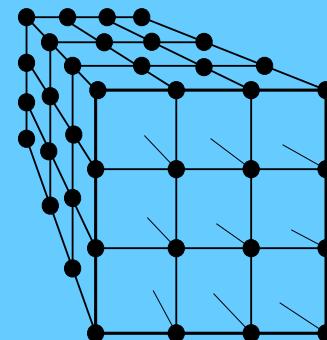
3D SOLID MEMBER



DISCRETE MODEL



CONTINUOUS MODELS



LINE ELEMENT

TYPICAL PLANAR ELEMENTS

TYPICAL SOLID ELEMENTS

# STRUCTURAL ELEMENTS

## FRAME elements

- beam elements
- axial elements
- beam-column elements

## SURFACE elements (slabs, plates, shells)

## SLID OBJECTS

# BEAM ELEMENTS

Defn: Members subject to bending and shear

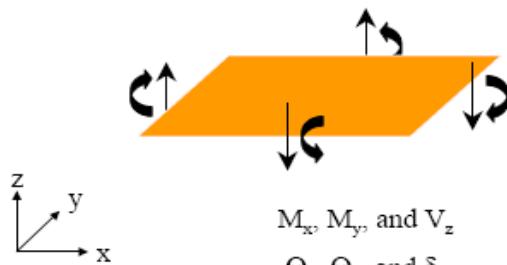


### Elastic Properties:

$$\begin{aligned} k_b &= f(EI/L^3) \text{ (bending)} & \sigma &= My/I \text{ (normal stress)} \\ k_s &= GA/L \text{ (shear)} & v &= VQ/Ib \text{ (shear stress)} \\ \delta_b &= f(\text{load, support conditions, } L, E, I) \text{ (bending)} \end{aligned}$$

# PLATE/SHELL ELEMENTS: slab elements

Defn: Members subject to bi-directional bending & shear



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Defn: Two force members, ie axial loads at nodes only



### Elastic Properties:

$$\begin{aligned} k_a &= EA/L \text{ (axial stiffness)} \\ \sigma &= F/A \text{ (normal stress)} \\ \delta &= FL / EA \text{ (deflection)} \end{aligned}$$

# BEAM-COLUMN ELEMENTS

Defn: Members subject to bending, shear, and axial

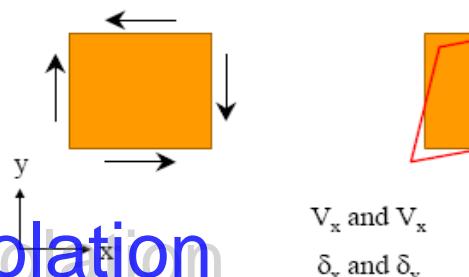


### Elastic Properties:

$$\begin{aligned} k_a &= EA/L \text{ (axial)} & \sigma_a &= F/A \text{ (normal stress)} \\ k_b &= f(EI/L^3) \text{ (bending)} & \sigma_b &= My/I \text{ (normal stress)} \\ k_s &= GA/L \text{ (shear)} & v &= VQ/Ib \text{ (shear stress)} \\ \delta_b &= f(\text{load, support conditions, } L, E, I, A) \text{ (normal)} \end{aligned}$$

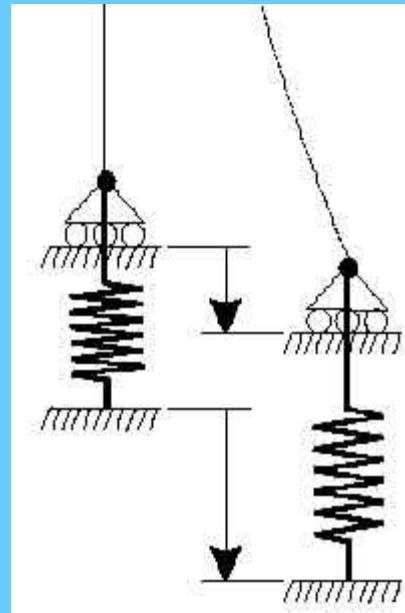
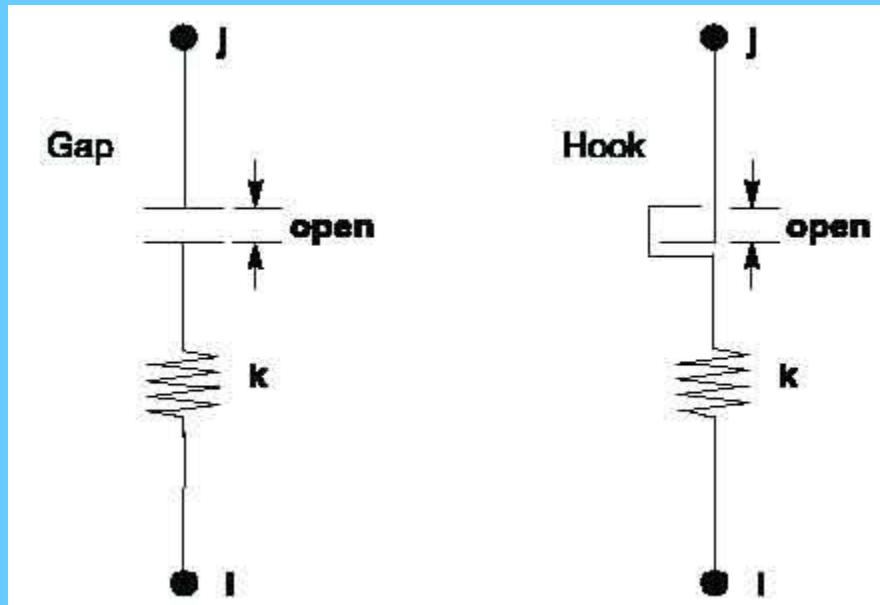
# MEMBRANE ELEMENTS: wall/diaphragm elements

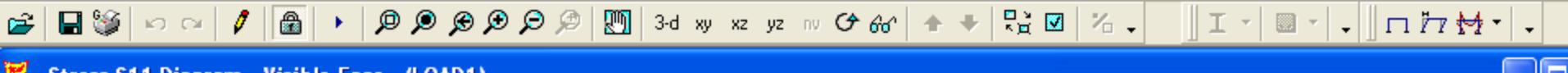
Defn: Members subject to shear



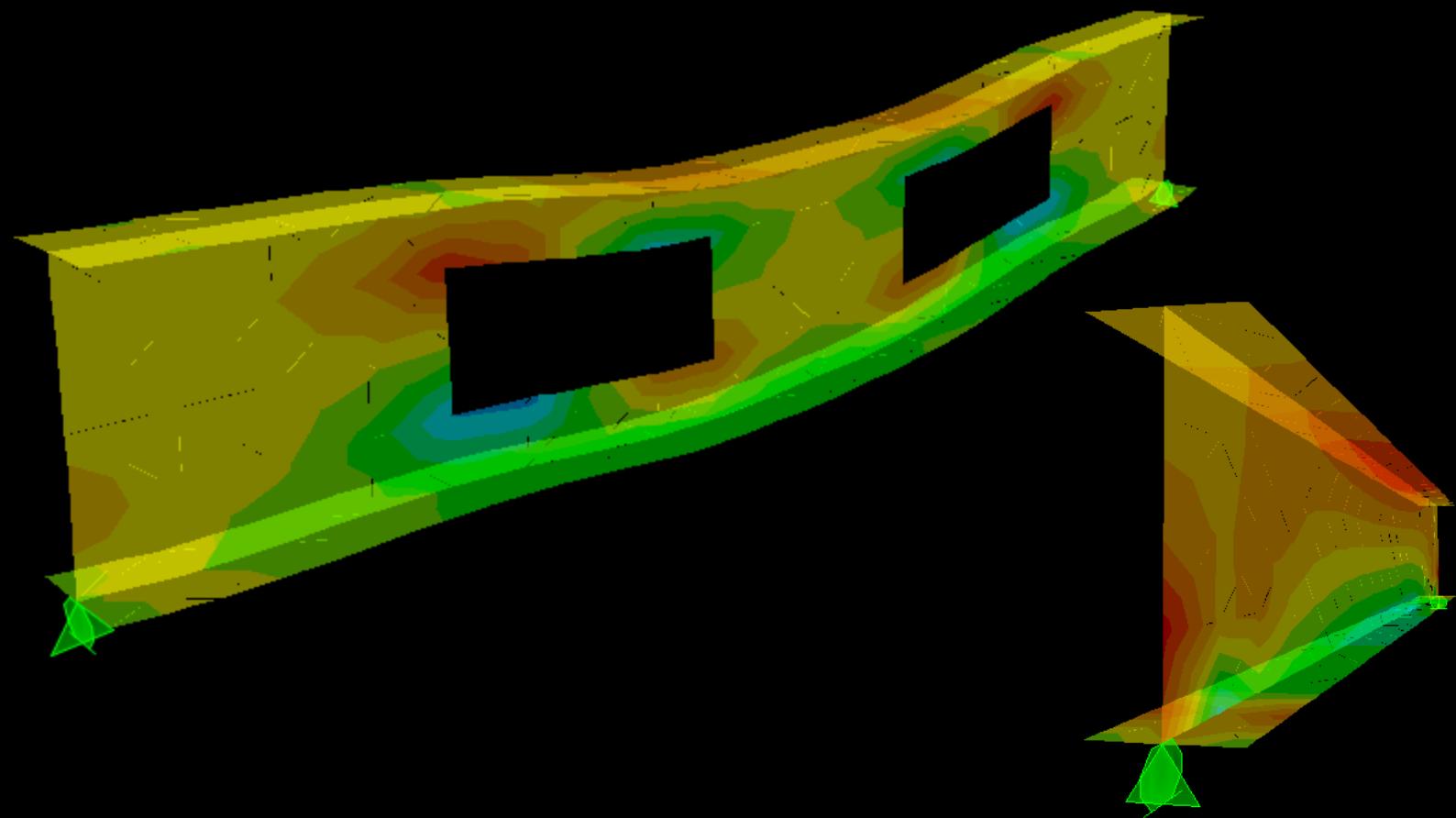
Other elements,

- Spring element
- Link element
- Cable element





The line object is modeled as a surface object



-35.0 -30.0 -25.0 -20.0 -15.0 -10.0 -5.0 0.0 5.0 10.0 15.0 20.0 25.0 30.0

=-35.325, MAX=32.784, Right Click on any Area Element for detailed diagram

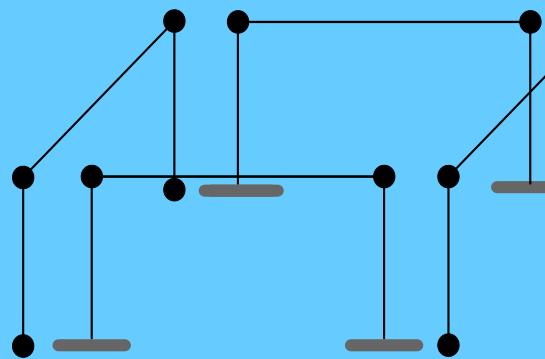
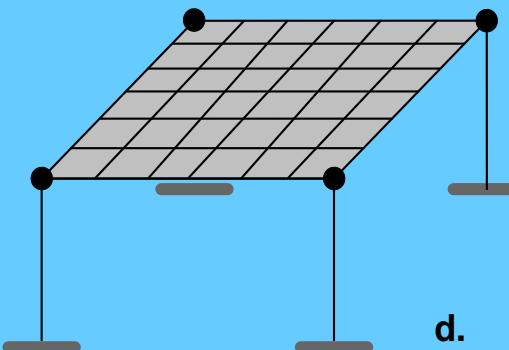
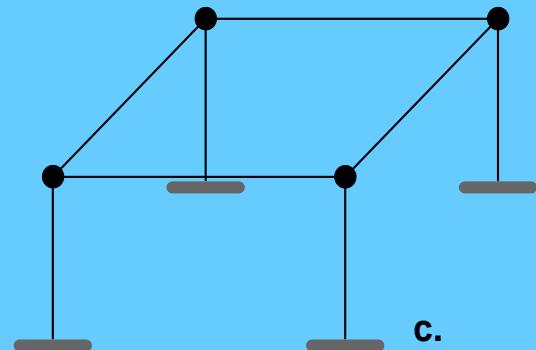
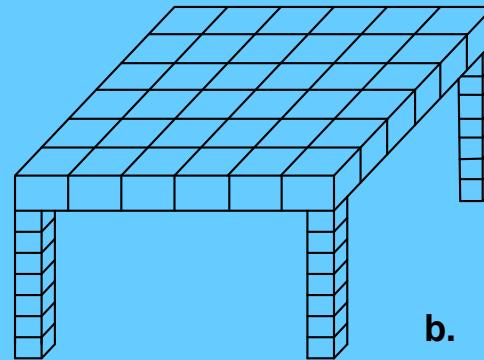
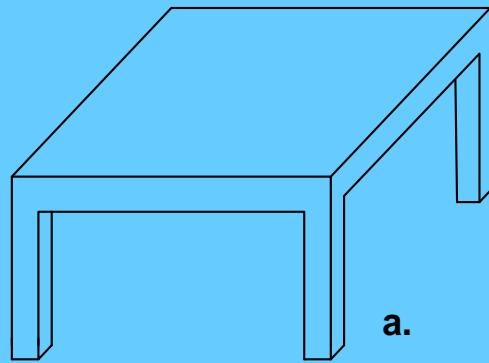
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Start Animation



GLOBAL

Kip, in, F



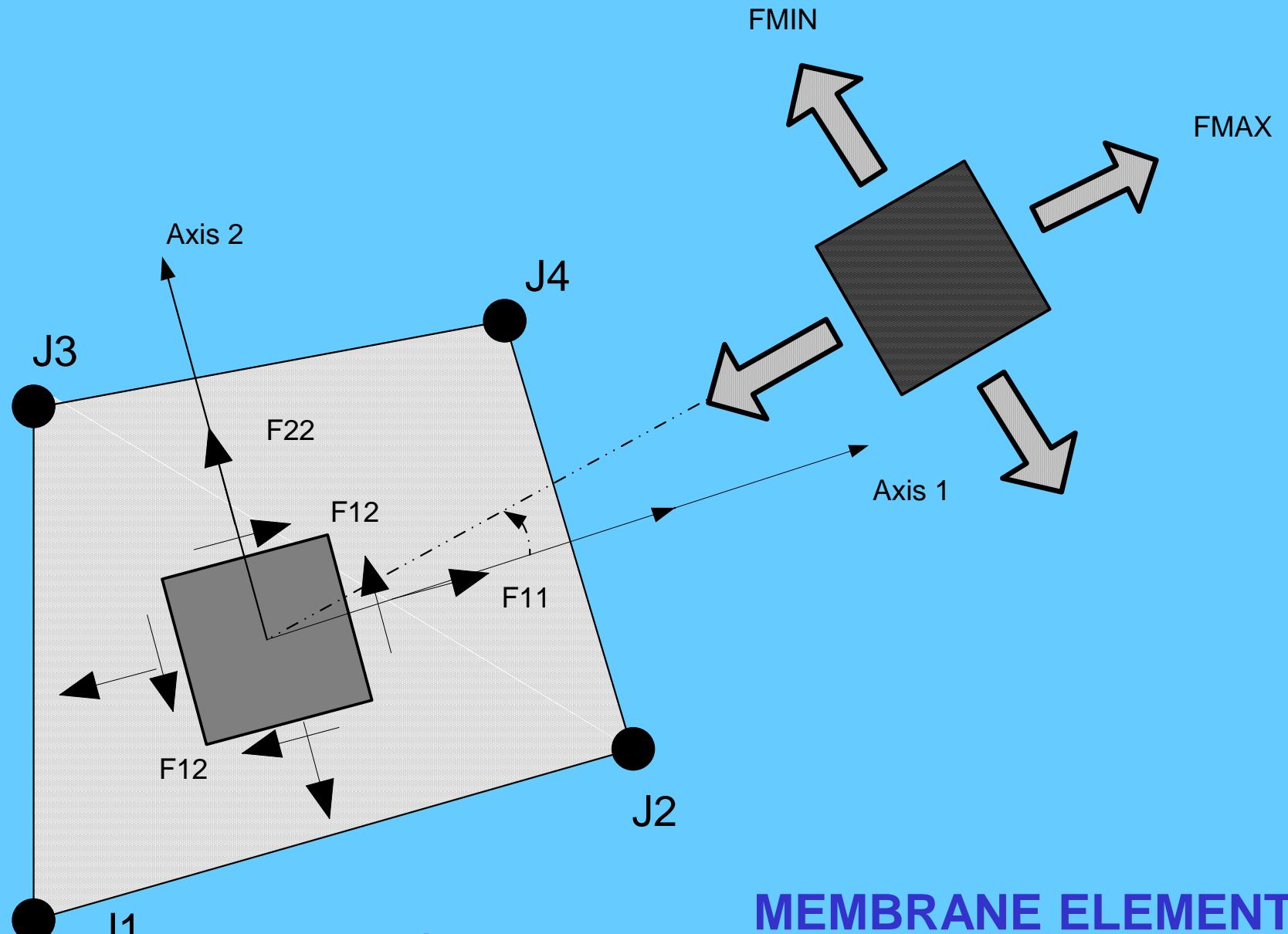
e.

POSSIBLE MODELS OF Seismic isolation STRUCTURE

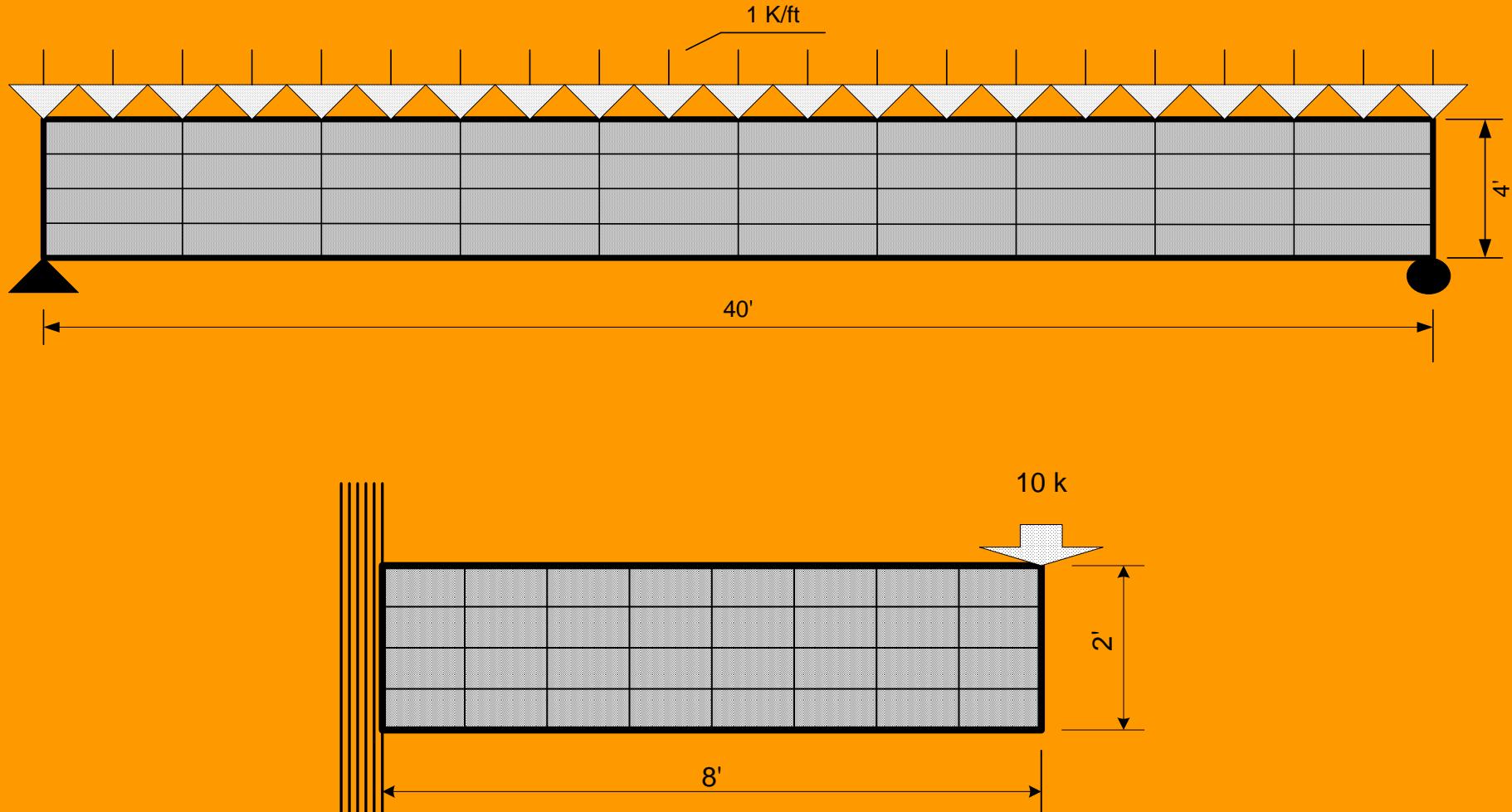
In the decomposed state, the **elements** (**deformable bodies**) are considered connected to **nodes** (infinitely small **rigid bodies**) typically located at the ends or corners of members (see previous figure).

The loads, or in more general terms the **action**, on the elements are converted to **nodal loads**, which in turn causes the nodes to respond by displacing. ***Displacement equations*** (i.e. using typically the **stiffness method**) are formulated for each element. When the elements are reassembled to form the overall structure, **compatibility at the boundaries** of the elements must exist. In other words, independent, simultaneous equations are set up, which are equal to the number of unknowns and can then be solved by computer so the unknown displacements, hence internal forces, are found. Extensive mathematical computations are required because of the huge number of finite elements especially in surface structures and the corresponding number of simultaneous equations to be developed which can only be handled by computers.

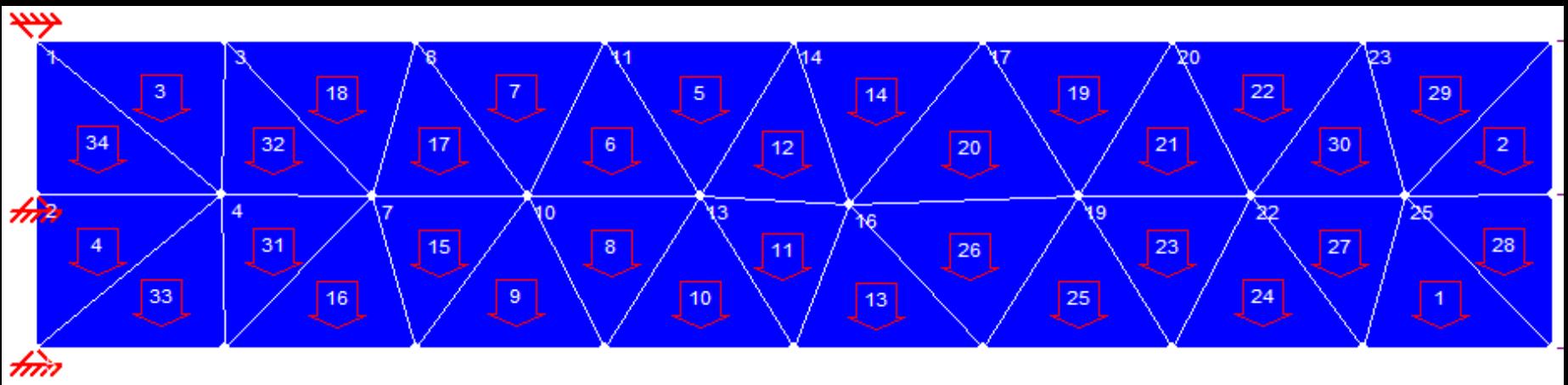
One must keep in mind, however, that the finite element analysis represents a method of approximation. It does not give exact results except for simple frame and membrane problems which are similar to theoretical closed-form solutions.



MEMBRANE ELEMENT  
@Seismicisolation



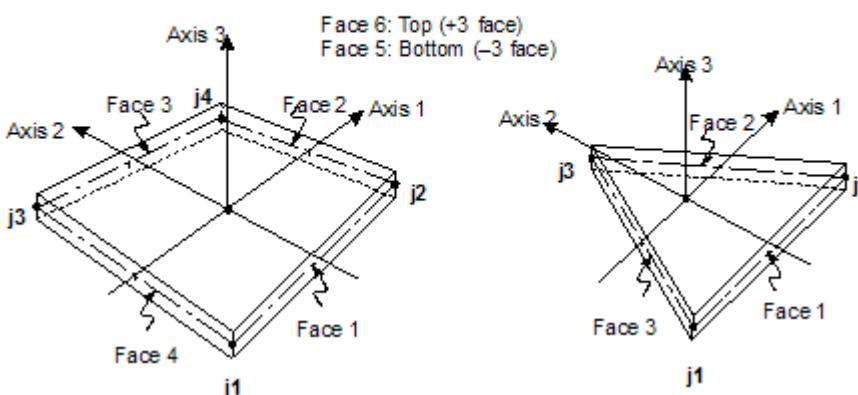
MECHANICAL ENGINEERING  
@Seismicisolation



## MEMBRANE ELEMENTS

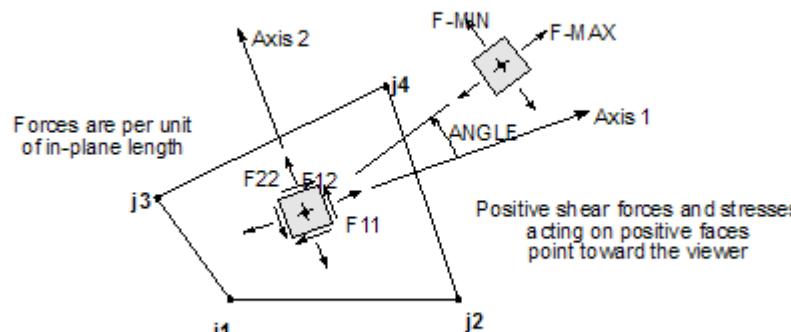
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## Shell Element



Four-Node Quadrilateral Shell Element

Three-Node Triangular Shell Element



Membrane Forces

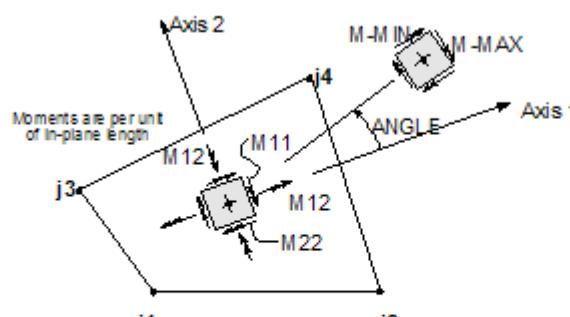


Plate Bending and Twisting Moments

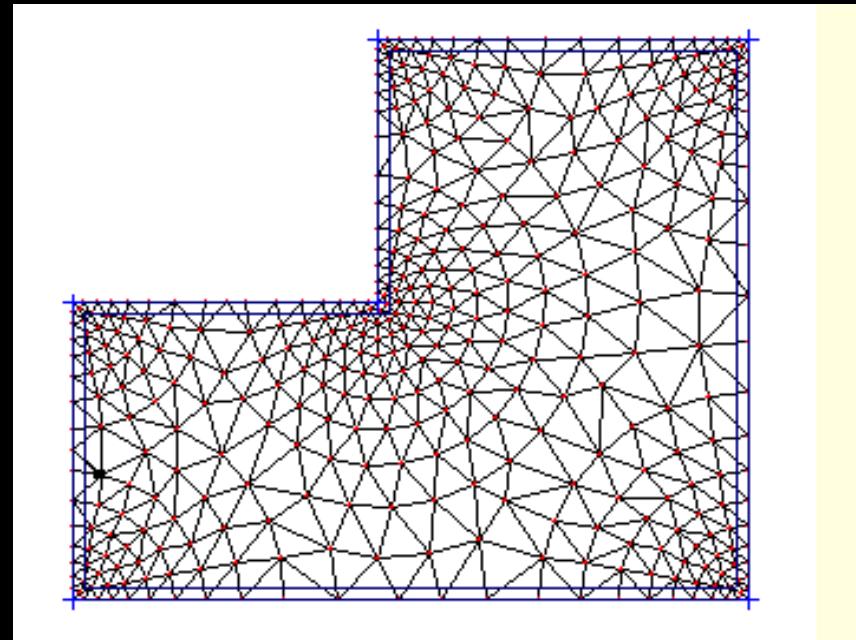
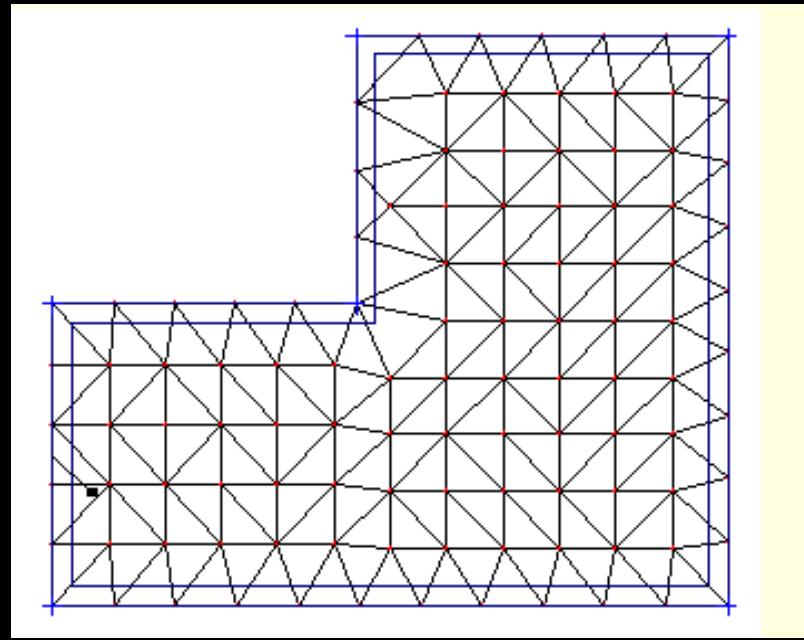
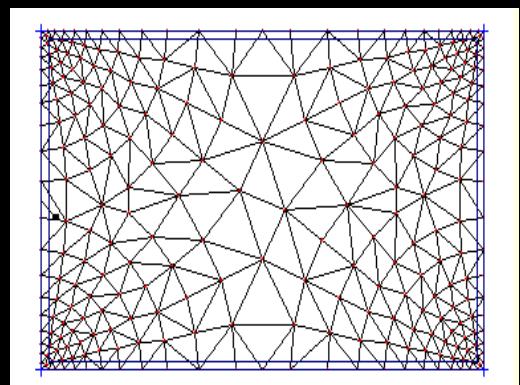
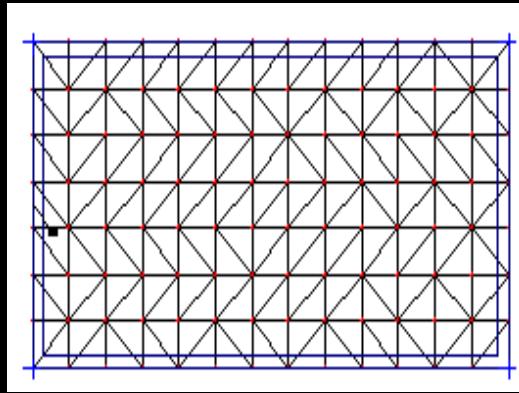
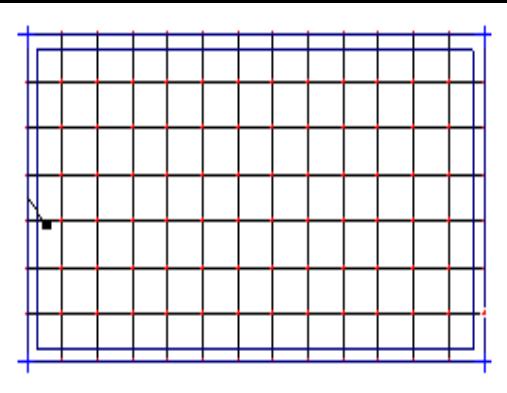
Shell Internal Forces

$$F_{INT} = F \text{ intermediate}$$

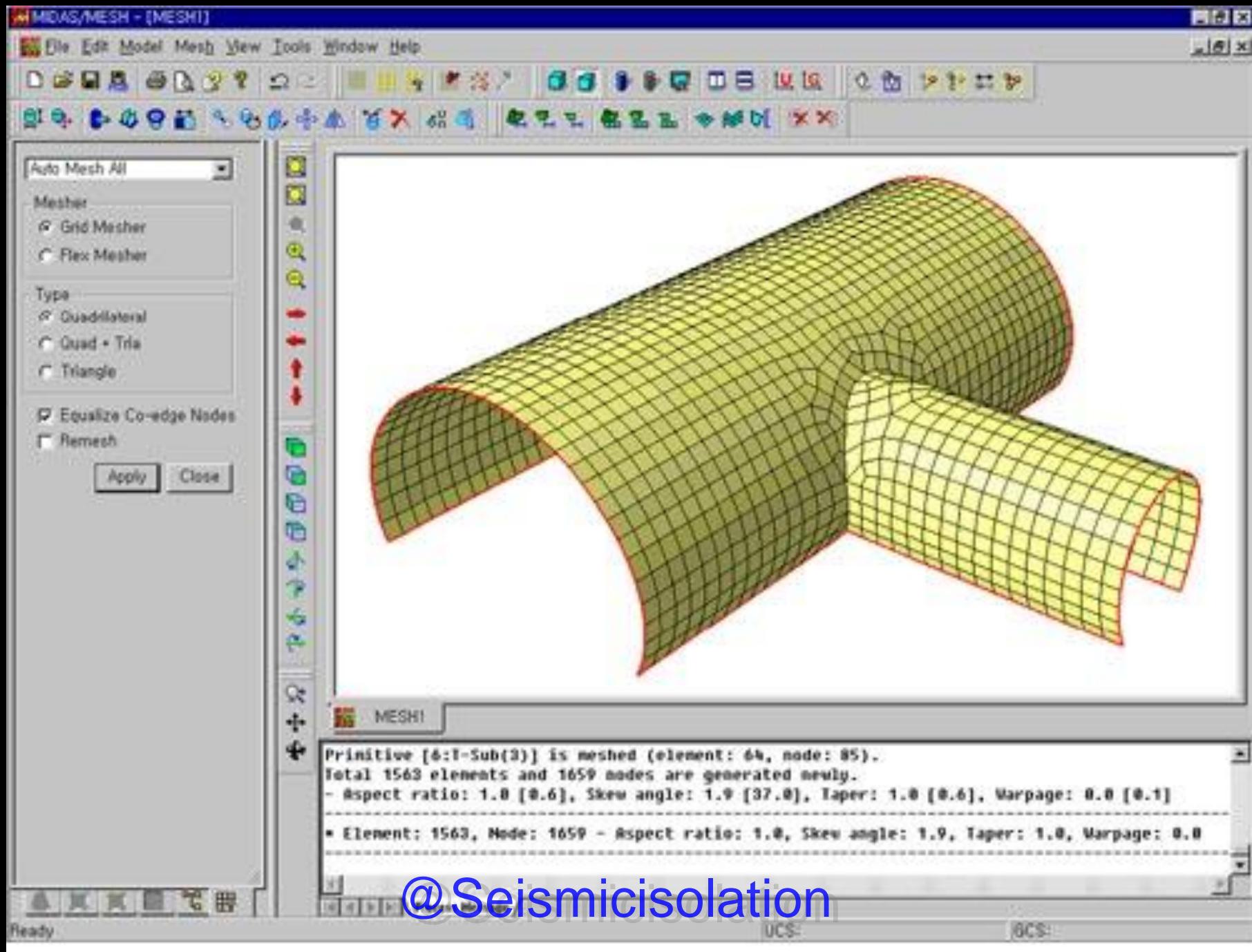
$$FVM = \sqrt{\frac{1}{2}[(F_{MAX} - F_{INT})^2 + (F_{MAX} - F_{MIN})^2 + (F_{INT} - F_{MIN})^2]}.$$

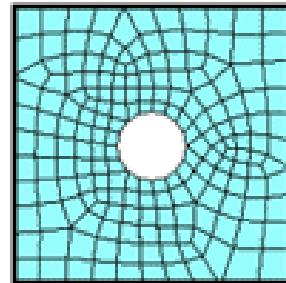
Von Mises Stress

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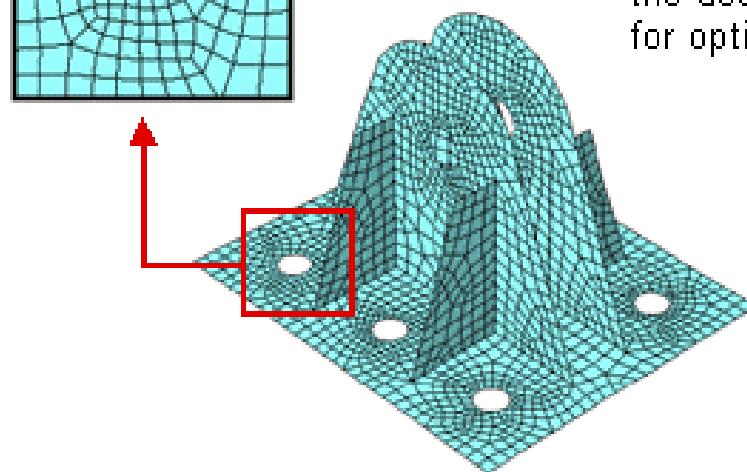
@Seismicisolation  
@PASEE\_ELEMENT





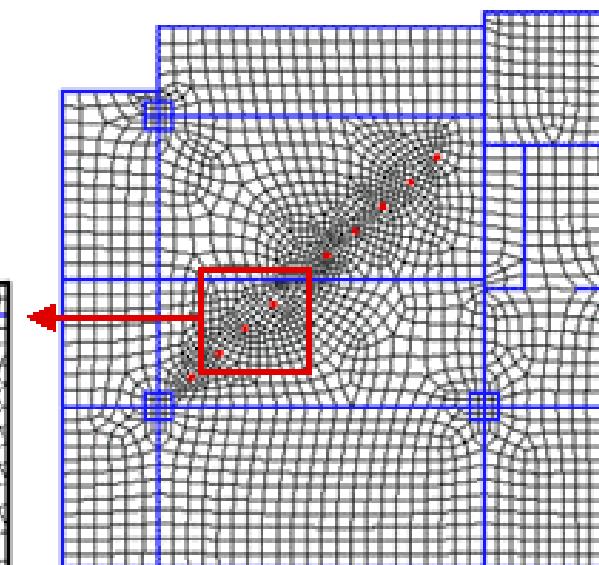
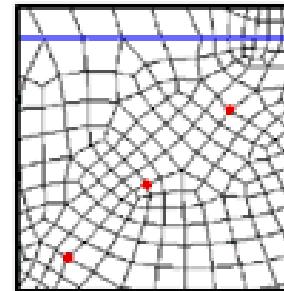
Top View

Where certain parts of a structure require detail investigation MESH enables the user to increase the mesh density for optimum meshing.

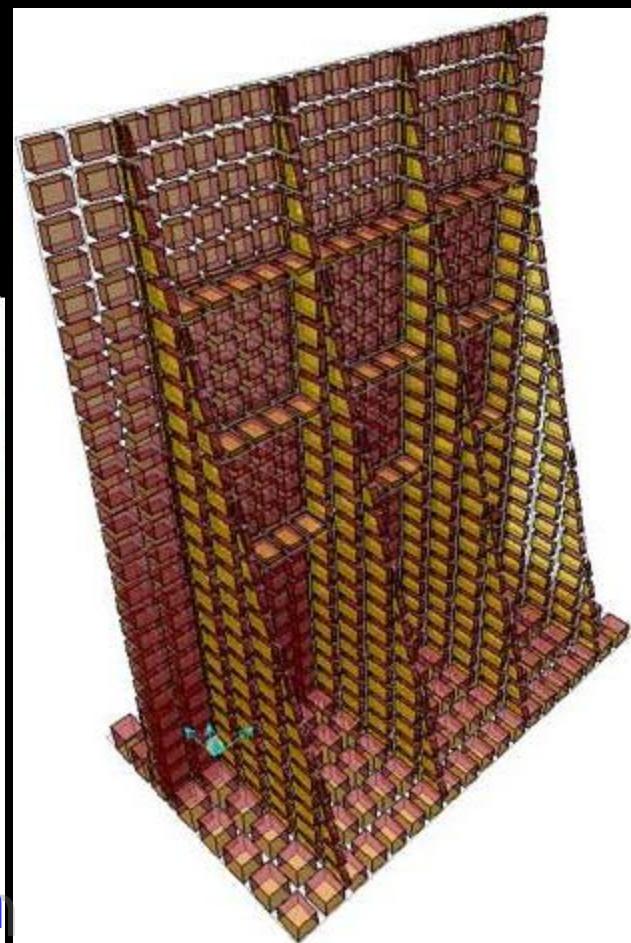
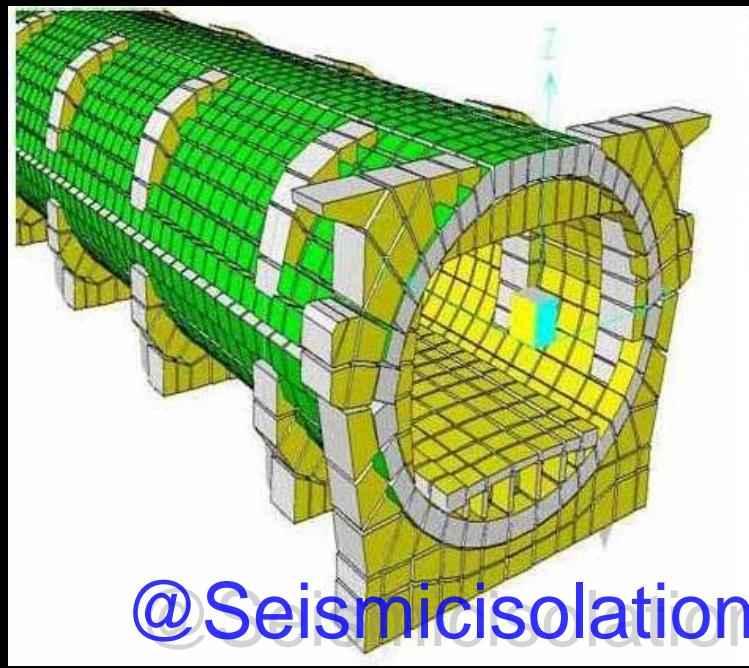
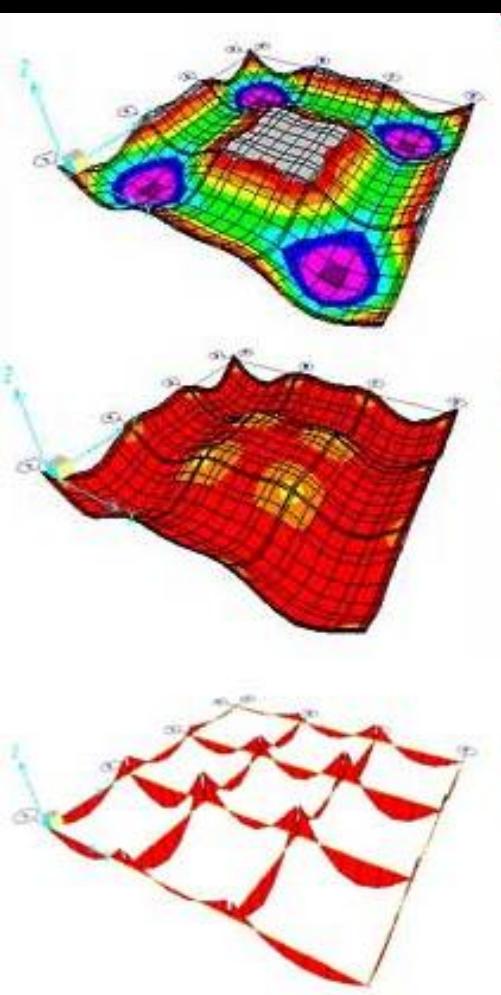
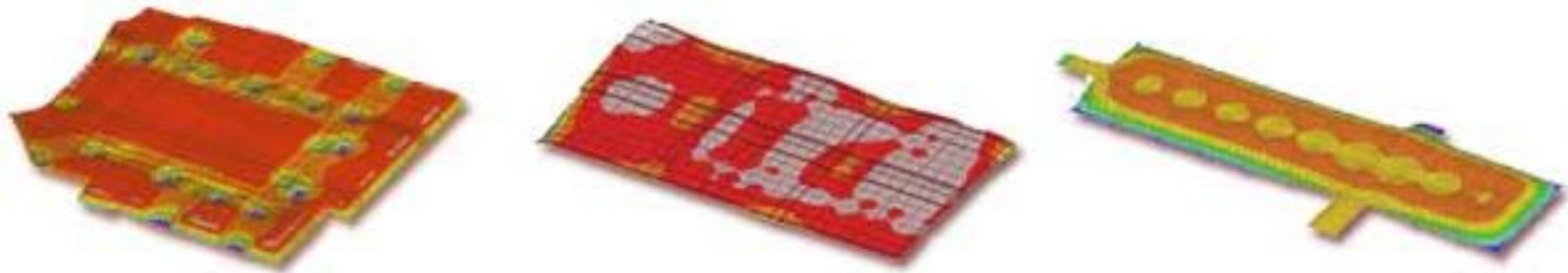


Base Plate Analysis Model  
(Mesh density control applied)

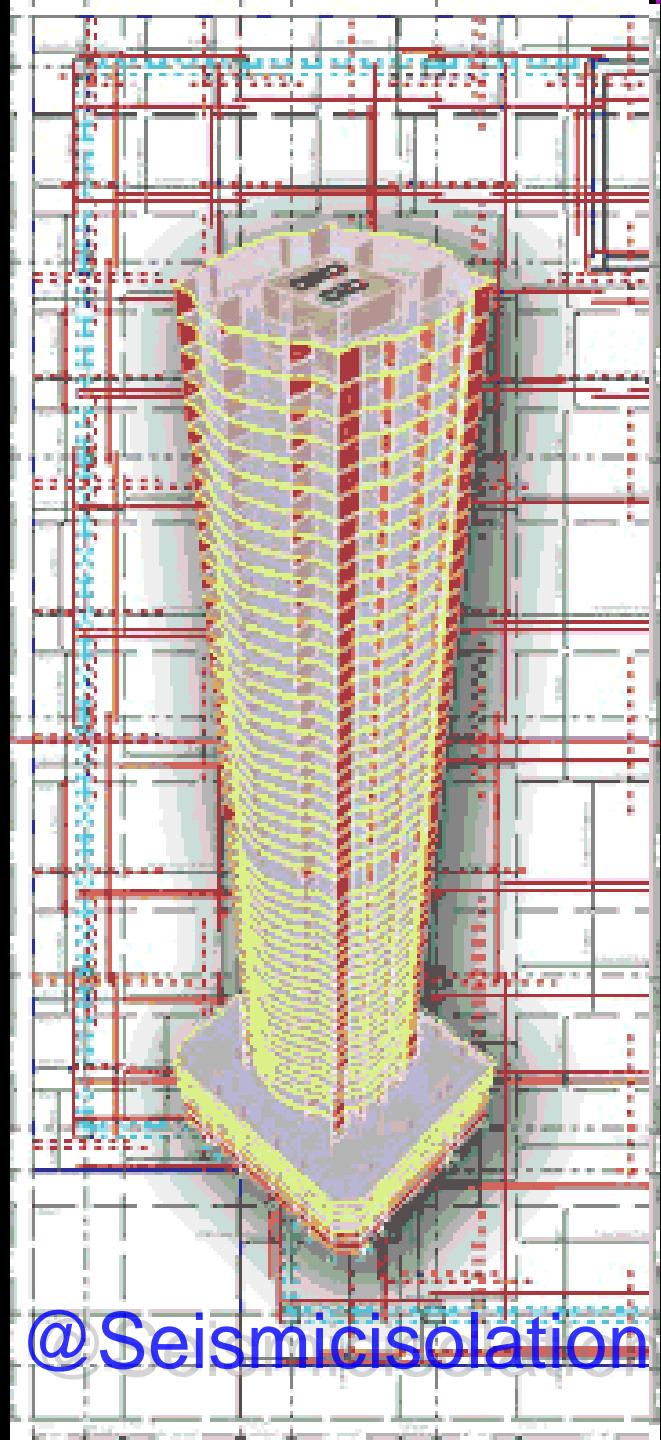
Locations of applied walking loads



Floor structure model for vibration analysis due to walking loads  
(Mesh density control applied to nodes)



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The following typical principal steps for performing a finite element analysis, considering only basic, simple structures for preliminary design purposes, are:

**CREATE** the finite element model by dividing the structure into finite elements – idealize the structure

## **DEFINE**

- GRID SIZE and UNITS
- MEMBER ELEMENTS and NODES: determine the type of elements which are appropriate for your model.  
In SAP2000 you only define elements - all nodes needed by the elements are automatically generated.
  - **Line elements**: truss, beam (beam column), cable
  - **Surface elements**: behavior (membrane-, plate-, shell-elements), shape (quadrilateral, triangular)
  - **3D elements**
- MATERIAL properties: steel, reinforced concrete, other
- MEMBER SECTIONS: geometric properties, shapes from library
- LOADING conditions: static load cases, load combinations

**DRAW GEOMETRY**: define the geometry of structure, i.e. the location of members on grid

**ASSIGN** properties, loading, and boundary conditions to the structure

- MATERIAL
- MEMBER SECTIONS
- EXTERNAL SUPPORTS: free, fixed, pinned, roller, spring
- NODAL RESTRAINTS: FRAME END RELEASES, constraints, springs
- EXTERNAL FORCES/DISPLACEMENTS on nodes and members
- DIAPHRAGM CONSTRAINTS
- MEMBER CHARACTERISTICS: clear length, unbraced length
- etc.

CHECK INPUT DATA

**ANALYZE**: select analysis type and run analysis

- DISPLAY the model (e.g. reactions, forces, stresses, deformations and animations)
- EVALUATE and CHECK important results

## **DESIGN**

- MEMBER PROPERTIES
- MODIFY MEMBER PROPERTIES

## **RE-ANALYZE and RE-DESIGN**

**PRINTING and PLOTTING** of results  
© Seismisolation

**Structure software** helps students to **VISUALIZE** the building as an assembly of,

- **linear members** (e.g. beams, columns, arches, cables),
- **planar members** (e.g. walls, slabs, shells, flexible membranes), or
- **spatial members** (e.g. solids).

**1.** First students have to **DEFINE**:

*geometry  
material  
member types  
member sections  
static load cases  
load combinations*

**2.** Then students set up the **mathematical model for the building support structure** by

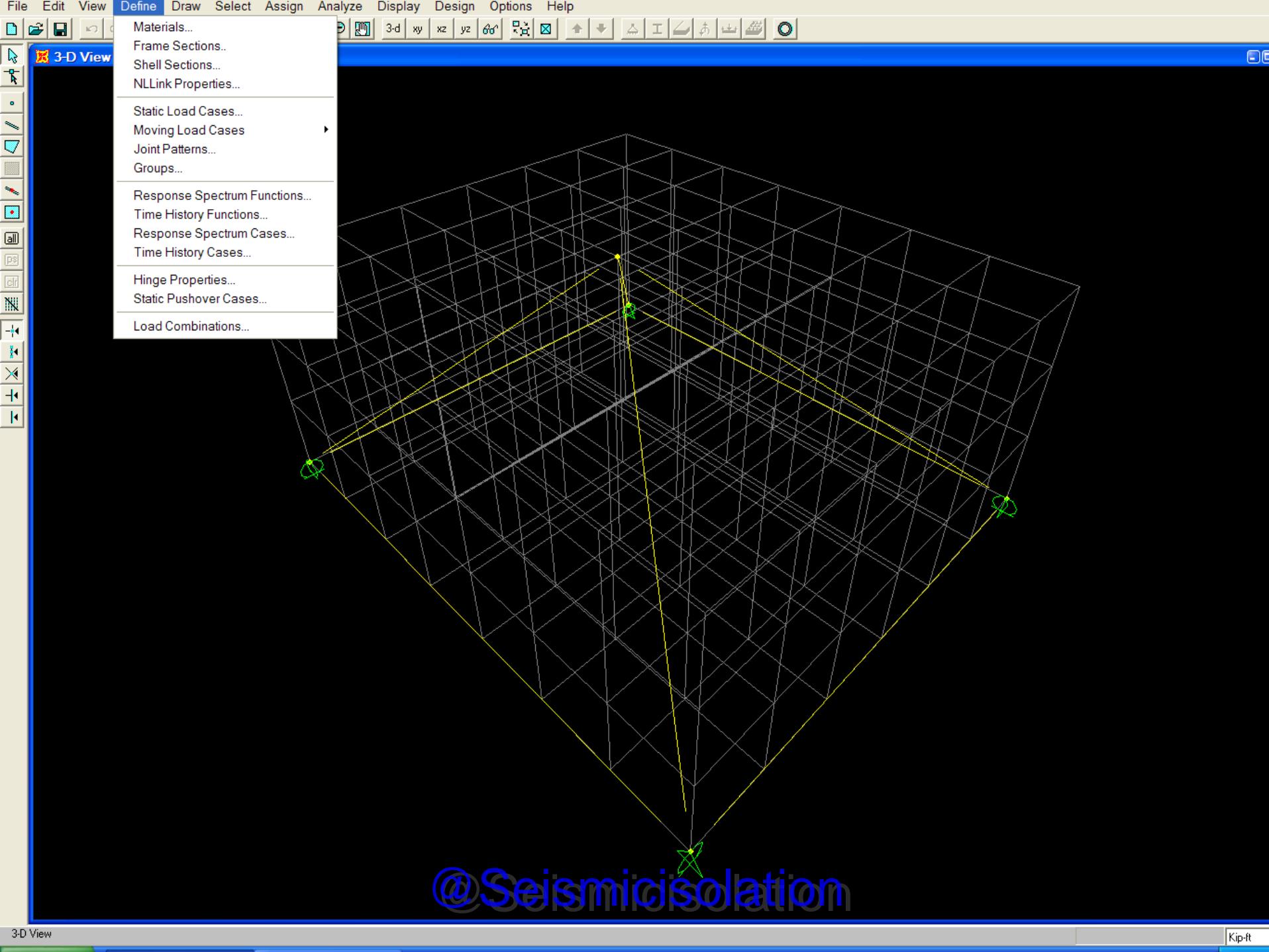
**DRAWING** the geometry of the structure and then to

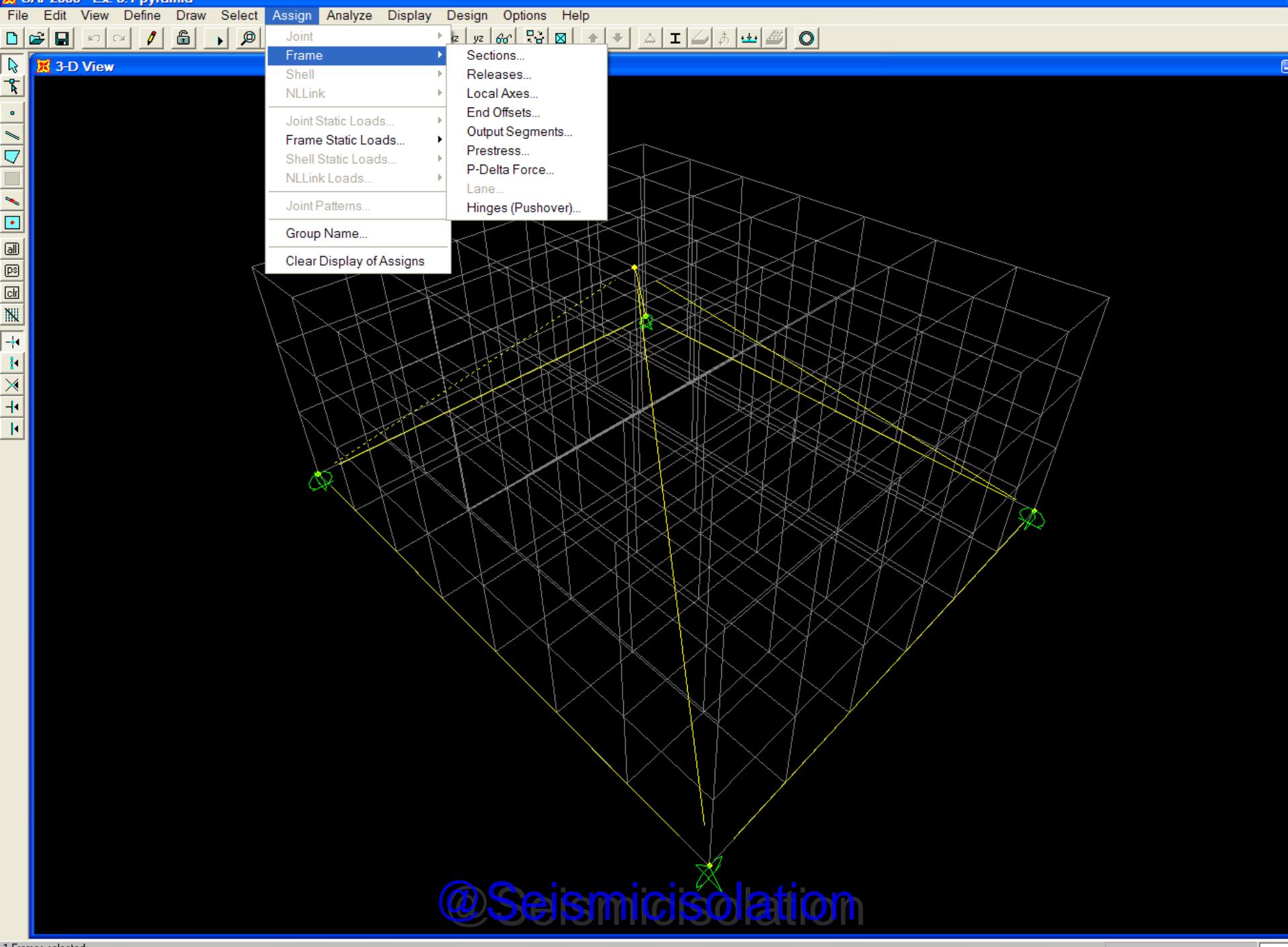
**3. ASSIGN** the

*member types and sections  
external support joints  
possibly constraints  
frame end releases (internal member joints)  
load types*

If you are upgrading from a previous version of SAP2000 (Version 11 and earlier), you should be aware that there has been a significant change in nomenclature regarding loading:

- The term "**load pattern**" replaces "**load cases**" of previous versions.
- The term "**load cases**" replaces "**analysis cases**" of previous versions.
- The term "**load combinations**" has not changed.





**To model a structure the designer must be familiar with the following criteria:**

- **building structure primary function**
- **types of loads**
- **building materials** (i.e. important material characteristics)
- **structural members**
- **member connections**
- **structural systems**

# **COMPUTER-BASED ANALYSIS AND DESIGN OF STRUCTURES**

## **INTRODUCTION**

- A. MODELING THE STRUCTURE**
- B. CHECK INPUT DATA**
- C. ANALYSIS**
- D. EVALUATE and CHECK RESULTS**
- E. DESIGN**
- F. MODIFY MEMBER PROPERTIES**
- G. RE-ANALYZE and RE-DESIGN**
- H. COMPUTER OUTPUT: display results (printing and plotting)**

## A. MODELING THE STRUCTURE

A typical computer input of mathematical models is presented to demonstrate the decomposition of the structure into its constituent parts. It is assumed for ordinary conditions that the original geometry of the structure does not change and that it remains **linearly elastic**. The initial modeling of the structure is defined by **geometry**, **members** and **nodes**, **material** and **member properties**, **supports**, **loads** and **type of analysis**.

Building structures are three-dimensional where the applied loads flow along the members and joints to the external supports usually the foundations. Since the applied loads cause internal forces in the members and joints, one can visualize the support structure as replaced by corresponding, complex three-dimensional force systems. In other words under the load action the support structure displaces and changes its geometry thereby causing deformations in the members which, in turn correspond to internal forces.

Ordinary buildings can generally be considered as an **assembly of independent horizontal** and **vertical planar structures** (at least for preliminary design purposes) so that force systems can be treated as two-dimensional or coplanar. Therefore, typically, the first approach towards modeling the structure will consist of a simplified version of the overall structure or it will be only a portion of it. The investigation of the various structure systems, in this context, is primarily concerned with planar structures and force systems rather than modeling the true spatial building structure.

- It is not the intention here to introduce a detailed discussion of the finite element method, since you will not write your own finite element programs but only use finite element programs. **It must be emphasized that a precise understanding of the finite element is not a prerequisite for working with structure finite element programs**  
 In other words, it is important not to see the finite element analysis as a numerical technique involving matrix mathematics to solve large sets of simultaneous algebraic equations, but to see it as an abstract model that makes it possible to learn about the behavior of structures in general. **To see it also as a powerful educational tool since it has the unique feature of integrating all the structural concepts of loads, language of forces, geometry, structure systems, member behavior, stability of building, materials, codes, etc., all necessary components of design, into one system. In the traditional approach, all those topics are treated more or less in separate courses.**
- Finite element analysis makes indeterminate structures now accessible to general designers during the preliminary design stage. **To study the effect of stiffness (e.g. member size) on force flow, as well as the effect of change of geometry, member arrangement and support locations, is most important for developing a feel of structure.** The same is true for studying the effect of the type of member assembly with respect to local and global stability by considering rigid joints and/or bracing. In addition, to become aware of **structural systems** (thereby developing a sense of what is possible), as well as a learning model that decomposes the building structure into geometry, its major parts of: **elements** and **nodes or joints, support joints, active member joints, material**, and **applied loads**.
- The primary difficulty in using structural computer programs is to not truly understand the physical action and boundary conditions as well as the behavior of the structure elements of the actual structure, which is especially true for the behavior of the joints. The true constraints (i.e. restraint to movement) that each member exerts on the adjacent ones, can only be approximated. **It is apparent, that the mathematical model may not represent the reality and true character of the structure at all.**

**Modeling the structure** involves the creation of an abstract mathematical model of the building structure and the loads acting on it. A building structure is defined by the

a. **GRID SIZE and UNITS**

**GEOMETRY:** define the geometry of structure, i.e. the location of members on grid

b. **MATERIAL properties:** aluminum, steel, reinforced concrete, etc.

c. **MEMBER ELEMENTS and NODES** (line elements, surface elements, 3D elements): in SAP2000 you only define elements - all nodes needed by the elements are automatically generated.

d. **MEMBER CHARACTERISTICS:** clear length, unbraced length

e. **MEMBER SECTIONS:** geometric properties, shapes from library

f. **EXTERNAL FORCES/DISPLACEMENTS** on nodes and members

g. **LOADING conditions:** static load cases, load combinations

h. **EXTERNAL SUPPORTS:** free, fixed, pinned, roller, spring

i. **NODAL RESTRAINTS: FRAME END RELEASES,** constraints, springs

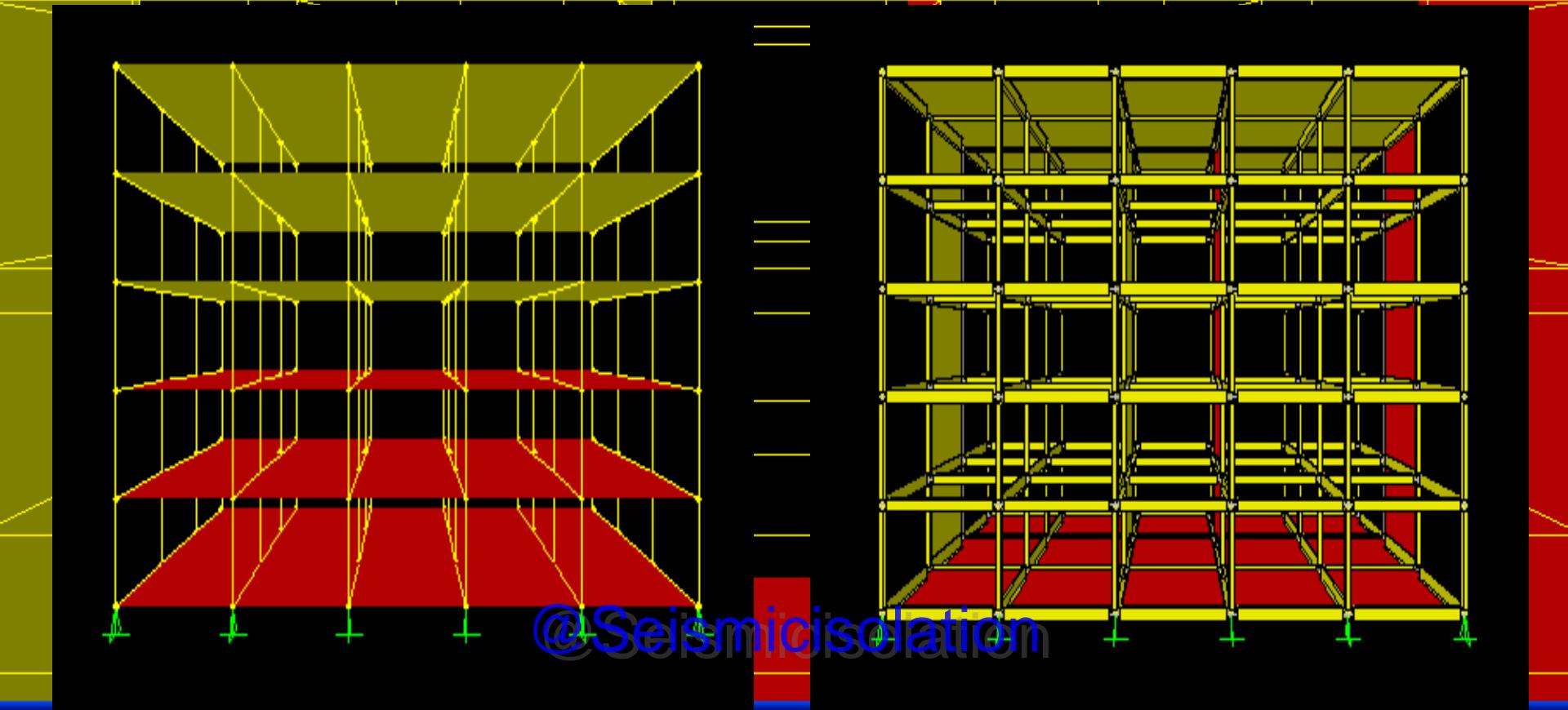
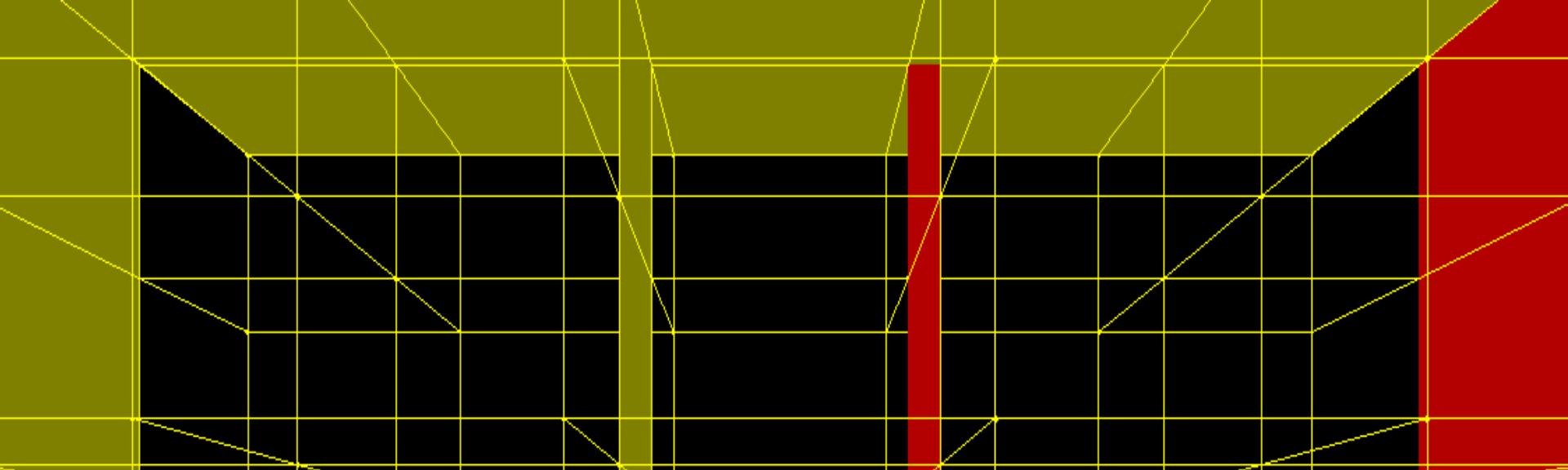
j. **DIAPHRAGM CONSTRAINTS**

k. etc.

# GEOMETRY

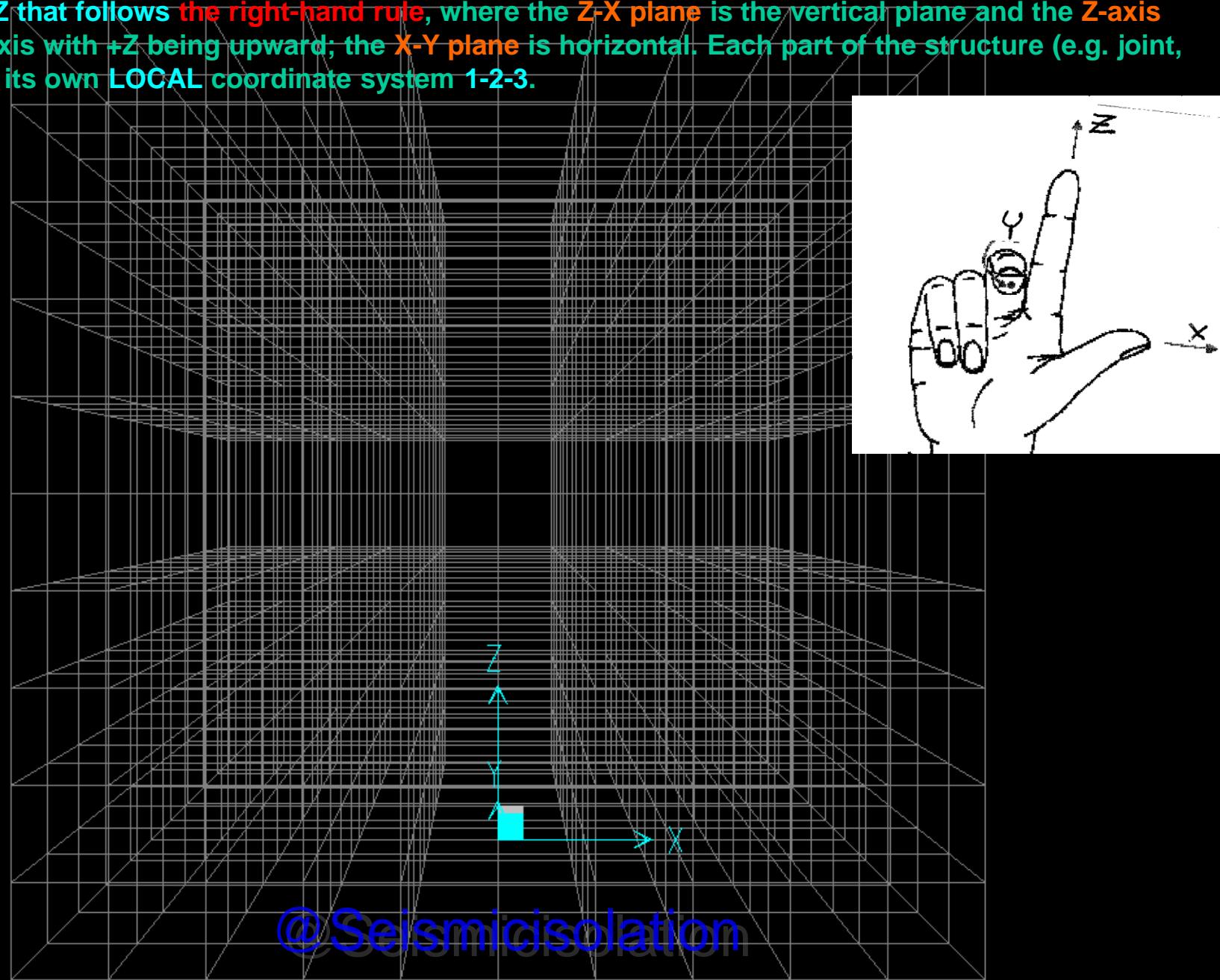
The **geometry of structures** may be generated in computers **graphically** and/or using **spreadsheets**, or importing **DXF files**. Depending on the structural engineering software, however, many structure types may be automatically generated with the assistance of templates. Most computer software packages provide a library of 2D and 3D **structure layouts** that include typical systems such as various types of beams, trusses, frames, and grids. Some software include in addition more specialized structures such as cables, shear walls, floor framing, as well as particular types of space frames, shells, geodesic and latticed domes, pre-stressed tensile membranes, and so on.

The geometry of the systems may be predetermined for line elements (e.g. as polygonal, circular, elliptical, parabolic, spiral), planar elements (possibly as derived for curvilinear surfaces from sphere, cone, cylinder, tube, hyperbolic paraboloid, torus, or other behavioral phenomena), and for spatial elements or blocks (as defined for instance for ordinary conditions by solid geometry for basic shapes of polyhedrons, prisms, pyramids, cylinders, cones, spheres, etc.). Large models can be generated automatically using templates. The models can then be sculptured with on screen editing to satisfy specific situations.



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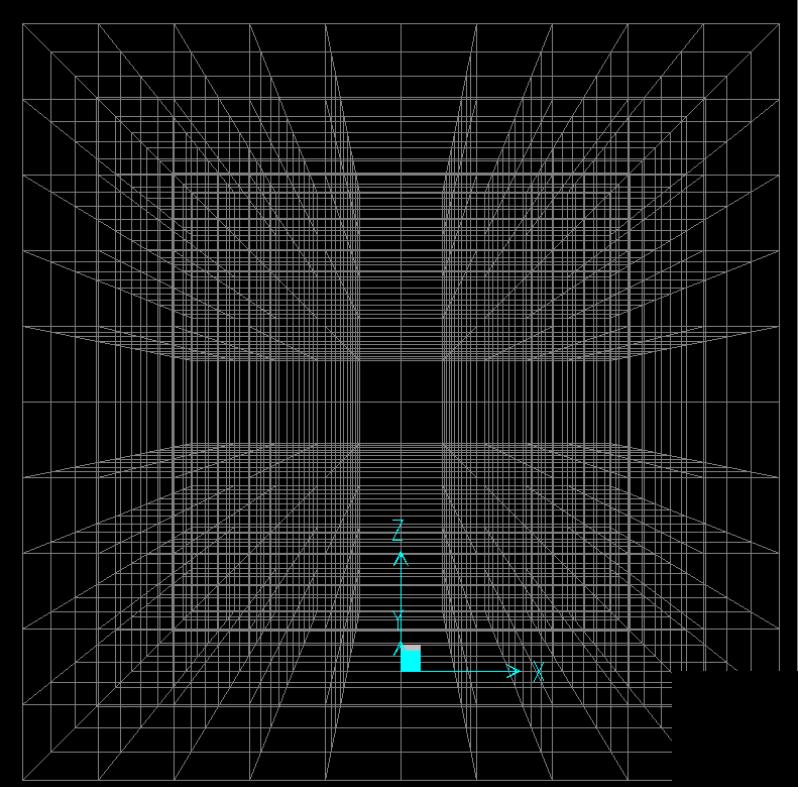
In general, a building structure is located in a spatial grid and defined by a single **GLOBAL** coordinate system X-Y-Z that follows the right-hand rule, where the Z-X plane is the vertical plane and the Z-axis the vertical axis with +Z being upward; the X-Y plane is horizontal. Each part of the structure (e.g. joint, element) has its own **LOCAL** coordinate system 1-2-3.



The building structure is located in a spatial grid and defined by a single **GLOBAL coordinate system X-Y-Z**, where the Z-X plane is the vertical plane and the Z-axis the vertical axis with +Z being upward; the X-Y plane is horizontal.

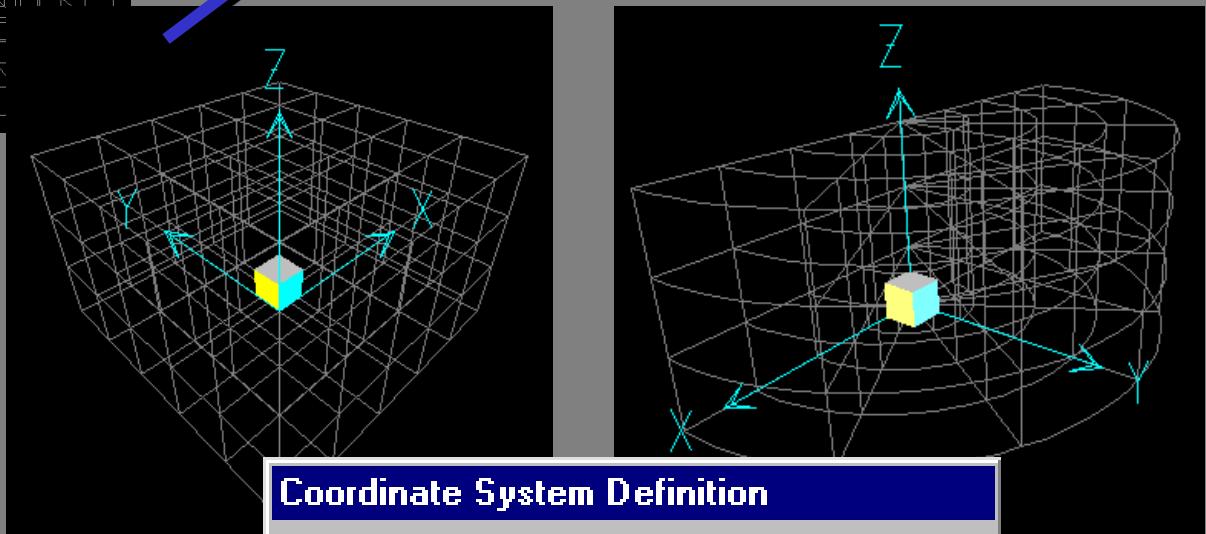
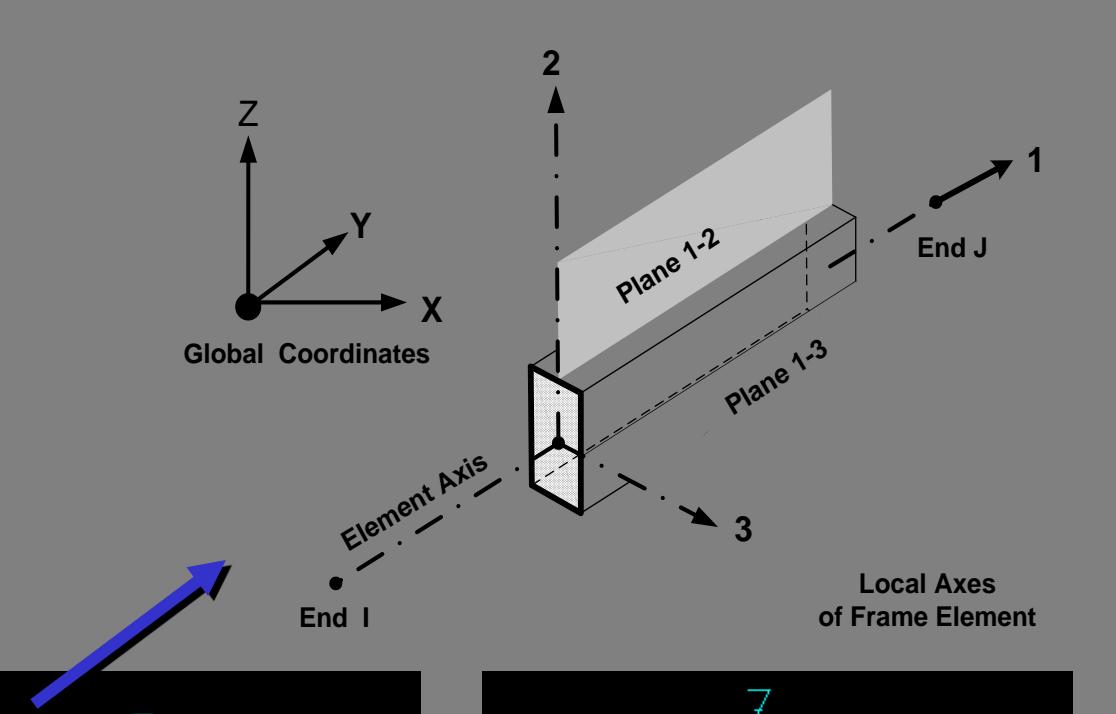
Each part of the structure (e.g. joint, element) has its own **LOCAL coordinate system 1-2-3**. In other words, each frame element has its own local coordinate system for defining section properties and loads, and for interpreting output data.

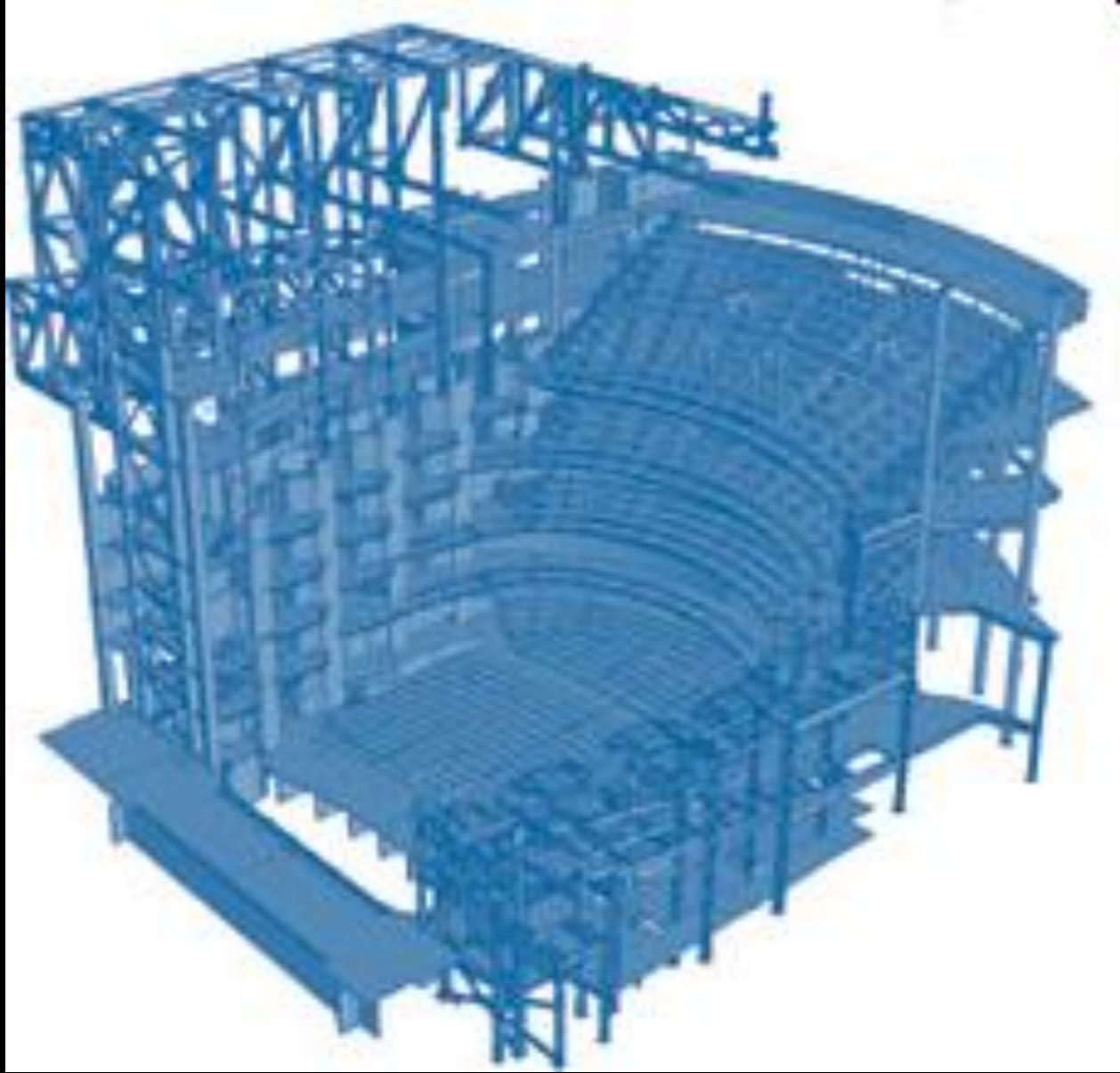
- The **joint local coordinate system** is normally the same as the global X-Y-Z coordinate system.
- For the elements, one of the **element local axes** is determined by the geometry of the individual element; the orientation of the remaining two axes is defined by specifying a single angle of rotation.  
For frame elements, for example, the **local axis 1** is always the longitudinal axis of the element with the positive direction from **I** to **J**.  
The default orientation of the **local 1-2 plane** is taken to be vertical (i.e. parallel to the Z-axis). The **local 3-axis** is always horizontal (i.e. lies in the X-Y plane).



## Guides and Grids

- Accurate dimensioning with guidelines and snapping
- Multiple rectangular and cylindrical coordinate systems with flexible grid systems





Structural design is gradually shifting from 2D drafting towards 3D modelling. Tekla has developed an innovative solution for Structural Building Information Modelling, a subset of the commonly used concept Building Information Modelling (BIM). **@Seismicisolation**

# MATERIAL

The typical material properties identified by structural engineering software are:

**Specific weight or weight density (weight per unit volume)**

**Mass density (mass per unit volume)**

**Modulus of elasticity:  $E$**

**Shear modulus:  $G$**

**Poisson's ratio:  $\nu$  ( $N_u$ )**

**Coefficient of thermal expansion:  $\epsilon_t$  or  $\mu$  ( $M_u$ )**

**Material strength, or yield strength for steel ( $f_y$ )**

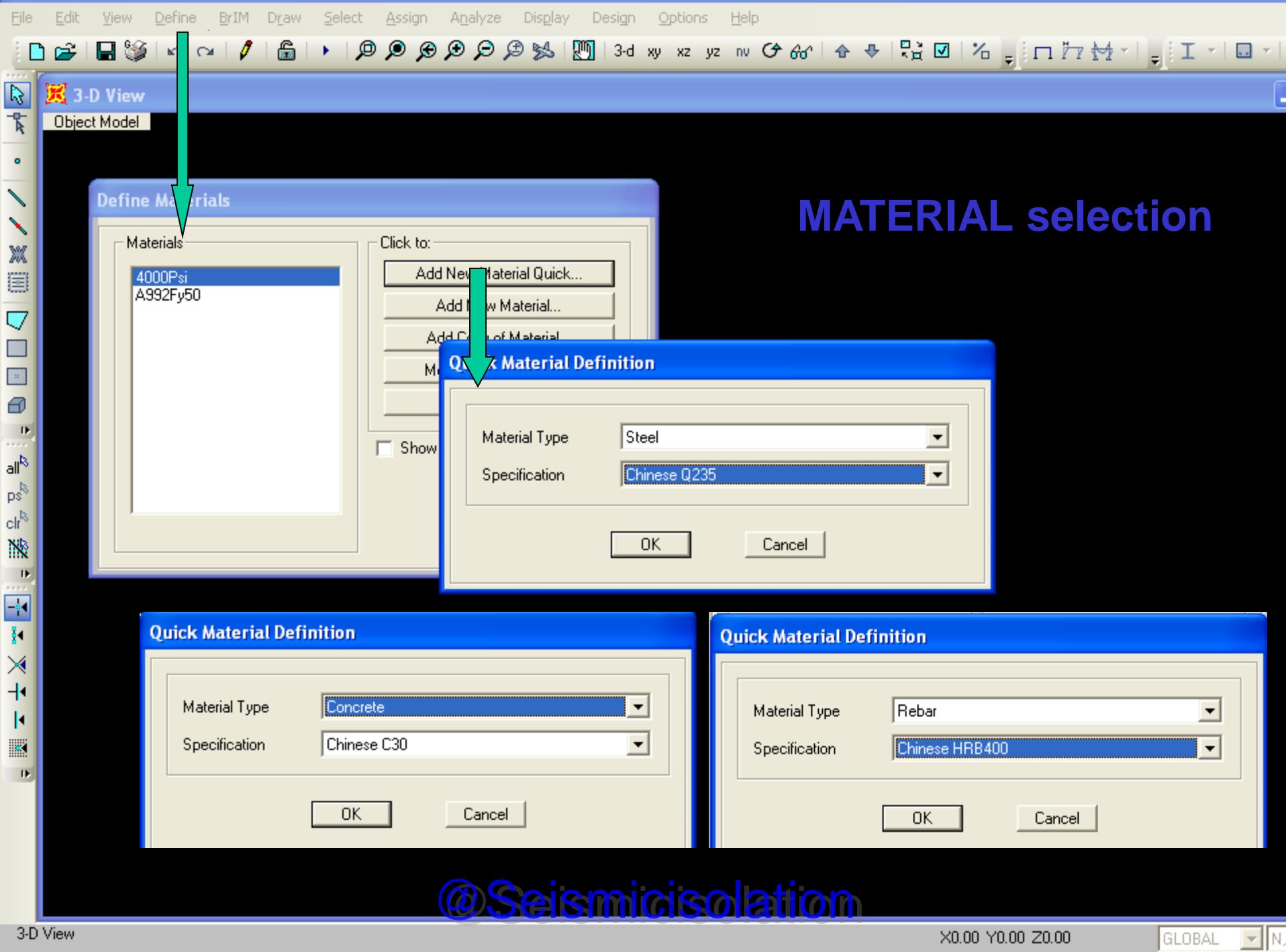
**Type of material: *isotropic* or *orthotropic***

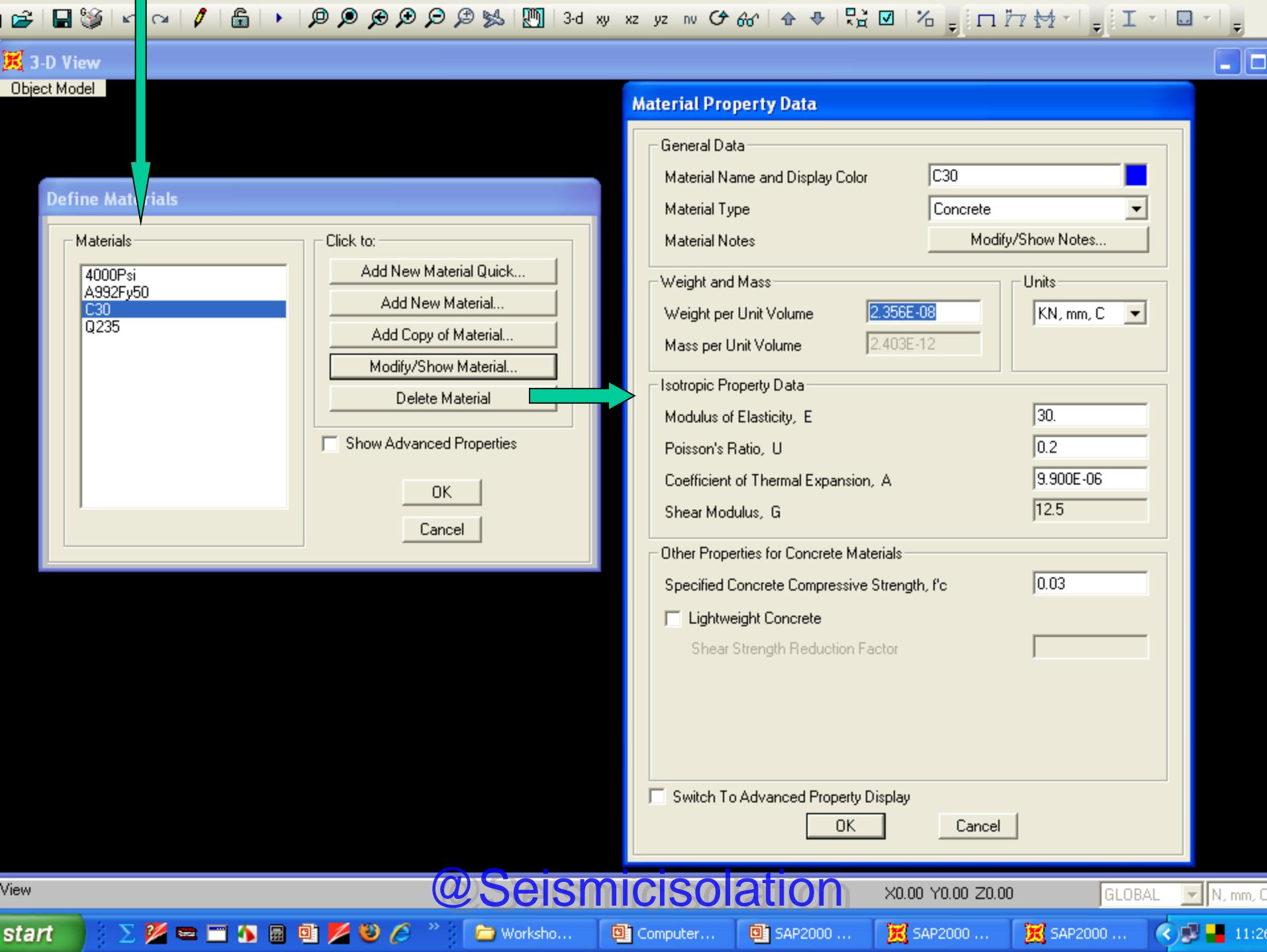
For orthotropic materials the values for  $E$ ,  $G$ ,  $\nu$ , and  $\mu$  must be defined for the three local axes of the material.

**Program defined materials** such as: aluminum, steel and reinforced concrete; possibly **user defined material** for wood and masonry but that depends on the structural engineering software

## Typical allowable stresses for ordinary materials:

	Permissible stress
	N/mm <sup>2</sup> (MPa) ksi
Steel	150 22
Aluminium	100 14.5
Concrete	10 1.5
Wood	7 1





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# MEMBER ELEMENTS

In the mathematical model the building structure is first modeled purely from a geometrical point of view using **line elements**, **planar elements** and possibly **three-dimensional elements**; these elements represent only geometry, they have no other properties (e.g. strength) assigned to them. For example, a building structure that consists of an assembly of **linear bar** or **frame elements** (e.g. beams, columns, arches, cables), the frames are idealized as *line diagrams* where the lines are located at the geometric axes of the structural elements. For the situation where the building structure is made up of **planar elements** (e.g. walls, slabs, rigid shells, flexible membranes), the mathematical model treats them as surface elements located at the mid-surface of the real structures.

**Three-dimensional** or **solid elements** are used to model spatial structures and solids. Each **joint element**, in the mathematical model, has its own symbol as is discussed in the next picture.

The element types, in turn, represent physical action in the real member as indicated in Fig. 3.2, such as for,

**LINE ELEMENTS:** **axial systems** (e.g. truss, tie, cable), **flexural systems** (e.g. beams), **axial-flexure systems** (e.g. beam-columns, frames, arches),

**SURFACE ELEMENTS:** **axial systems** (e.g. tensile membranes, **axial-shear systems** (e.g. thin shells), **axial flexure systems** (e.g. plates, walls, slabs, thick shells)).

In SAP2000, the physical members are represented by objects, where the geometry of the object corresponds to that of the member. The typical objects are as follows:

- **Point objects:** (1) joint objects, which are automatically created at the ends or corners of objects,  
(2) support objects
- **Line objects:** (1) frame/cable/tendon objects,  
(2) link objects
- **Area objects:** shell type objects
- **Solid objects**

By assigning properties and loads to the object, the physical member is modeled. SAP2000 converts automatically the object-based model into a finite element-based model that is used for analysis.

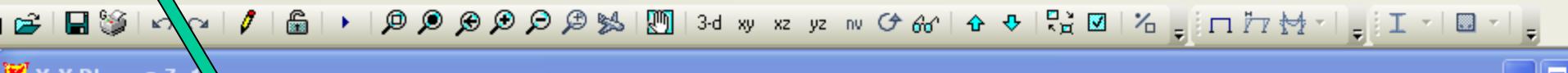
# FRAME MEMBER SECTIONS

**Frame or shell sections** (section properties such as outline, reinforcement, cross-sectional area, moment of inertia, etc.)

Standard structural steel shapes are the wide-flange shape (**W**), American standard beams (**S**), miscellaneous shapes (**M**), bearing pile shapes (**HP**), channel shapes (**C**, **MC**), equal leg and unequal leg angles (**L**), structural tees split from **W**, **M** and **S** shapes (**WT**, **MT** and **ST**), round steel pipe and structural tubing (**TS**).

The typical geometric section properties for prismatic (i.e. not tapered members) used in structural engineering software are:

- **section dimensions**
- **cross-sectional area ( $A$ )**
- **moment of inertia ( $I_x$ ,  $I_y$ )**
- **section modulus ( $S_x$ ,  $S_y$ )**
- **radius of gyration ( $r_x$ ,  $r_y$ ,  $r_z$ )**
- **plastic section modulus ( $Z_x$ ,  $Z_y$ )**
- **shear area ( $A_{sx}$ ,  $A_{sy}$ )**
- **torsional constant ( $J$ )**



X-Y Plane @ Z=1

Object Model

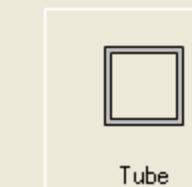
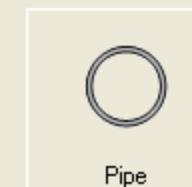
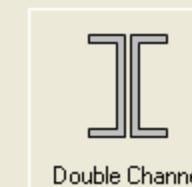
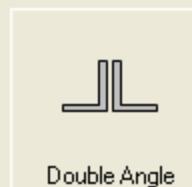
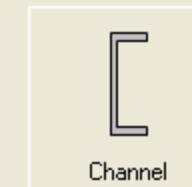
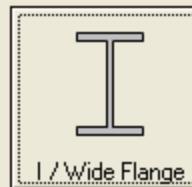
### Add Frame Section Property

Select Property Type

Frame Section Property Type

Steel

Click to Add a Steel Section



Cancel

Plane @ Z=12

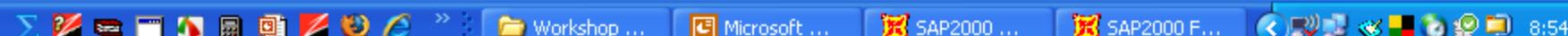
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X-2.92 Y19.46 Z12.00

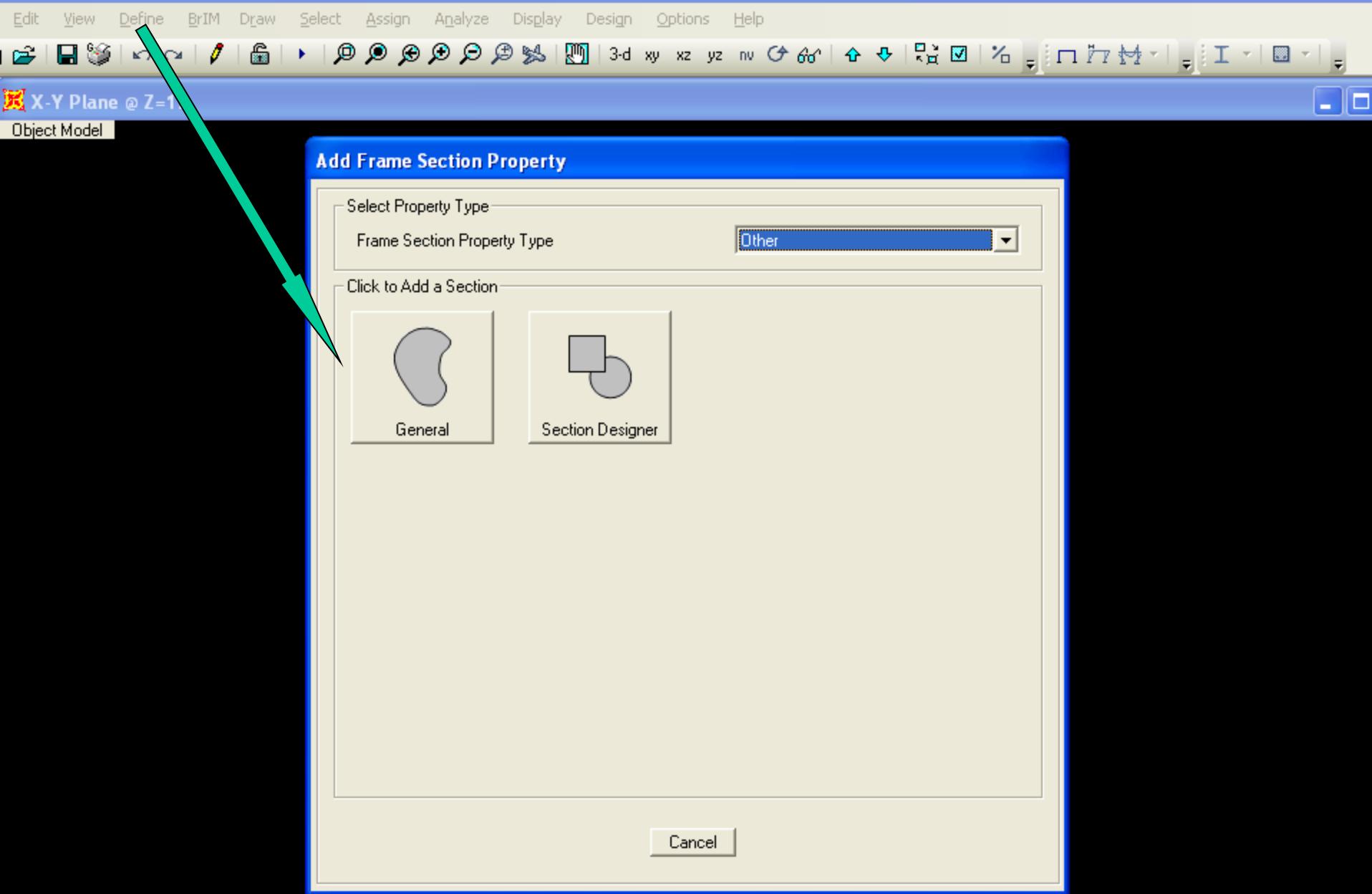
GLOBAL

KN, m, C

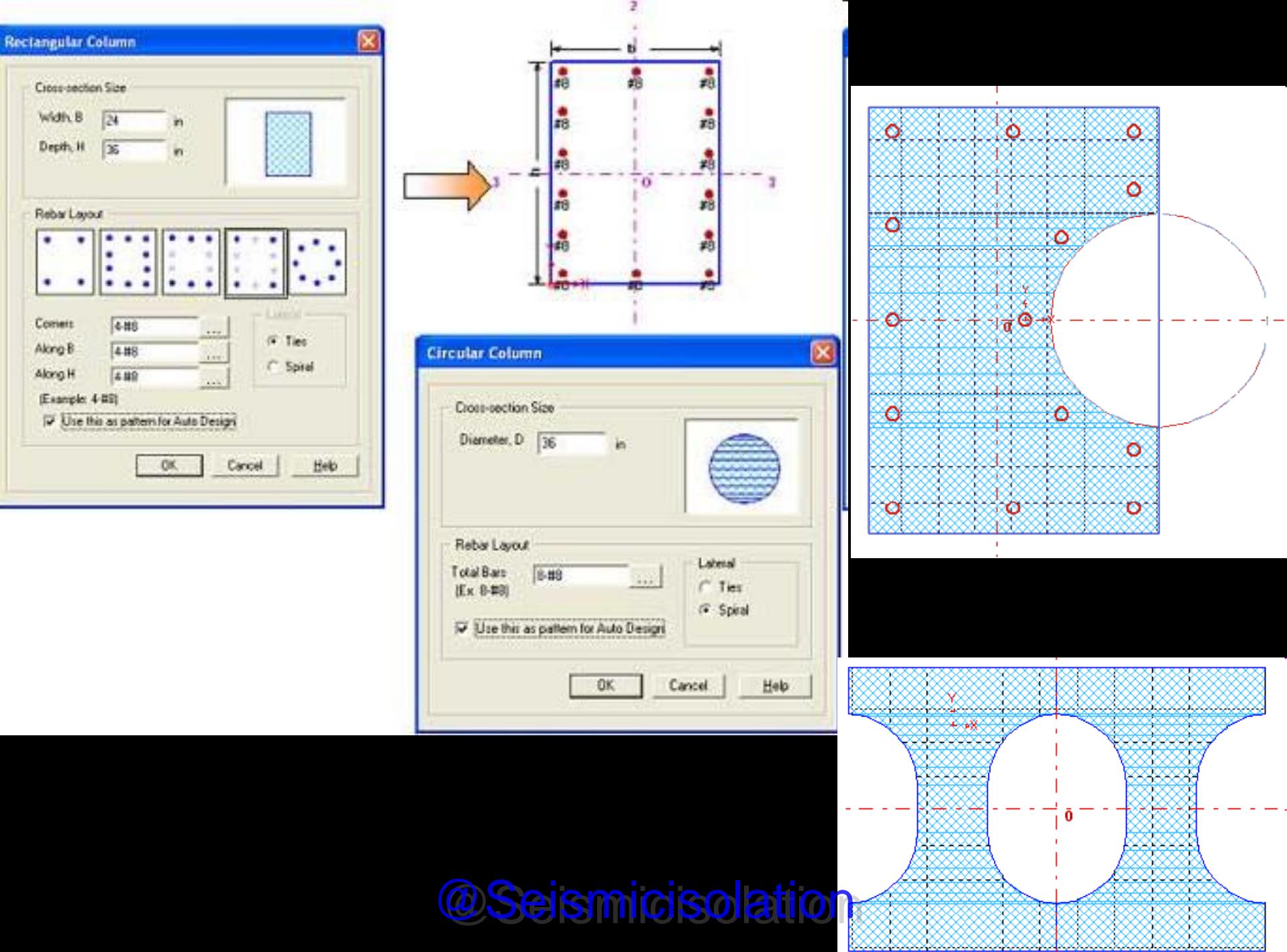
start



8:54

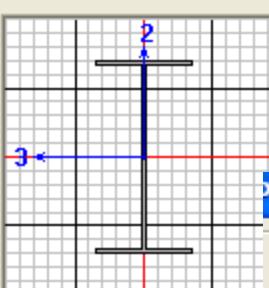


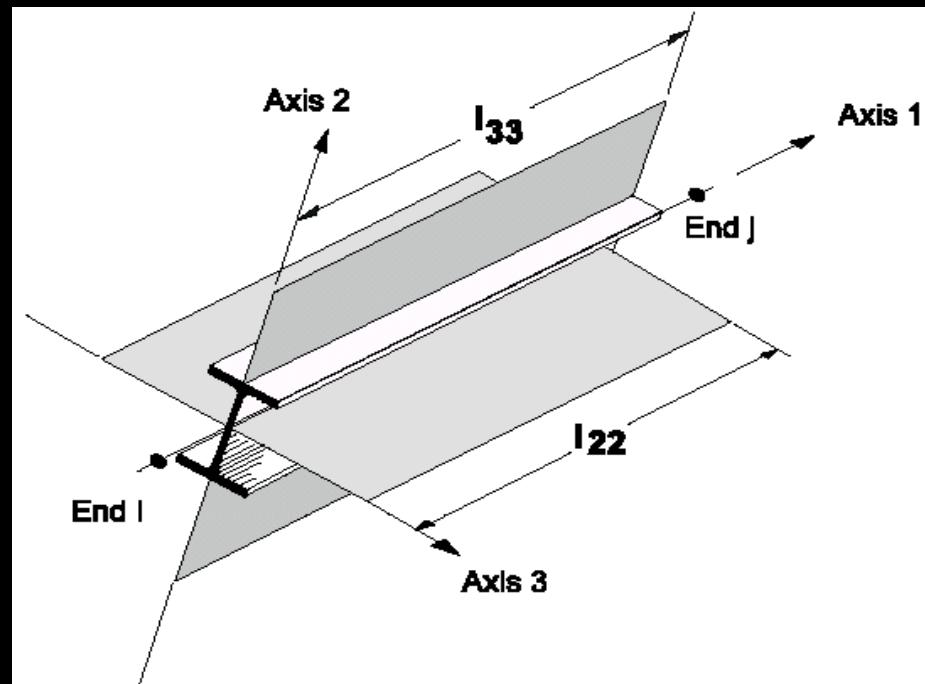
**FRAME ELEMENTS** in general.  
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# STEEL MEMBER PROPERTIES

**Wide Flange Section**

Section Name	GB-HN300X150X5.5X8
Section Notes	<a href="#">Modify/Show Notes...</a>
Extract Data from Section Property File	
<a href="#">Open File...</a>	c:\program files\computers and
<a href="#">Import...</a>	
Properties	
<a href="#">Section Properties...</a>	<a href="#">Property Modifiers...</a>
Material	
<a href="#">+</a> Q235	
Dimensions	
Outside height (t3)	298.
Top flange width (t2)	149.
Top flange thickness (tf)	8.
Web thickness (tw)	5.5
Bottom flange width (t2b)	149.
Bottom flange thickness (tfb)	8.
	
Display Color	<input type="checkbox"/>

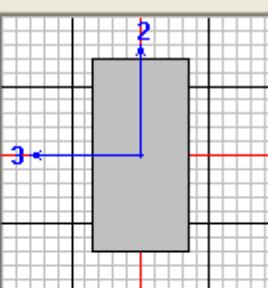


**Property Data**

Section Name	GB-HN300X150X5.5X8		
Properties			
Cross-section (axial) area	4155.	Section modulus about 3 axis	433557.
Torsional constant	64585.43	Section modulus about 2 axis	59463.09
Moment of Inertia about 3 axis	64600000	Plastic modulus about 3 axis	457225.5
Moment of Inertia about 2 axis	4430000.	Plastic modulus about 2 axis	91057.63
Shear area in 2 direction	1639.	Radius of Gyration about 3 axis	124.6897
Shear area in 3 direction	1986.6666	Radius of Gyration about 2 axis	32.6525

# CONCRETE MEMBER PROPERTIES

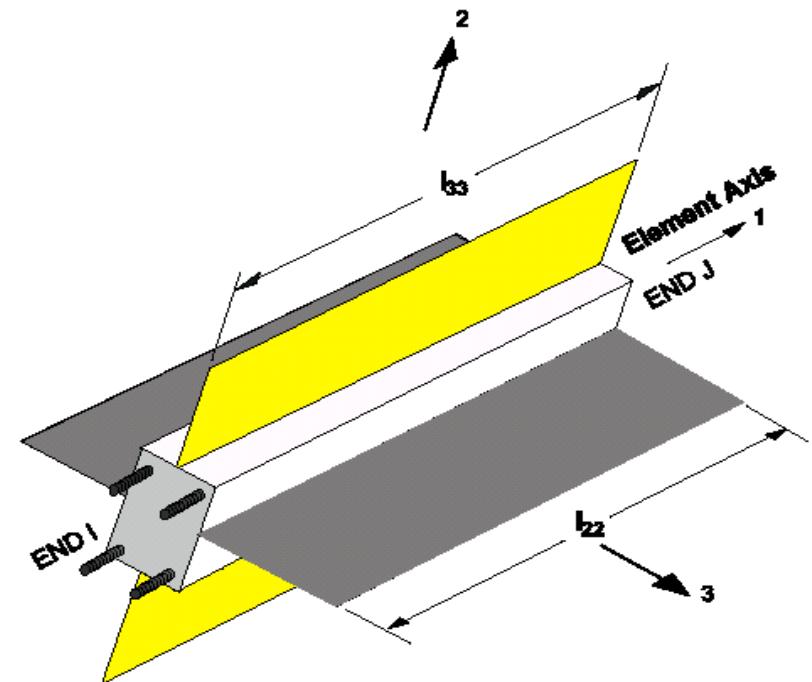
## Rectangular Section

Section Name	CONBM
Section Notes	<a href="#">Modify/Show Notes...</a>
Properties	Property Modifiers
<a href="#">Section Properties...</a>	<a href="#">Set Modifiers...</a>
Material	+ C30
Dimensions	
Depth (t3)	500.
Width (t2)	250.
 Display Color <span style="background-color: gray; border: 1px solid black; padding: 2px;"> </span>	
<a href="#">Concrete Reinforcement...</a>	

## Property Data

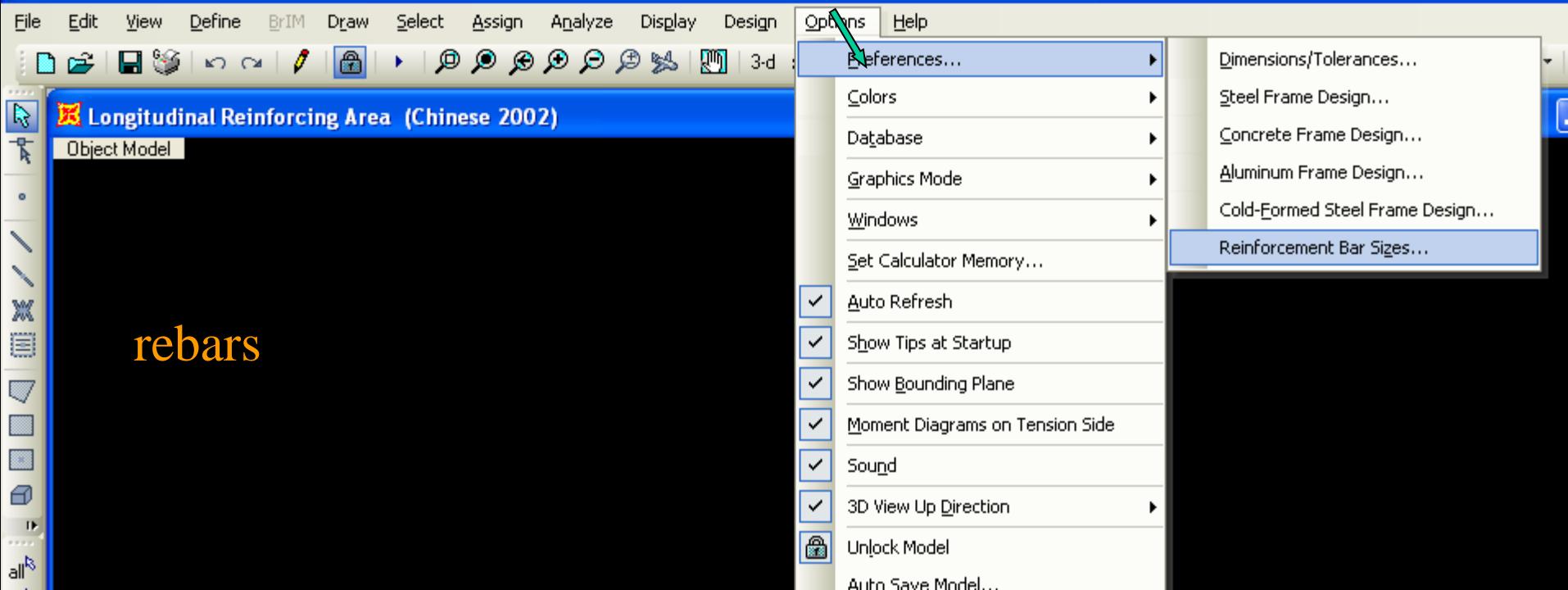
Section Name	CONBM
Properties	
Cross-section (axial) area	125000.
Torsional constant	1.788E+09
Moment of Inertia about 3 axis	2.604E+09
Moment of Inertia about 2 axis	6.510E+08
Shear area in 2 direction	104166.67
Shear area in 3 direction	104166.67
Section modulus about 3 axis	10416667
Section modulus about 2 axis	5208333.
Plastic modulus about 3 axis	15625000
Plastic modulus about 2 axis	7812500.
Radius of Gyration about 3 axis	144.3376
Radius of Gyration about 2 axis	121.688

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## Reinforcement Data

Rebar Material	Longitudinal Bars	+ HRB400
	Confinement Bars (Ties)	+ HRB400
Design Type	<input type="radio"/> Column (P-M2-M3 Design) <input checked="" type="radio"/> Beam (M3 Design Only)	
Concrete Cover to Longitudinal Rebar Center		
Top	60.	
Bottom	60.	
Reinforcement Overrides for Ductile Beams		
Left	Top	Right
	0.	0.
Bottom	0.	0.



rebars

Nominal Dimensions						
Bar Size <sup>a</sup> (SI) <sup>b</sup>	Diameter in mm	Cross-Sect. Area in <sup>2</sup> mm <sup>2</sup>	Weight lbs/ft kg/m	Mass kg/m		
#3 #10	0.375 9.5	0.11 71	0.376	0.560		
#4 #13	0.500 12.7	0.20 129	0.668	0.944		
#5 #16	0.625 15.9	0.31 199	1.043	1.552		
#6 #19	0.750 19.1	0.44 284	1.502	2.235		
#7 #22	0.875 22.2	0.60 387	2.044	3.042		
#8 #25	1.000 25.4	0.79 510	2.670	3.973		
#9 #29	1.128 28.7	1.00 645	3.400	5.060		
#10 #32	1.270 32.3	1.27 819	4.303	6.404		
#11 #36	1.410 35.8	1.56 1006	5.313	7.907		

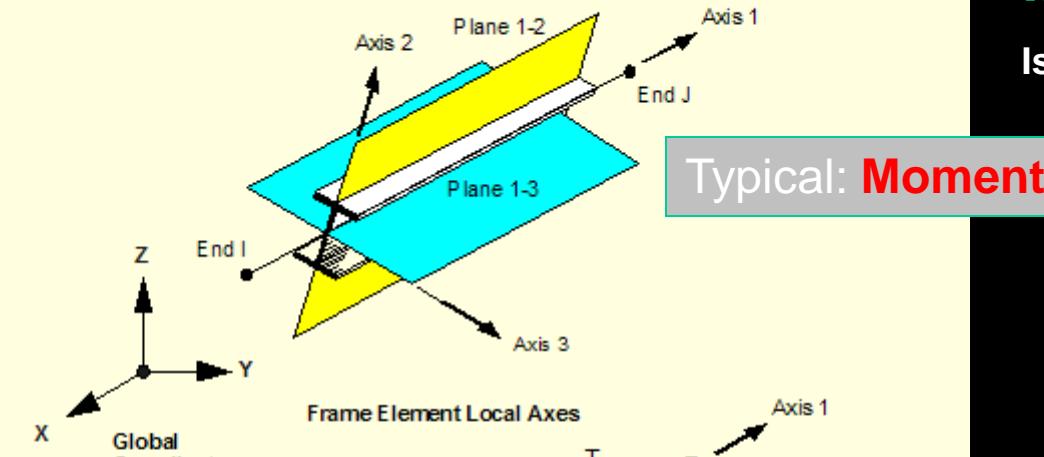
10M	100.	11.3
15M	200.	16
20M	300.	19.5
25M	500.	25.2
30M	700.	29.9
35M	1000.	35.7
45M	1500.0001	43.7
55M	2500.0001	56.4
6d	28.3	6

Rebar	Bar ID	Bar Area	Bar Diameter
#2	32.258	6.35	
6d	28.3	6	
8d	50.3	8	
10d	78.5	10	
12d	113.	12	
14d	154.	14	
16d	201.	16	
20d	314.	20	
25d	491.	25	
26d	531.	26	

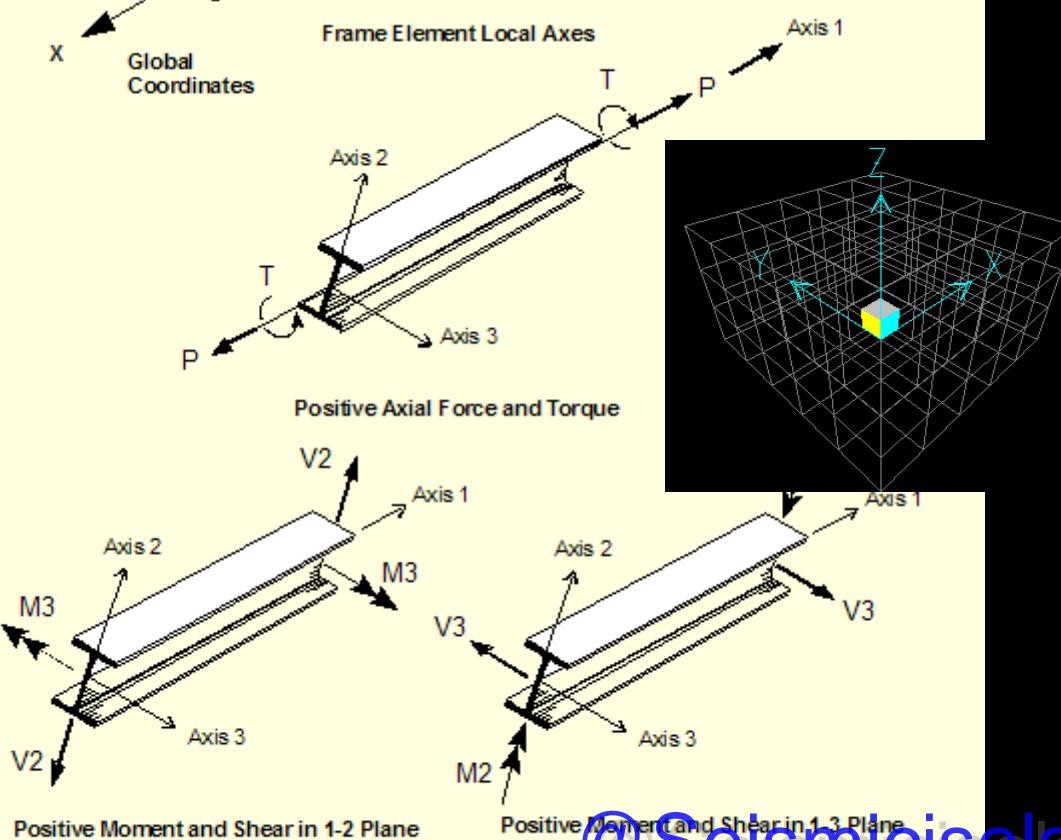
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## Frame and Nllink Element

FRAME element follows the designer's convention.



Typical: **Moment 3-3, Shear 2-2**



Frame Element Internal Forces

# MEMBER ORIENTATION

Is defined by local coordinate system

Each part of the structure (e.g. joint, element) has its own **LOCAL** coordinate system **1-2-3**.

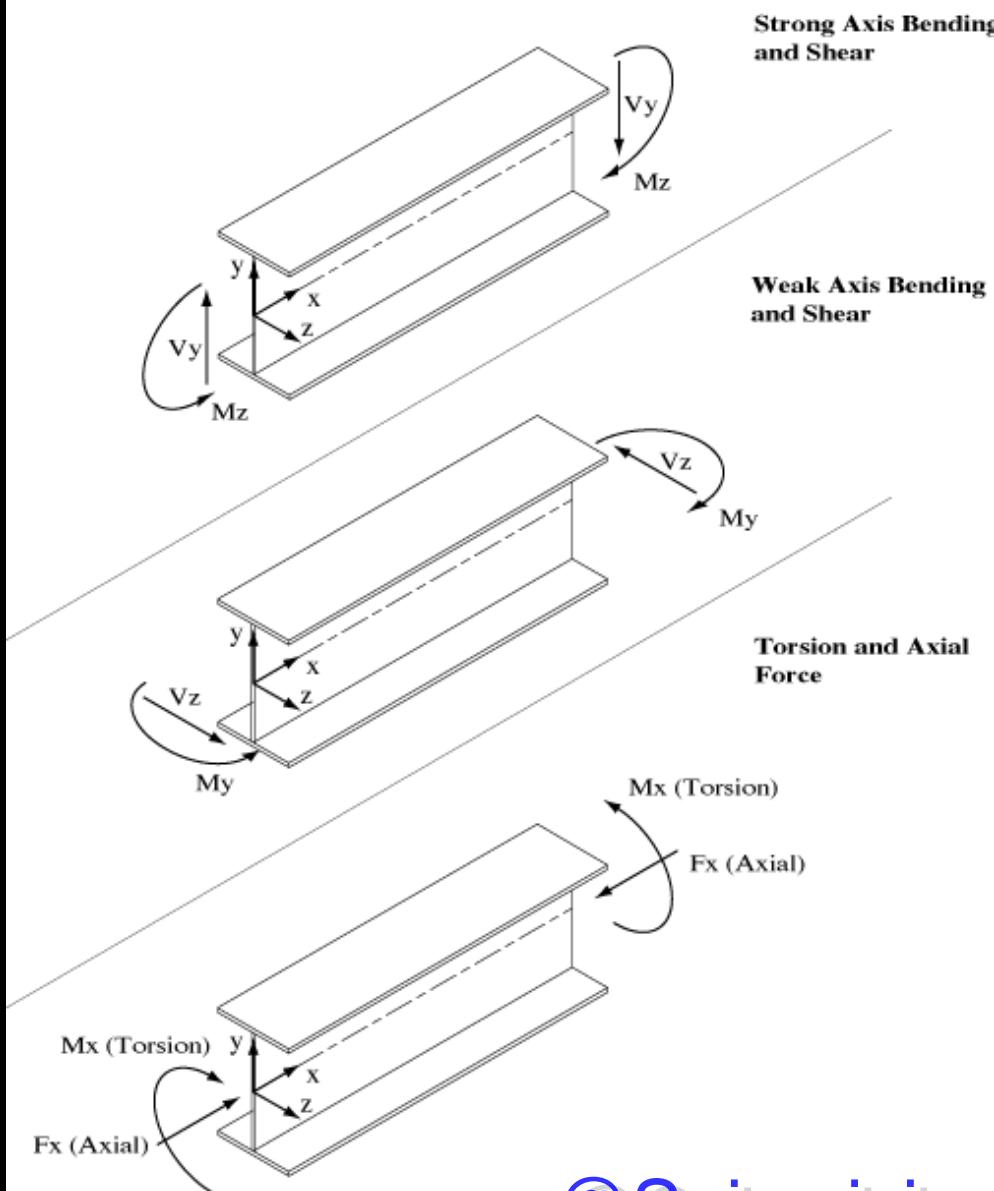
The **joint local coordinate system** is normally the same as the global X-Y-Z coordinate system.

For the **elements**, one of the element local axes is determined by the geometry of the individual element; the orientation of the remaining two axes is defined by specifying a single angle of rotation.

For frame elements, for example, the **local axis 1 is always the longitudinal axis of the element with the positive direction from I to J**. The default orientation of the local **1-2** plane in SAP is taken to be vertical (i.e. parallel to the Z-axis). The local **3**-axis is always horizontal (i.e. lies in the X-Y plane).

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## Force Sign Conventions for 3D Beam-Column Elements

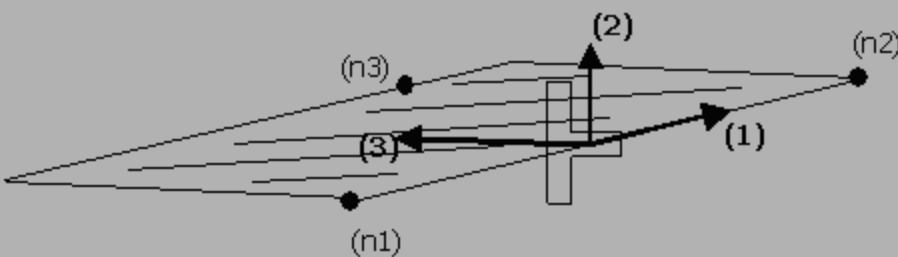
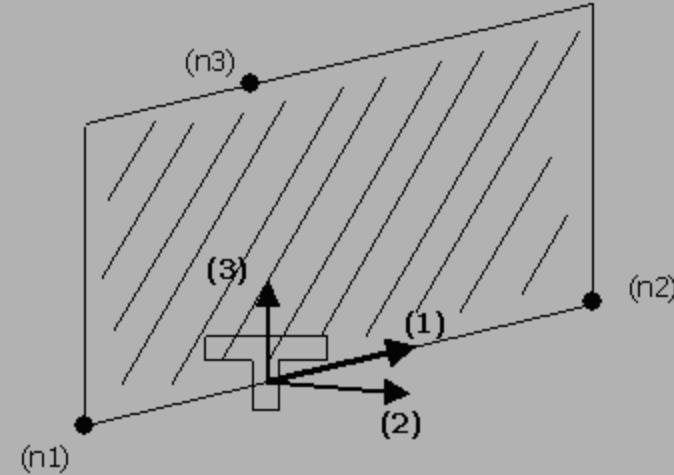
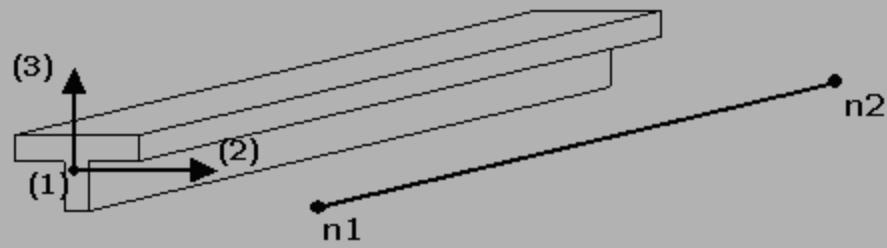


# INTERNAL MEMBER FORCES

A member has six potential internal force quantities in three dimensional space. Internal moments are defined as positive according to the **right-hand rule** acting at the i-node end.

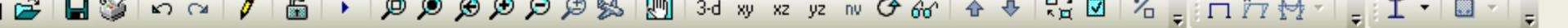
In two dimensional or planar space, there are only three force quantities:

- **Bending moment**
- **Shear**
- **Axial force.**



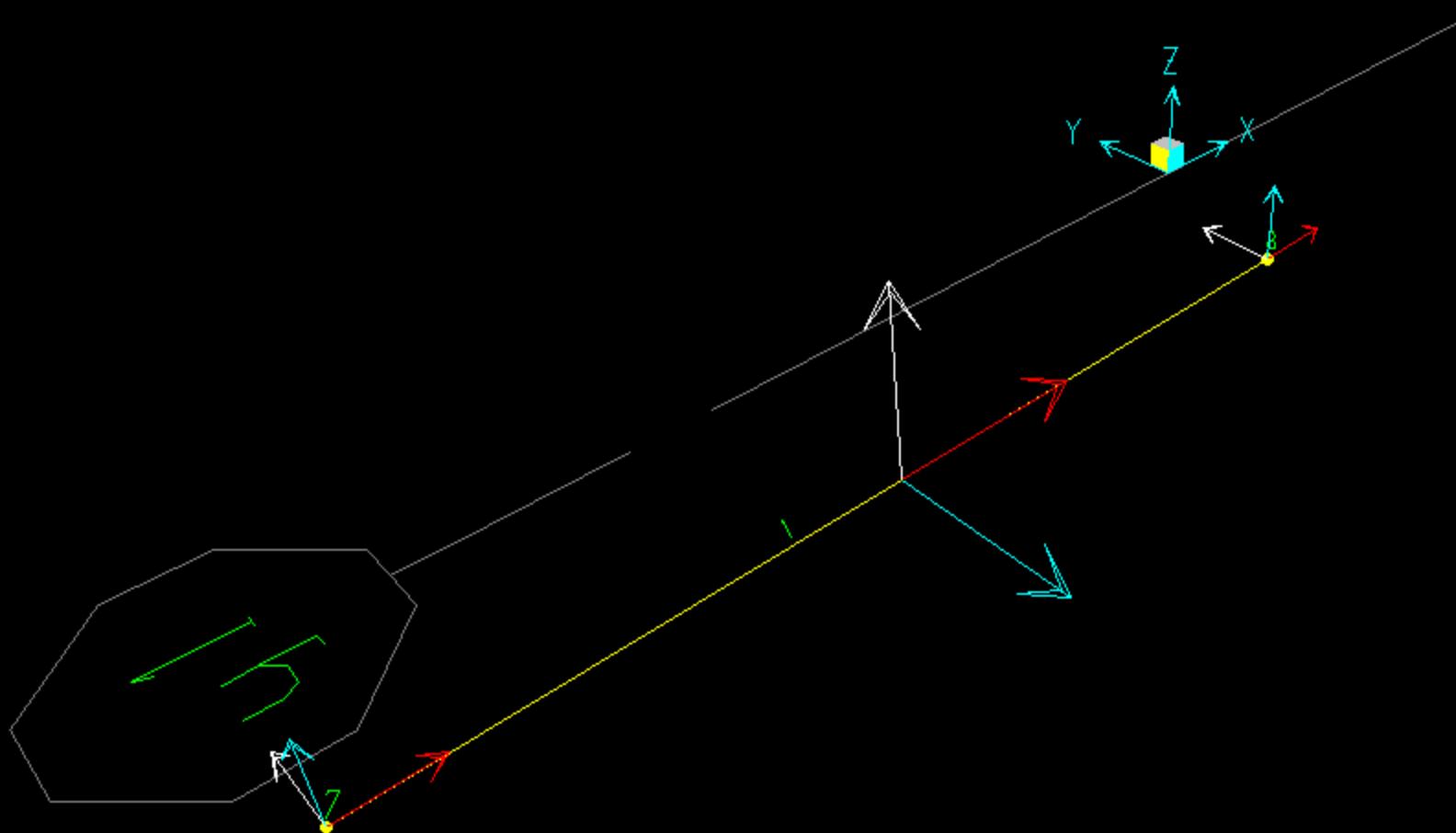
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Edit View Define BrIM Draw Select Assign Analyze Display Design Options Help



3-D View

Object Model

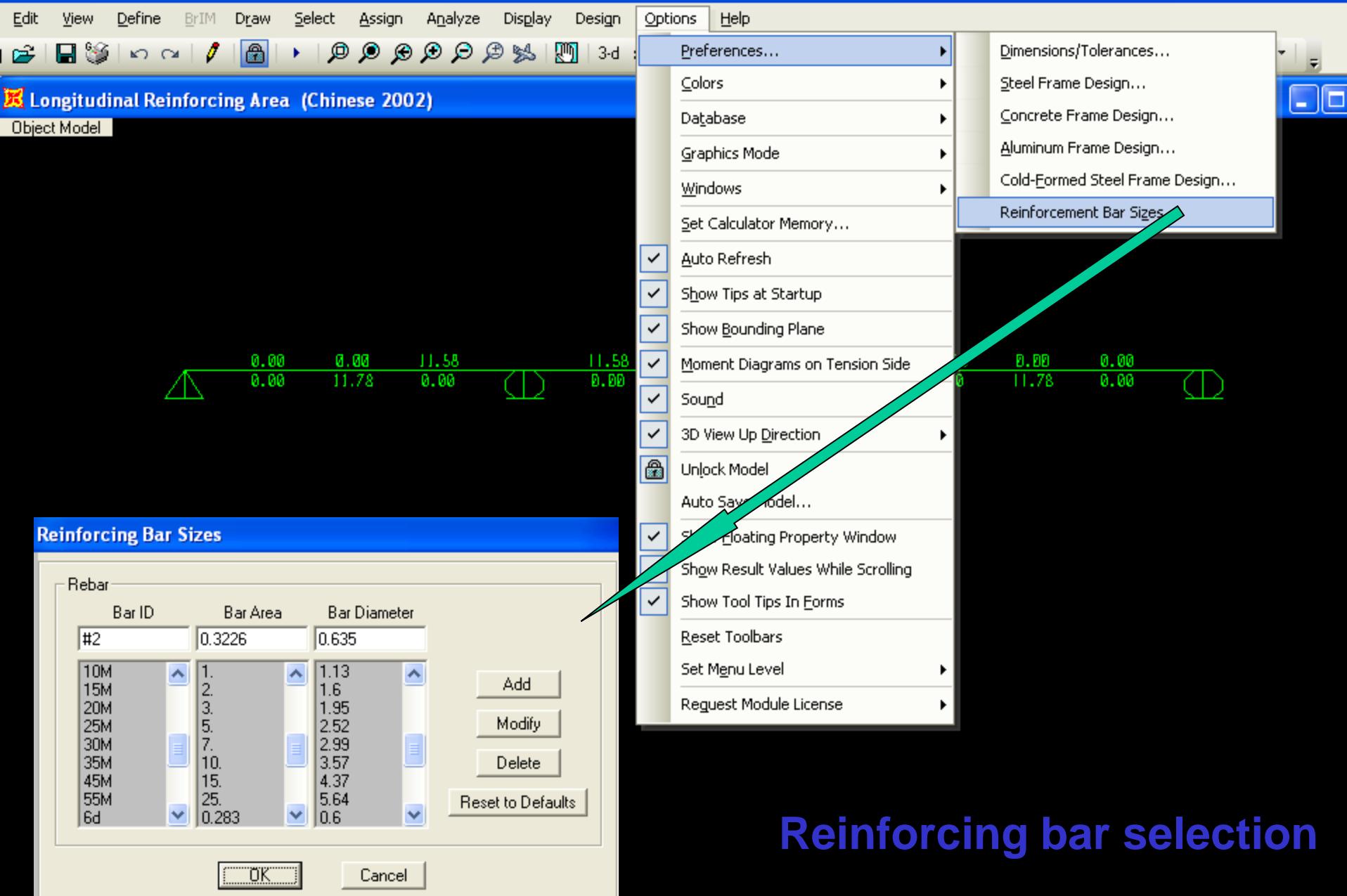


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X0.00 Y0.00 Z20.00

GLOBAL

KN, m, C



Reinforcing bar selection

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# LOADS

**Joint (nodal) loads, element loads** (concentrated forces and moments, and distributed loads on frames and surfaces): **static loads** (e.g. gravity, pressure, prestress, torsional, thermal) and **displacements** applied to nodes or in elements, **thermal loads** and other *indirect loads* due to *creep, shrinkage or settlement, moving loads, vibrational loads, seismic loads*; sloped loads vs. projected loads, **local loads** versus **global loads**.

**Self-weight** is included automatically in structural software programs

The common loads used in structural engineering software correspond to the typical loading conditions.

**Dead load** and **self-weight**

**Live load**

**Snow load, roof live load**

**Wind and earthquake loads**

**Water and earth pressure loads**

**Loads due to restrained volume change:**

**thermal loads, joint and support displacement loads, prestress loads**

**Dynamic loads** including **time-history loads**

**Abnormal loads** including **accidental loads**

**Moving vehicle and lane loading**

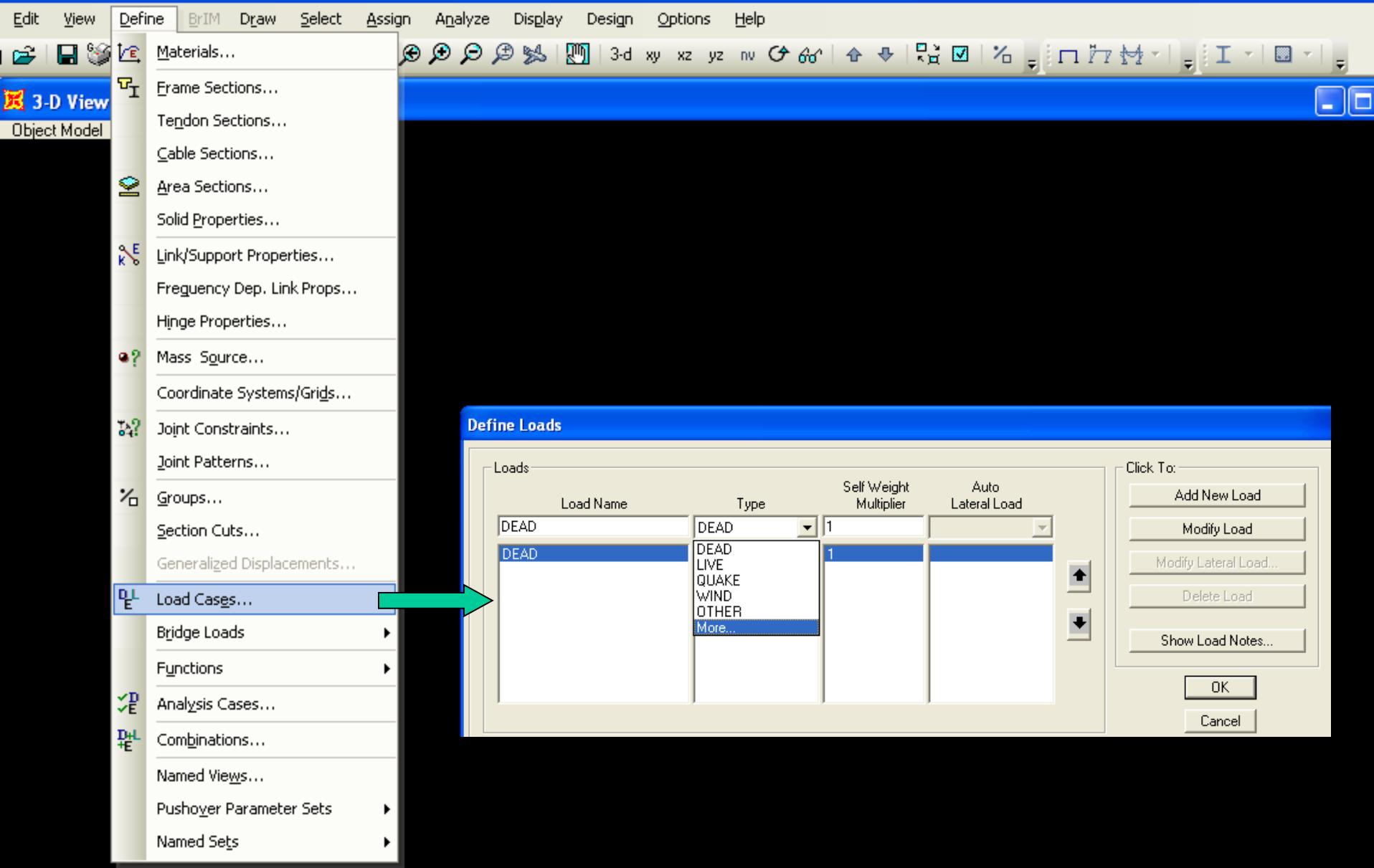
Some of the structural engineering software provides automatic wind load generation from user-specified wind intensity as well as UBC Seismic load generation for calculation and automatic distribution of base shear according to code stipulation

## Introduction to:

- (i) Self-weight of the structure
- (ii) Vertical loads (dead loads and live loads)
- (iii) Horizontal loads (wind loads)
- (iv) Total loads

## Typical loads

	Dead loads kN/m <sup>2</sup> (kPa) psf	Live loads kN/m <sup>2</sup> psf	Snow loads kN/m <sup>2</sup> psf	Wind loads kN/m <sup>2</sup> psf
Floors	3.0 - 4.0 60 - 80	2.0 - 4.0 40-80	#	#
Roofs	2.00 - 2.50 40 - 50	1.0 – 2.0 20 - 40	1.00 20	#
Walls	#	#	#	1.0 - 1.5 20 - 30

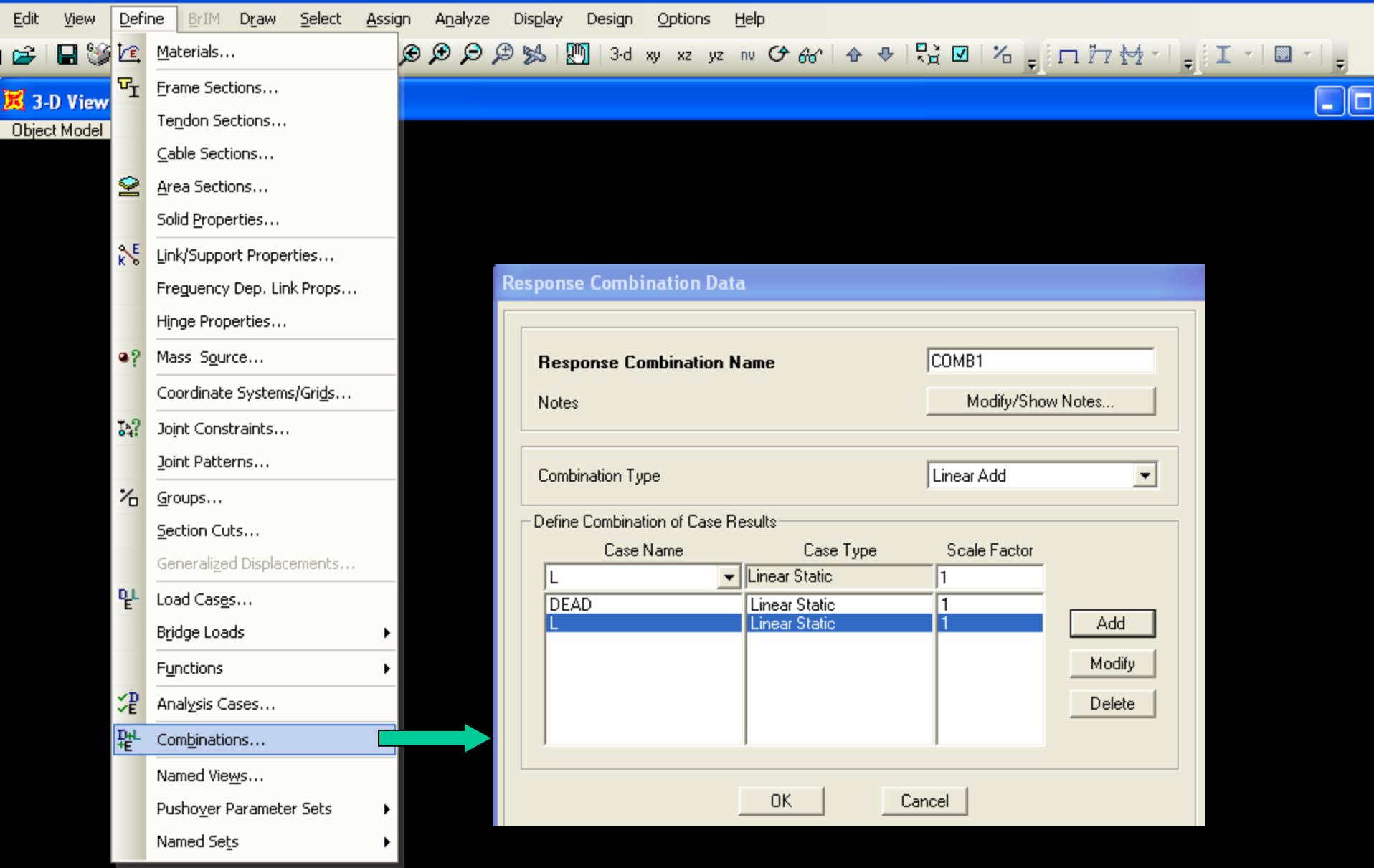


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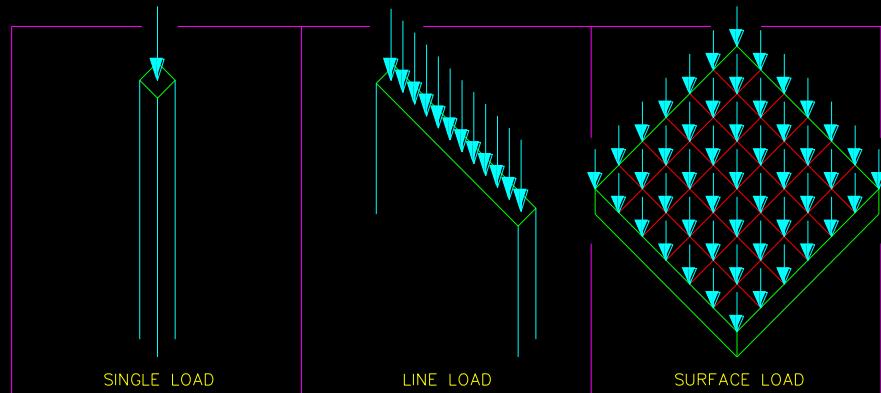
# LOAD COMBINATION

In structural engineering software programs the user defines first the various load cases and then may set up the **load combinations** depending on the type of program. It may be the responsibility of the user to determine which combination will control the design.

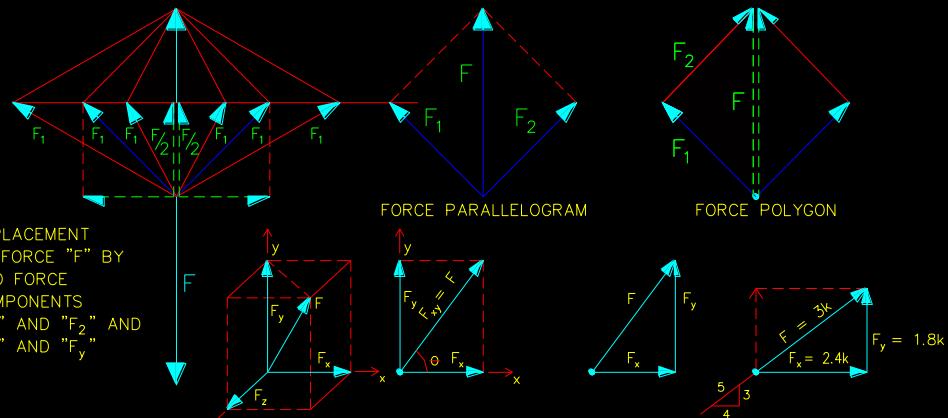
Some programs assemble automatically basic load cases into load combinations; they define also the load factors for each load according to codes and type of analysis. In other words, they provide already sets of default design load combinations so that the user only defines the required combinations according to the selected code (e.g. model codes, **ASCE 7-98**, **AISC-ASD 89**, **AISC-LRFD 99**, **ACI 318-99**, **NDS 1997**). Then the program may select automatically the critical load combination for design.



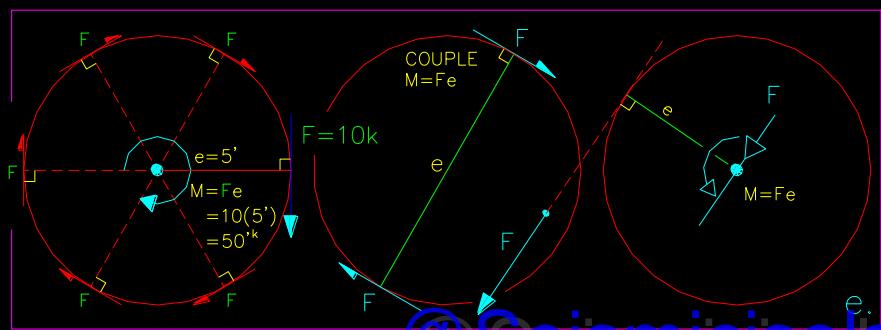
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a.



b.



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## Properties of Forces

- magnitude
- direction
- location

# BOUNDARY CONDITIONS

: **nodal restraints** (frame end releases),  
**external supports**

The deflection of the structural model is governed by the displacements of the nodes where **every node may have six displacement components**. The node may **translate** along its three local axes as denoted by **U1, U2, U3**, and it may **rotate** about its three local axes as denoted by **R1, R2, R3**. The **six** displacement components are known as the **degrees of freedom** of the node. For **planar structures** you may want to restrict the available degrees of freedom to **three** such as for the **vertical X-Z** or **horizontal X-Y planes**.

Each degree of freedom must be one of the following types:

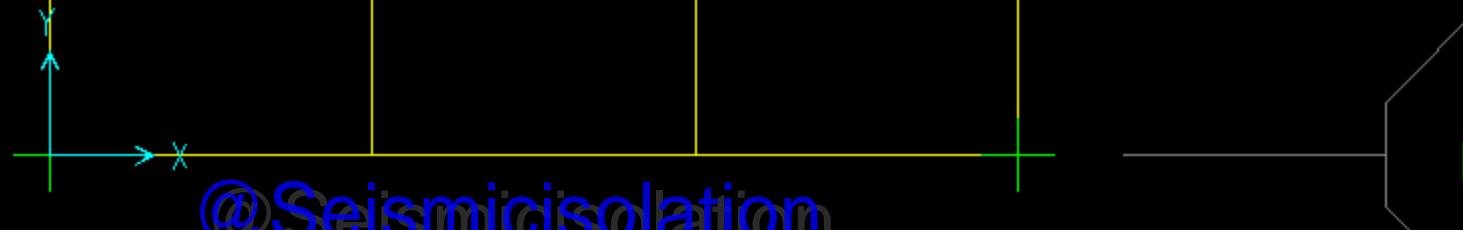
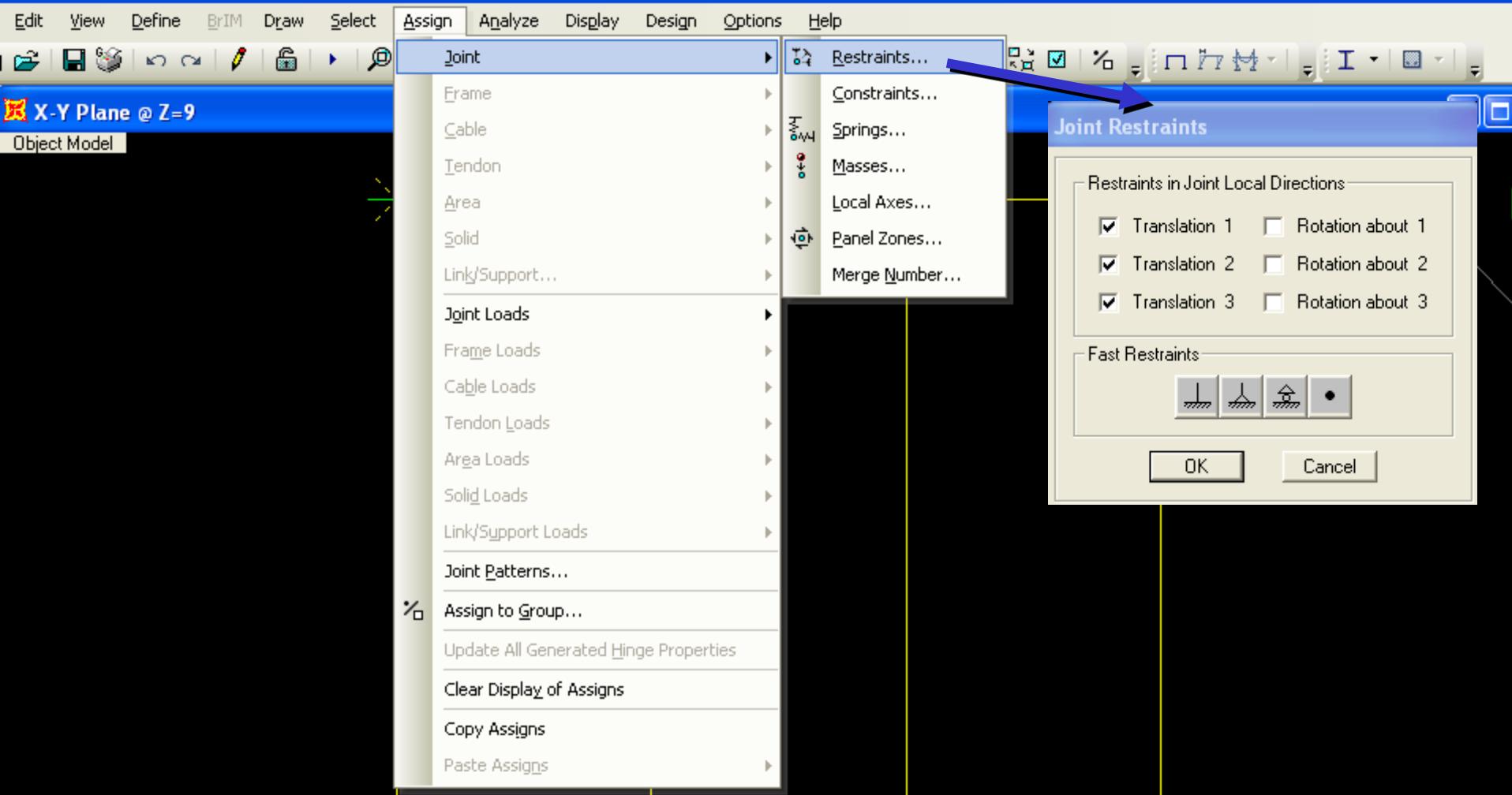
**Active**, where the displacement is computed during the analysis

**Restrained**, where the displacement is specified, and the corresponding **reaction** is computed during the analysis (e.g. **external supports**)

**Constrained**, where the displacement is determined from the displacements at other degrees of freedom, as applied to rigid body behavior (e.g. **rigid diaphragms**) to connect together different parts of the model, and to impose certain types of symmetry conditions. Using constraints reduces the number of degrees of freedom in the model, which can make the analysis run faster.

**Null**, where the displacement does not affect the structure and is ignored by the analysis

**Unavailable**, where the displacement has been explicitly excluded from the analysis



## B. CHECK INPUT DATA

## C. ANALYSIS

The **ANALYSIS** determines the **reactions**, **internal forces**, and **displacements** of the model in response to the **loads**.

Three key concepts are underlying the theory of structural analysis:

- **equilibrium of forces**
- **relationship between forces and displacements**
- **compatibility of displacements**

Depending on the loading conditions and type of structure either a **static** or **dynamic analysis** must be performed. Some structural engineering software also offer in addition other types of analyses for special situations such as: **moving load analysis**, **buckling analysis**, and **fatigue analysis**.

**TYPE OF ANALYSIS:** many different types of analysis are available such as the common given below

## **STATIC ANALYSIS:**

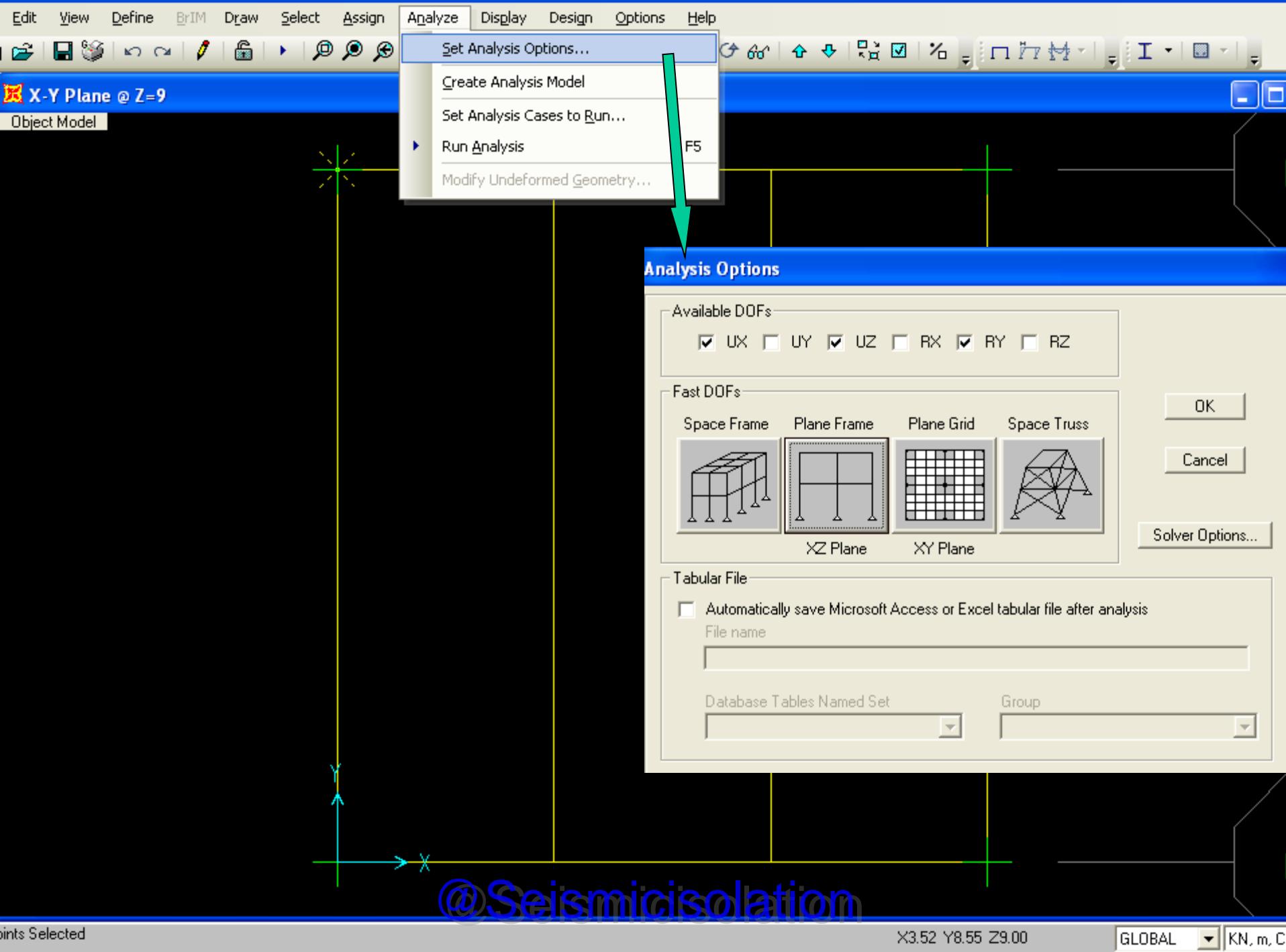
- **Linear Static Analysis**
- **Nonlinear Static Analysis:**
  - P-Delta Analysis
  - Pushover Analysis
- **Seismic Analysis: Equivalent Lateral Force Analysis**

## **DYNAMIC ANALYSIS:**

- **Modal Analysis** for vibration modes:
  - Eigenvalue Analysis
- **Harmonic Steady-State Analysis**
- **Seismic Analysis:**
  - Modal Response-Spectrum Analysis
  - Linear Time-history Analysis
  - Nonlinear Time-history Analysis

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Other load analyses include: MOVING LOAD ANALYSIS, LINEAR BUCKLING-MODE ANALYSIS



Design Options Help

Steel Frame Design

Concrete Frame Design

Aluminum Frame Design

Cold-Formed Steel Frame Design

Lateral Bracing

Overwrite Frame Design Procedure...

Select Design Groups...

Select Design Combos...

**View/Revise Overwrites...**

Set Displacement Targets...

Set Time Period Targets...

Start Design/Check of Structure

Interactive Steel Frame Design

Display Design Info...

Make Auto Select Section Null...

Change Design Section...

Reset Design Section to Last Analysis...

Verify Analysis vs Design Section...

Verify all Members Passed...

Reset All Steel Overwrites...

Delete Steel Design Results...

Item Description

Unbraced Length Ratio (Minor, LTB): Unbraced length factor for buckling about the frame object minor axis. This item is specified as a fraction of the frame object length. This factor times the frame object length gives the unbraced length for the object. Specifying 0 means the value is program determined. This factor is also used for determining the length for lateral-torsional buckling.

For symmetrical sections minor bending is bending about the local 2-axis. For unsymmetrical sections (e.g., angles) minor bending is the bending about the section principal axis with the smaller moment of inertia.

Explanation of Color Coding for Values

**Blue:** All selected items are program determined

**Black:** Some selected items are user defined

**Red:** Value that has changed during the current session

Moment 3-3 Diagram (COMB1)

Analysis Model

Frame Design Overwrites for AISC-ASD89

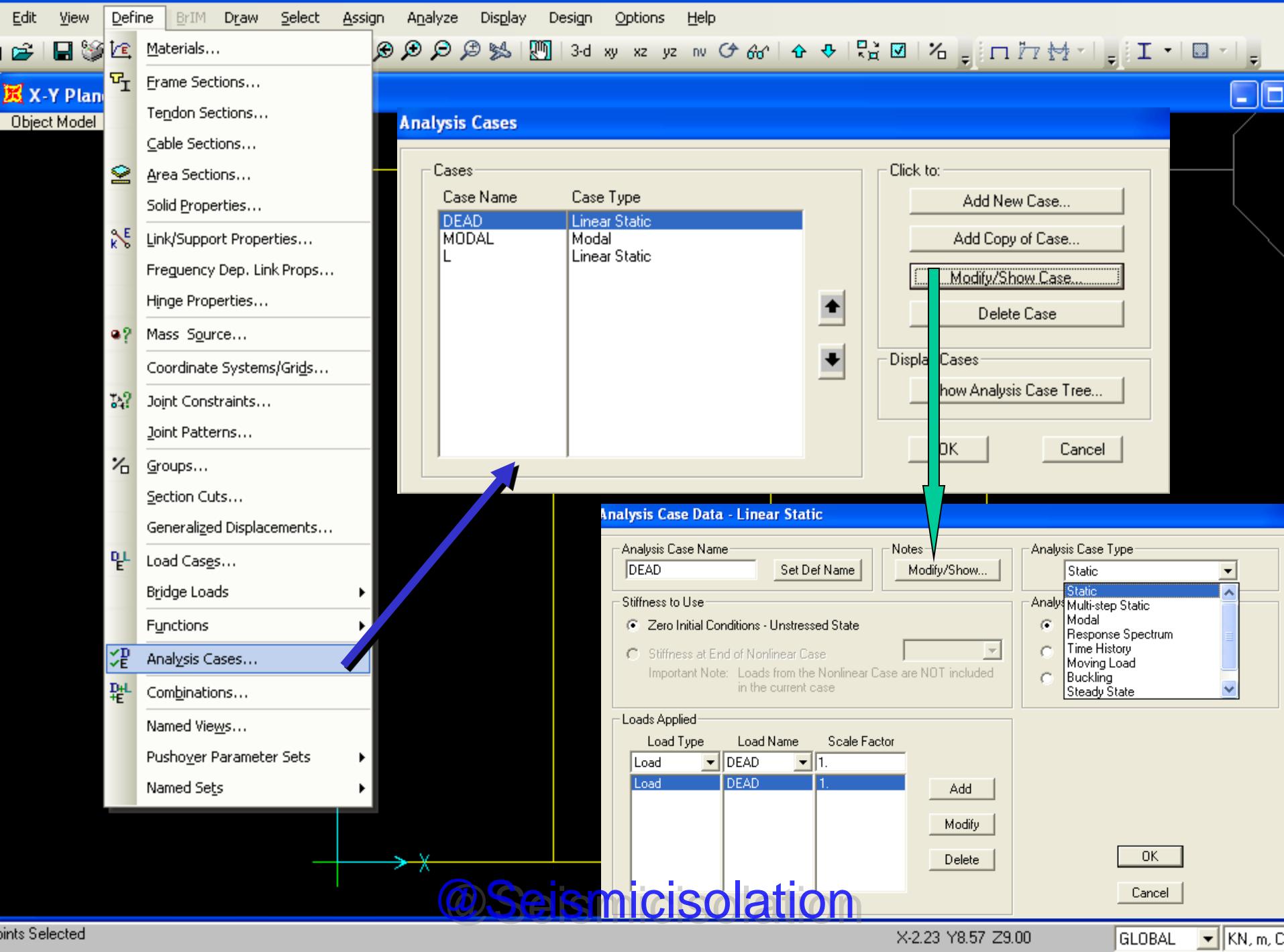
Item	Value
1 Current Design Section	Program Determined
2 Framing Type	Program Determined
3 Consider Deflection?	No
4 Deflection Check Type	Program Determined
5 DL Limit, L /	Program Determined
6 Super DL+LL Limit, L /	Program Determined
7 Live Load Limit, L /	Program Determined
8 Total Limit, L /	Program Determined
9 Total-Camber Limit, L /	Program Determined
10 DL Limit, abs	Program Determined
11 Super DL+LL Limit, abs	Program Determined
12 Live Load Limit, abs	Program Determined
13 Total Limit, abs	Program Determined
14 Total-Camber Limit, abs	Program Determined
15 Specified Camber	Program Determined
16 Net Area to Total Area Ratio	Program Determined
17 Live Load Reduction Factor	Program Determined
18 Unbraced Length Ratio (Major)	Program Determined
19 Unbraced Length Ratio (Minor, LTB)	0.1
20 Effective Length Factor (K Major)	Program Determined
21 Effective Length Factor (K Minor)	Program Determined
22 Moment Coefficient (Cm Major)	Program Determined
23 Moment Coefficient (Cm Minor)	Program Determined
24 Bending Coefficient (Cb)	Program Determined

All Items Selected Items

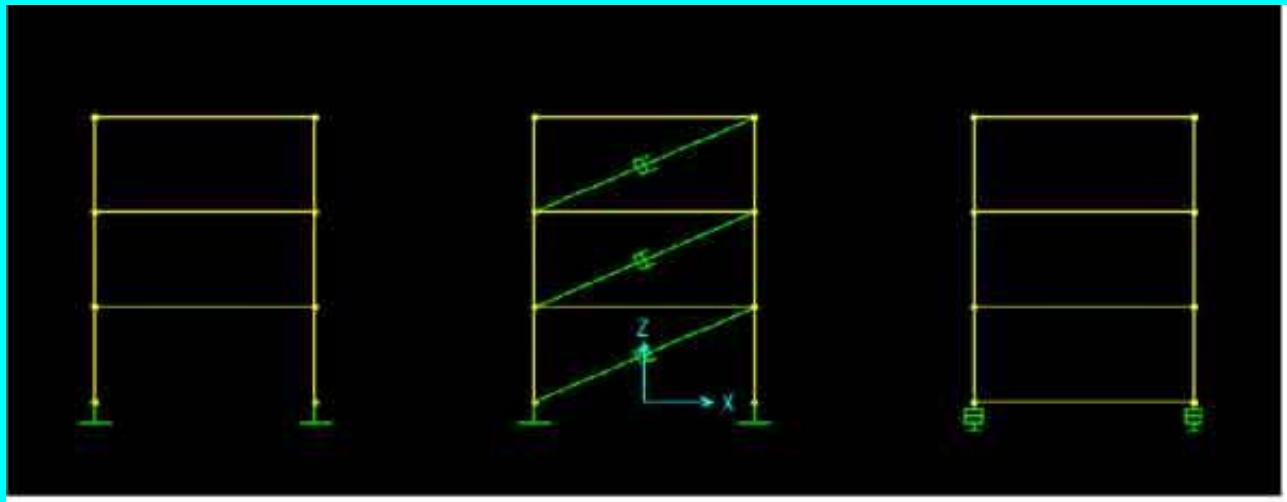
All Items Selected Items

GLOBAL KN, m, C

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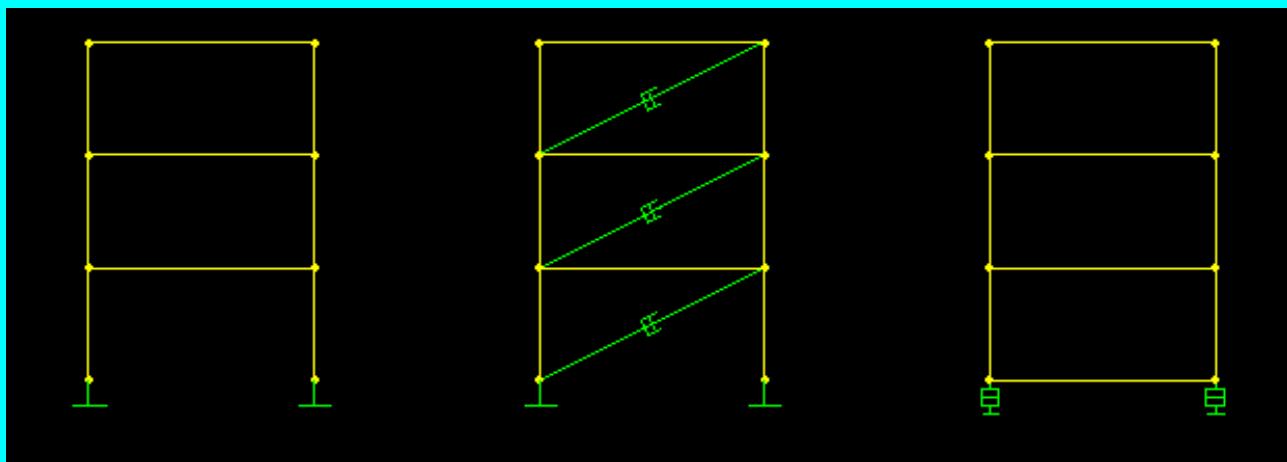
# Static linear & nonlinear analysis



Linear Structure

Structure with Dampers

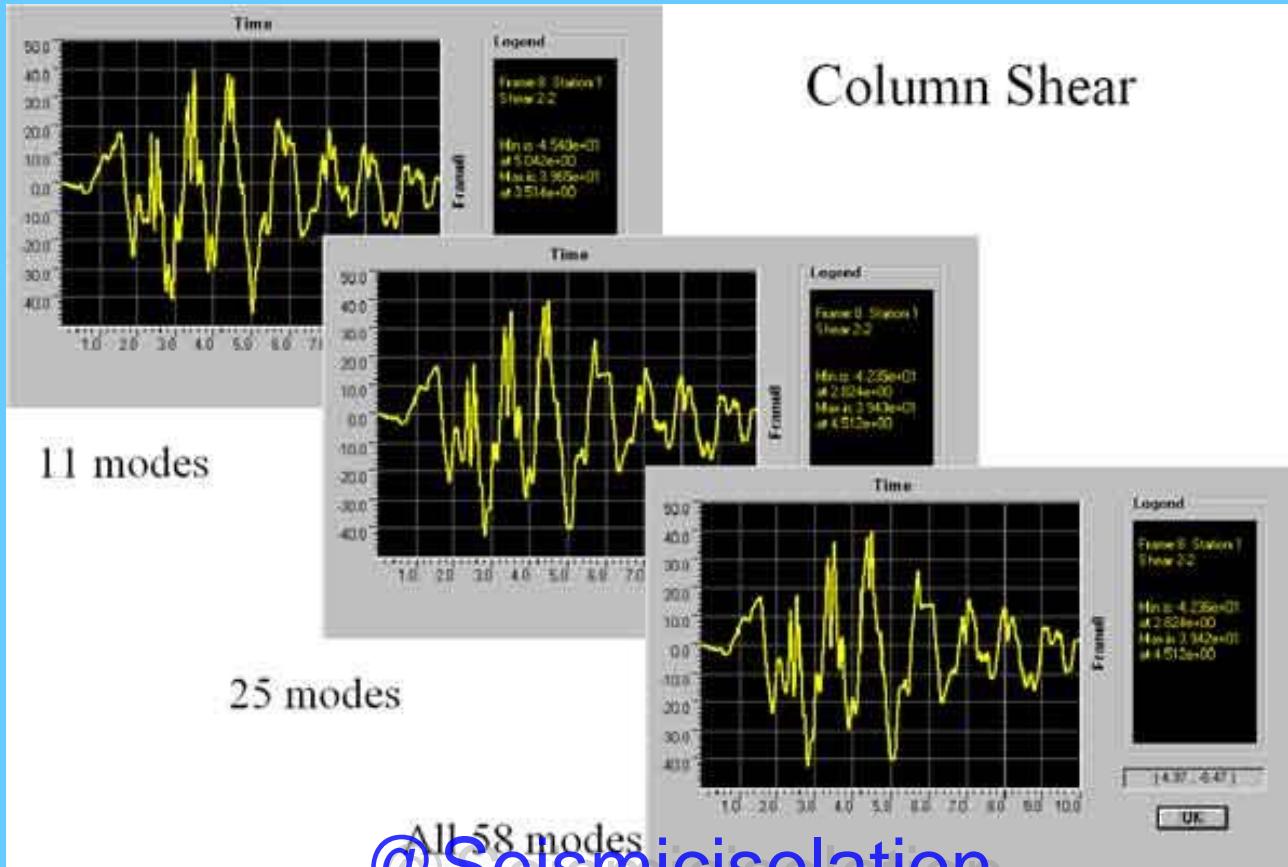
Base-isolated Structure



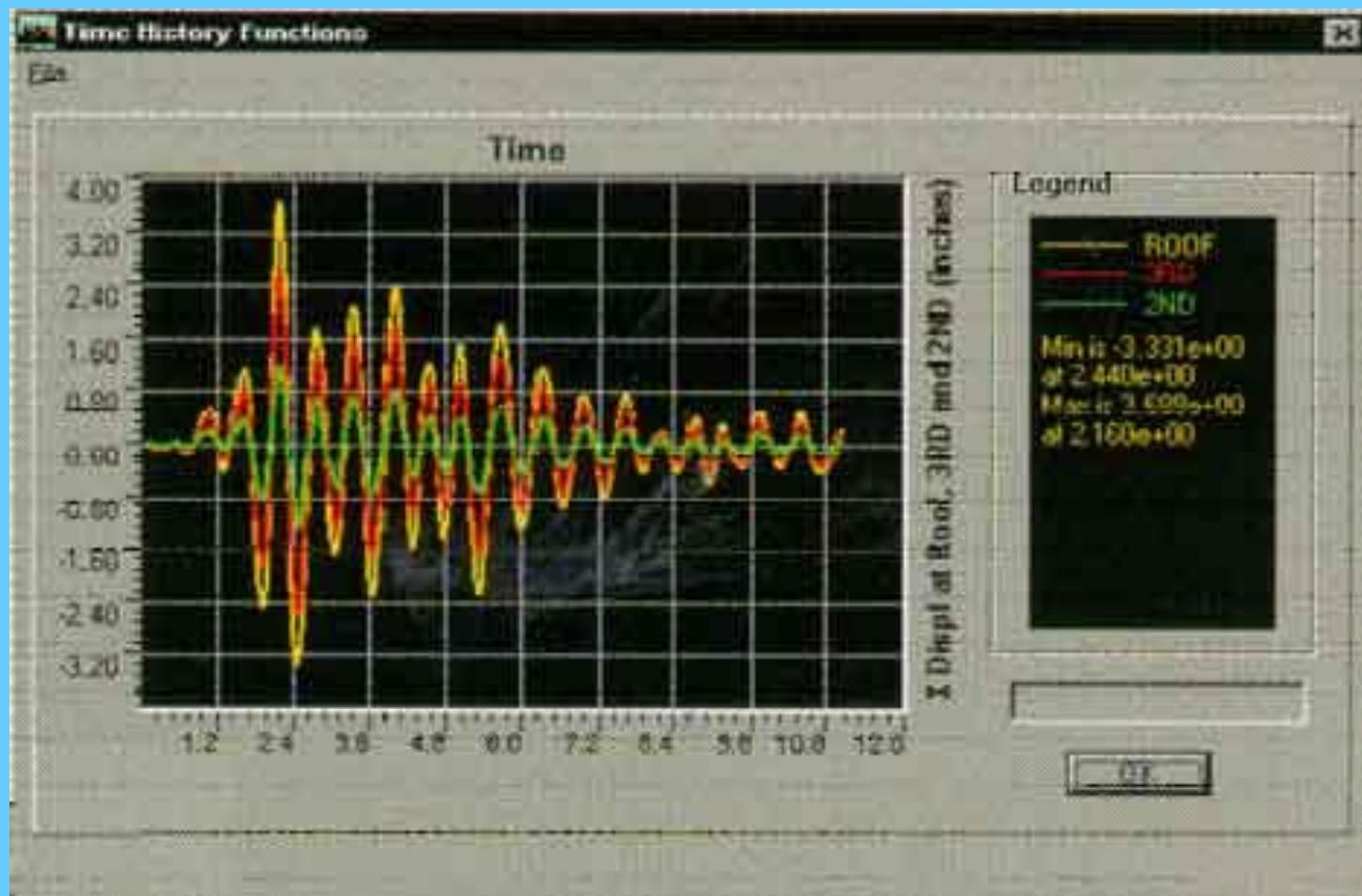
@Seismicisolation

# Modal analysis

- Eigenvalue analysis with an accelerated subspace iteration algorithm
- Ritz analysis for optimal mode superposition basis
- Modes can include P-delta, large displacement and construction effects

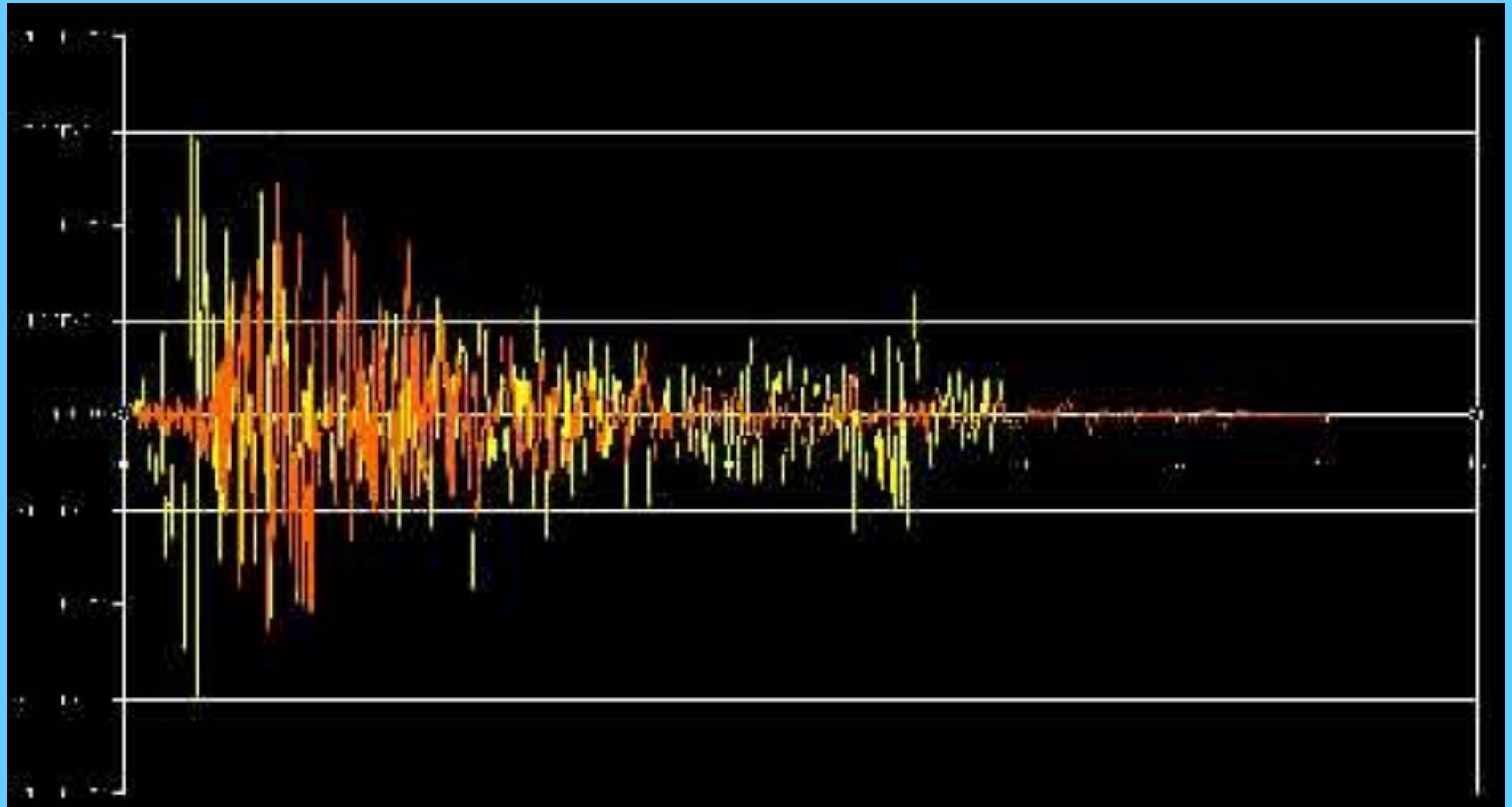


# Time history analysis



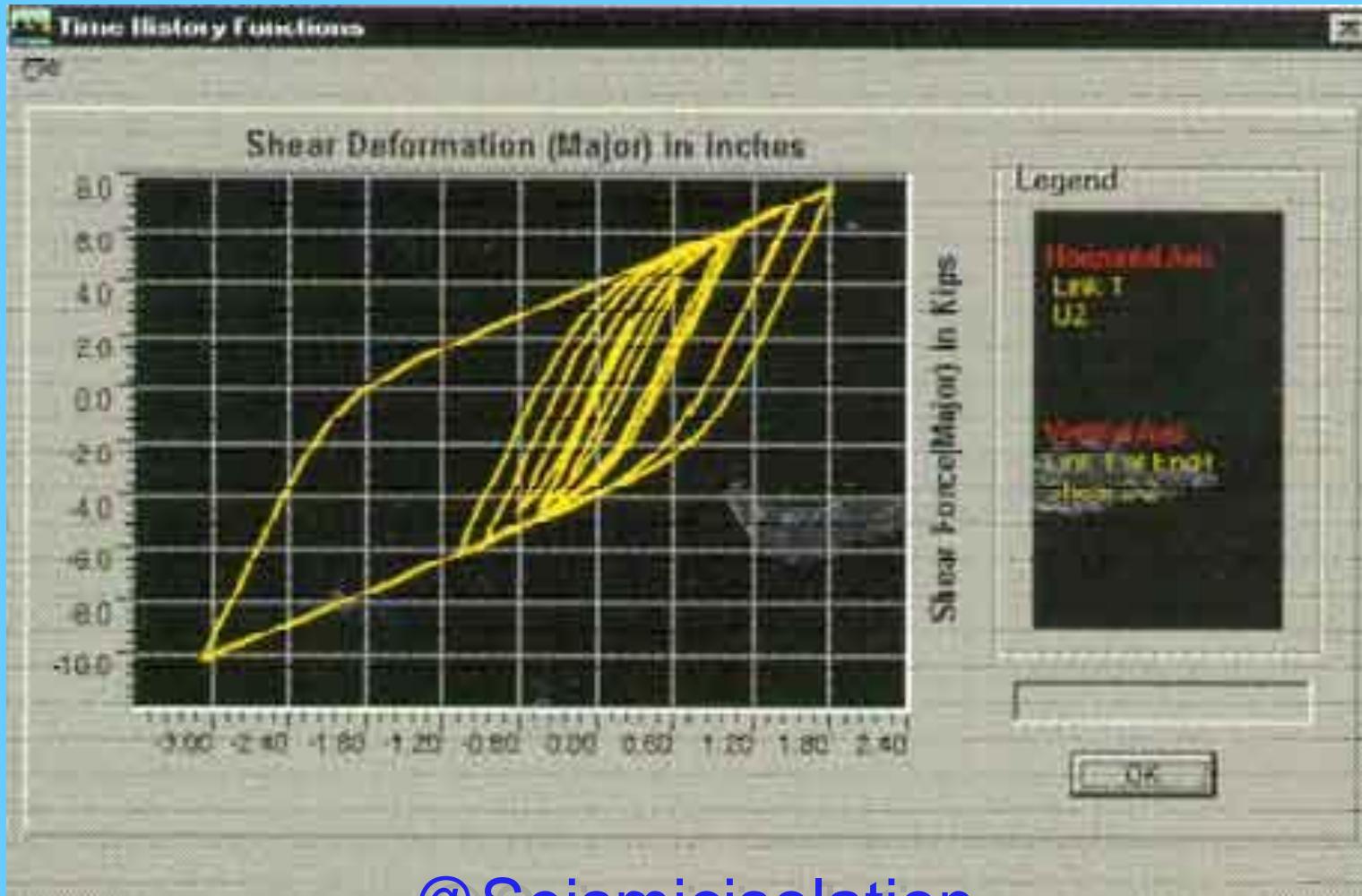
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# Dynamic response spectrum analysis



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# Nonlinear element analysis



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## D. EVALUATE and CHECK RESULTS

The primary difficulty in using structural computer programs is to not truly understand the physical action and boundary conditions as well as the behavior of the structure elements of the **real structure**, which is especially true for the behavior of the joints. The true constraints (i.e. restraint to movement) that each member exerts on the adjacent ones, can only be approximated. It is apparent, that the **mathematical model** may not at all represent the reality and true character of the structure. Therefore it is necessary to evaluate whether the results are correct and check for errors and possibly develop an improved model. It is also necessary to verify computer answers with hand calculations.

Finite element computer programs report the results of *nodal displacements*, *support reactions* and *member forces or stresses* in graphical and numerical (tabular) form immediately. It is apparent that the meaning of the results must be checked as discussed above.

It is apparent that during the preliminary design stage the graphical results are more revealing. A check of the deformed shape superimposed upon the un-deflected shape gives an immediate indication whether there are any errors. Stress (or forces) are reported as stress components of principal stresses in contour maps, where the various colors clearly reflect the behavior of the structure as indicated by the intensity of stress flow and the distribution of stresses.

### DISPLAY THE MODEL :

View the model and results of analysis for a typical frame structure,

- REACTION FORCES
- MEMBER AXIAL FORCES
- MEMBER BENDING MOMENT
- DEFLECTED SHAPE

## E. DESIGN

Any type of material can be defined and assigned to a frame element. You can run the analysis and get forces for that frame element (not stresses). If the material type is **Steel** or **Cold-Formed Steel** or **Concrete** or **Aluminum** then you can design the element using the built-in design post processors. For **Wood** you can put in member sections and get forces but not stresses. You only get stresses for frame elements in a design post processor, which SAP2000 does not have for wood.

SAP2000 v. 8 is structured to support a wide variety of design codes for the automated design and check of aluminum, concrete and steel frame members. The program currently supports the following design codes (go Options/Preferences/Steel or Concrete):

### **REINFORCED CONCRETE:**

- US: (ACI 318-99, AASHTO Concrete 97)
- Canadian: CSA-A23.3-94 (CSA 1994)
- British: (BS 8110-89)
- European: EUROCODE 2 – 1992 (CEN 1992)
- New Zealand design code: NZS 3101-95
- Indian IS
- Italian DM
- Mexican RCDF 2001

### **STEEL:**

- US: (AISC-ASD89, AISC-LRFD93, AASHTO Steel 97)
- Canadian: CAN/CSA-S16.1-94 (CISC 1995)
- British: BS 5950 (BSI 1990)
- European: EUROCODE 3 – 1993 (CEN 1993)
- Italian UNI

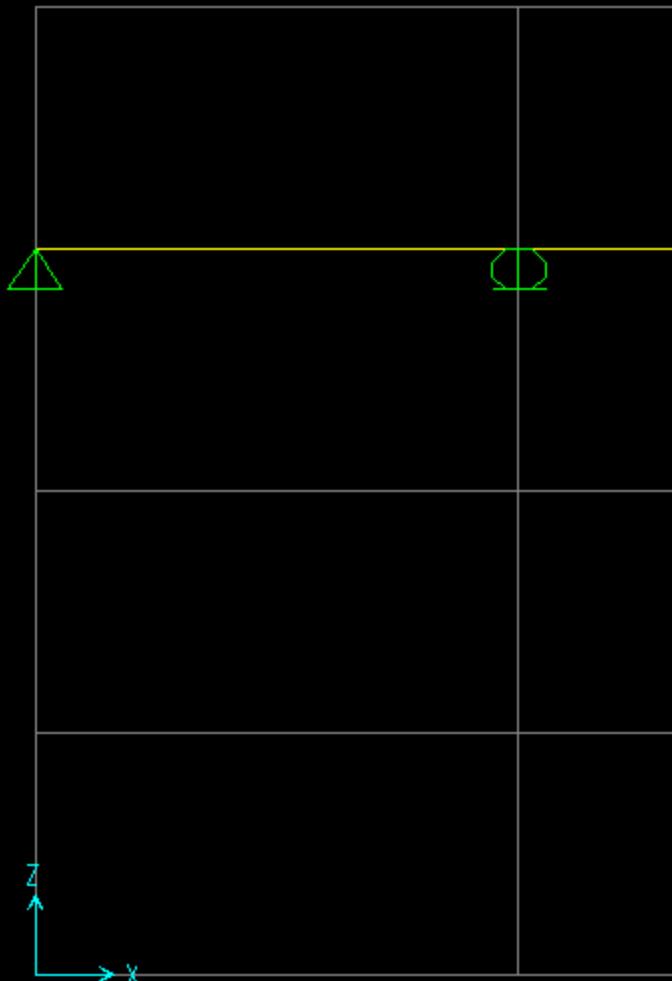
View Define BrIM Draw Select Assign Analyze Display Design

Options Help



-Z Plane @ Y=18

ect Model



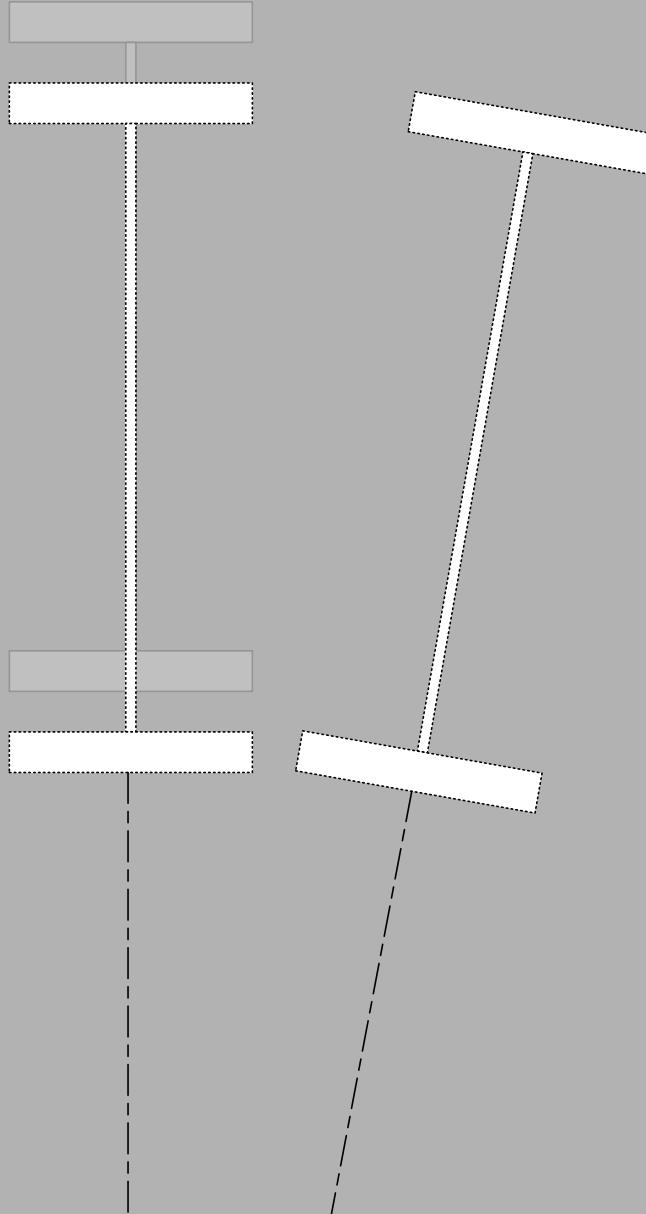
- Preferences...
- Colors
- Database
- Graphics Mode
- Windows
- Set Calculator Memory...
- Auto Refresh
- Show Tips at Startup
- Show Bounding Plane
- Moment Diagrams on Tension Side
- Sound
- 3D View Up Direction
- Lock Model
- Auto Save
- Show Float
- Show Result
- Show Tool
- Reset Tool
- Set Menu L
- Request M

- Dimensions/Tolerances...
- Steel Frame Design...
- Concrete Frame Design...
- Aluminum Frame Design...
- Cold-Formed Steel Frame Design...
- Reinforcement Bar Sizes...

### Steel Frame Design Preferences for Chinese 2002

Item	Value
1 Design Code	Chinese 2002
2 Time History Design	Chinese 2002
3 Framing Type	CISC 95
4 Gamma0	CAN/CSA-S16-01
5 Ignore B/T Check?	EUROCODE 3-1993
6 Classify Beam as Flexo-Compression Member	Indian IS:800-1998
7 Consider Deflection?	Italian UNI 10011
8 DL Limit, L /	UBC97-ASD
9 Super DL+LL Limit, L /	UBC97-LRFD
10 Live Load Limit, L /	120.
11 Total Limit, L /	500.
12 Total-Camber Limit, L /	400.
13 Pattern Live Load Factor	500.
14 Demand/Capacity Ratio Limit	0.
	0.95

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## Modeling Steel Members using SAP2000 (see also Appendix A)

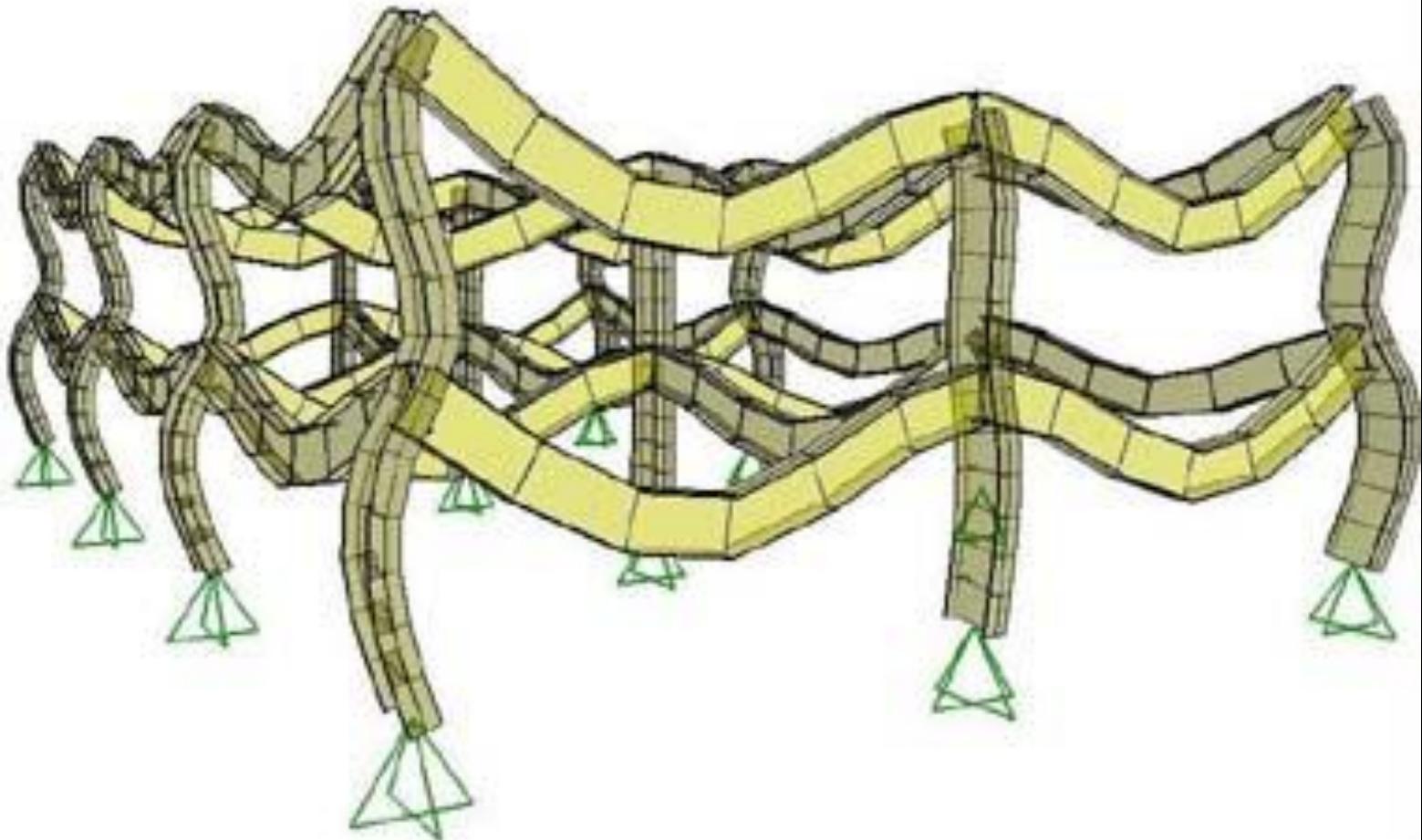
SAP2000 assumes by default that **frame elements** (i.e., beams and columns) **are laterally unsupported for their full length**. But beams are generally laterally supported by the floor structure (Fig. 4.1). Therefore, assume an unsupported length of say  $L_b = 2 \text{ ft}$  for preliminary design purposes, or when in doubt, take the spacing between the filler beams. For example, for a beam span of,  $L = 24 \text{ ft}$ , assume an **unbraced length ratio** about the minor axis of  $L_b/L = 2 \text{ ft}/24 \text{ ft} = 0.083$ , or say **0.1**; that is, take the minor direction unbraced length as **10%** of the actual span length.

## **F. MODIFY MEMBER PROPERTIES**

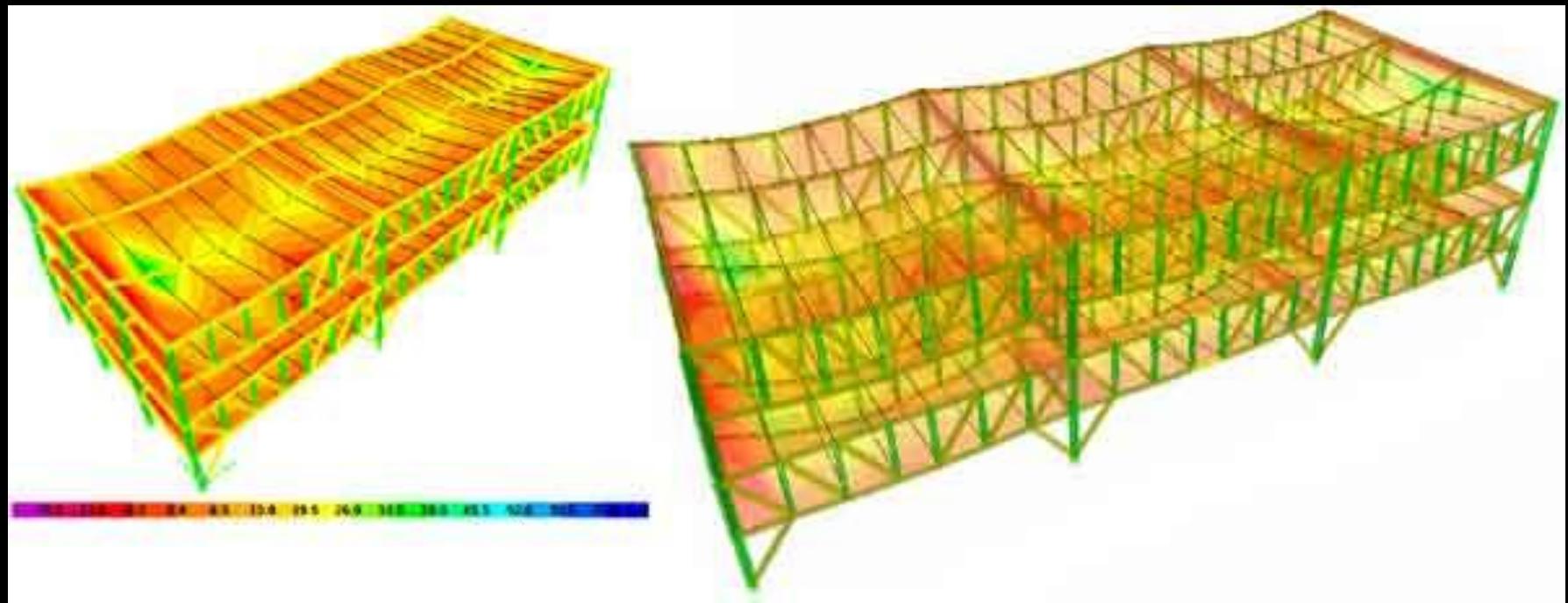
## **G. RE-ANALYZE and RE-DESIGN** with updated model

The design changes must be taken into account in the analysis portion that is the redistribution of member forces due to change of stiffness, therefore a re-run of the analysis is necessary

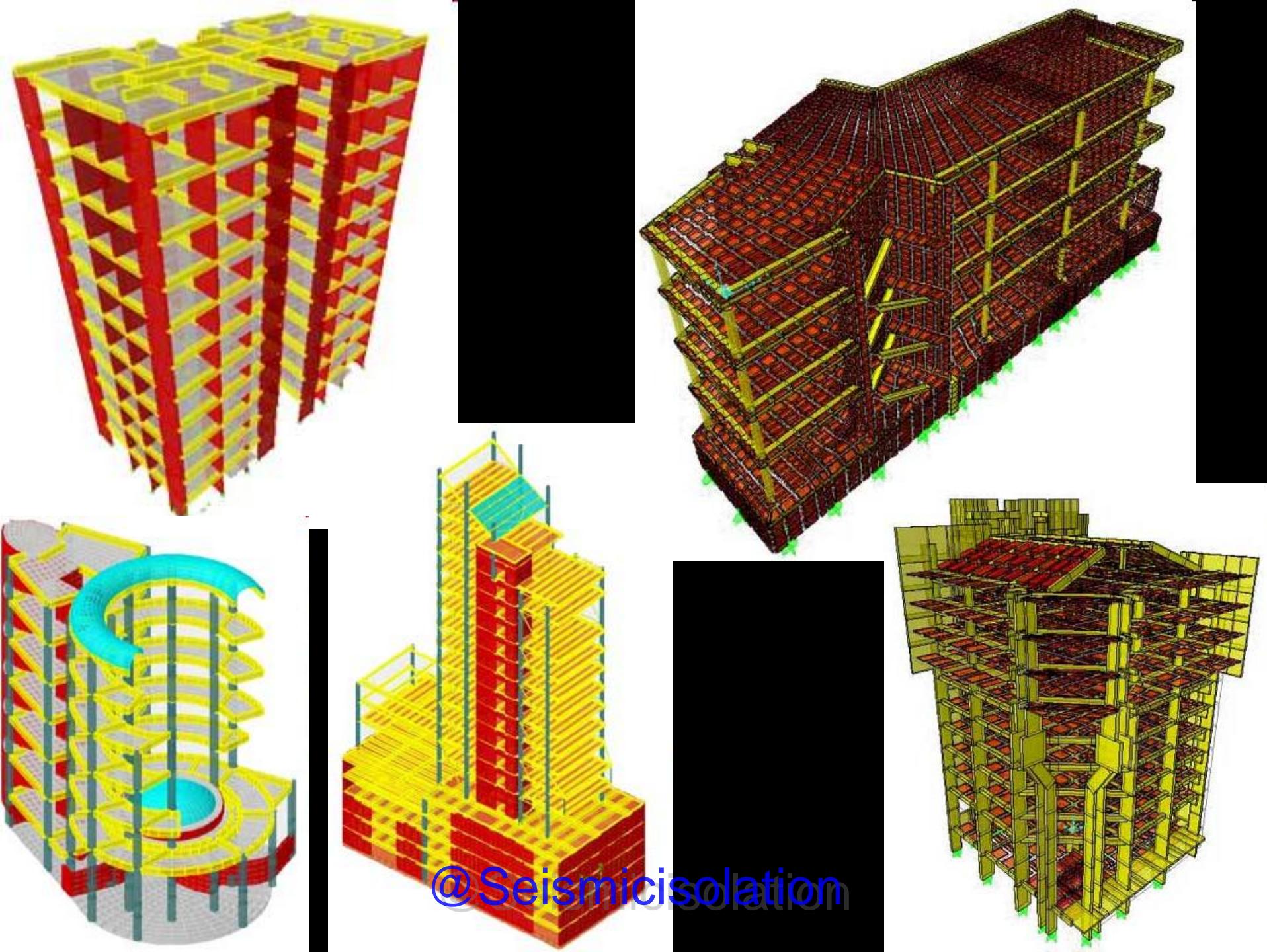
## **H. COMPUTER OUTPUT: display results (printing and plotting)**



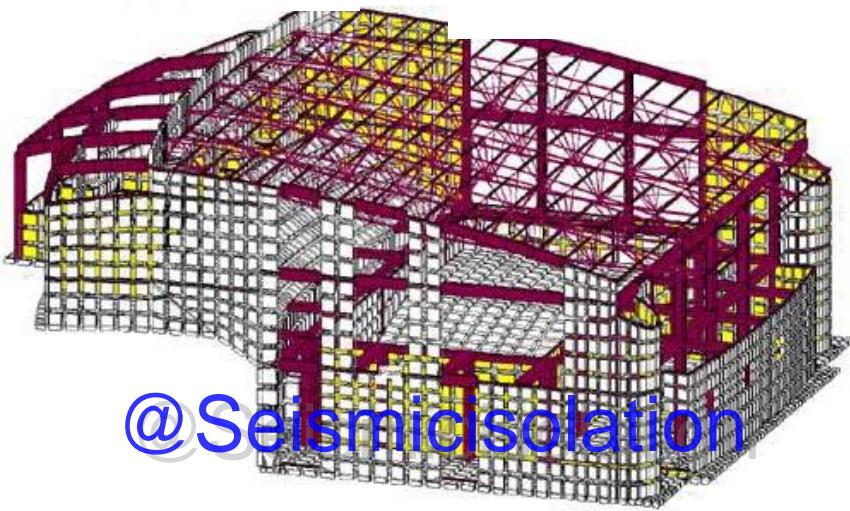
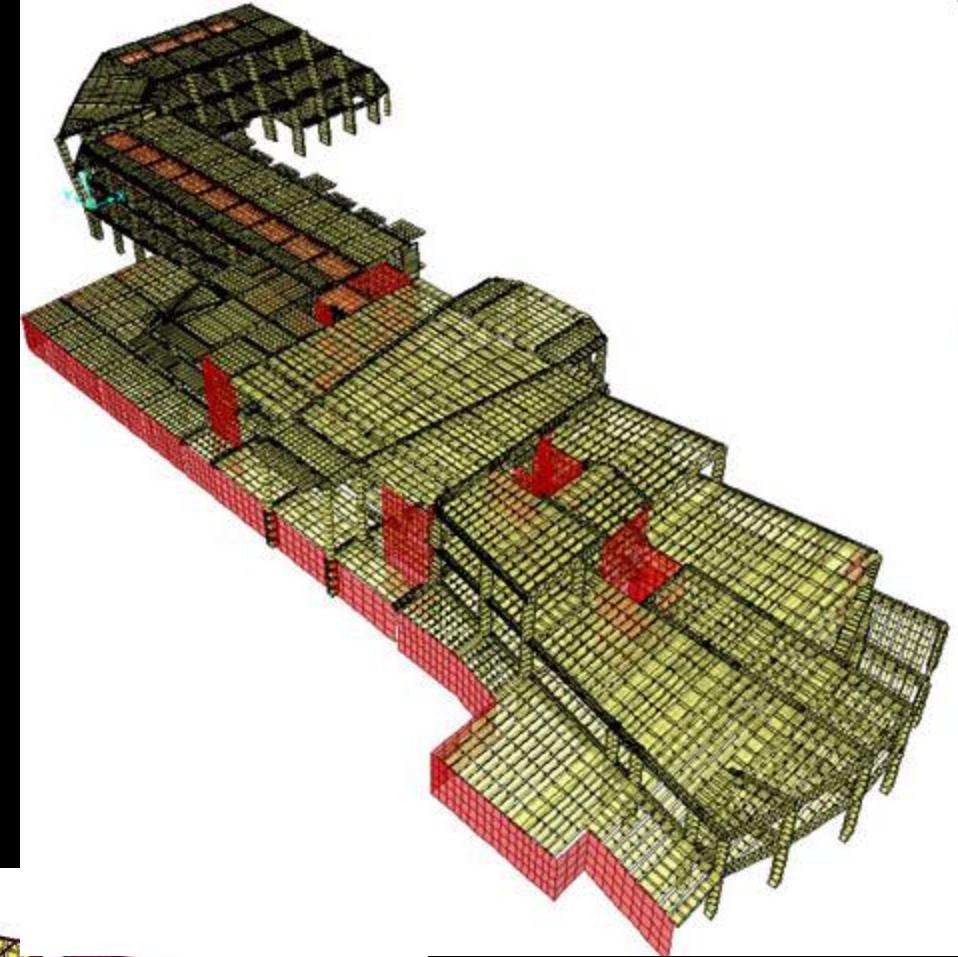
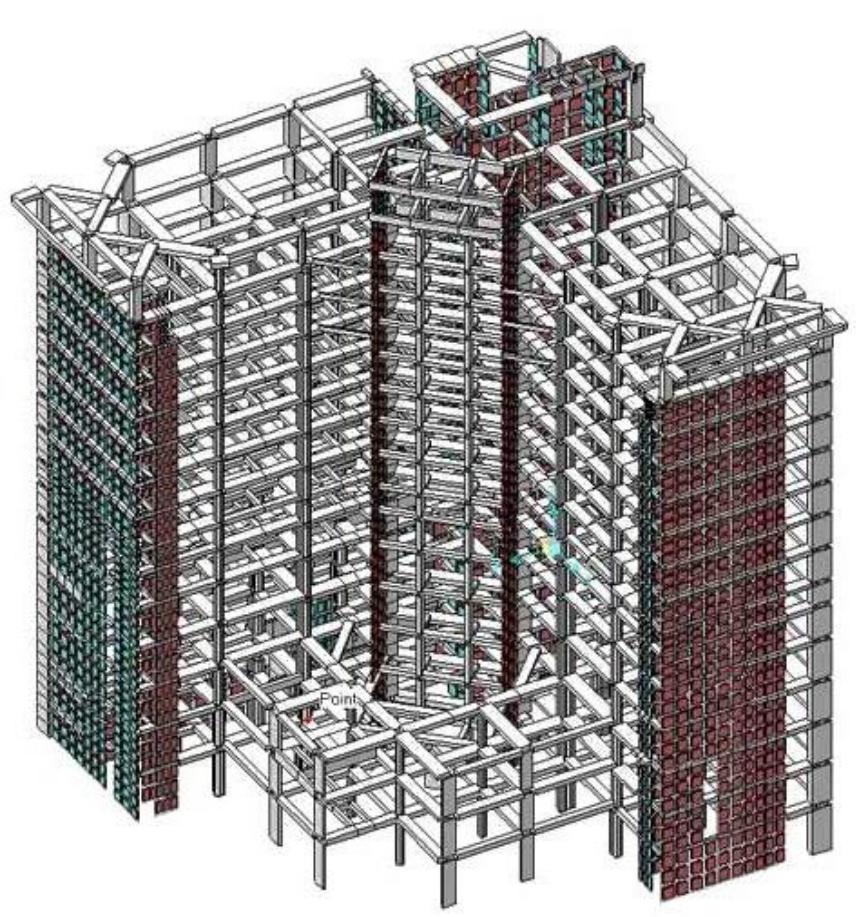
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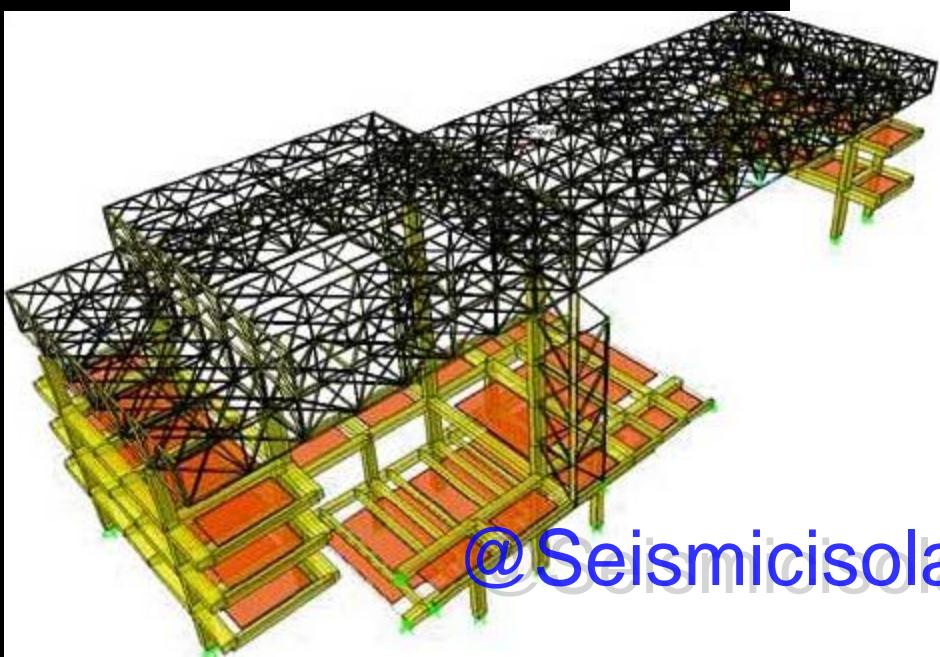
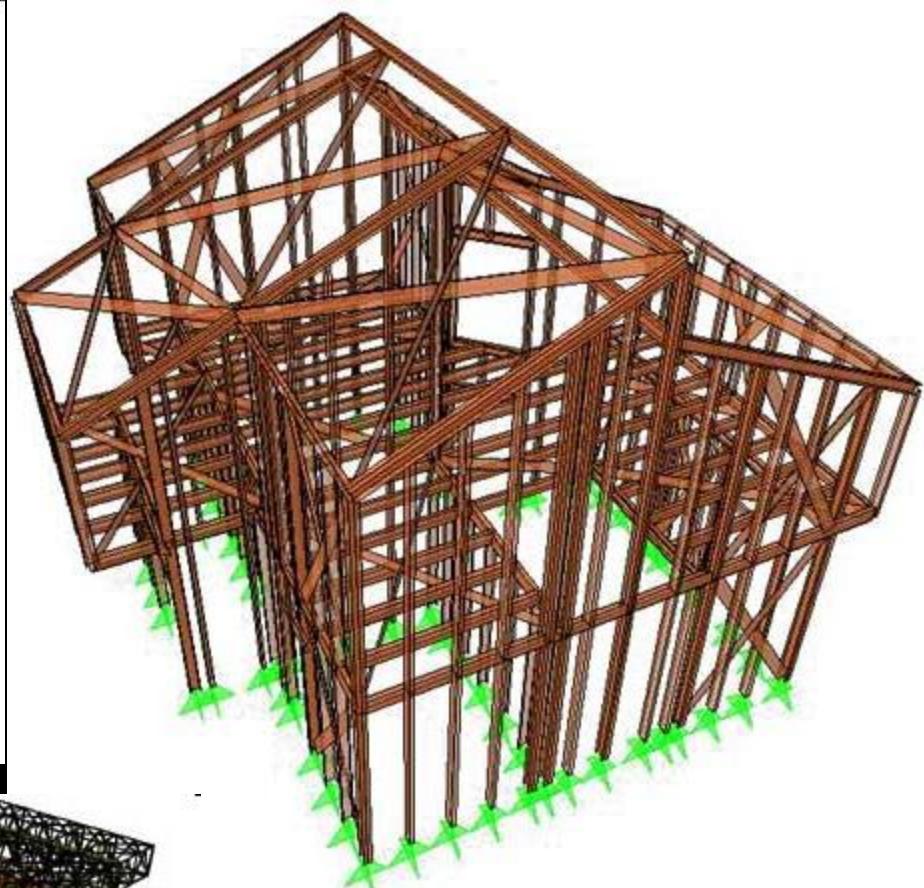
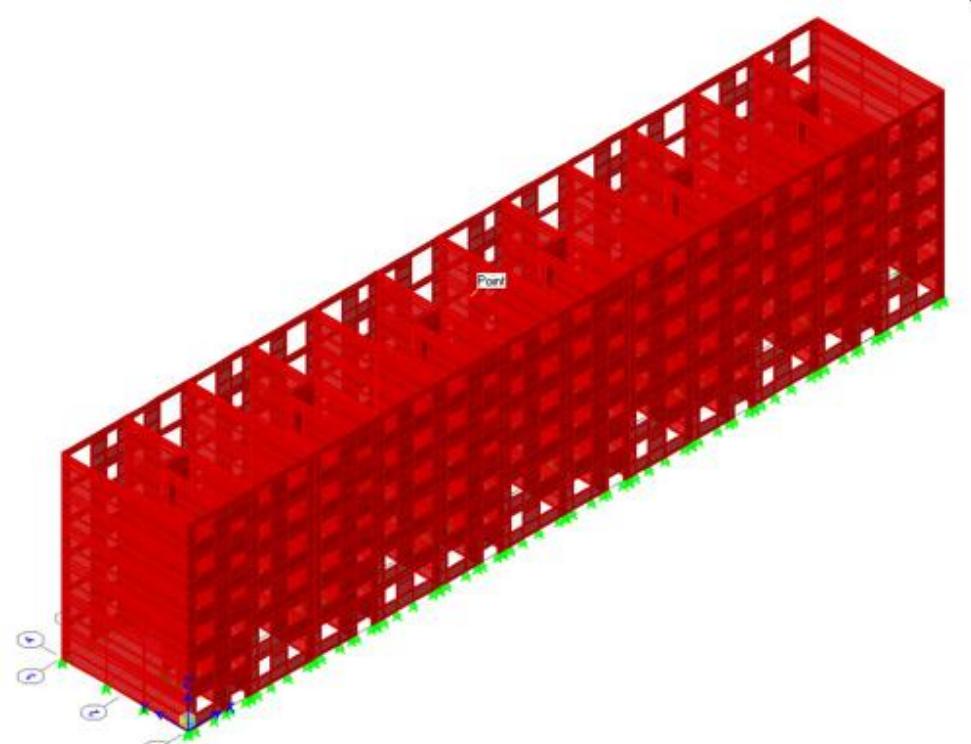


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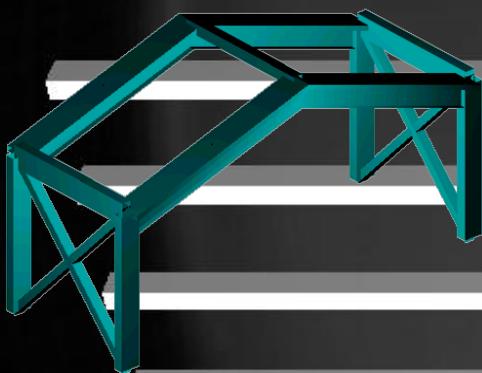




@Seismicisolation

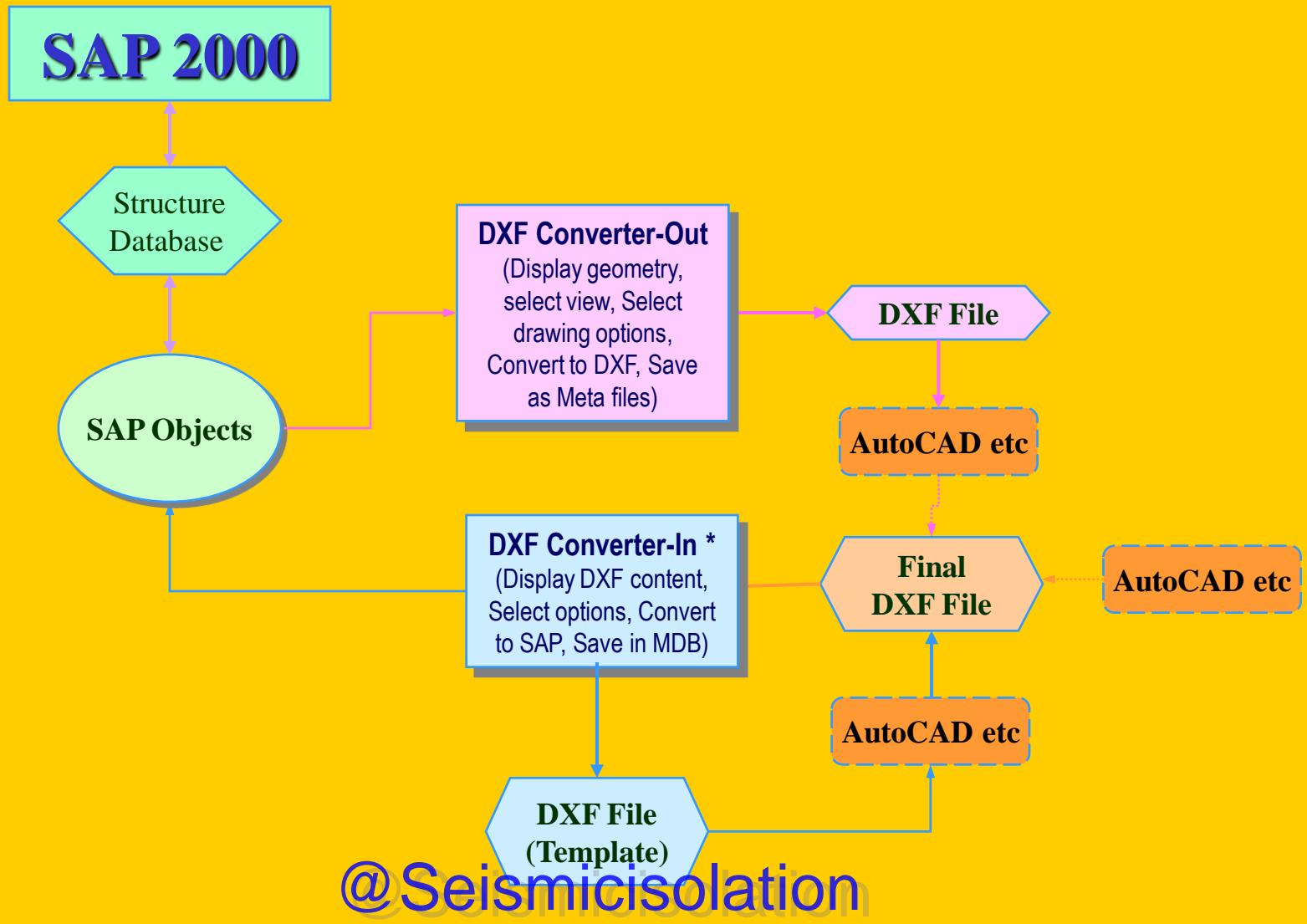
# **New Productivity Tools**

## **DXF Interface and Reporter**



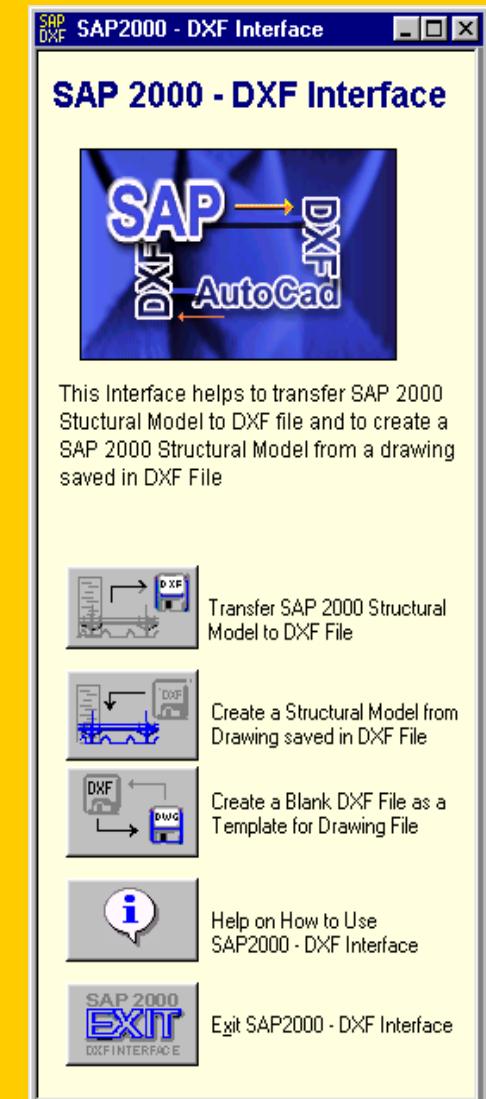
**@Seismicisolation**

# SAP200-DXF-AutoCAD Cycle

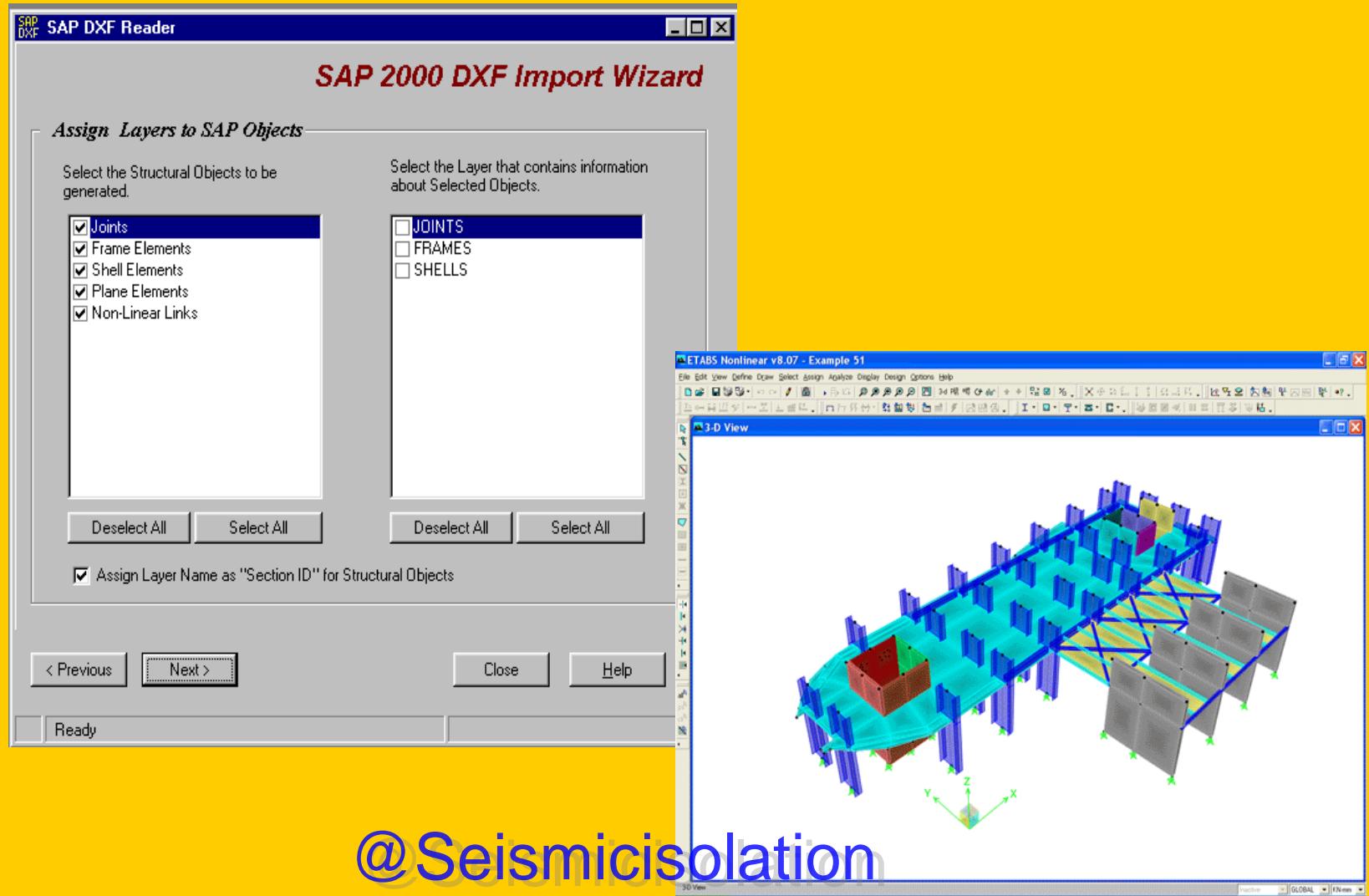


# Transfer Model To AutoCAD

- Complete control of the elements and attributes to be transferred
- Generate wire-frame or full solid models



# Get Model From AutoCAD



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**Complex geometry may be modeled first in SketchUp or Rhinoceros and the imported to AutoCAD.**

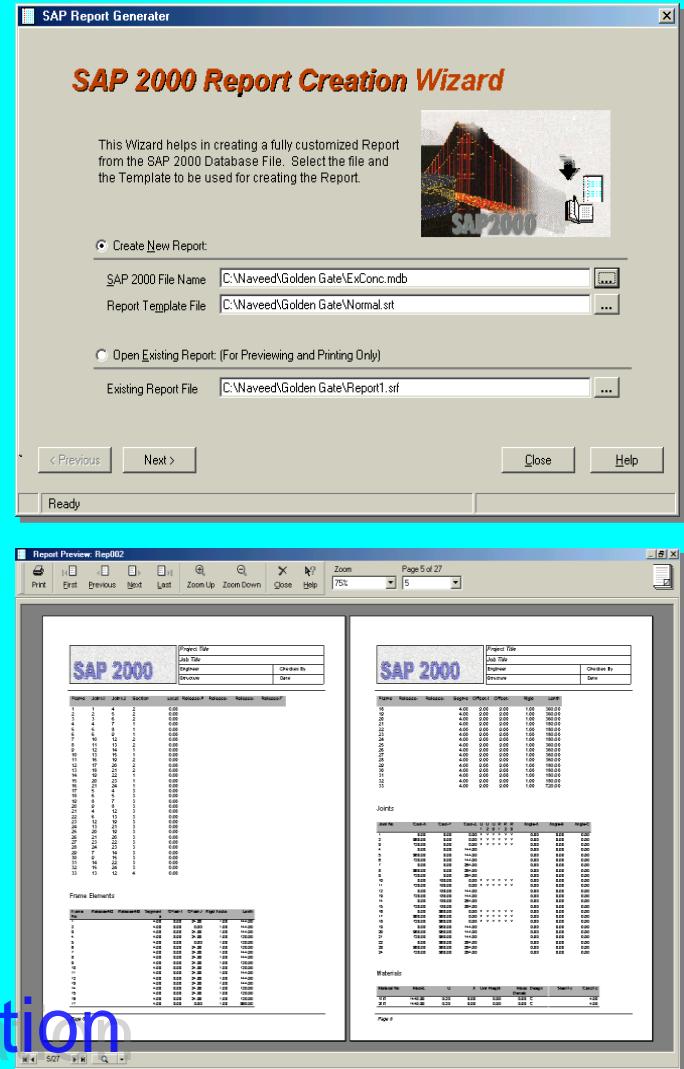
## **Import AutoCAD into SAP2000.**

For example for a frame system frame:

- 1**-adjust your units and coordinates on AutoCAD
- 2**-create layer for your lines. because when you export your shape on sap 2000 it will be important.
- 3**-draw your shape only using line.
- 4**-save your shape as a dxf file.
- 5**-go to sap 2000 and file menu and go to import
- 6**-you will see a list about what kind of drawing will import from AutoCAD to SAP2000
- 7**-answer all questions and click ok.  
you will see that all shape in AutoCAD you can import in SAP2000.

# SAPReporter

- Create fully customized and formatted reports for selected items including text, tables and graphics.
- Output can be
  - *Previewed on screen*
  - *Printed on paper*
  - *Saved in RTF files*
  - *Saved in HTML format for viewing in web browsers.*



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# Structure Systems

## Structure Behavior

### A.. PLANAR STRUCTURES

Axial force systems

- TRUSSES
- STAYED STRUCTURES

Flexural force systems

- BEAMS

Flexural-axial force systems

- SKELETONS: frames, arches

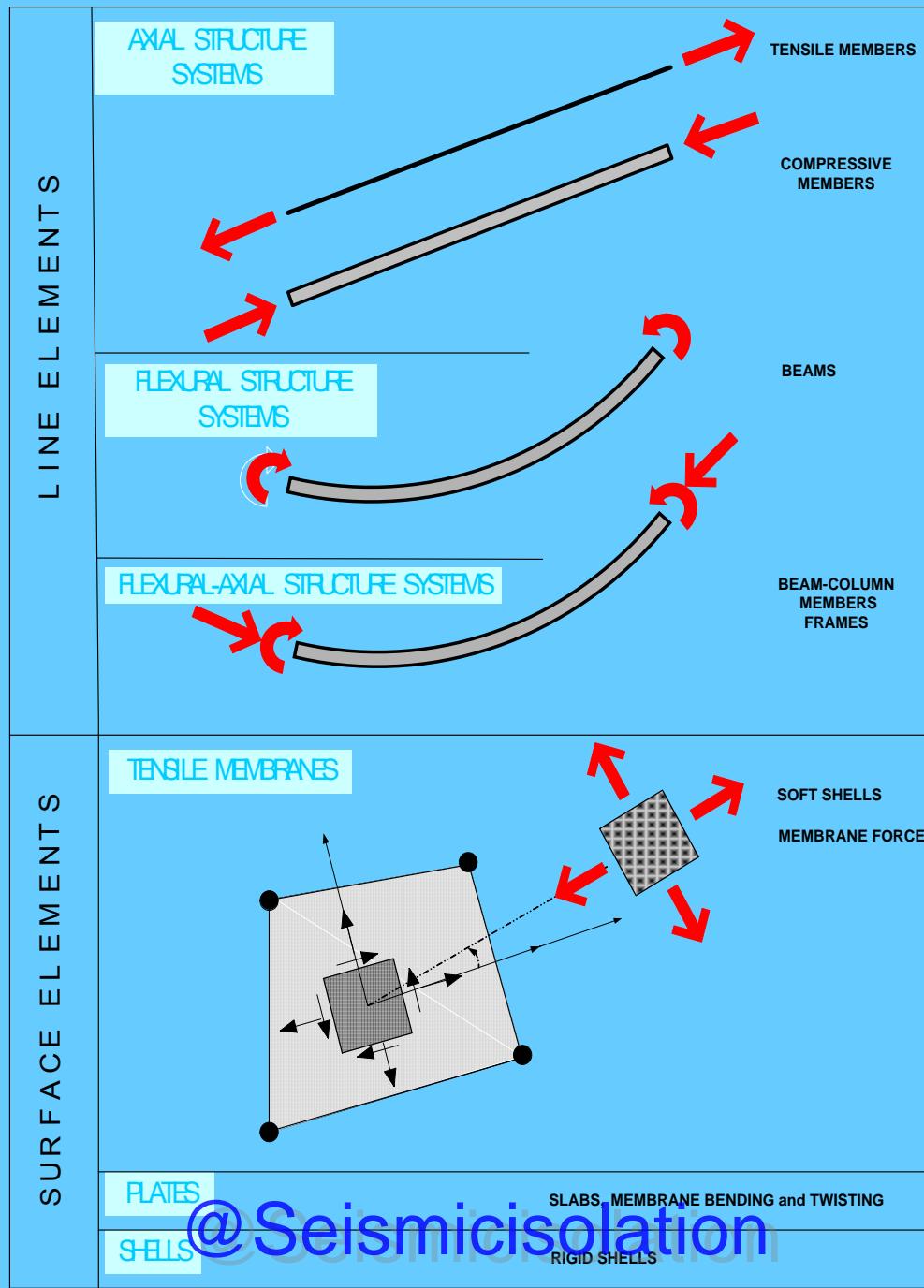
### B. SPATIAL STRUCTURES

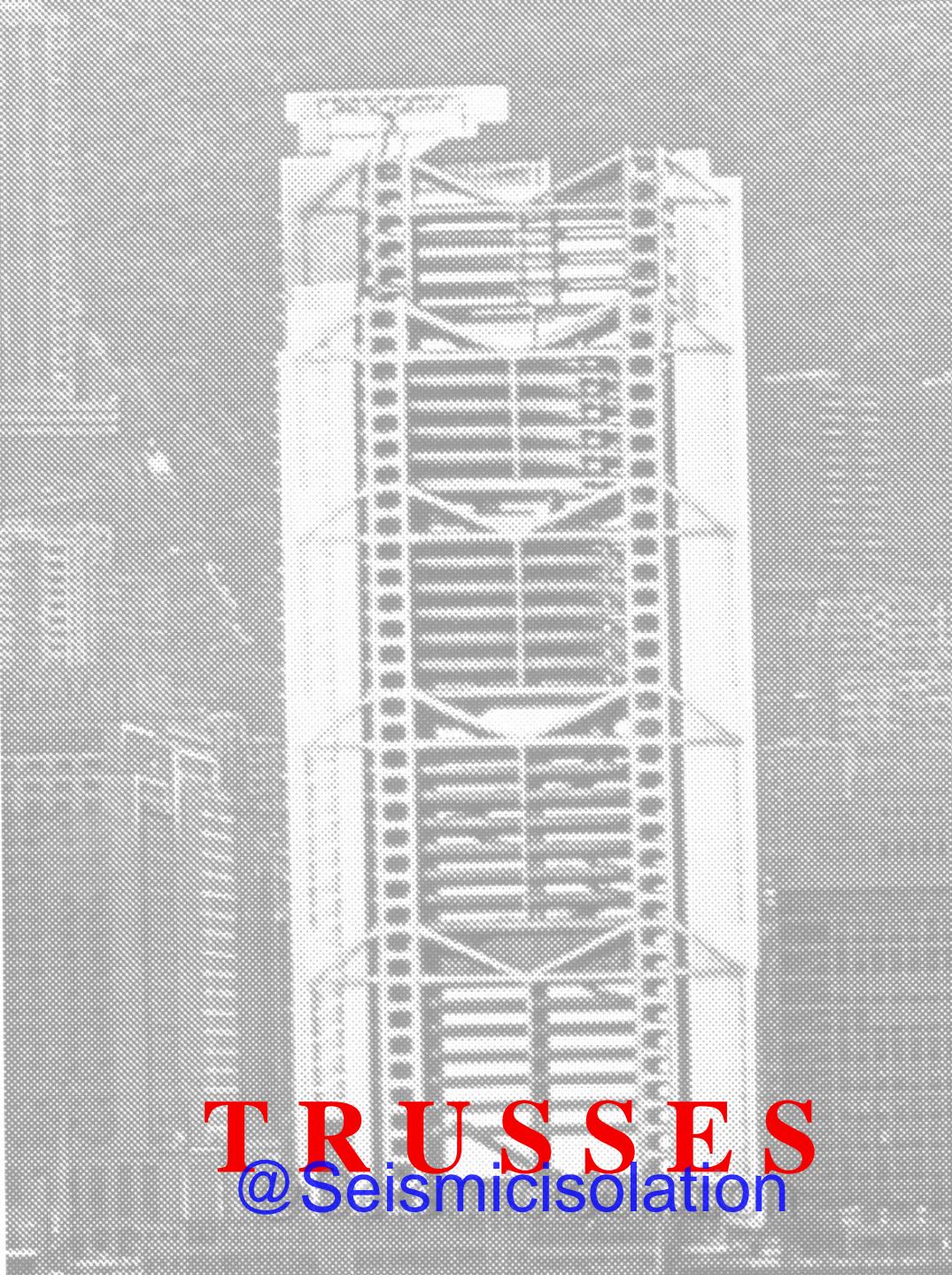
- LATERAL STABILITY OF BUILDINGS
- SPACE FRAMES
- CABLE STRUCTURES
- SURFACE STRUCTURES

### C. CASE STUDY

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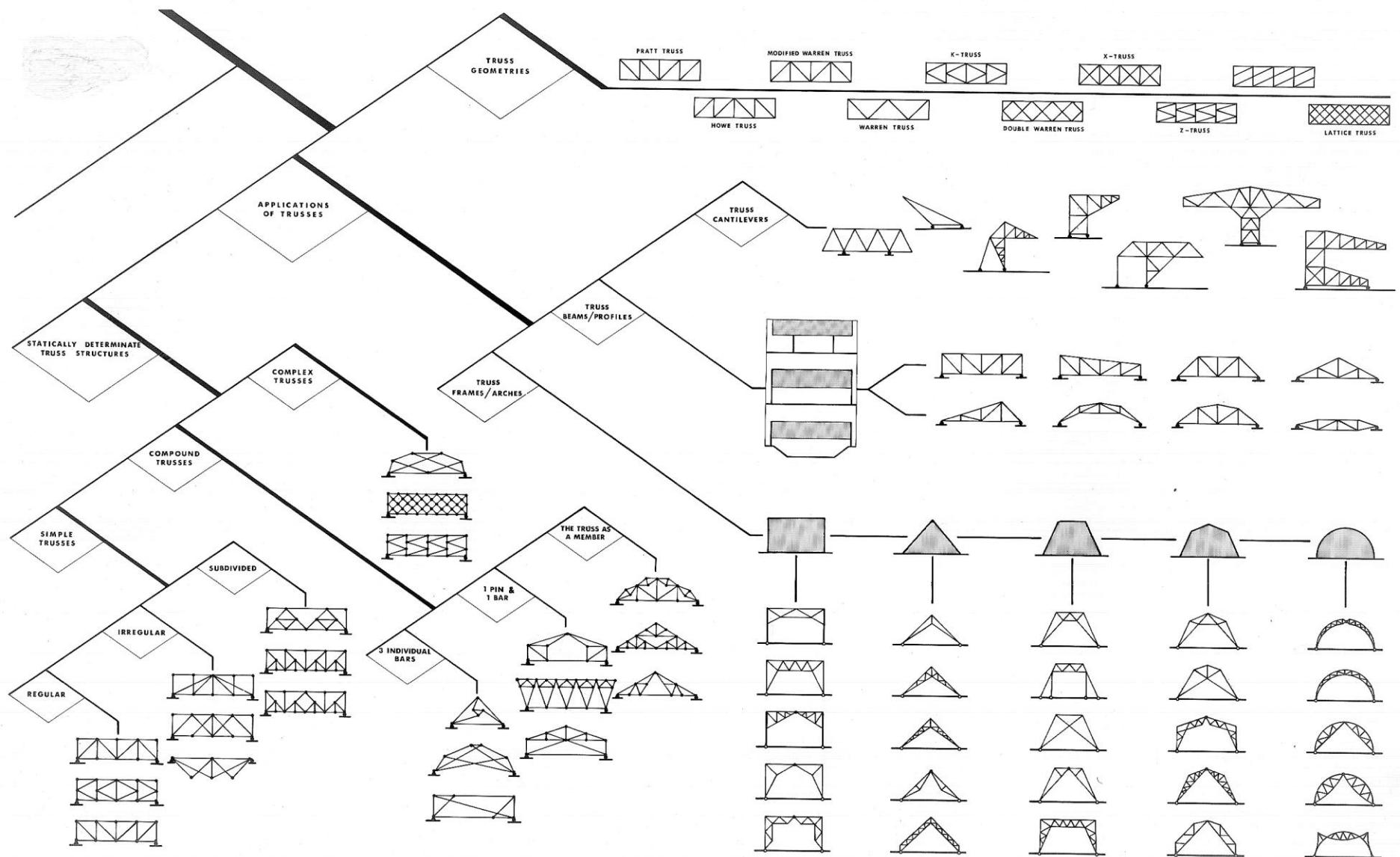




# TRUSSES

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Hongkong  
Bank, Hong  
Kong, 1985,  
Foster/Arup



@Seismicisolation



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John Hancock Center  
(100 stories, 344 m),  
Chicago, 1968, Bruce  
Graham/ Fazlur Kahn  
of SOM





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Library Gainesville, FL



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Mercedes-Benz Zentrale Berlin, 2000 Jamm, Weber, Donath und Partner



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**Sony Center, Potzdamer Platz,  
Berlin, 2000, Helmut Jahn  
Arch., Ove Arup USA Struct.  
Eng**

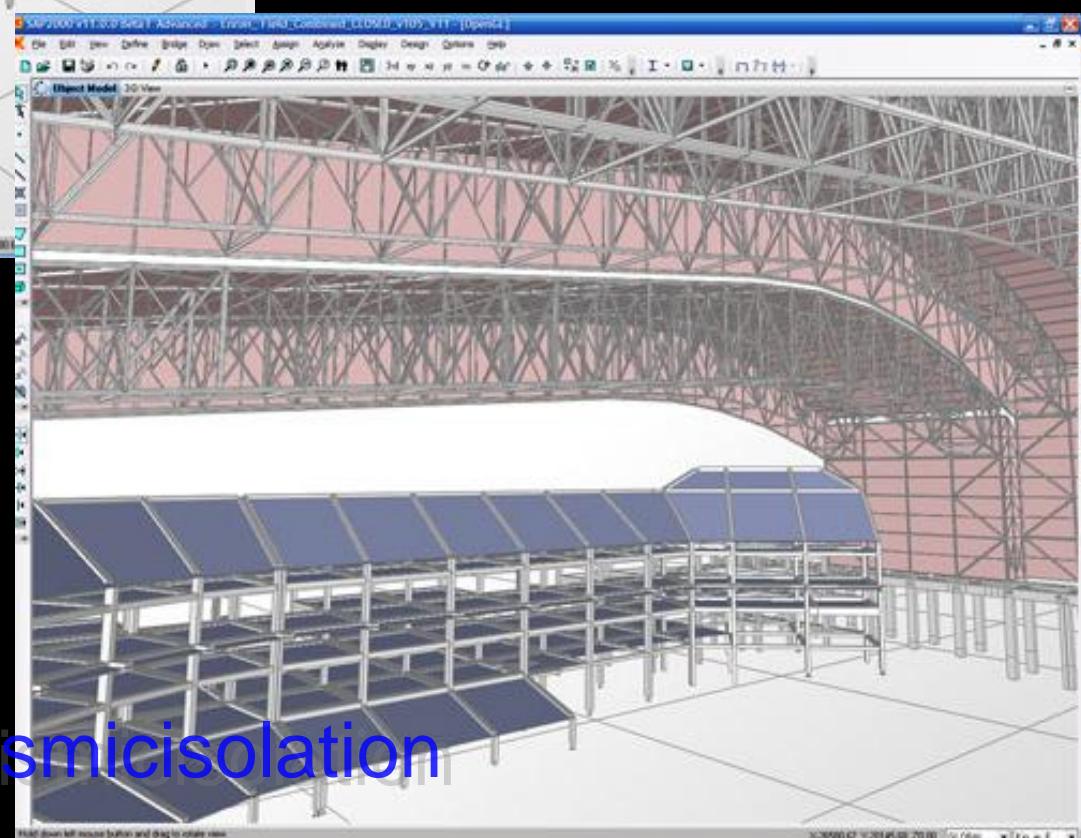
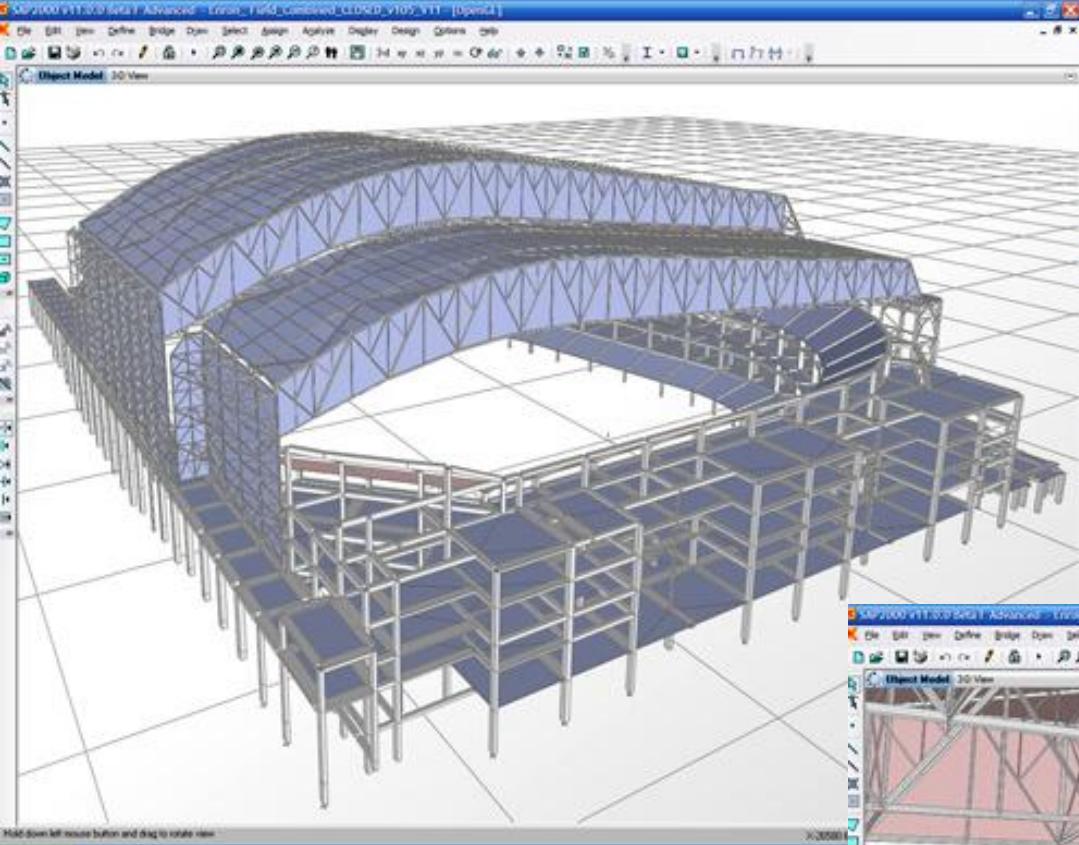
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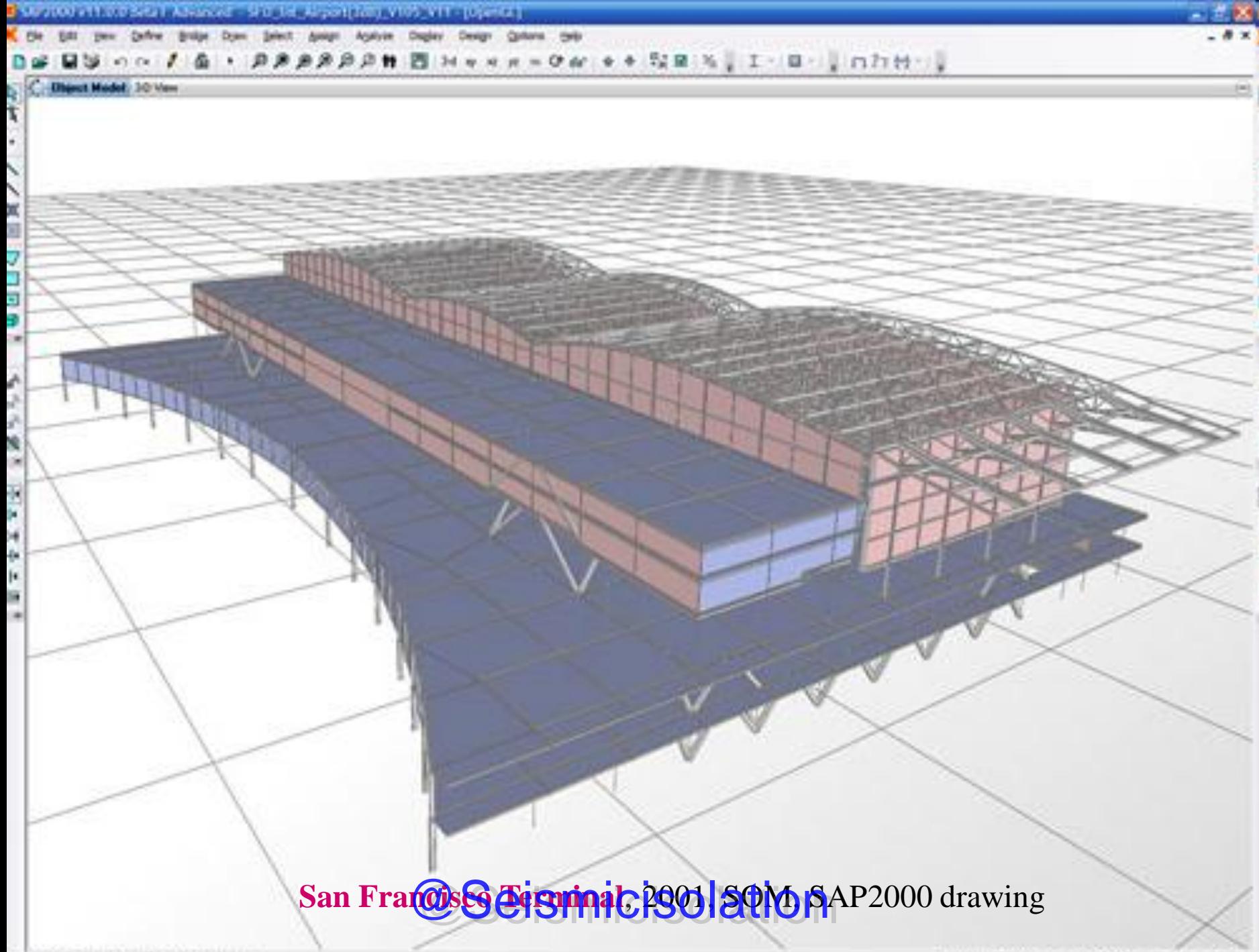
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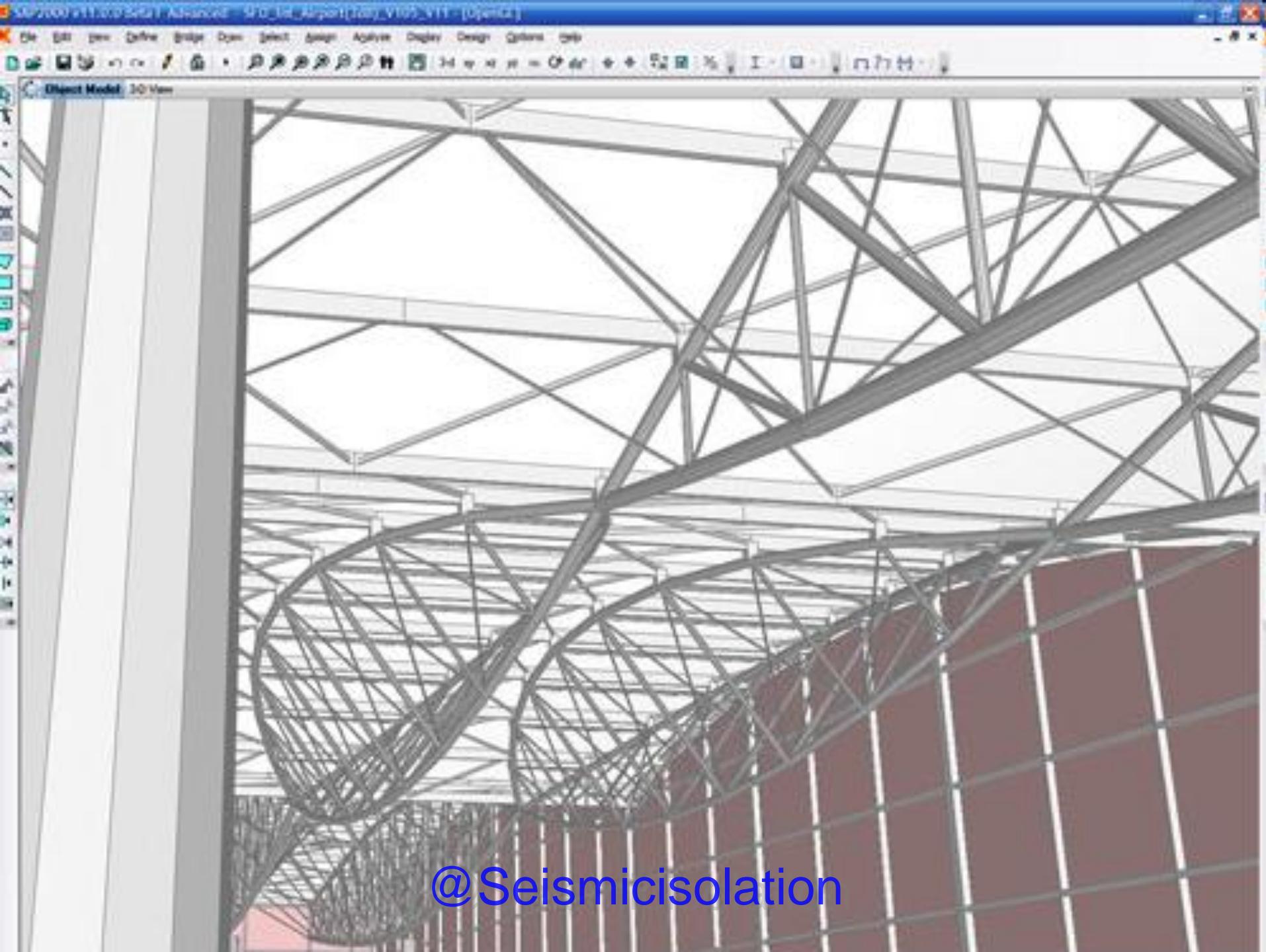
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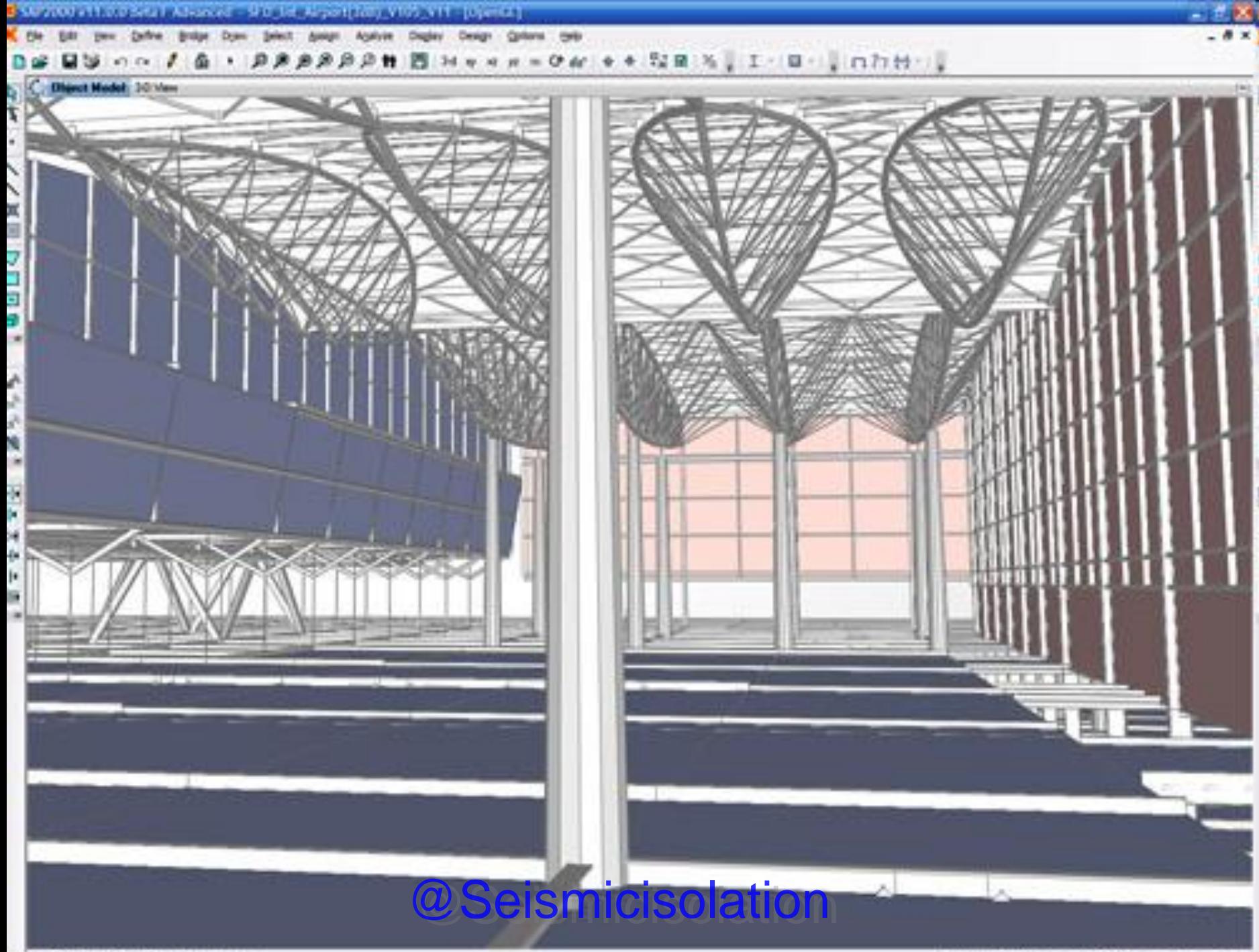
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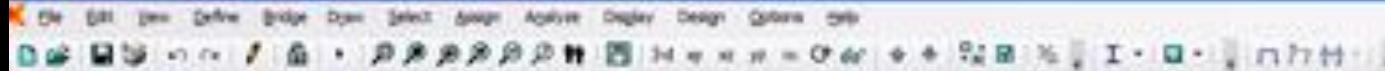
San Francisco International Airport Terminal 2001 Seismic Isolation System SAP2000 drawing



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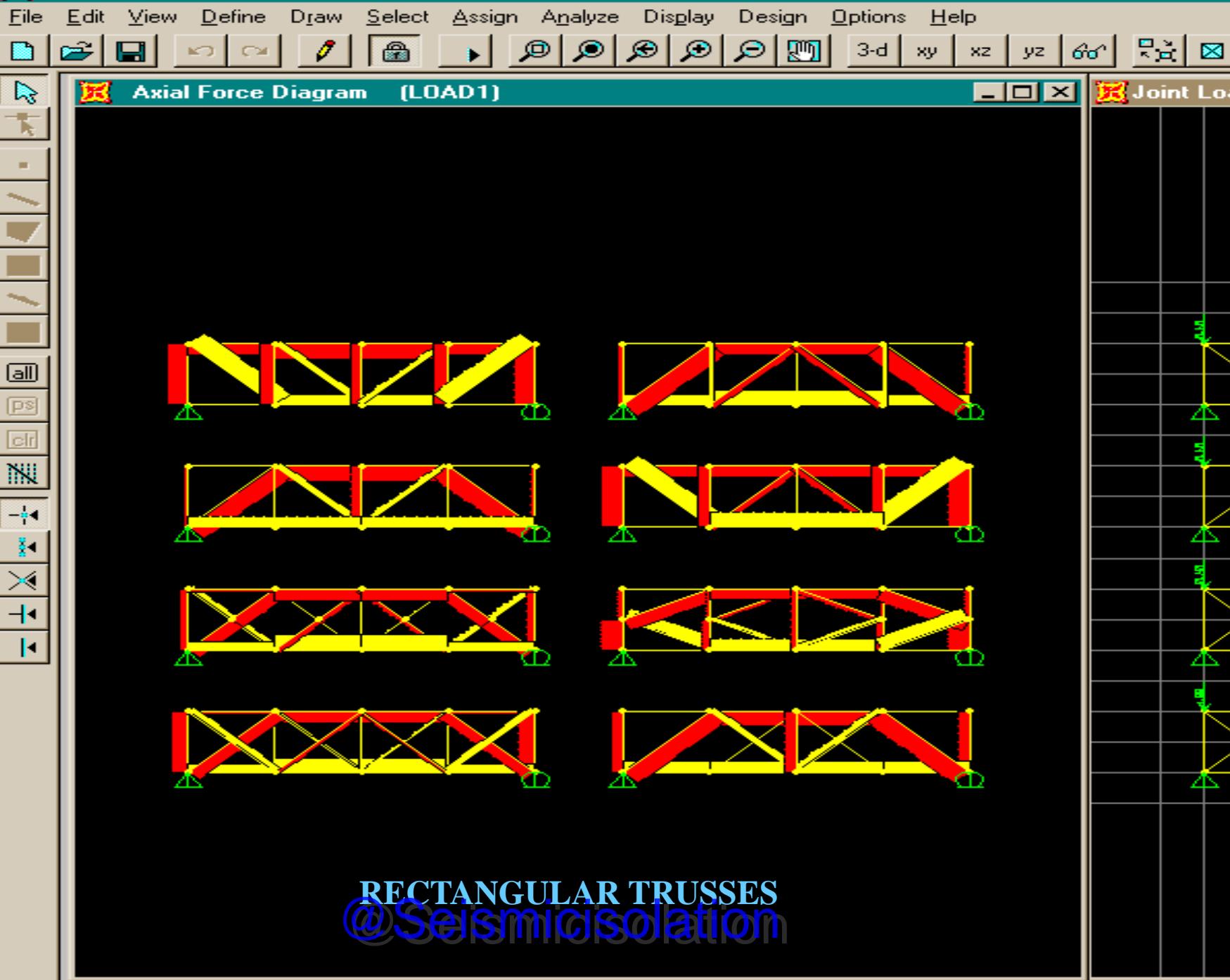
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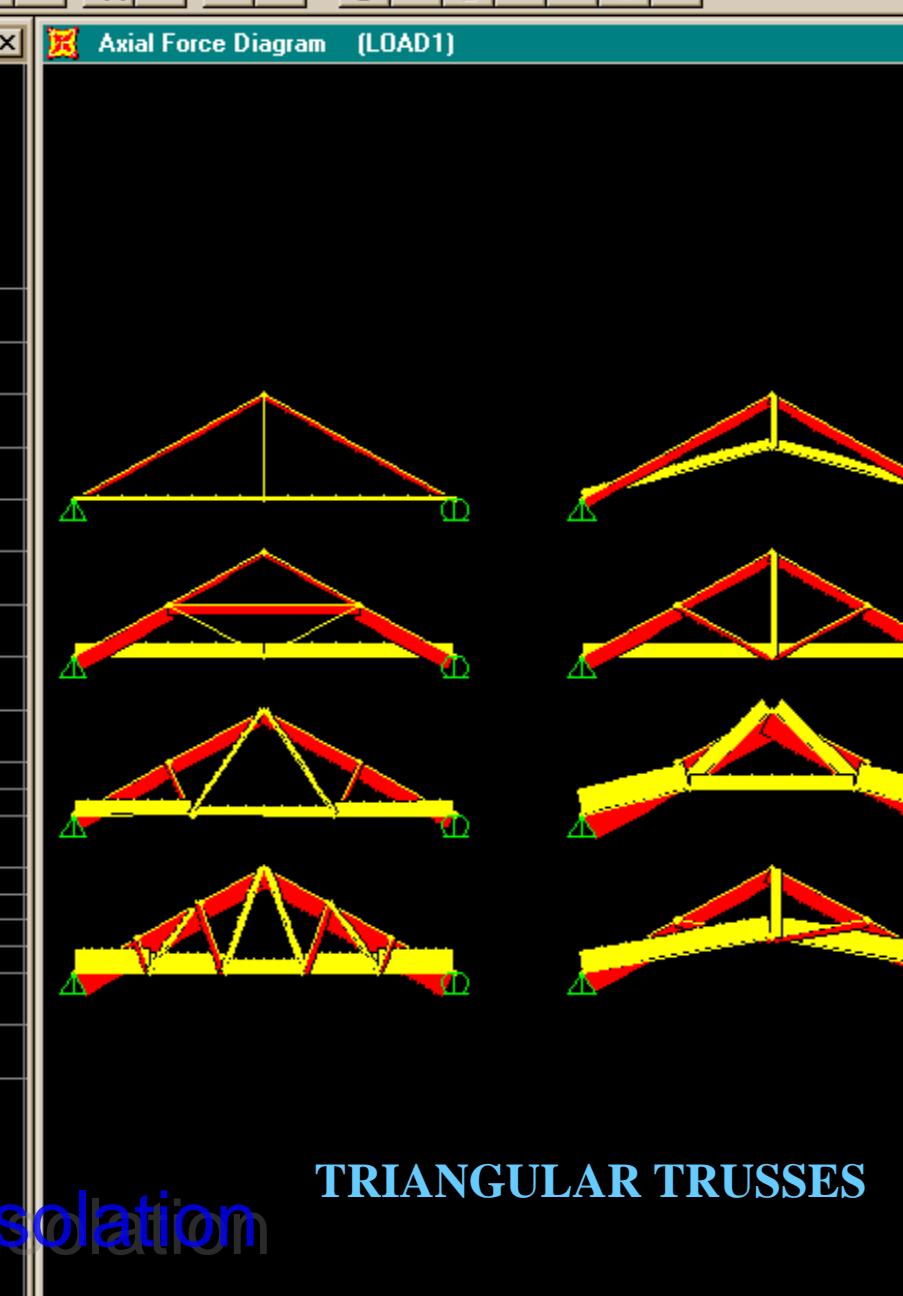
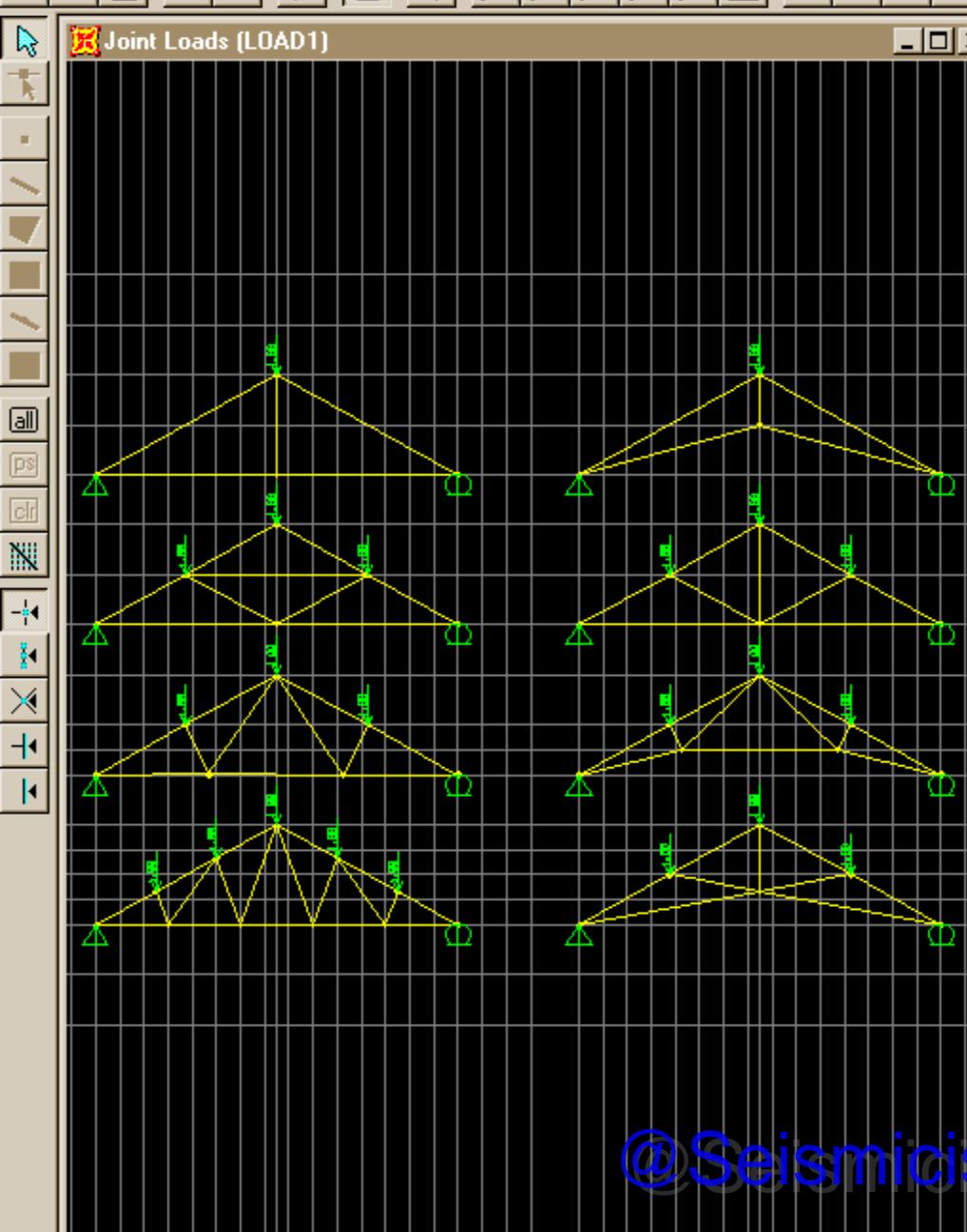
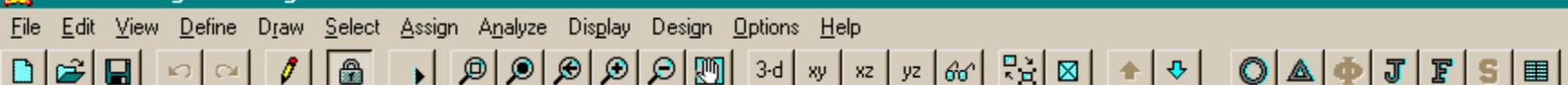
Direct Model 3D View

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# SAP2000 - Fig. 5.5 simple rectangular trusses



# SAP2000 - Fig. 5.4 triangular trusses

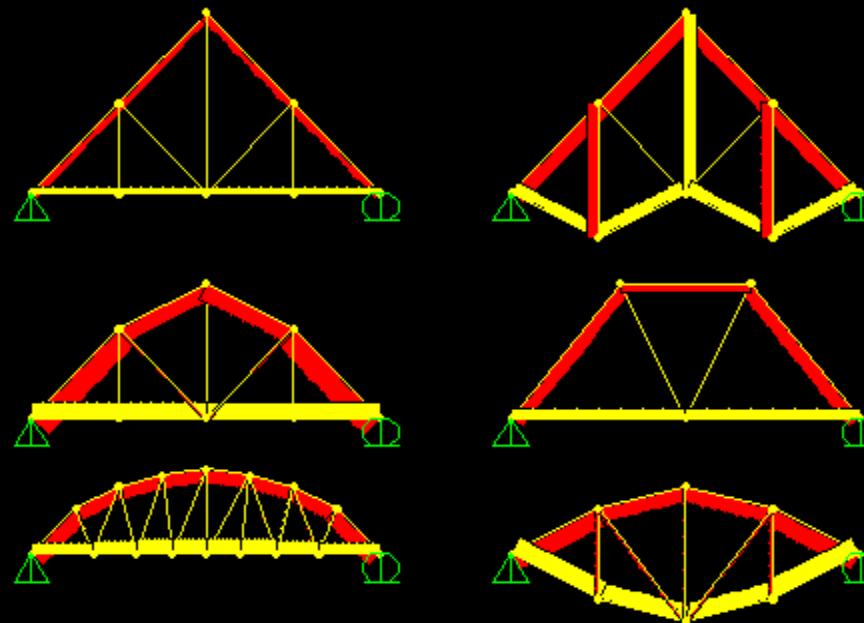


TRIANGULAR TRUSSES

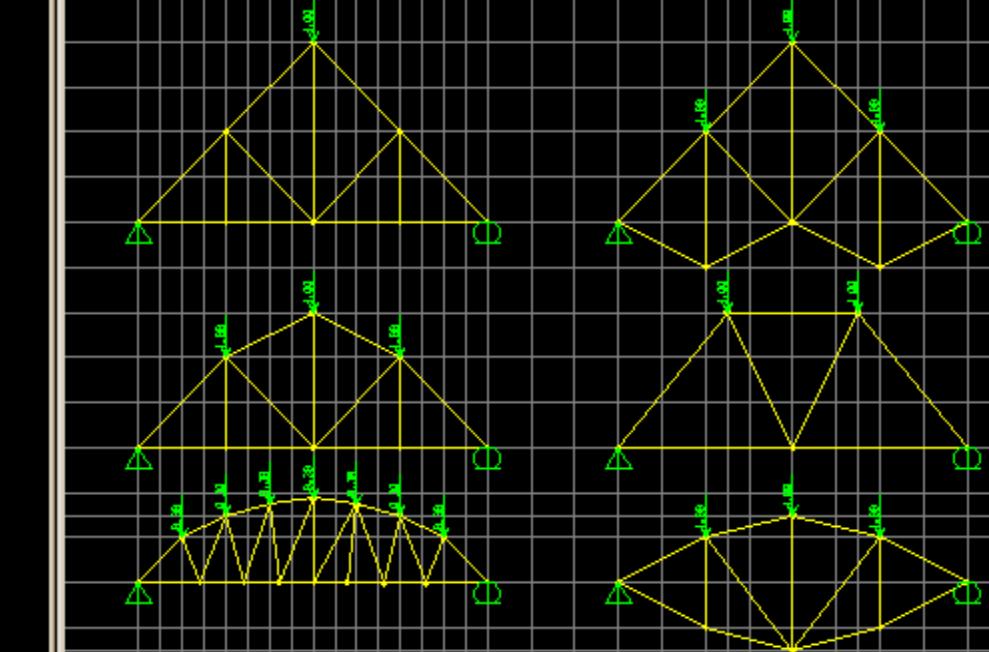
@Seismicisolation



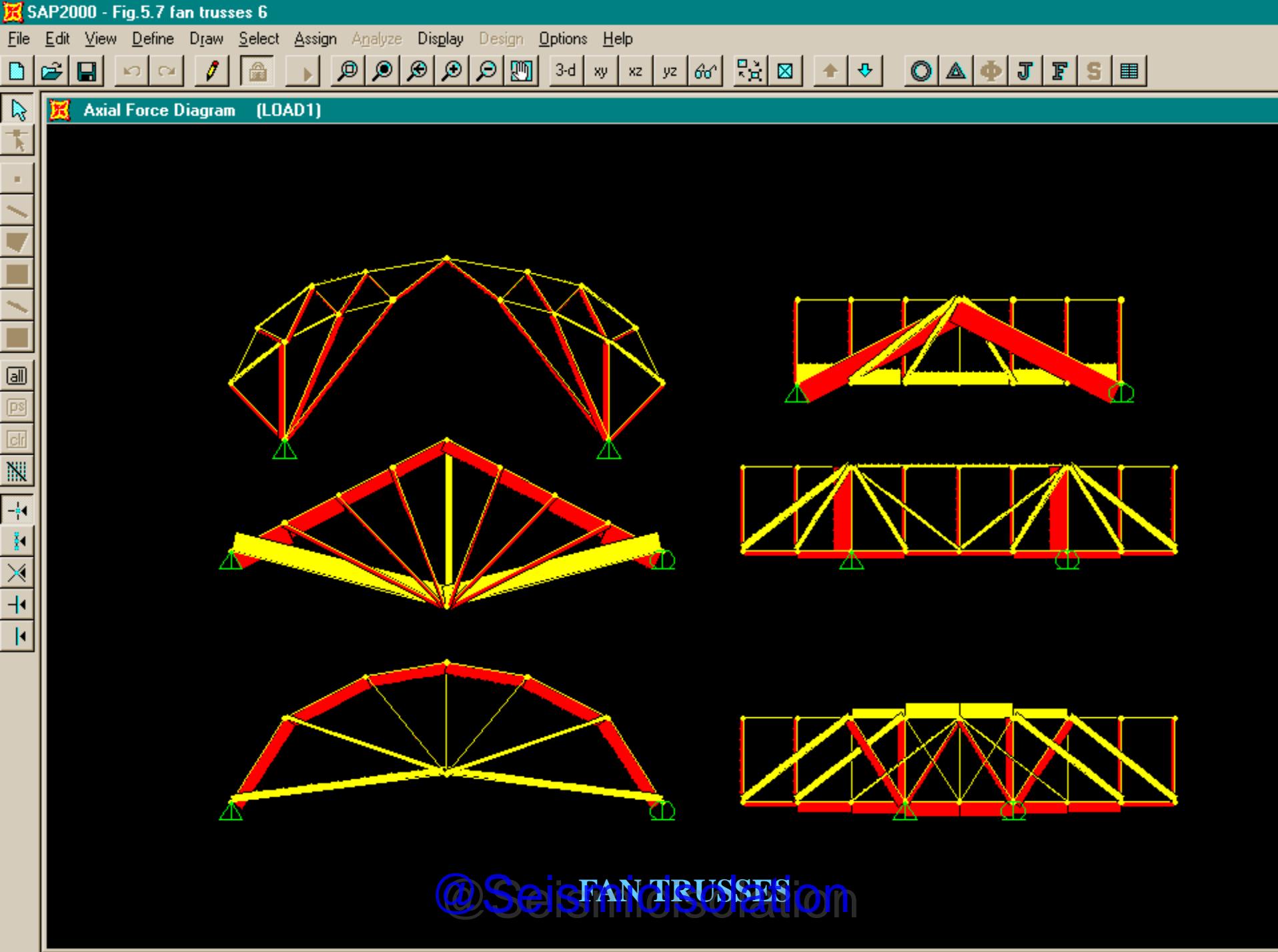
Axial Force Diagram (LOAD1)



Joint Loads (LOAD1)

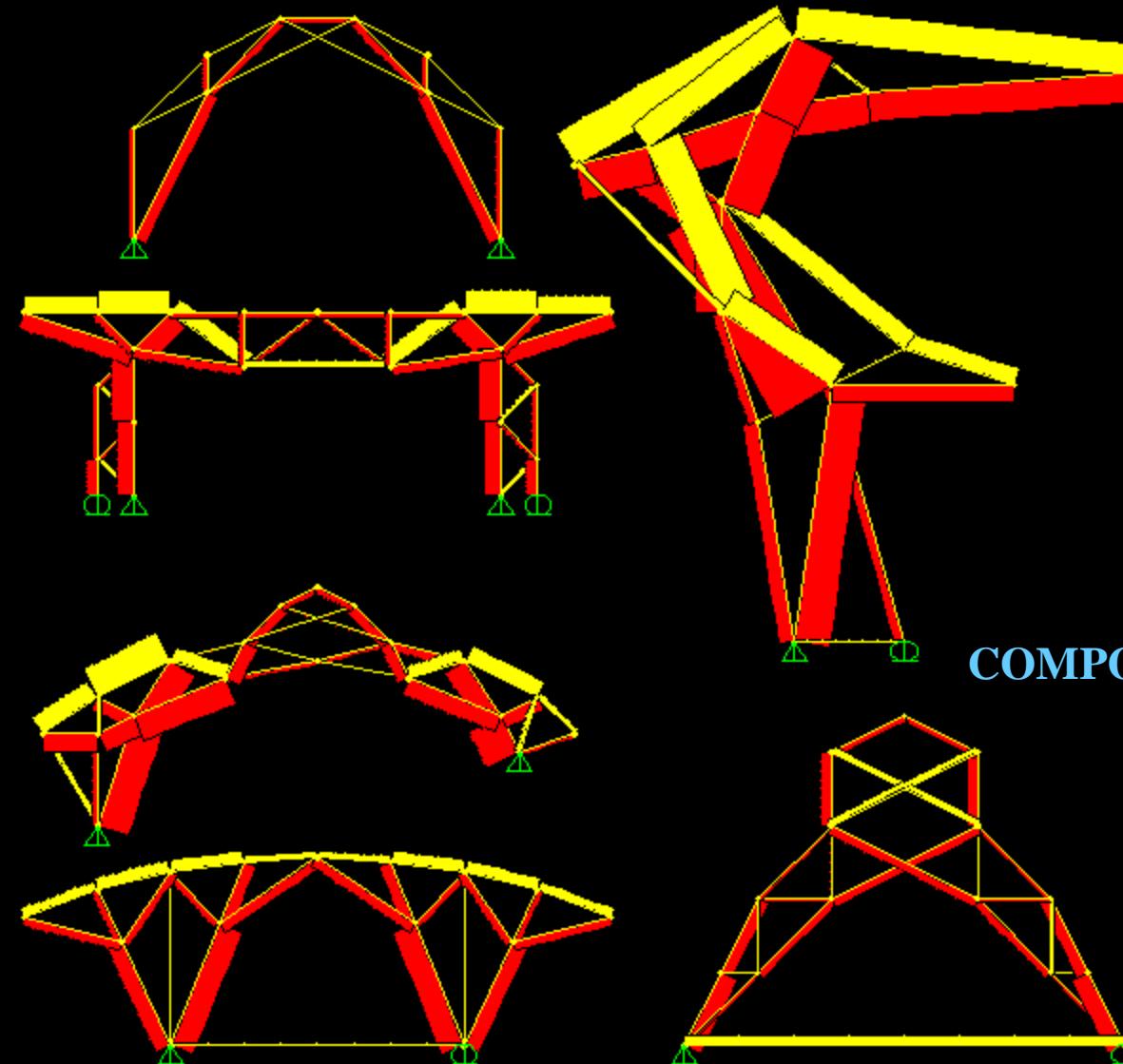


FUNICULAR TRUSSES @ Seismicisolation





Axial Force Diagram (LOAD1)



COMPOUND TRUSSES

@Seismicisolation

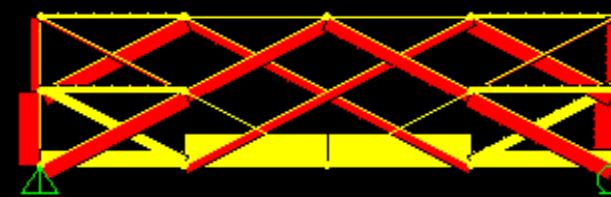
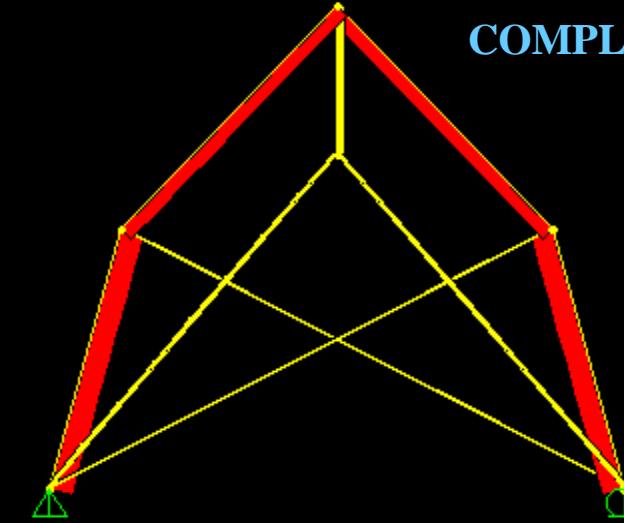
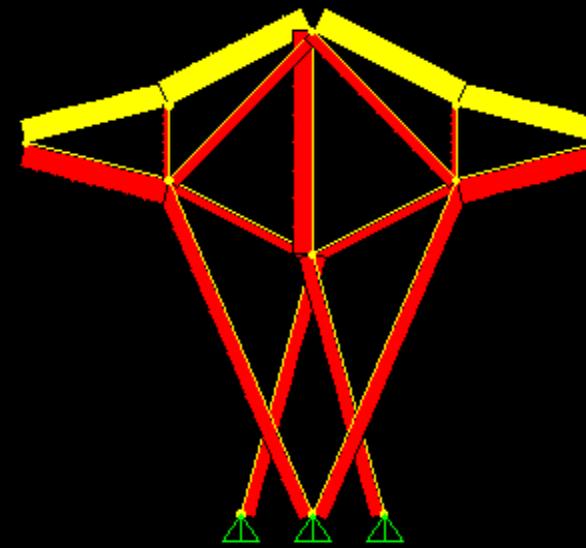
Click on any Frame Element for detailed diagram

Kip-It

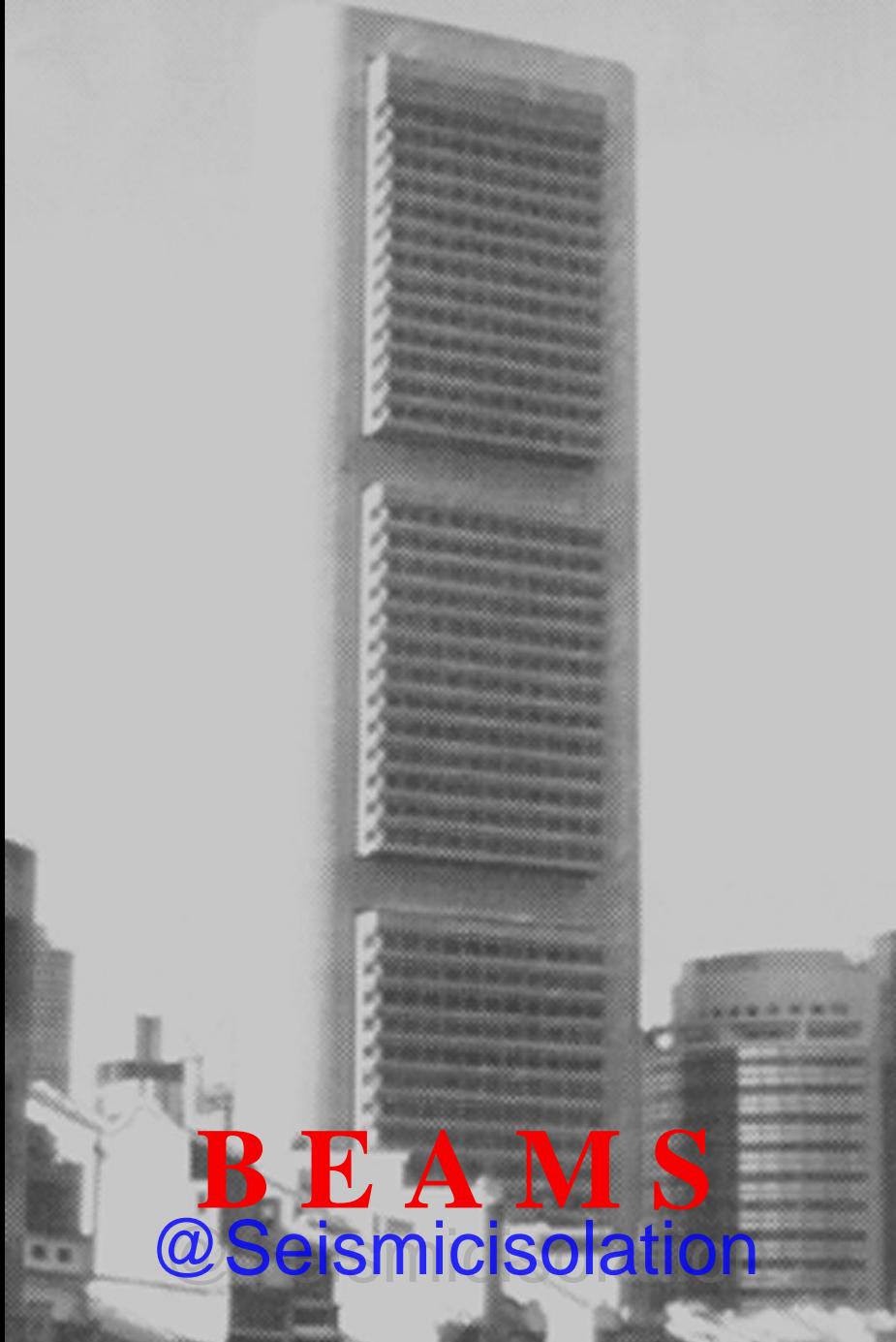


Axial Force Diagram (LOAD1)

COMPLEX TRUSSES

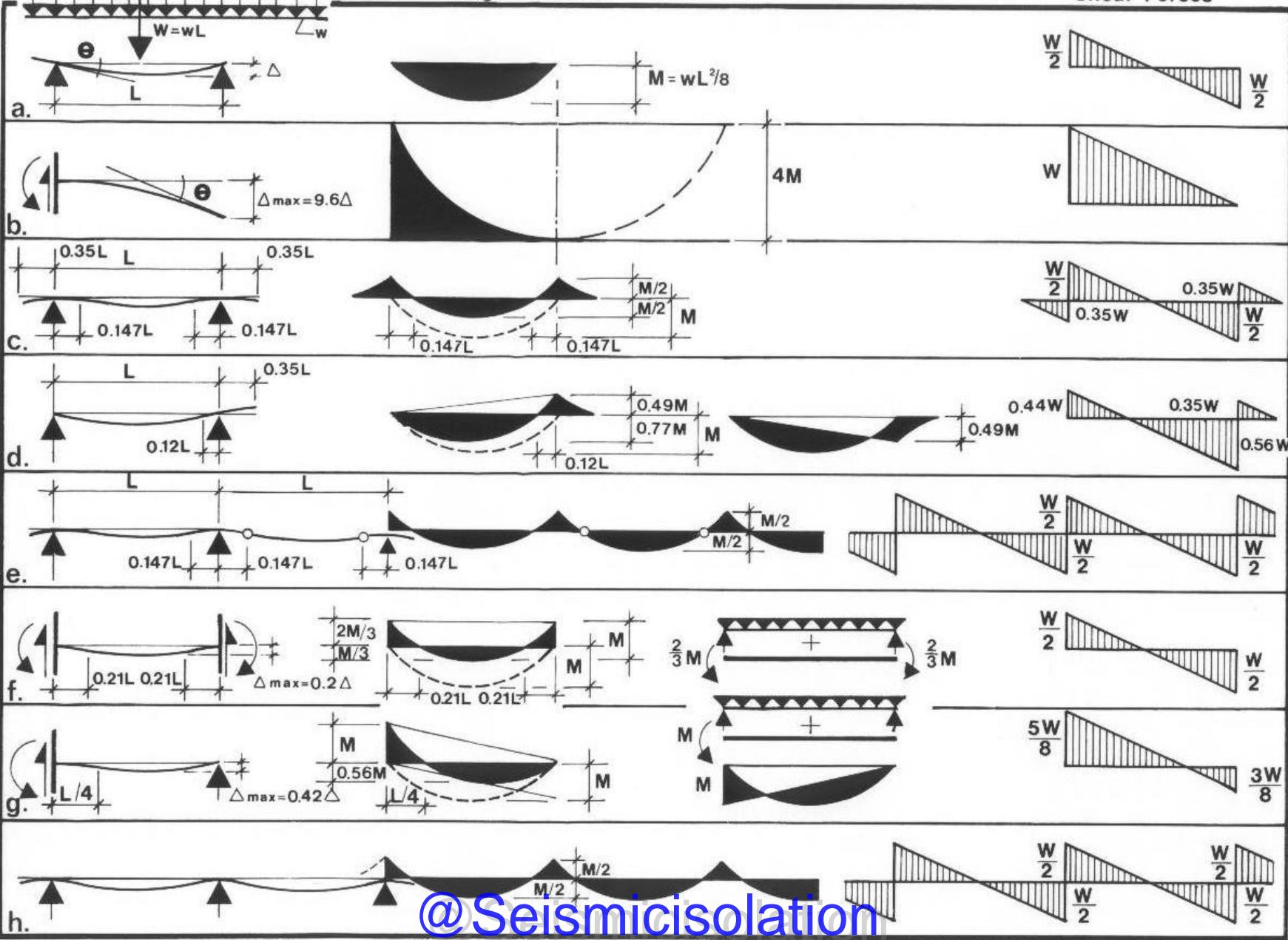


@Seismicisolation



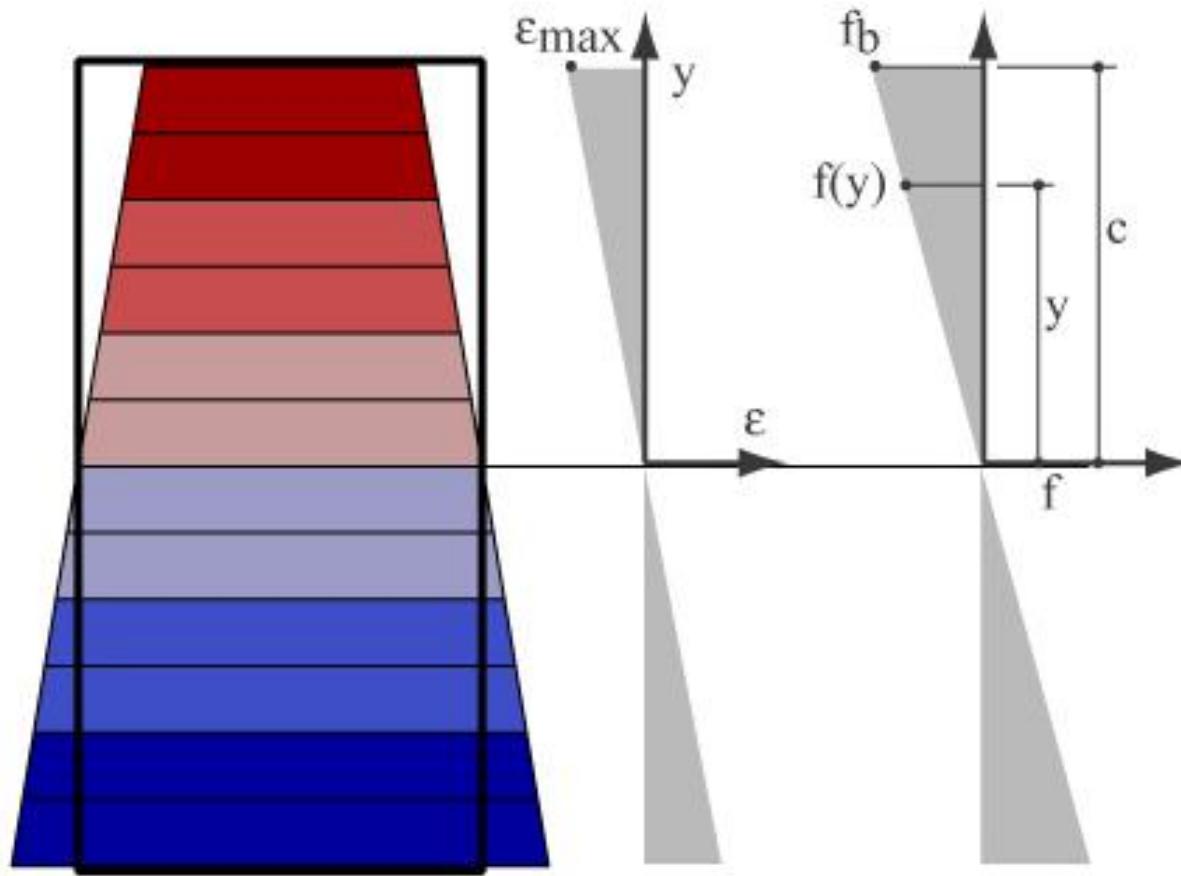
**B E A M S**  
@Seismicisolation

OBCB Center,  
Singapore, 1976,  
I.M. Pei/ Ove  
Arup

**Beam Types****Bending Moments****Shear Forces**

@Seismicisolation

$$\frac{f(y)}{y} = -\frac{f_b}{c} \rightarrow f(y) = \frac{f_b}{c} y$$



$$M = \sum_i F_i y_i$$

$$M = \sum_i (f_i A_i) y_i$$

$$M = \sum_i \left( \frac{f_b y_i}{c} A_i \right) y_i$$

$$M = \frac{f_b}{c} \sum_i A_i y_i^2$$

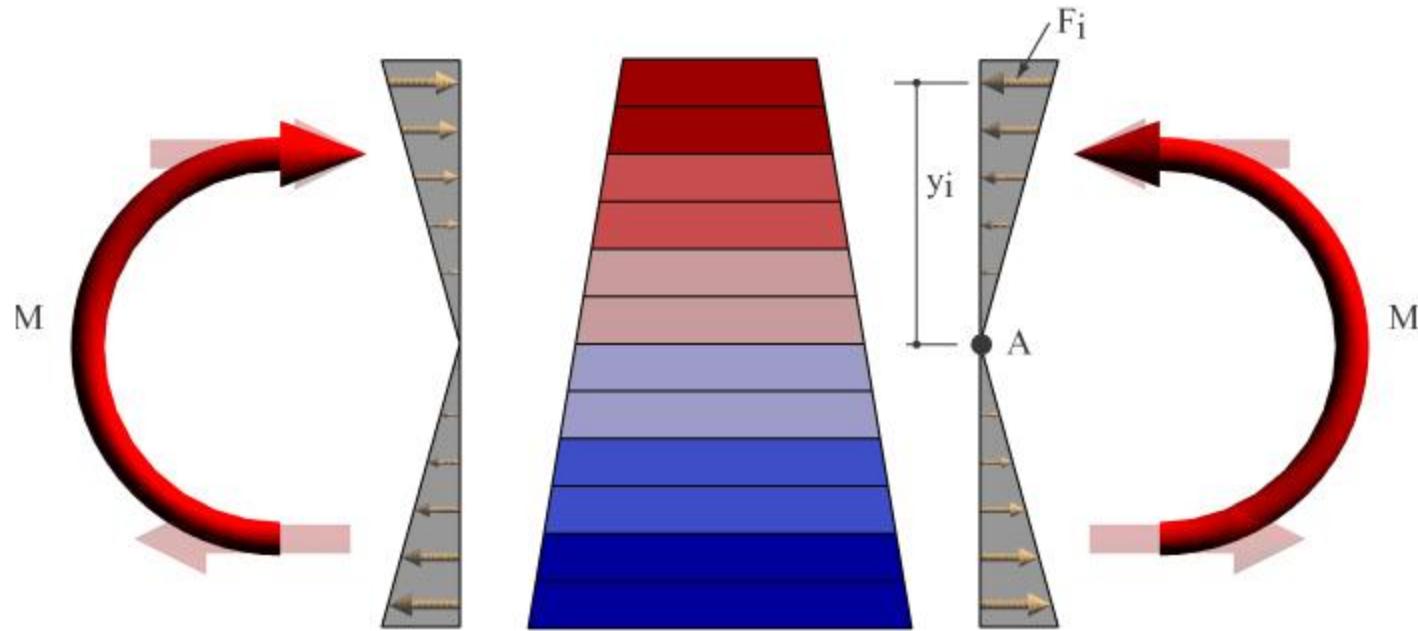
$$M = \frac{f_b}{c} I$$

$$f_b = \frac{Mc}{I}$$

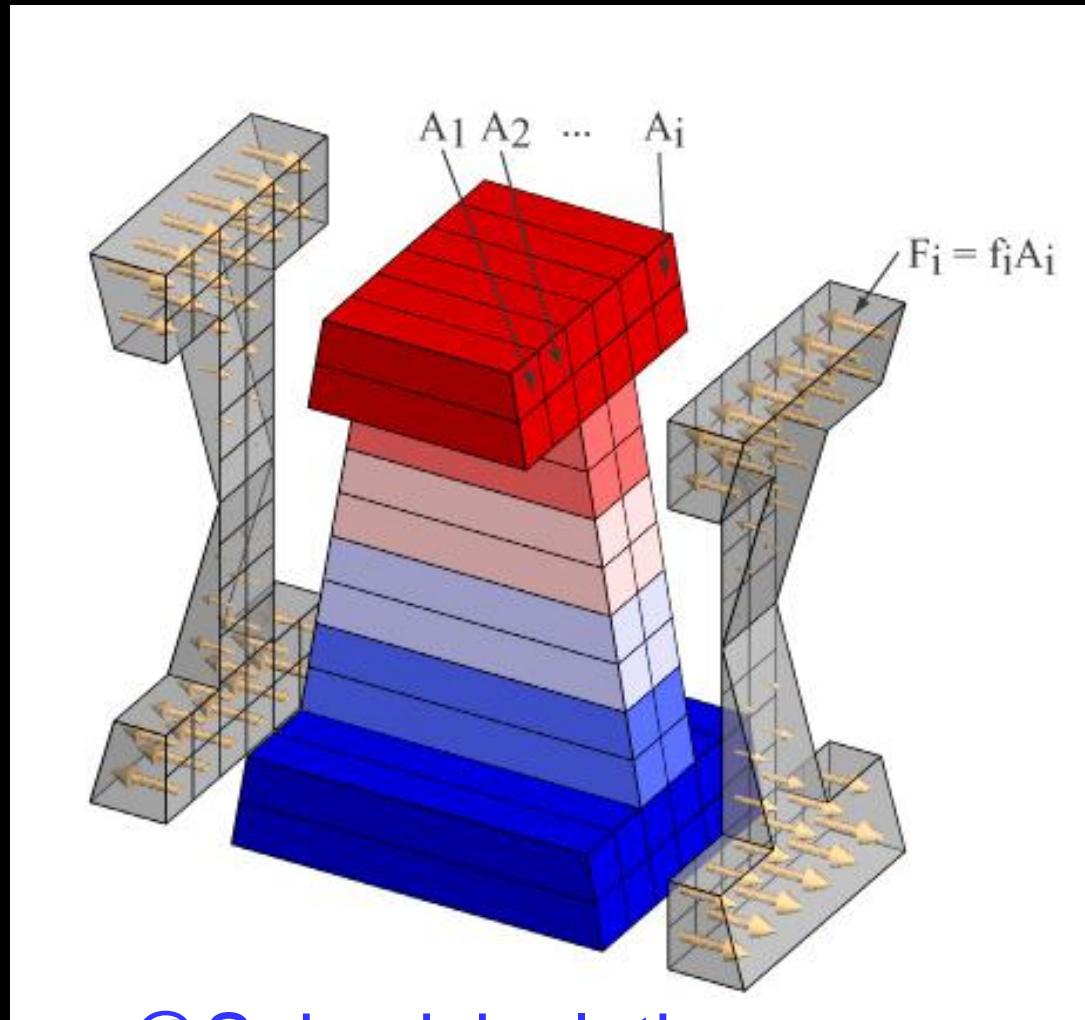
$$I = \sum_i A_i y_i^2$$

$$S = \frac{I}{c}$$

$$f_b = \frac{M}{S}$$



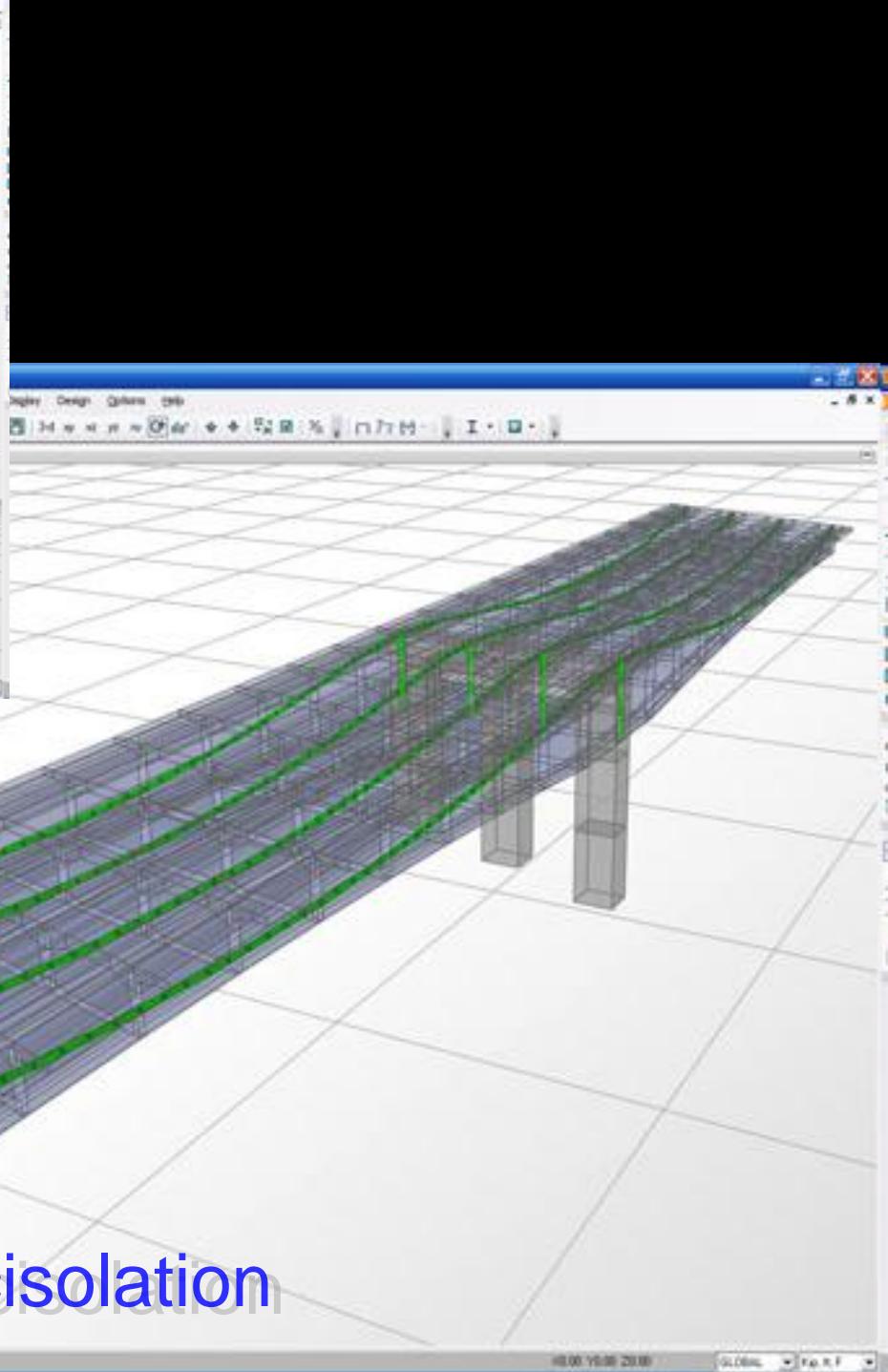
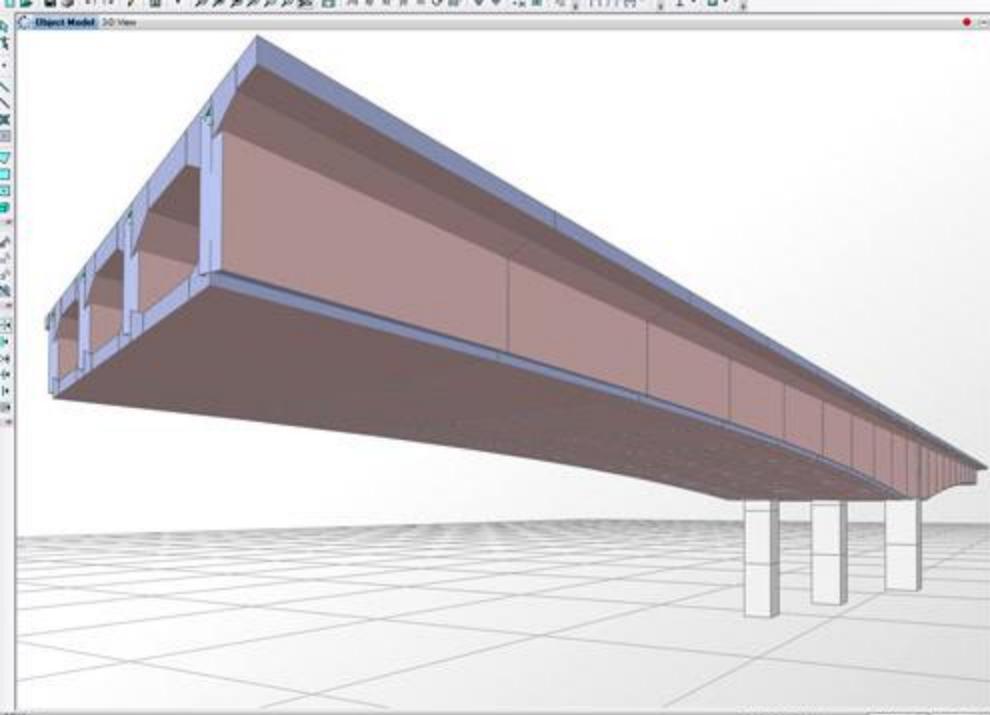
## General Form of the Flexure Formula



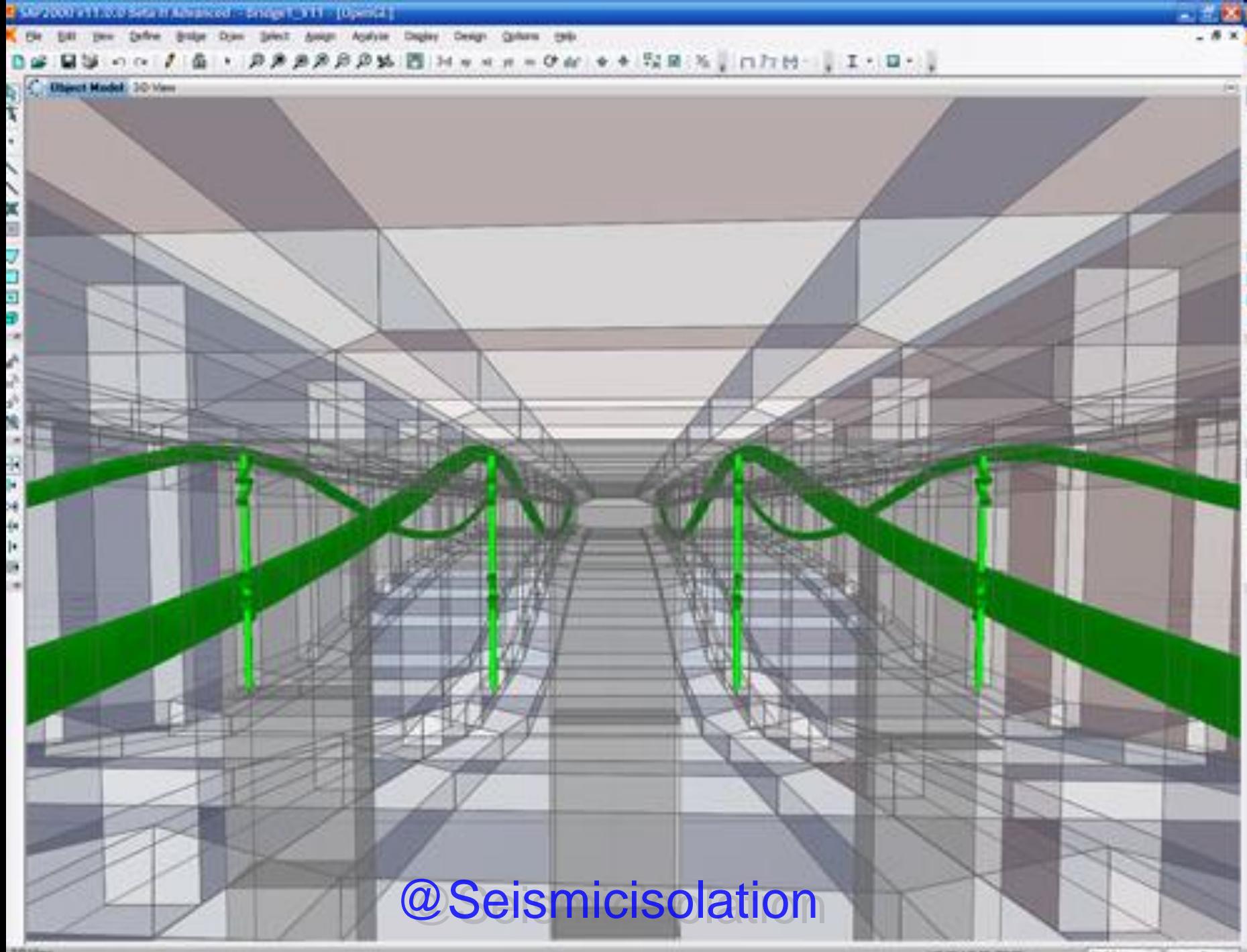
@Seismicisolation



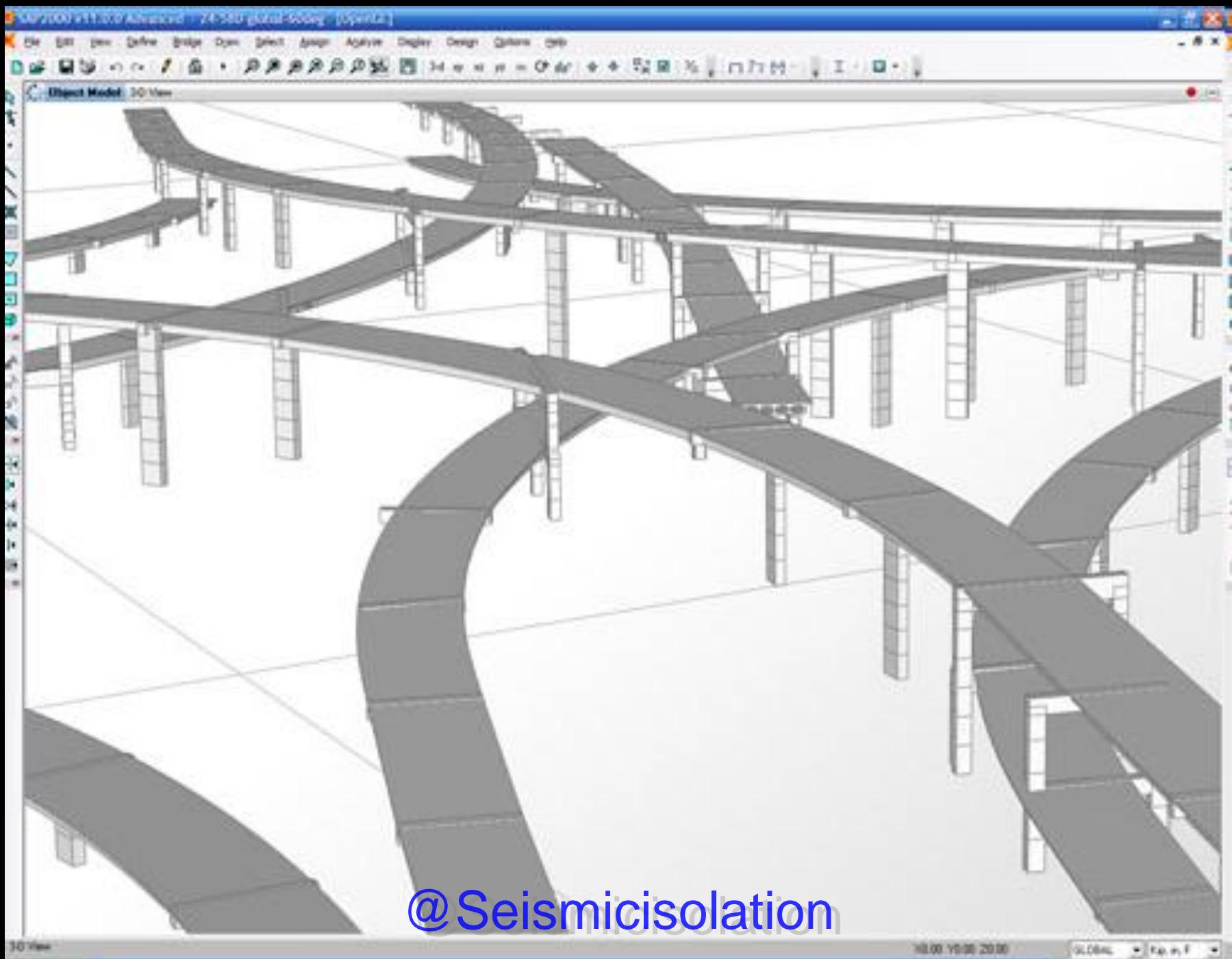
Incheon International Airport, Seoul, 2001, Fentress Bradburn Arch.  
Seismic isolation



@Seismicisolation



@Seismicisolation

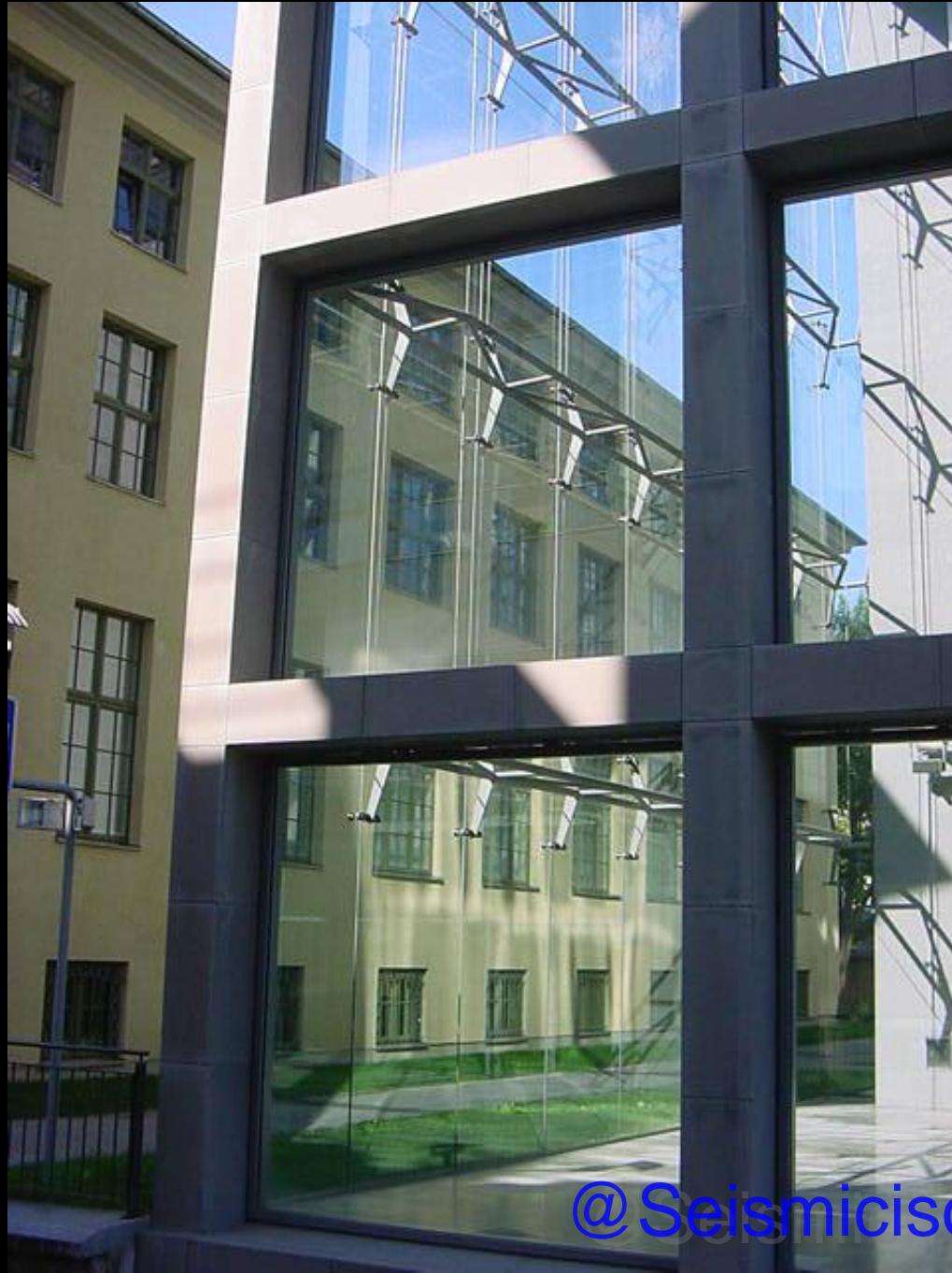


@Seismicisolation



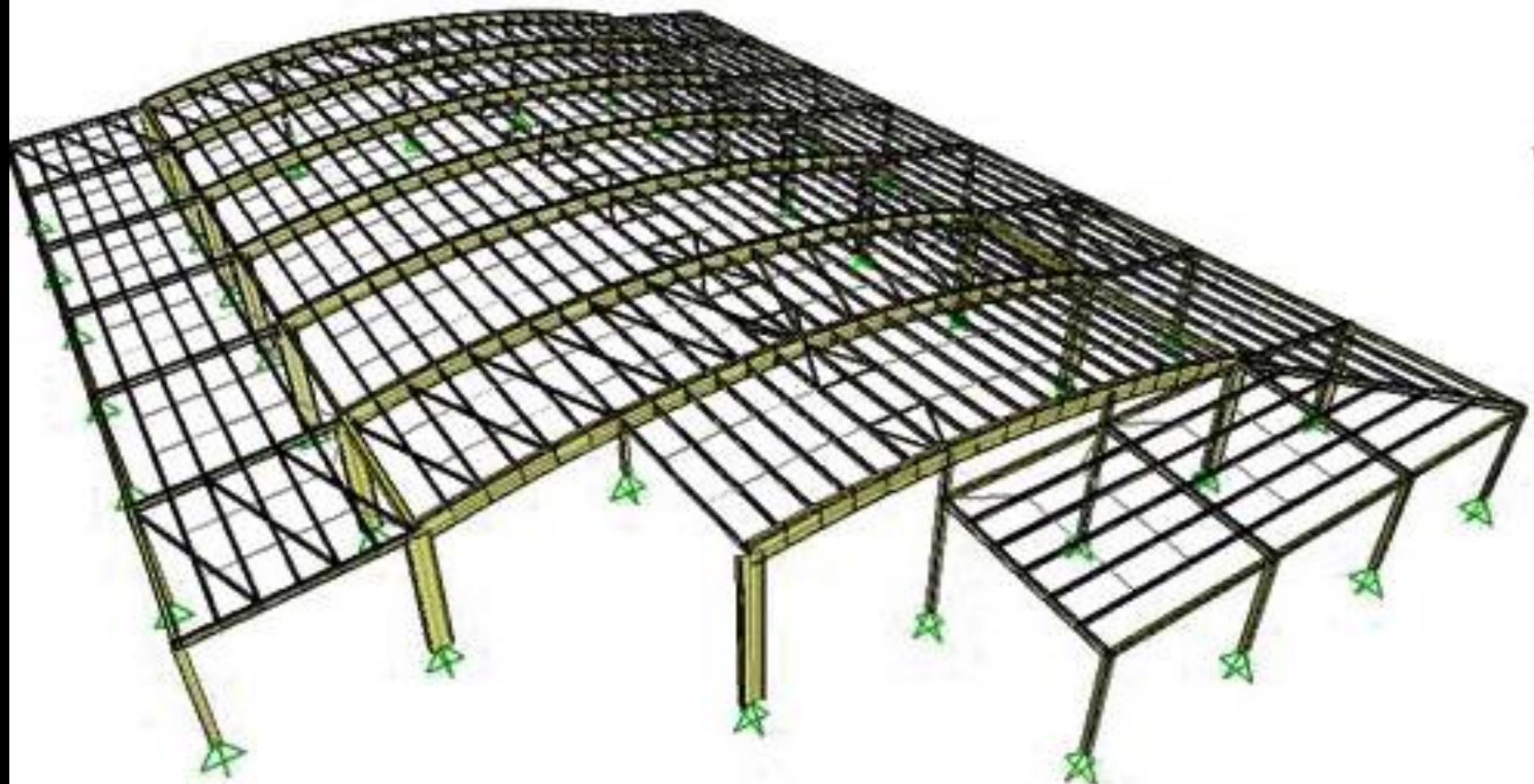
@Seismicisolation

Ningbo downtown,  
2002, Qingyun Ma



**Dresdner Bank,  
Verwaltungszentrum, Leipzig,  
1997, Engel und Zimmermann**

@Seismicisolation



@Seismicisolation



National Gallery @Seismicisolation  
of Art, East Wing, Washington, 1978, I.M. Pei

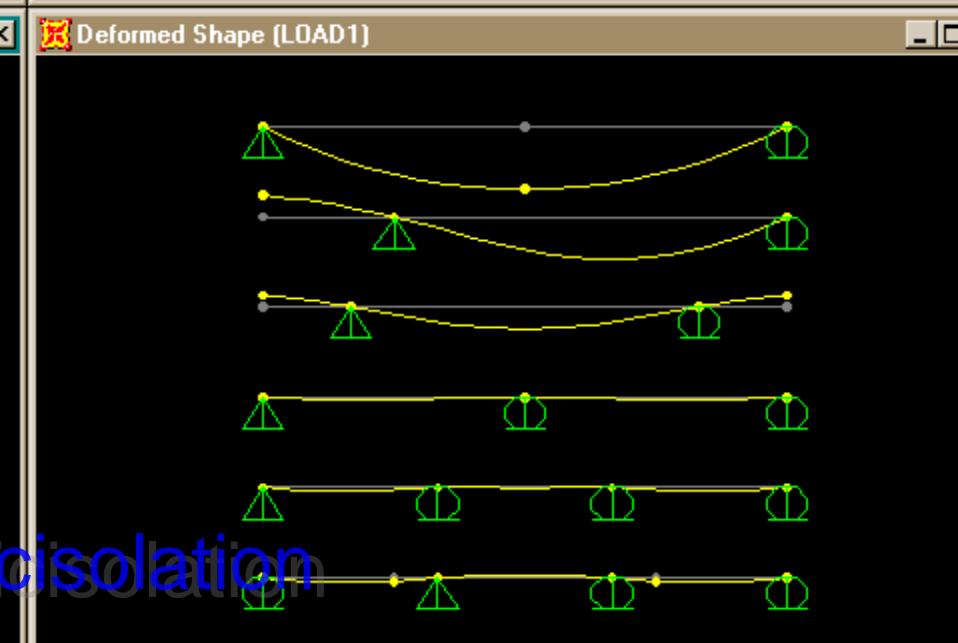
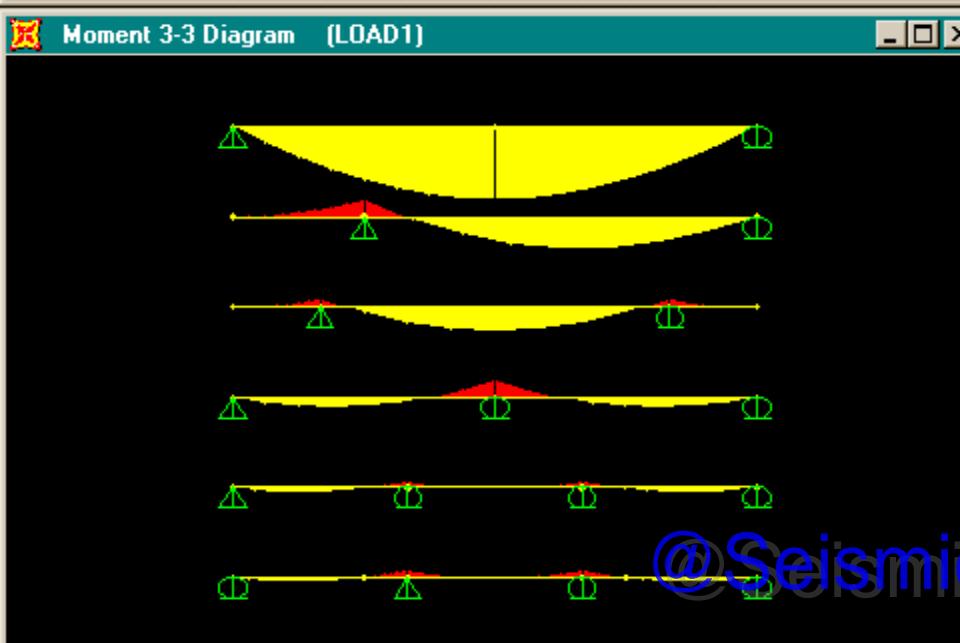
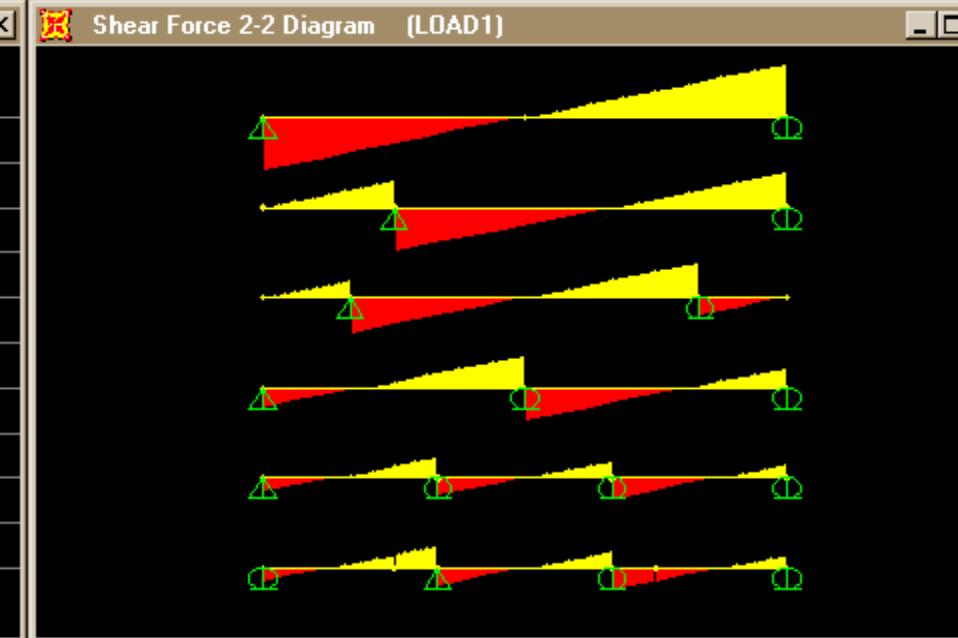
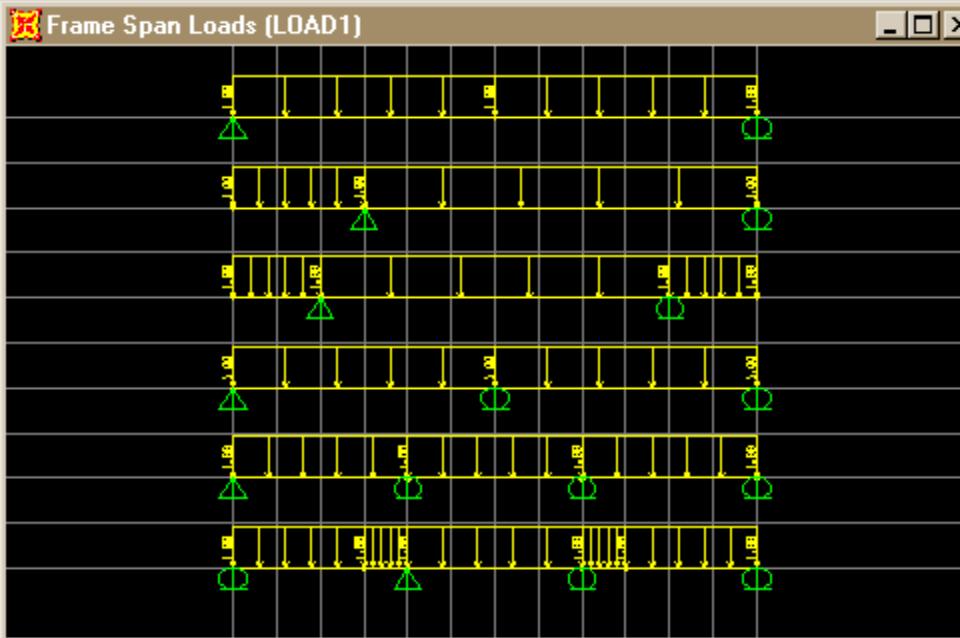
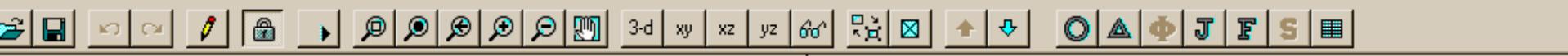


@Seismicisolation  
Atrium, Germanisches Museum, Nuremberg, Germany

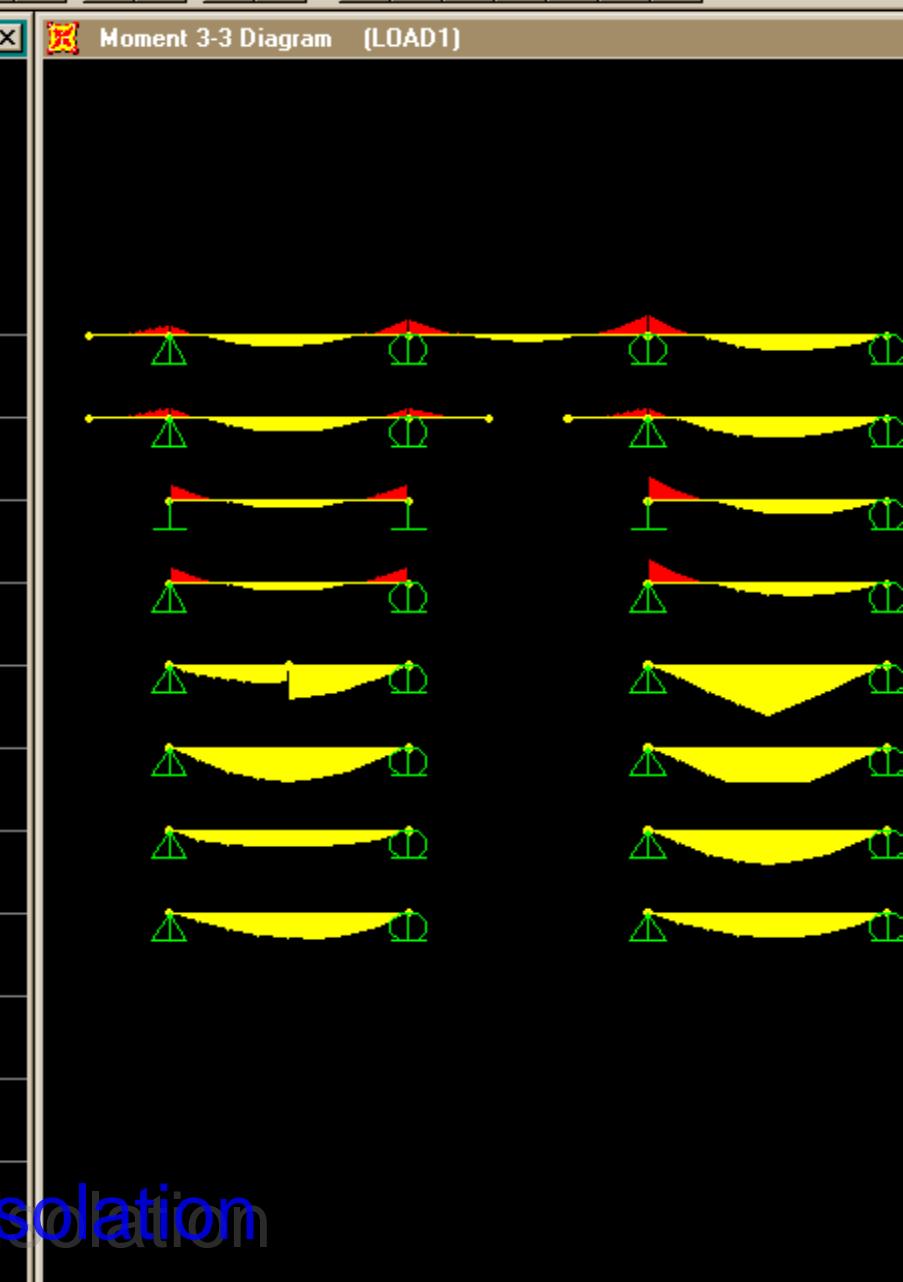
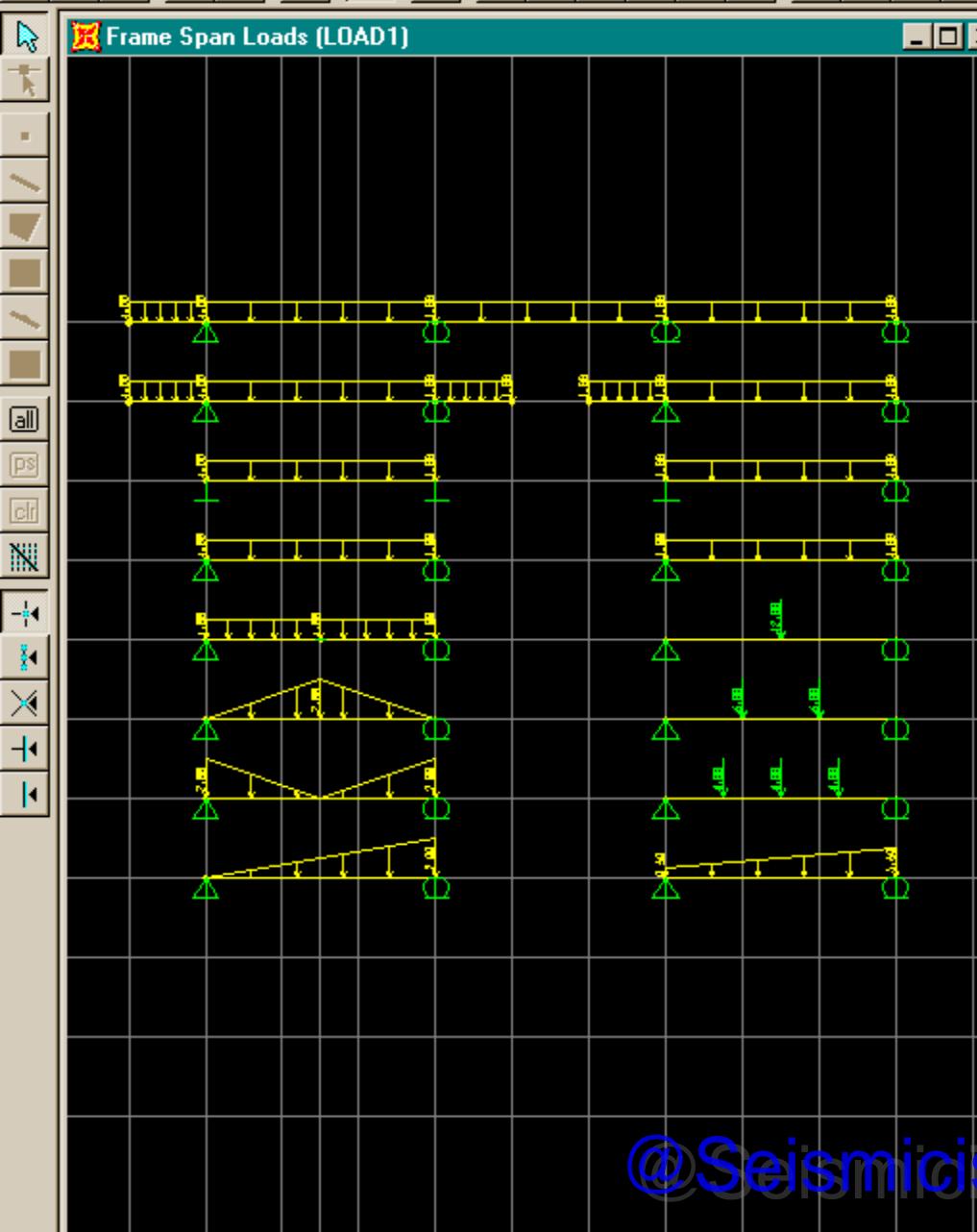


@Seismicisolation

**Shanghai-Pudong  
International Airport,  
2001, Paul Andreu  
principal architect, Coyne  
et Bellier structural  
engineers**



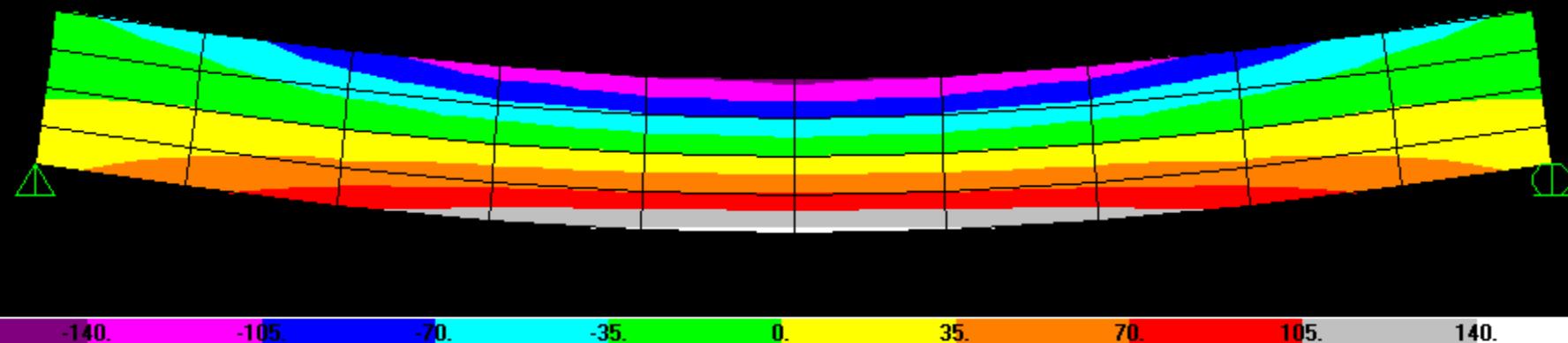
@Seismicisolation



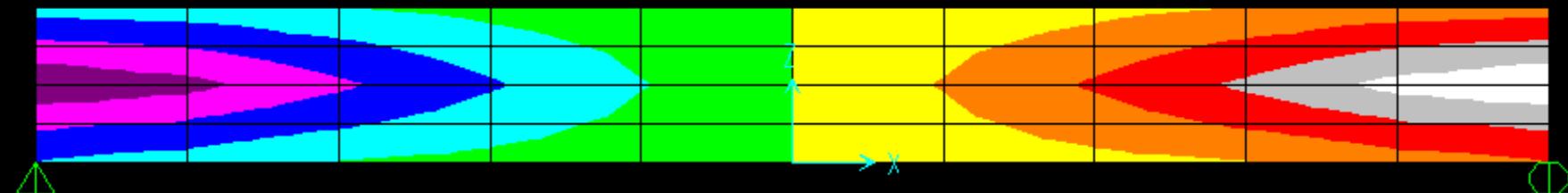
@Seismicisolation



Stress S11 Diagram (LOAD1)



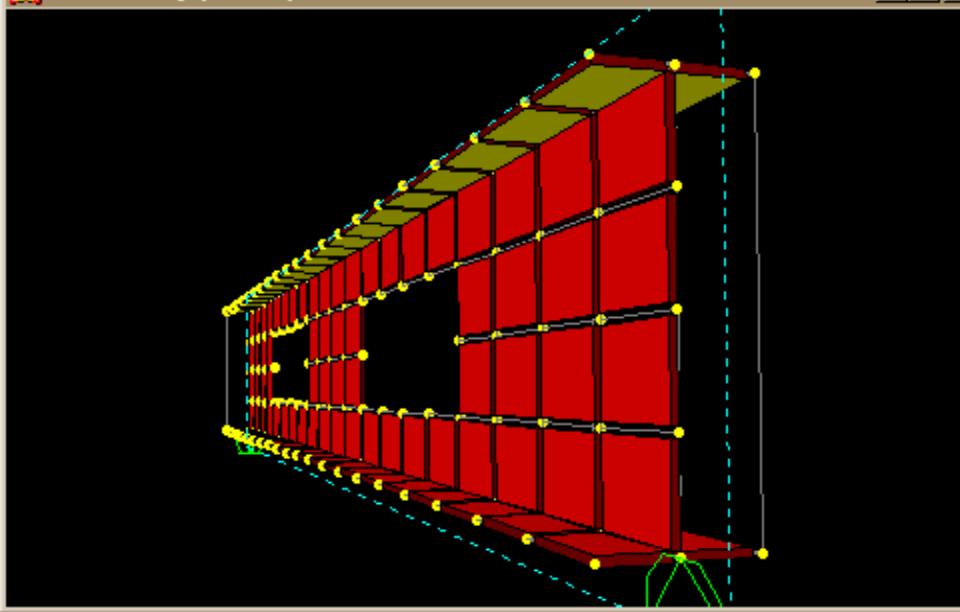
Stress S12 Diagram (LOAD1)



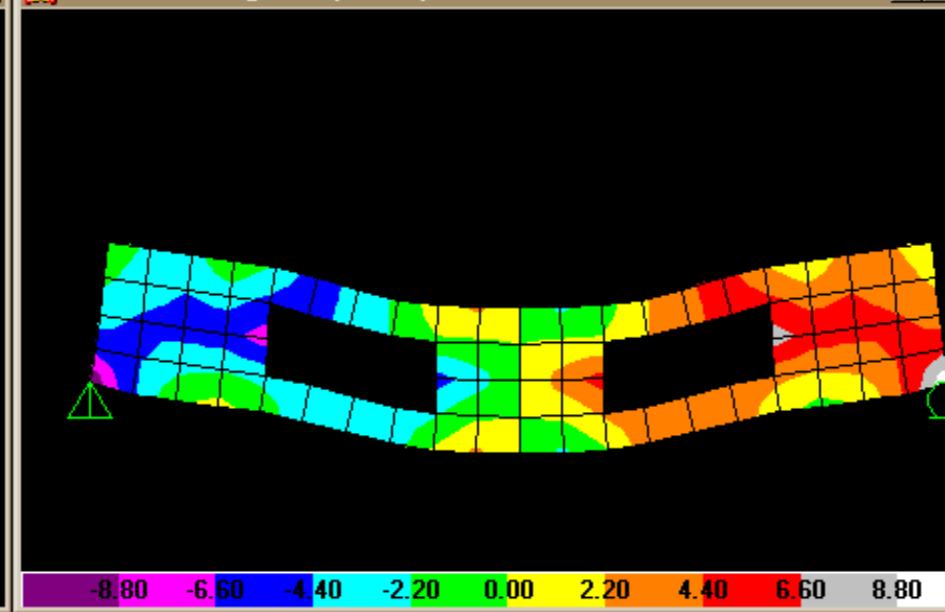
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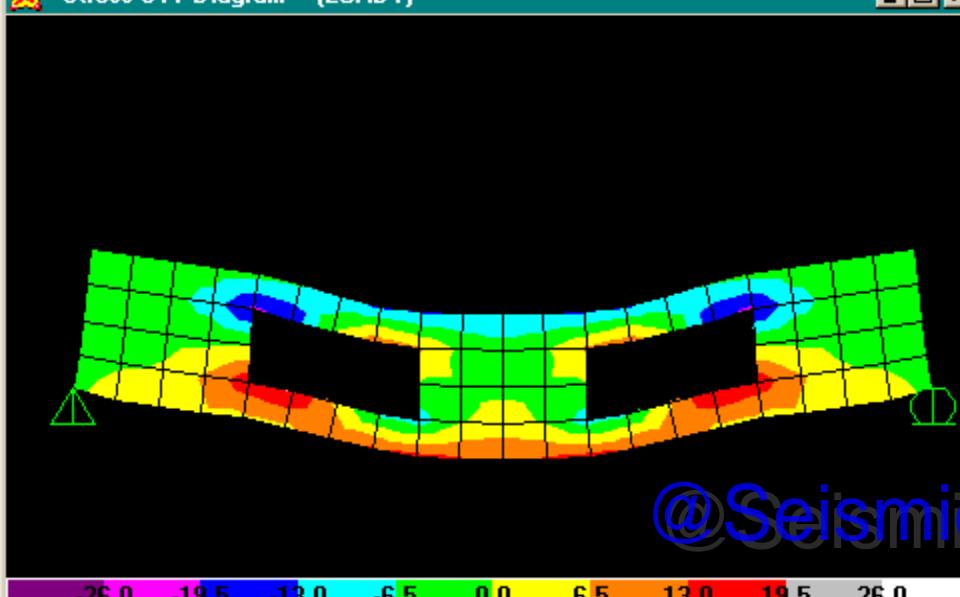
Shell Gravity (LOAD1)



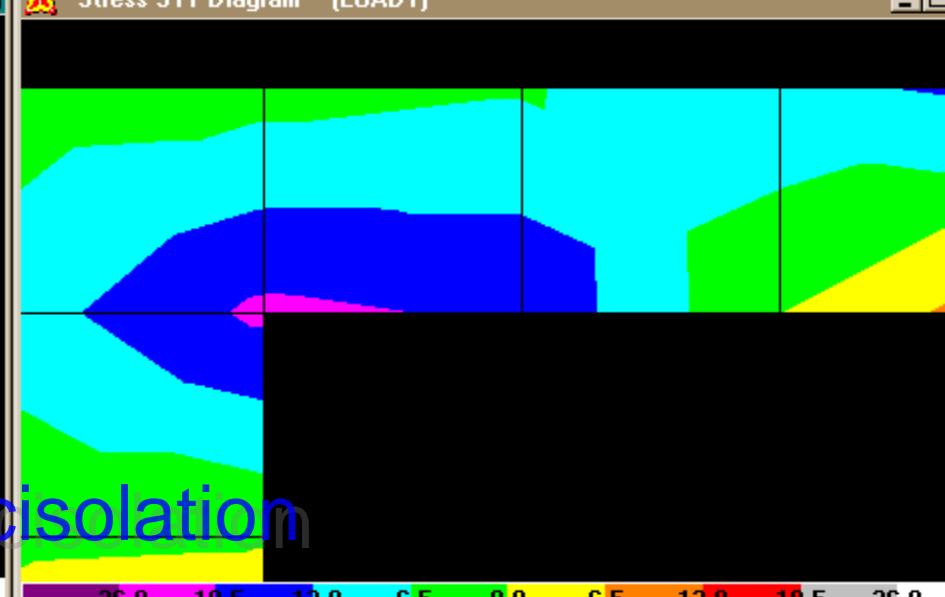
Stress S12 Diagram (LOAD1)



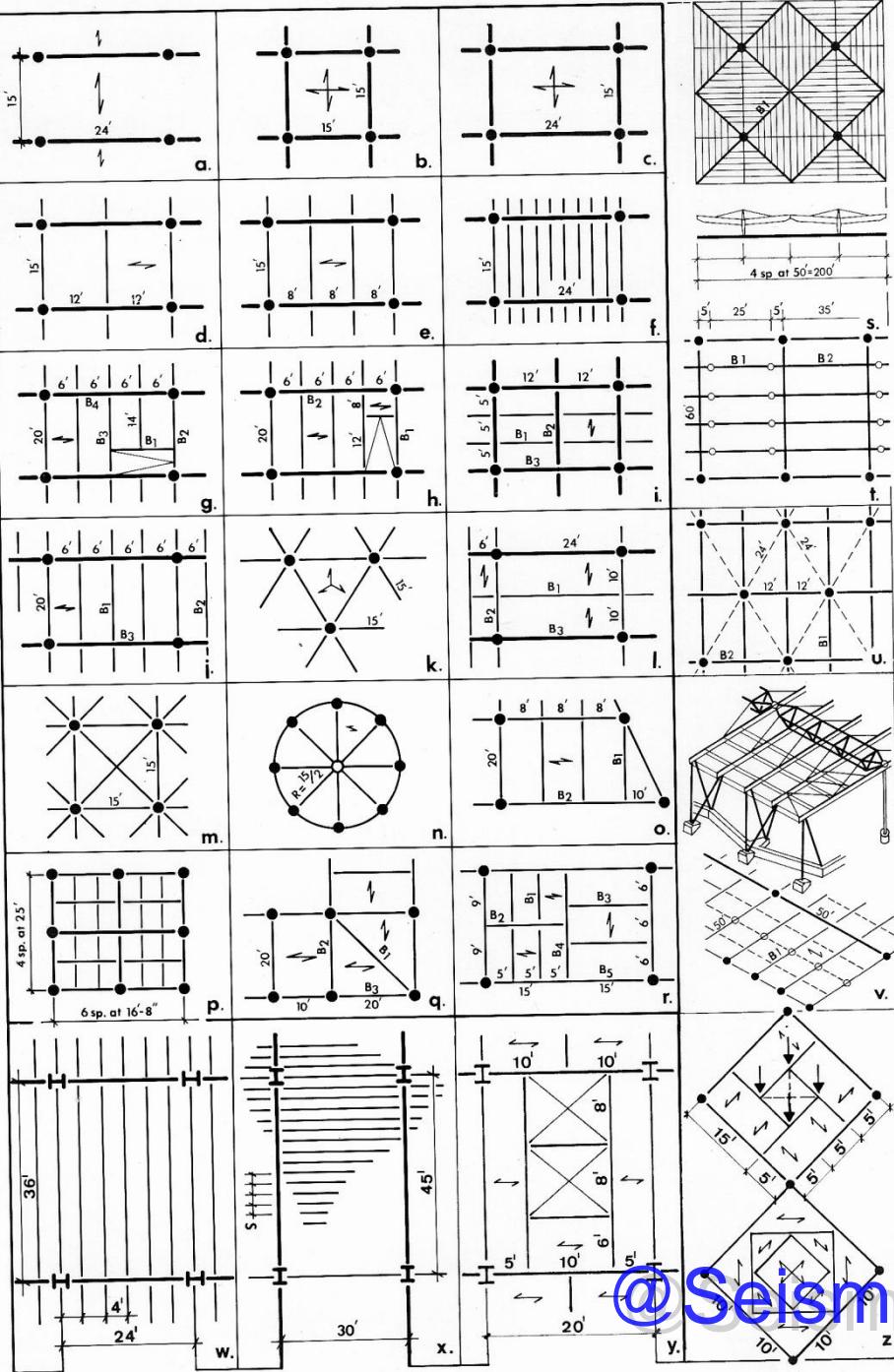
Stress S11 Diagram (LOAD1)



Stress S11 Diagram (LOAD1)



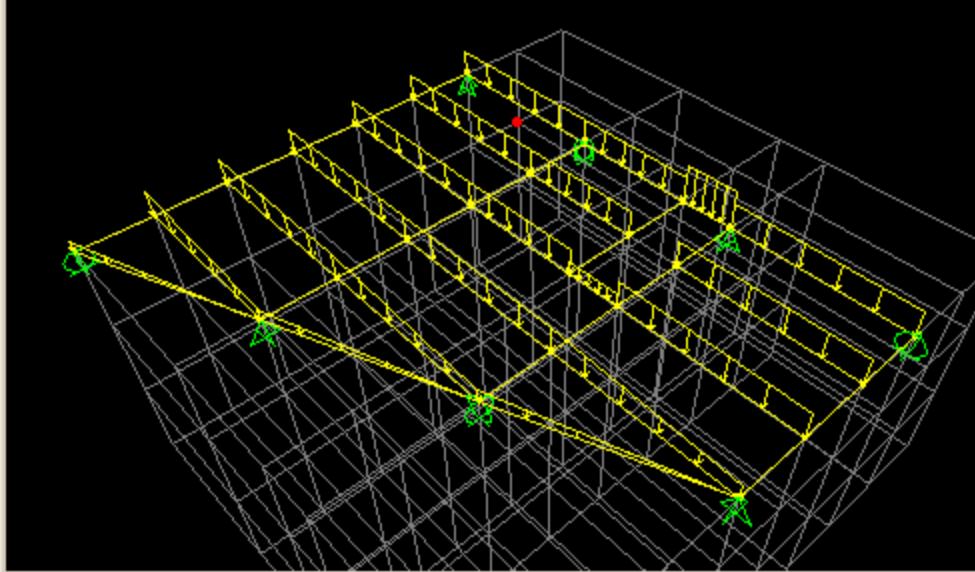
@Seismicisolation



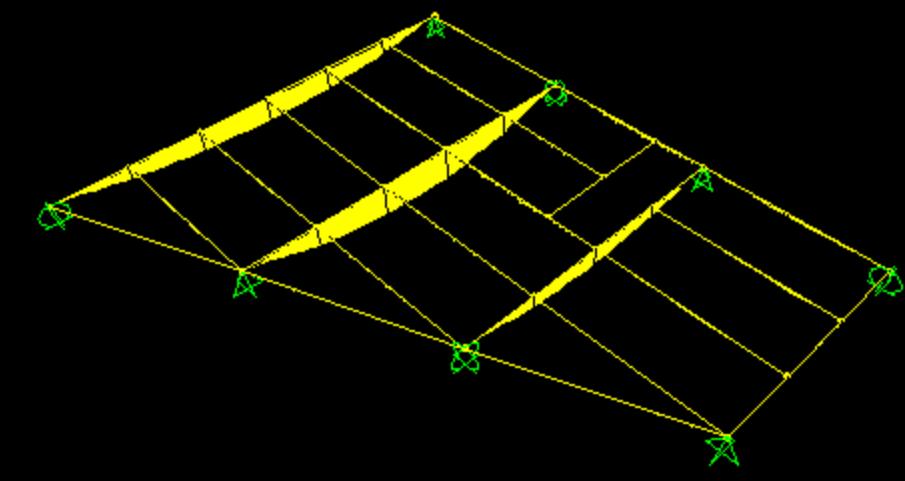
FLOOR-ROOF FRAMING SYSTEMS  
@Seismicisolation



Frame Span Loads (LOAD1)



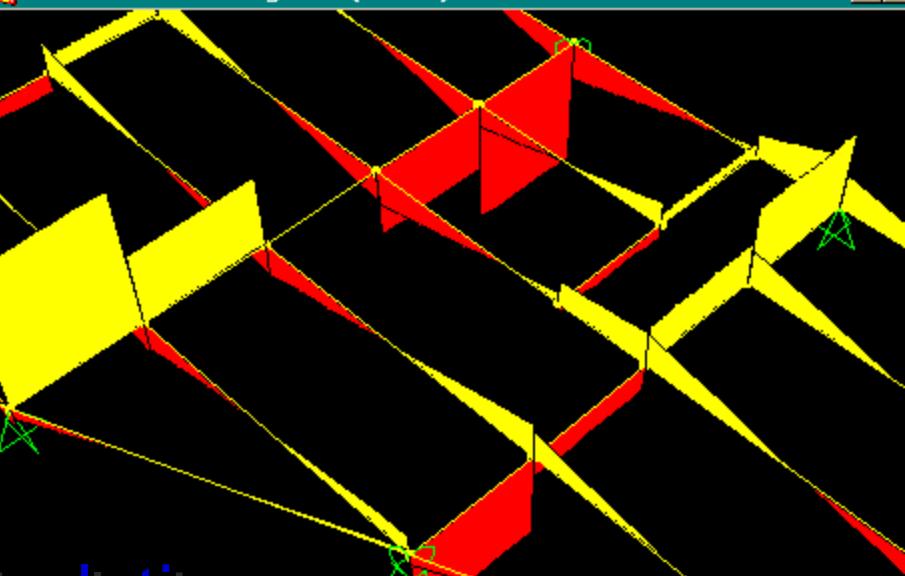
Moment 3-3 Diagram (LOAD1)



Moment 3-3 Diagram (LOAD1)



Shear Force 2-2 Diagram (LOAD1)



Click on any Frame Element for detailed diagram

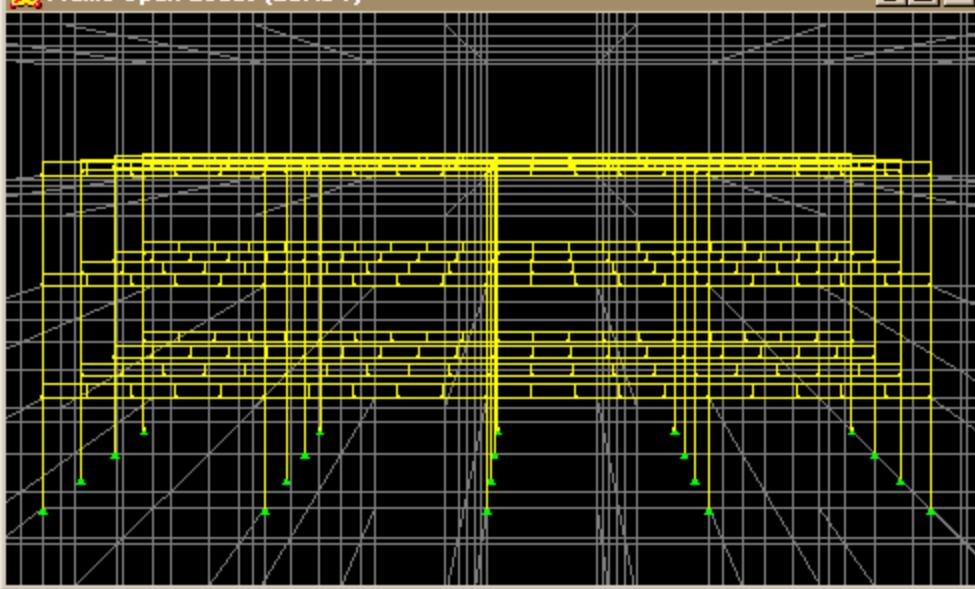
@Seismicisolation

X20.00 Y30.00 Z22.00

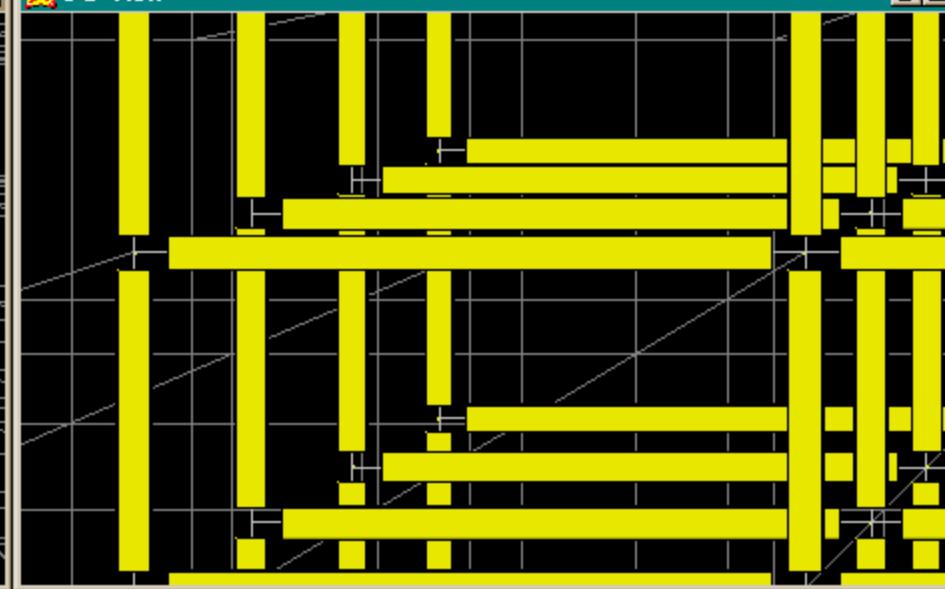
Kip-It



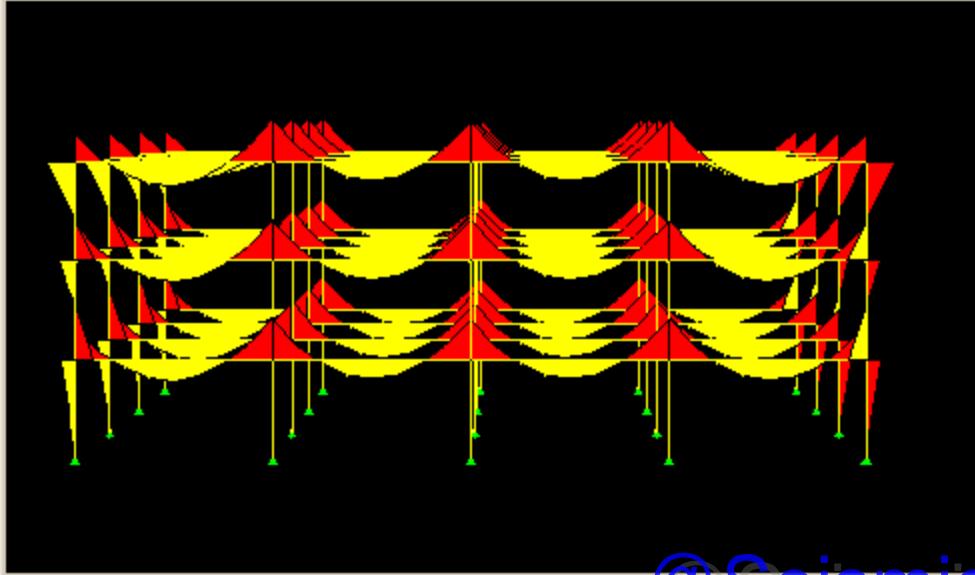
Frame Span Loads (LOAD1)



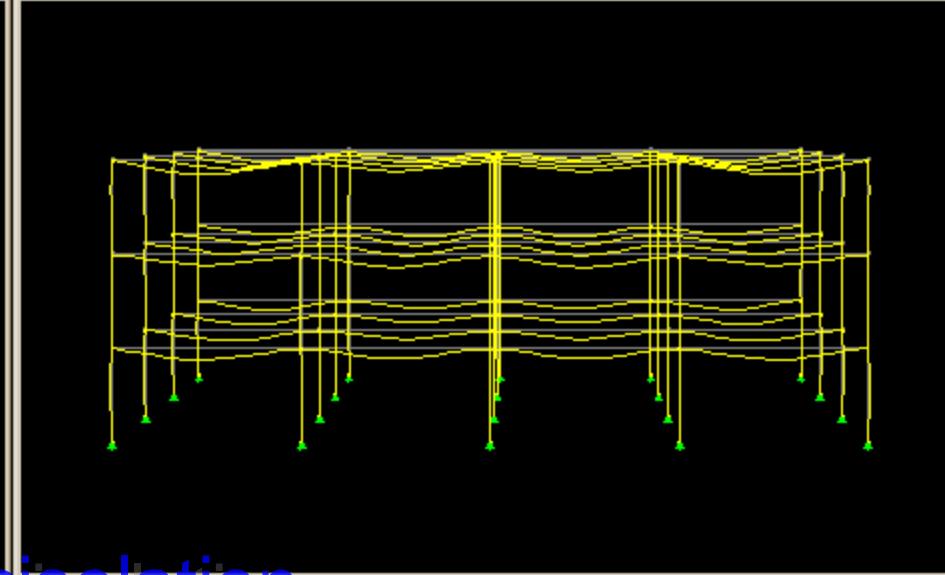
3-D View



Moment 3-3 Diagram (LOAD1)



Deformed Shape (LOAD1)



@Seismicisolation



# CABLE STRUCTURES

@Seismicisolation

Lufthansa hangar, Munich, Germany, 1992, Guenther Buechl + Fred Angerer Arch

# CABLE STRUCTURES

## Single-layer, simply suspended cable roofs

Single-curvature and dish-shaped (synclastic) hanging roofs

## Prestressed tensile membranes and cable nets

Edge-supported saddle roofs

Mast-supported conical saddle roofs

Arch-supported saddle roofs

Air supported structures and air-inflated structures (air members)

## Cable-supported structures

cable-supported beams and arched beams

cable-stayed bridges

cable-stayed roof structures

## Tensegrity structures

Planar open and closed tensegrity systems: **cable beams**, **cable trusses**, **cable frames**

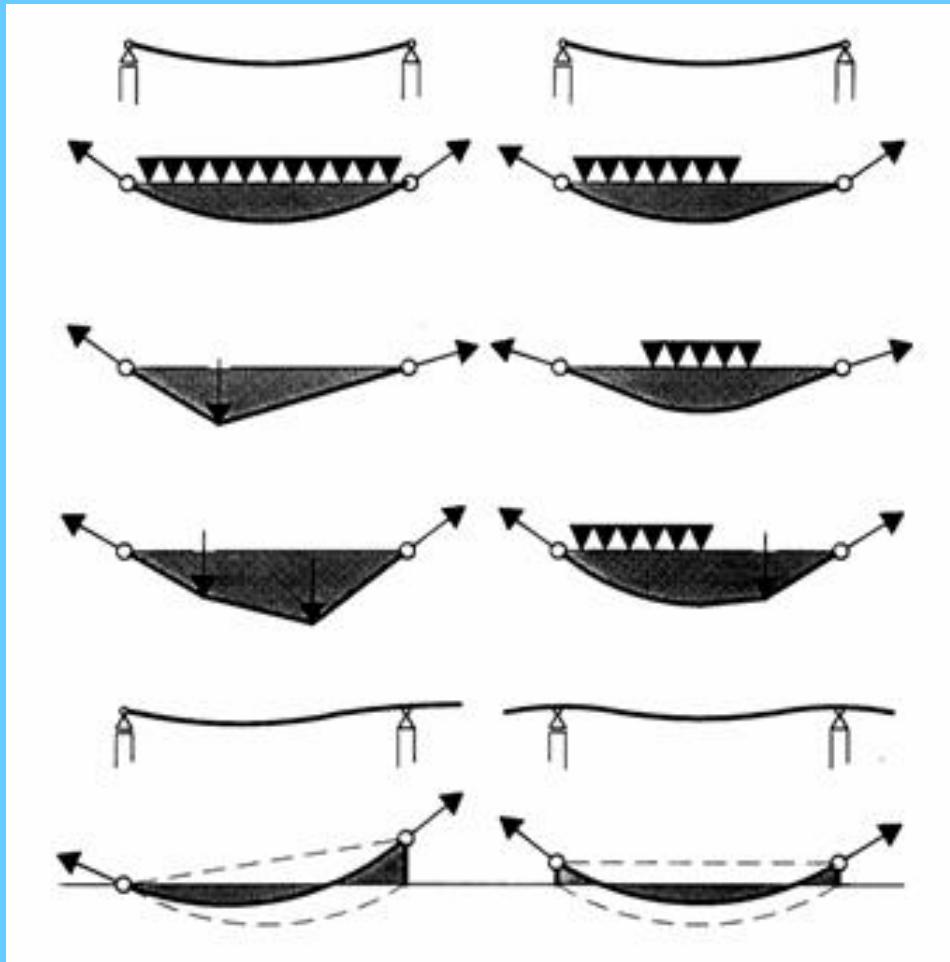
Spatial open tensegrity systems: **cable domes**

Spatial closed tensegrity systems: **polyhedral twist units**

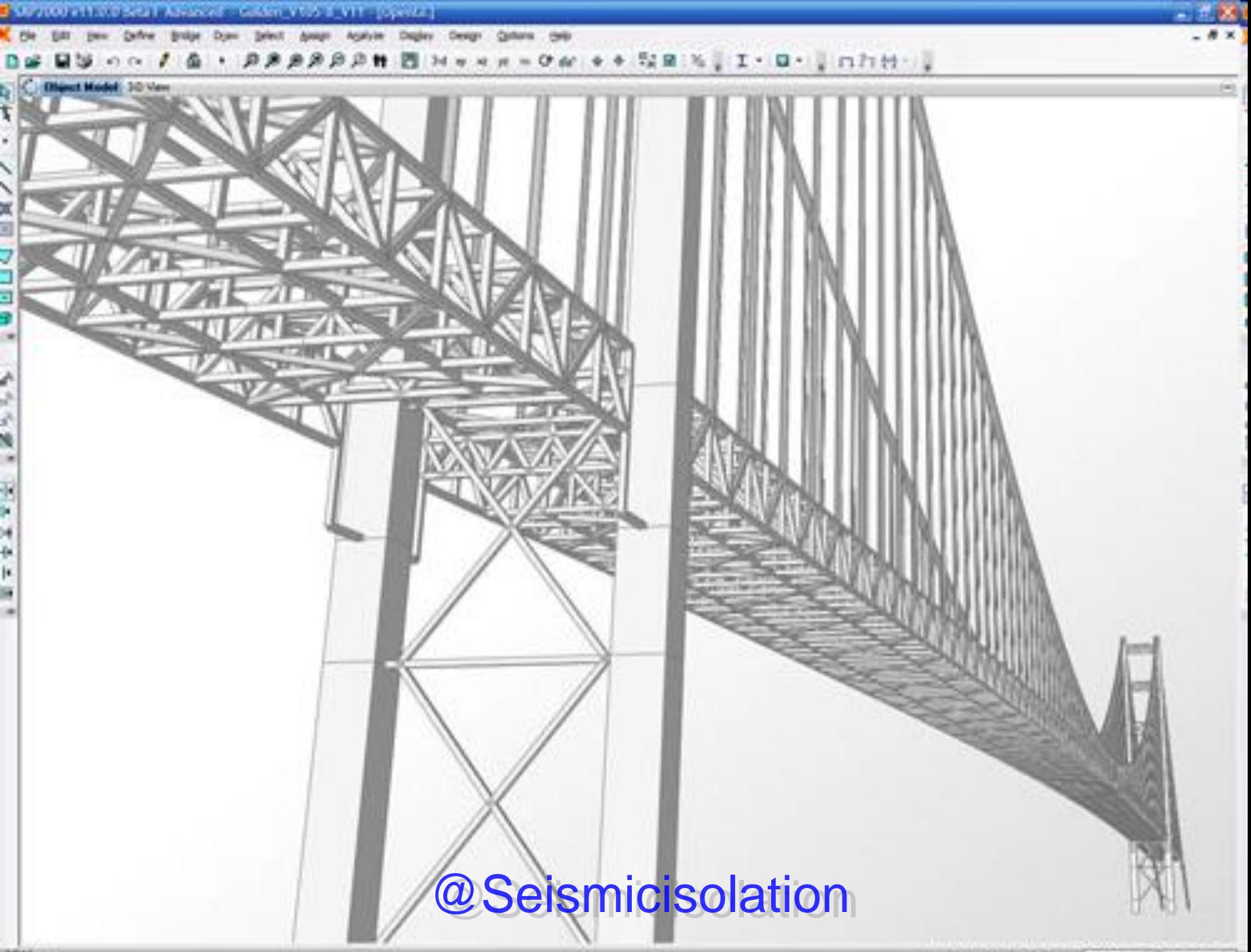
## Hybrid structures

Combination of the above systems

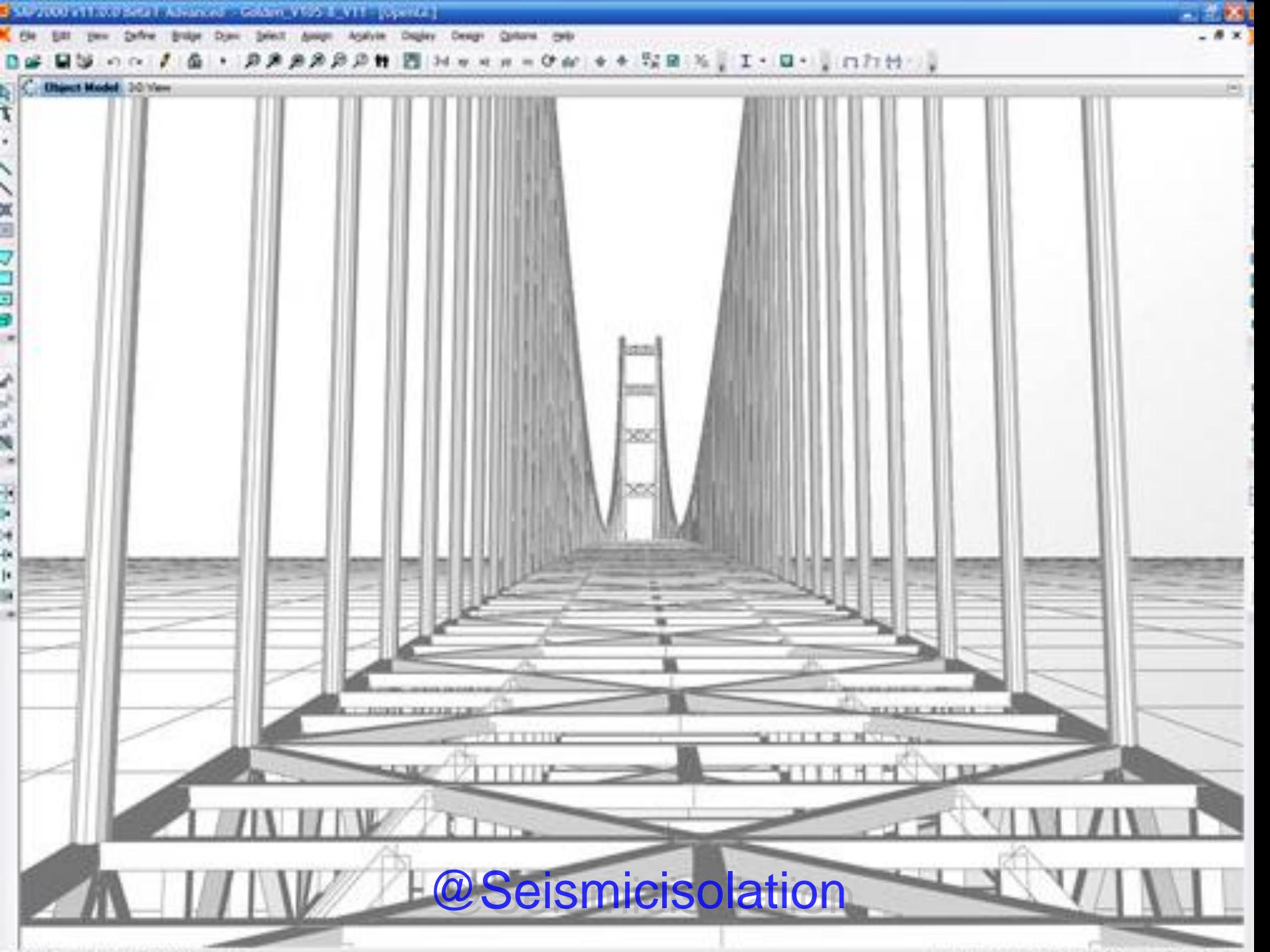
@Seismicisolation



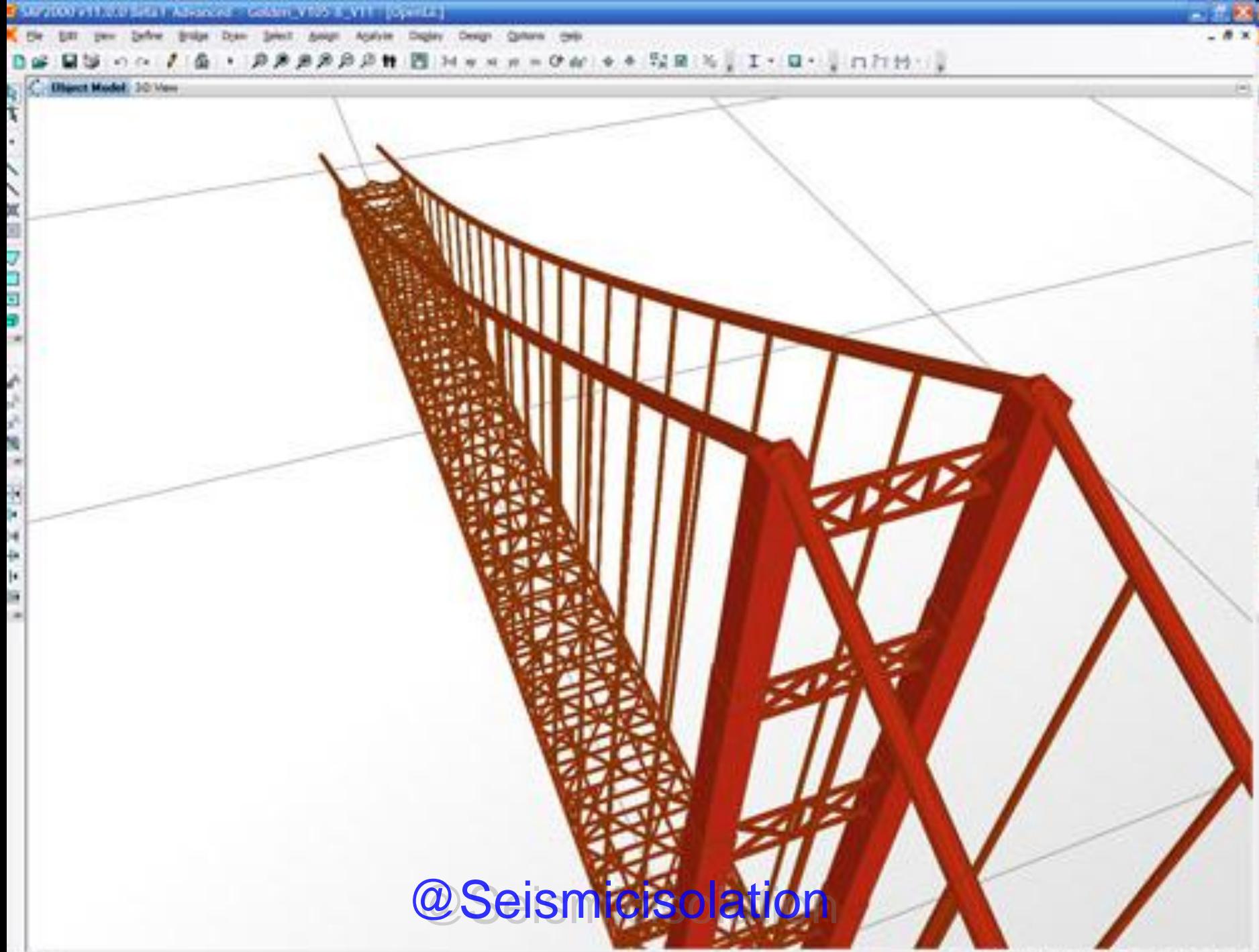
The deformation of a cable under its loads takes the shape of a **funicular curve** that is produced by only **axial forces** since a cable has negligible bending strength: **polygonal** and **curved shapes** (e.g. catenary shapes, parabolic shapes, circular shapes)



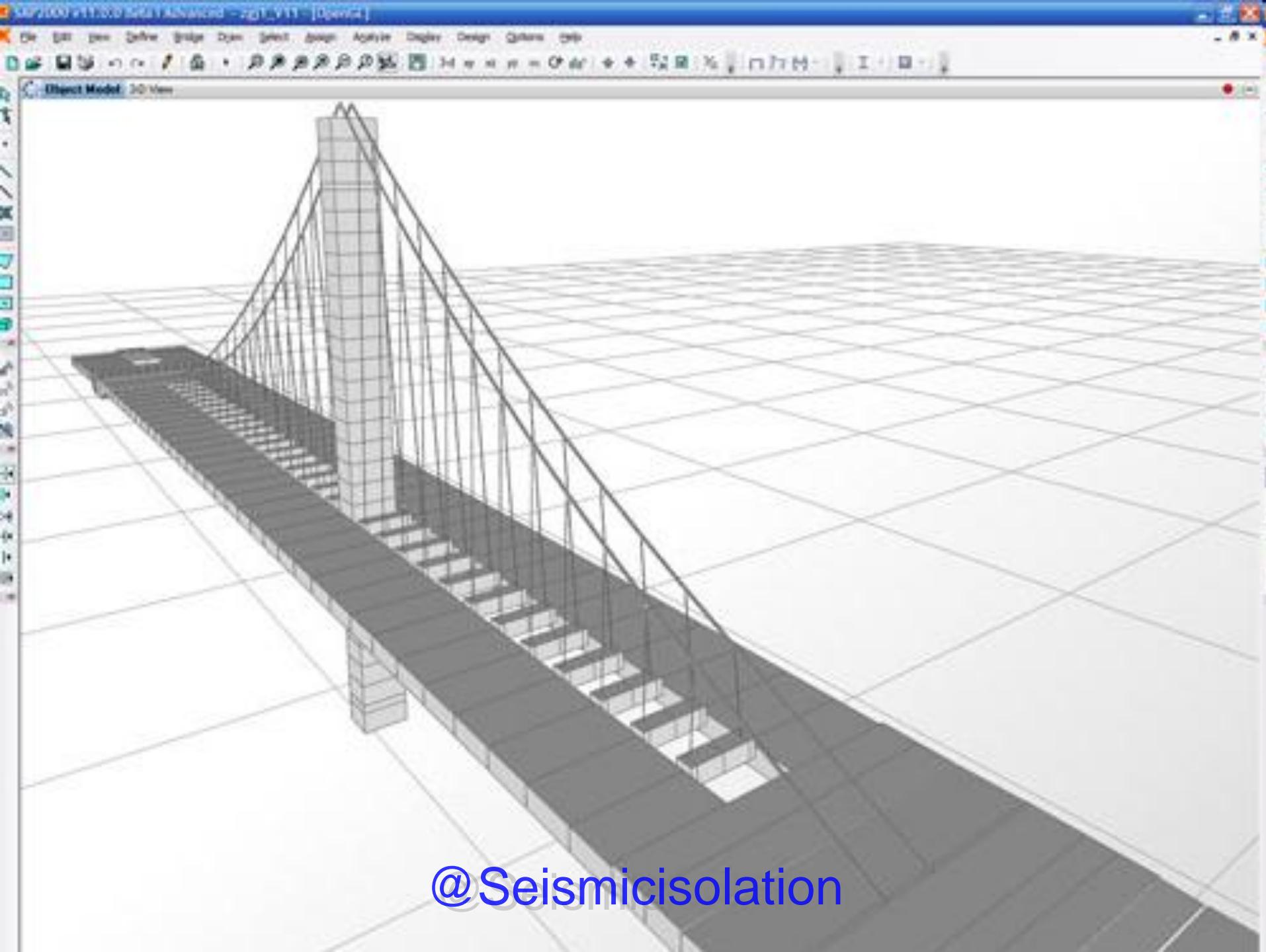
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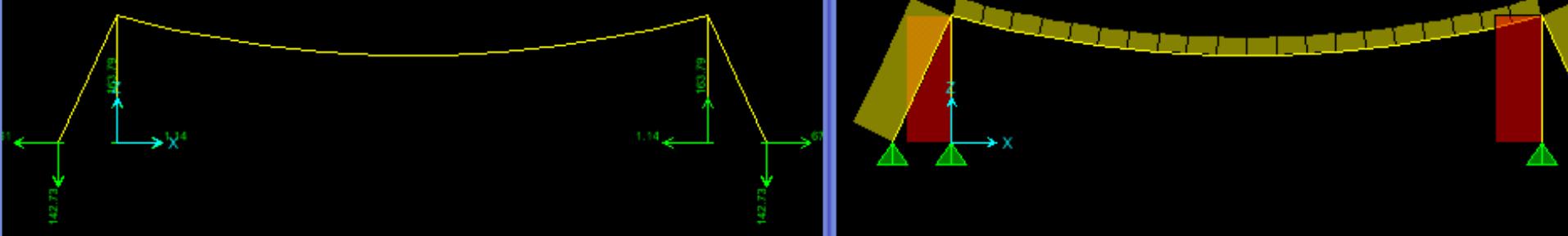
@Seismicisolation



@Seismicisolation



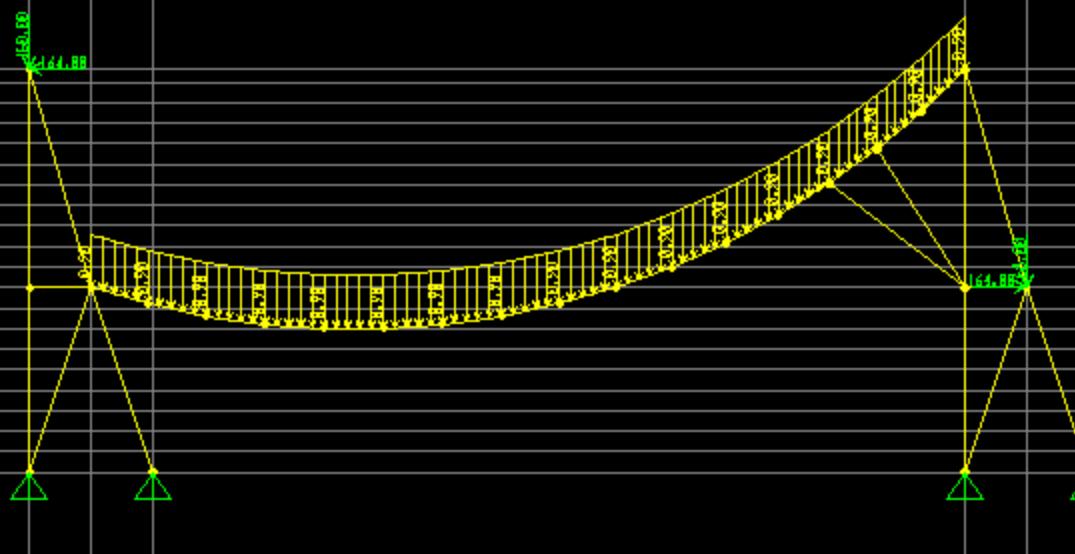
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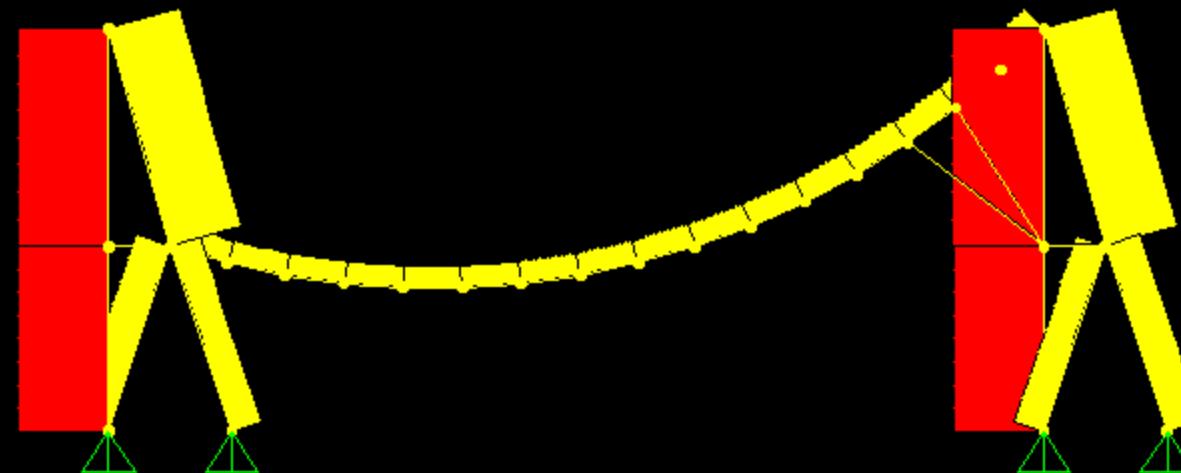
Edit View Define Draw Select Assign Analyze Display Design Options Help



Frame Span Loads (DL)



Axial Force Diagram (DL)



@Seismicisolation

Plane @ Y=0

X170.49 Y0.00 Z38.55

Kip-It

Start

Book

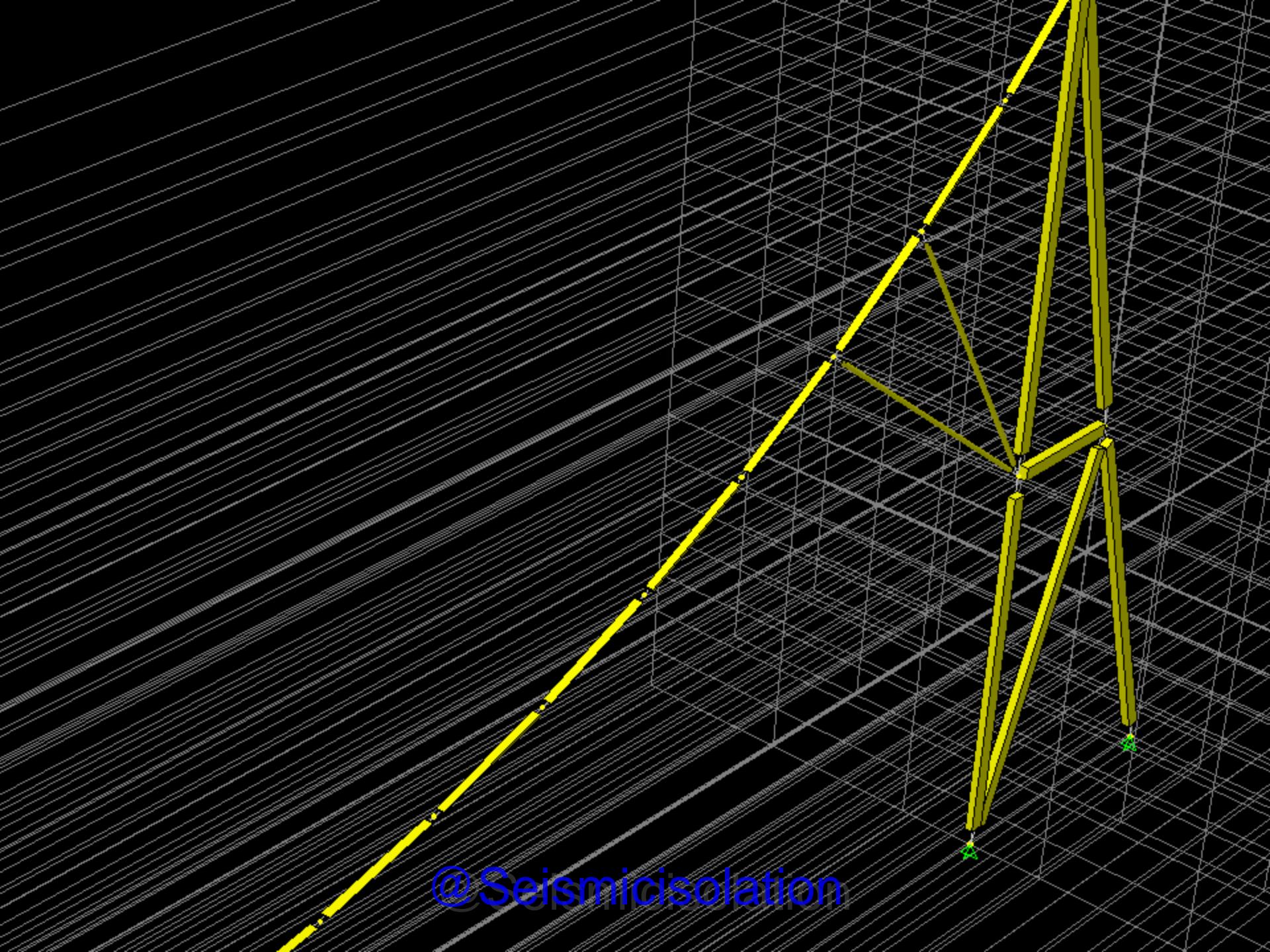
Microsoft PowerPoint - Vis...

Ch.6.doc - Microsoft Word

SAP2000 - Ex.9.2 sus...

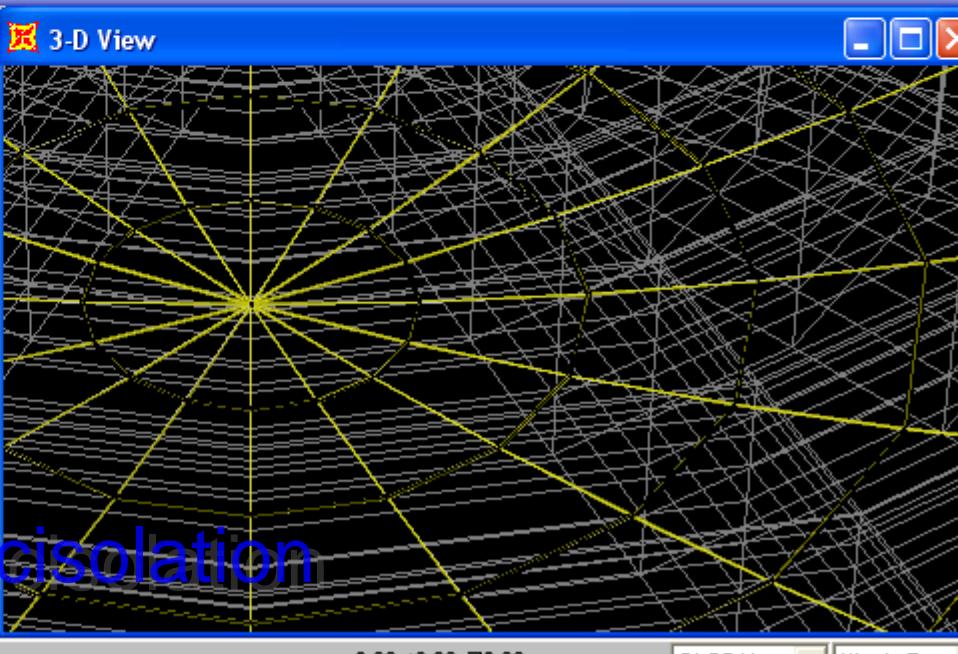
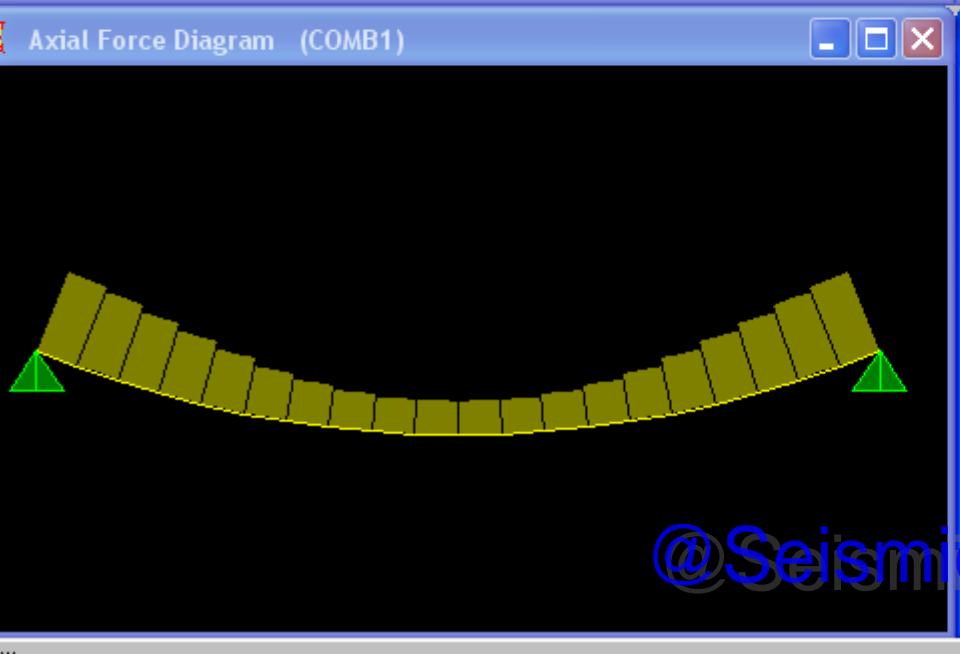
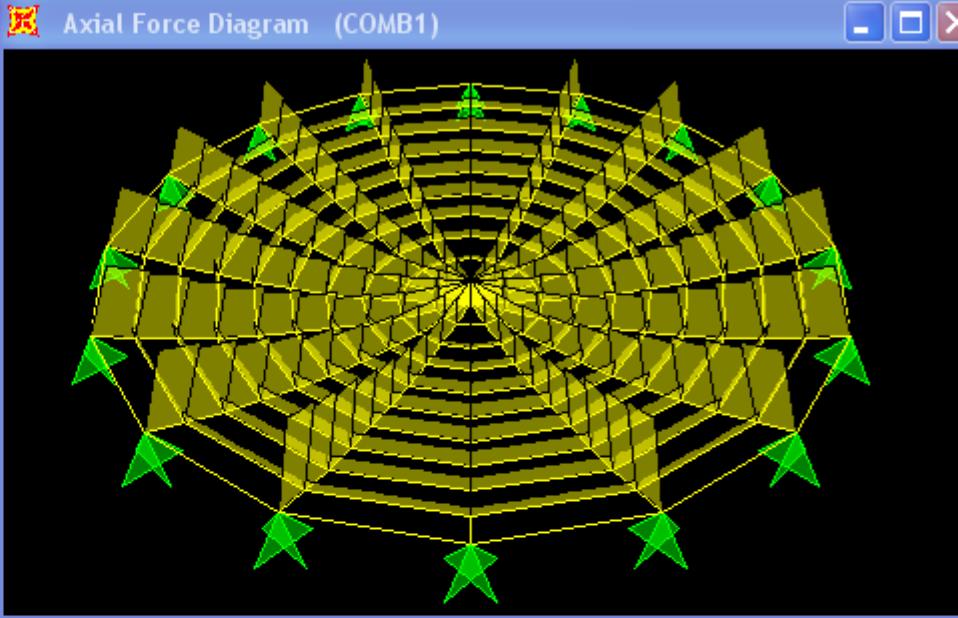
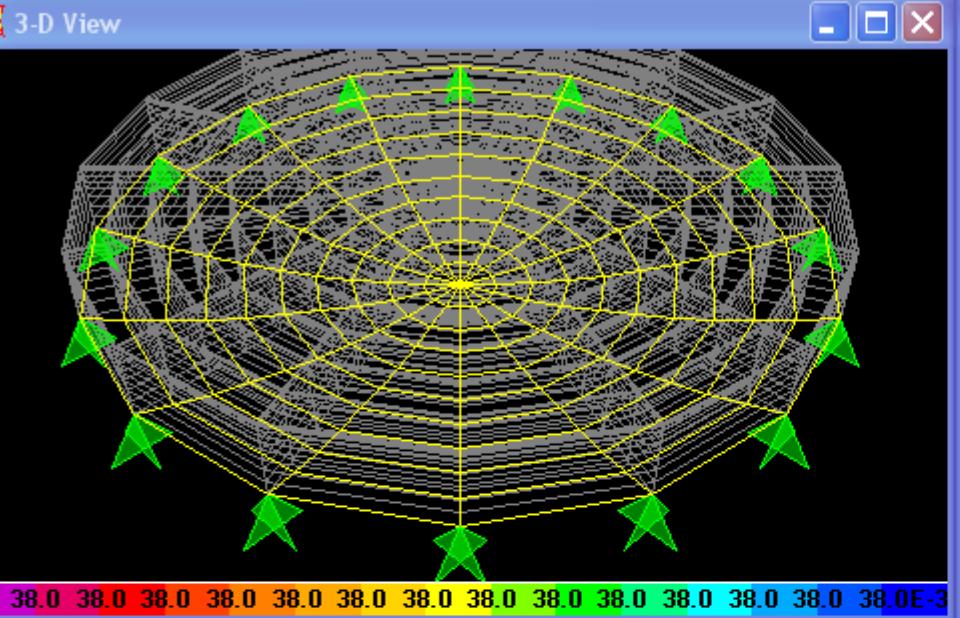


10:08

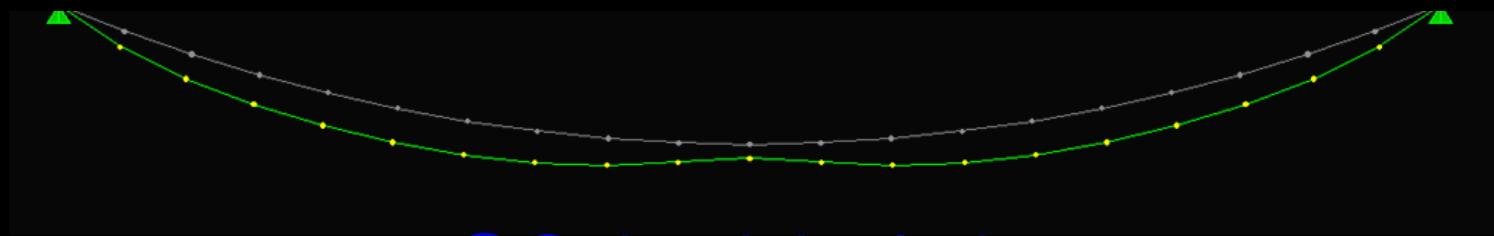
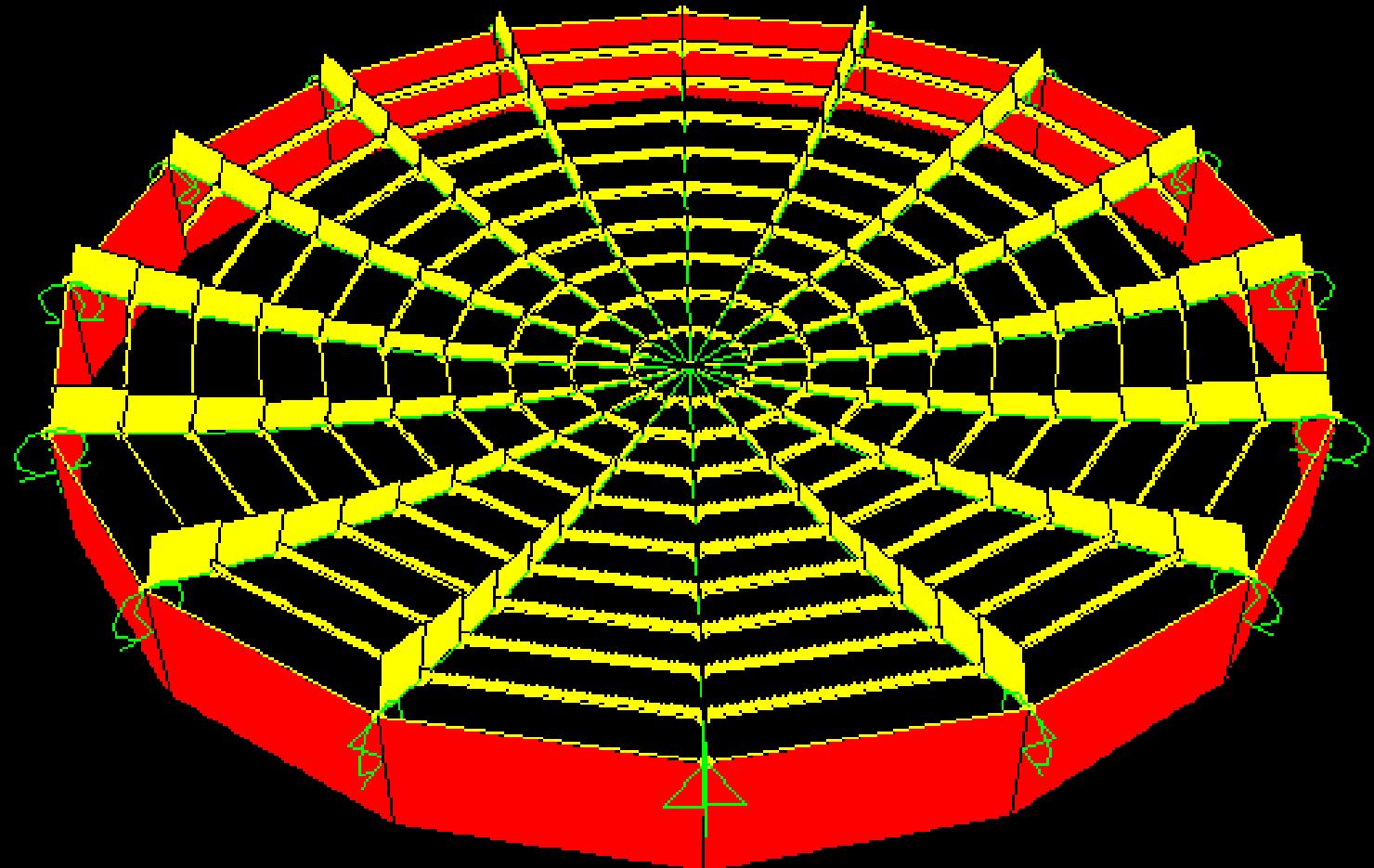


@Seismicisolation

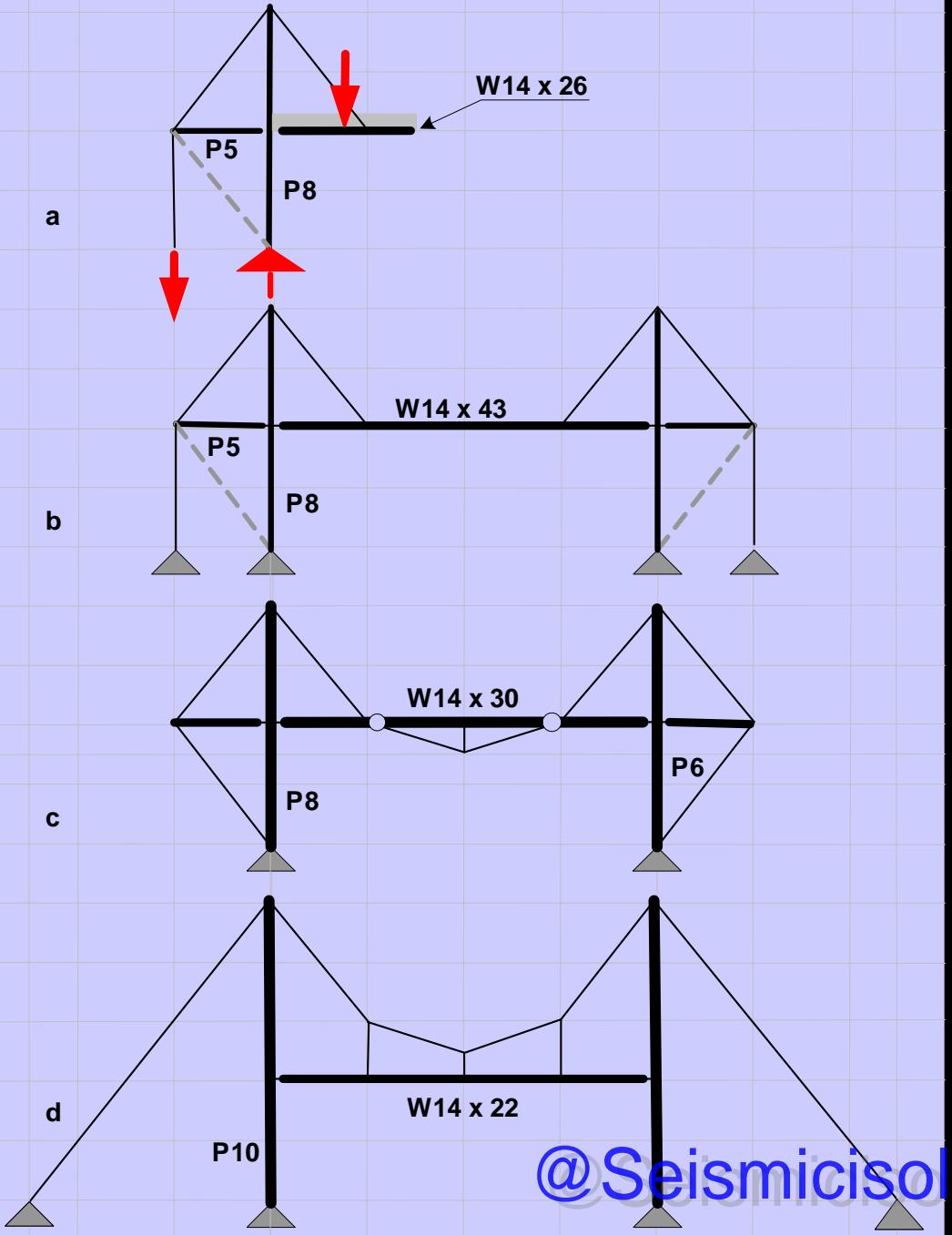
Edit View Define Draw Select Assign Analyze Display Design Options Help



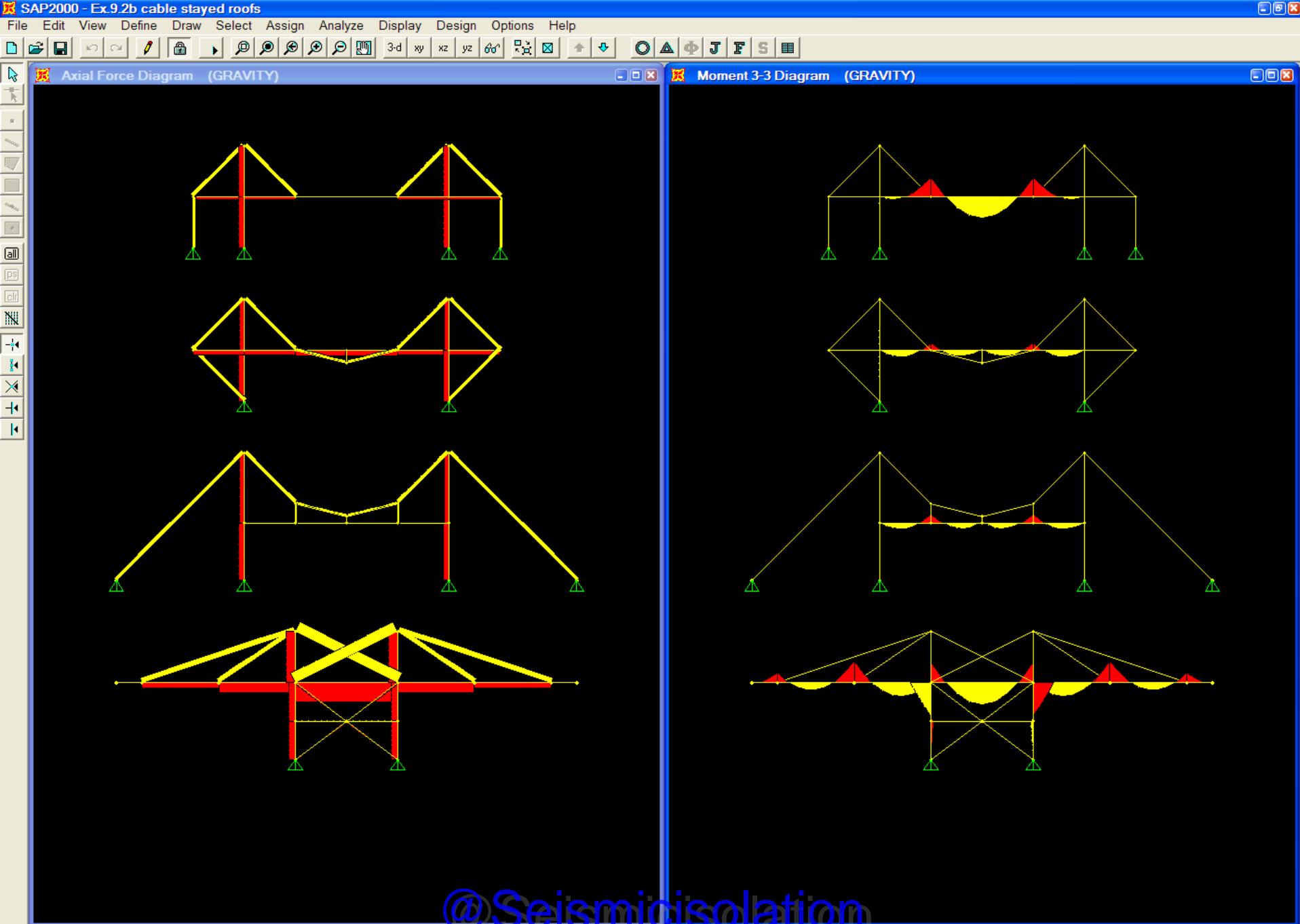
@Seismicisolation



@Seismicisolation



## Cable-Stayed Roof Structures



@Seismicisolation

Right Click on any Frame Element for detailed diagram



Ch.9, Cable-Supp...

SAP2000 - Ex.9.2...

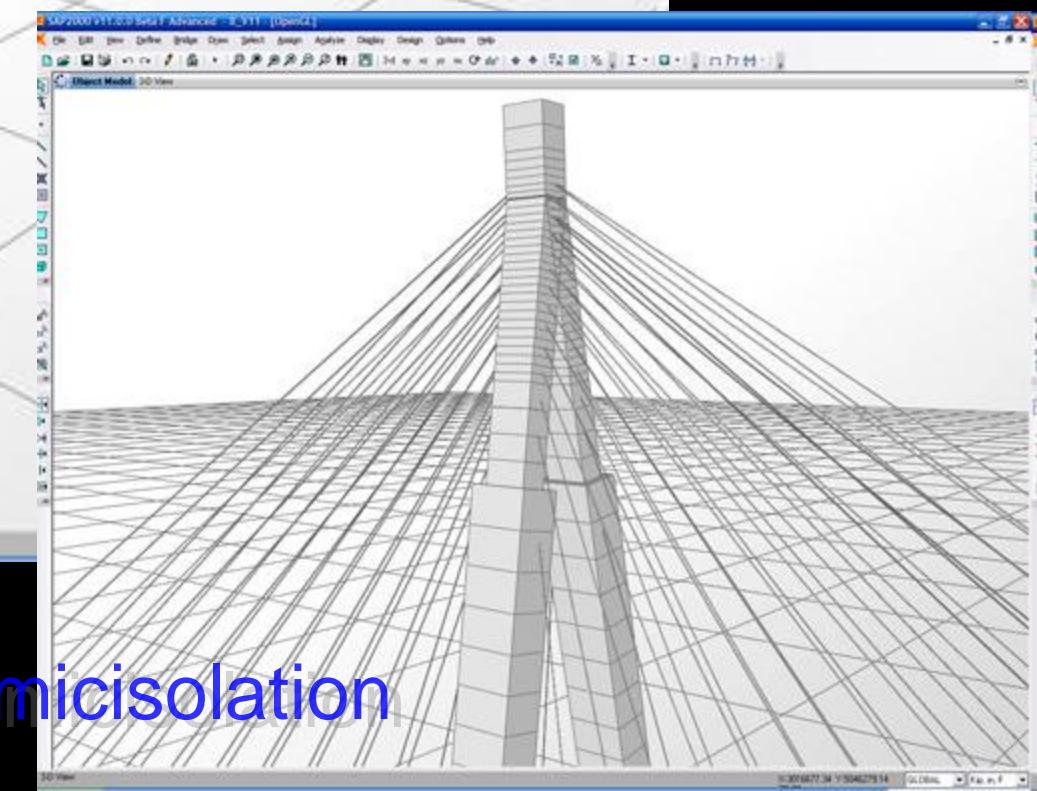
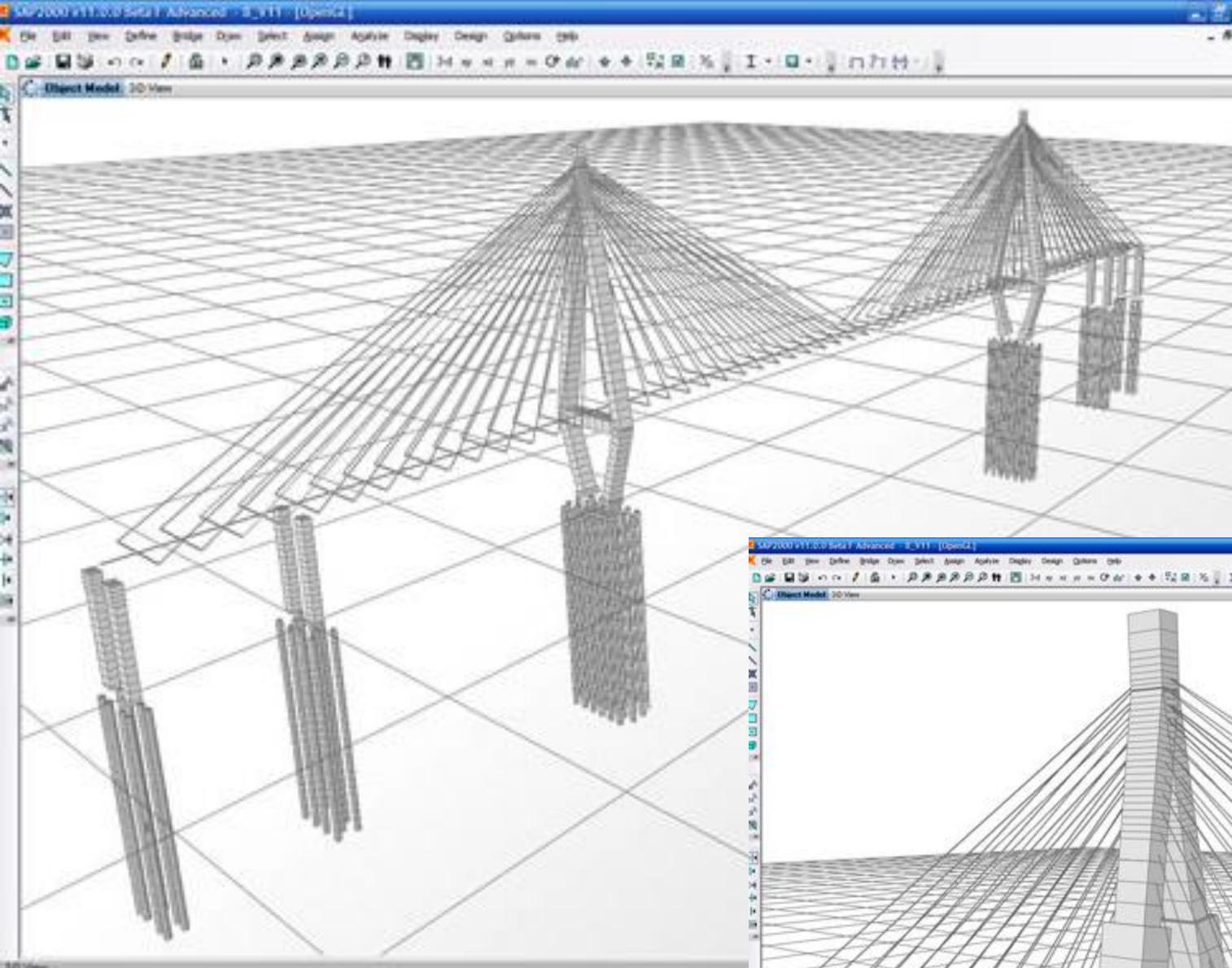
Microsoft Visio - [...]

Microsoft

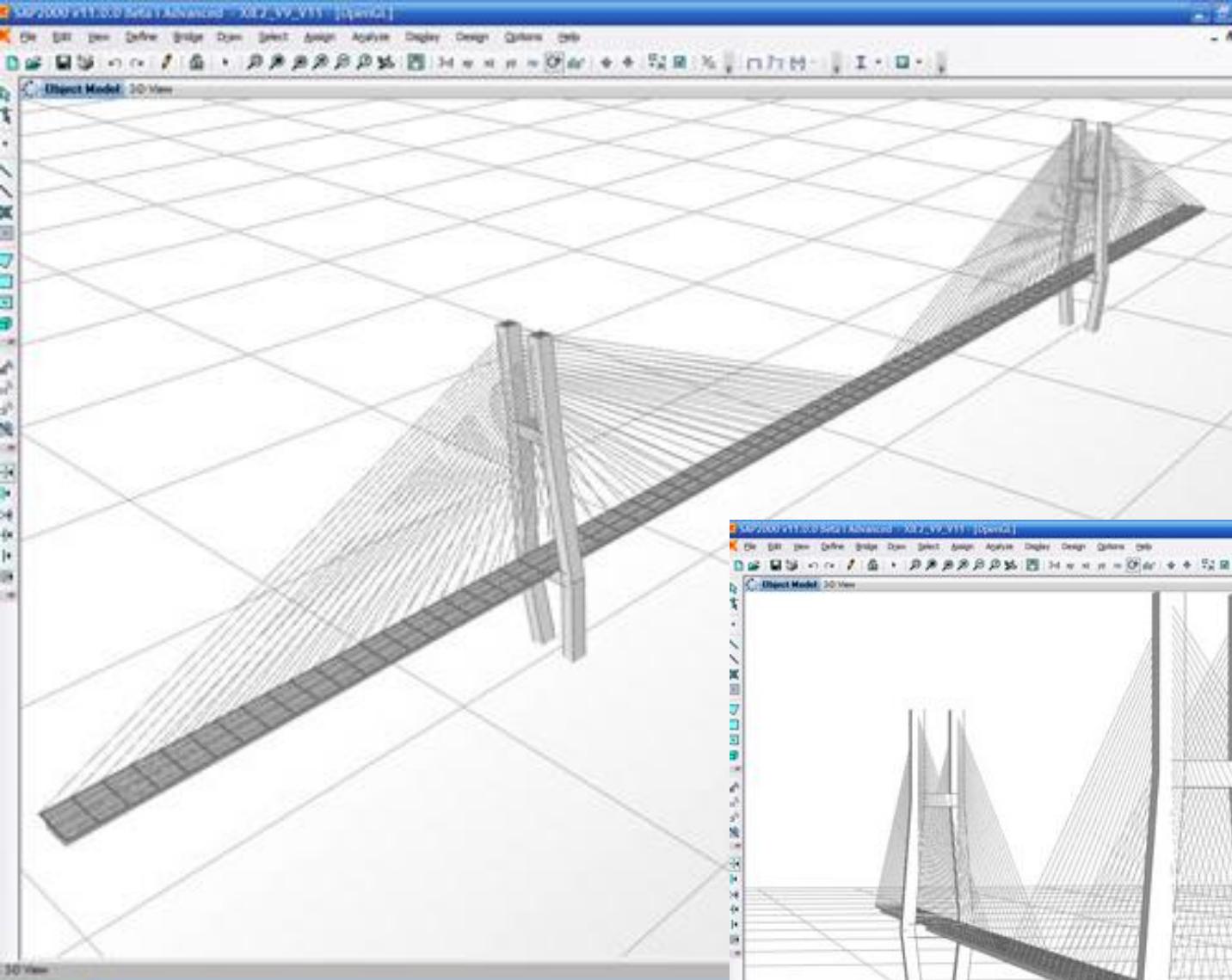
Microsoft PowerP...

Kip-f

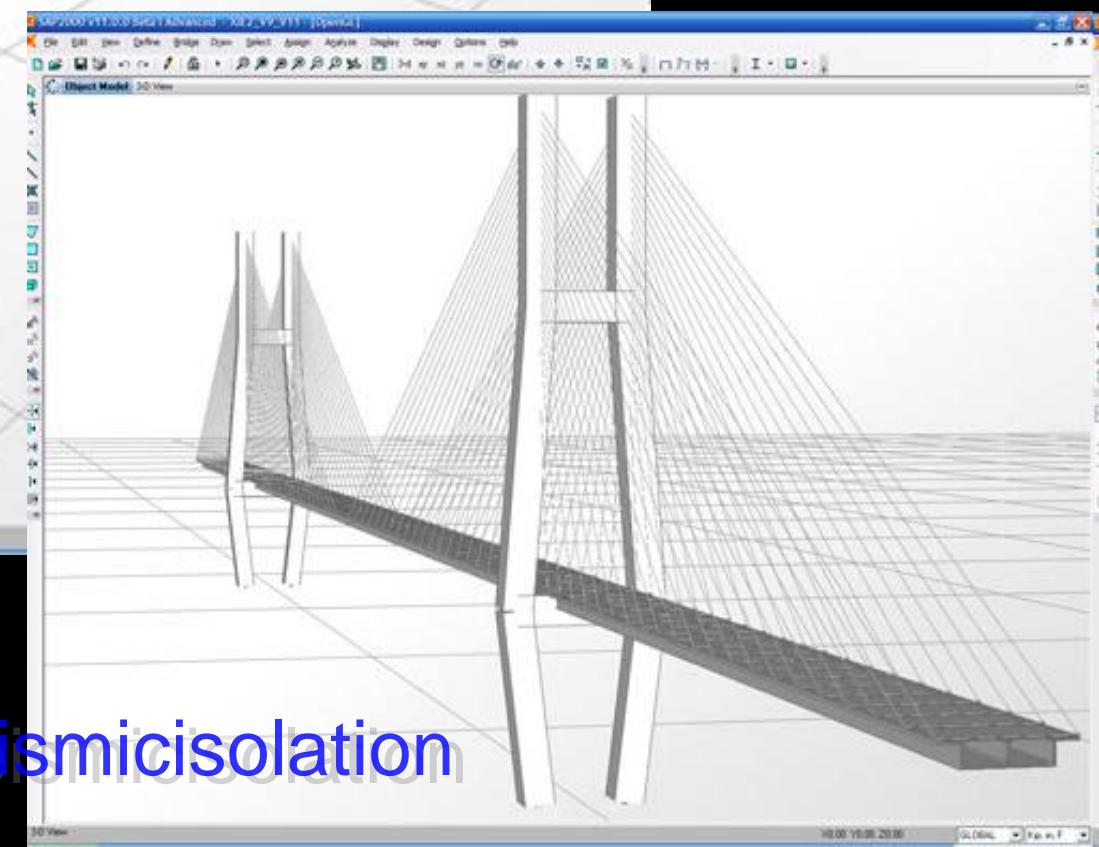
1:05 PM



@Seismicisolation



3D View

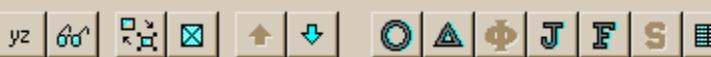


@Seismicisolation

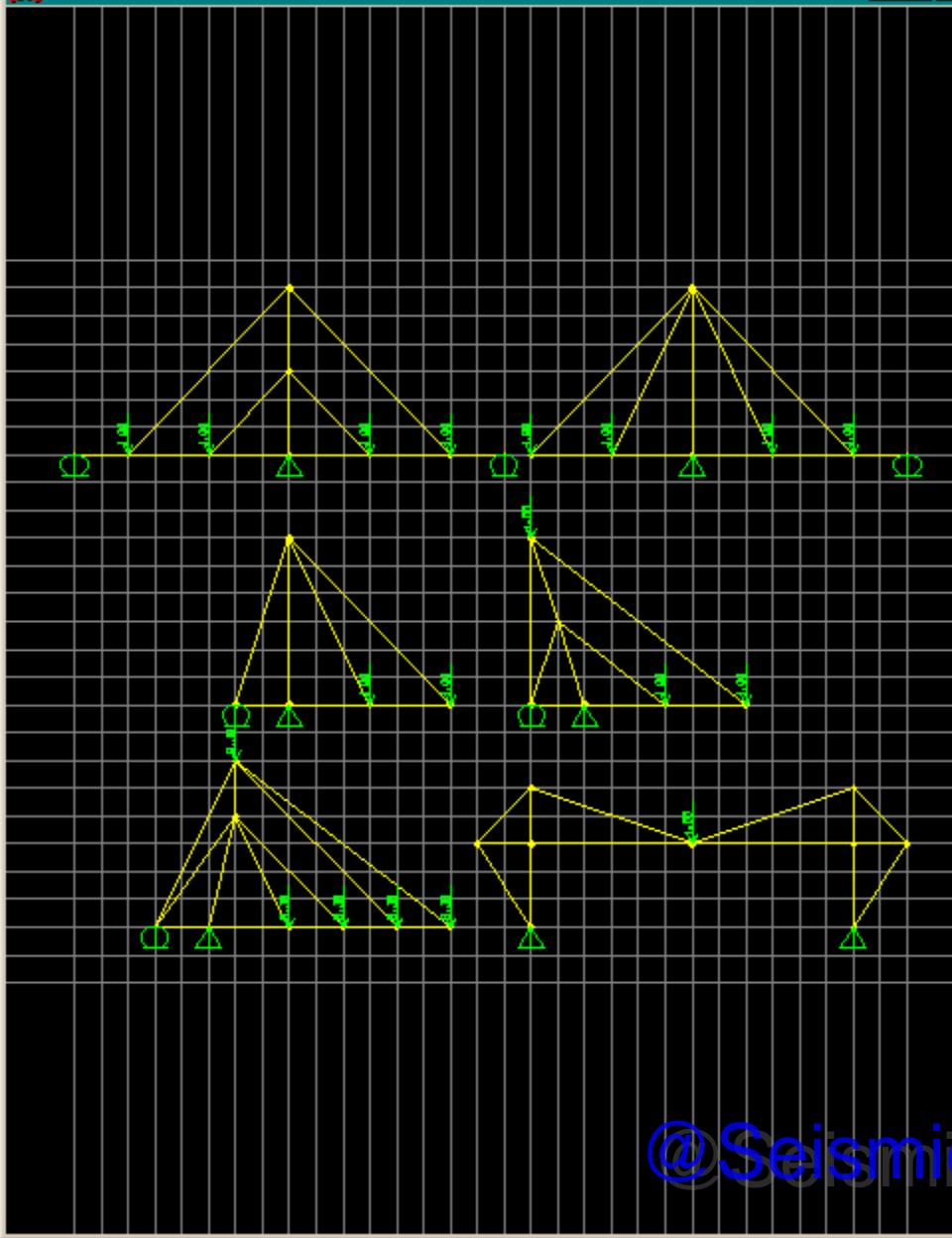
3D View

10.00 10.00 20.00

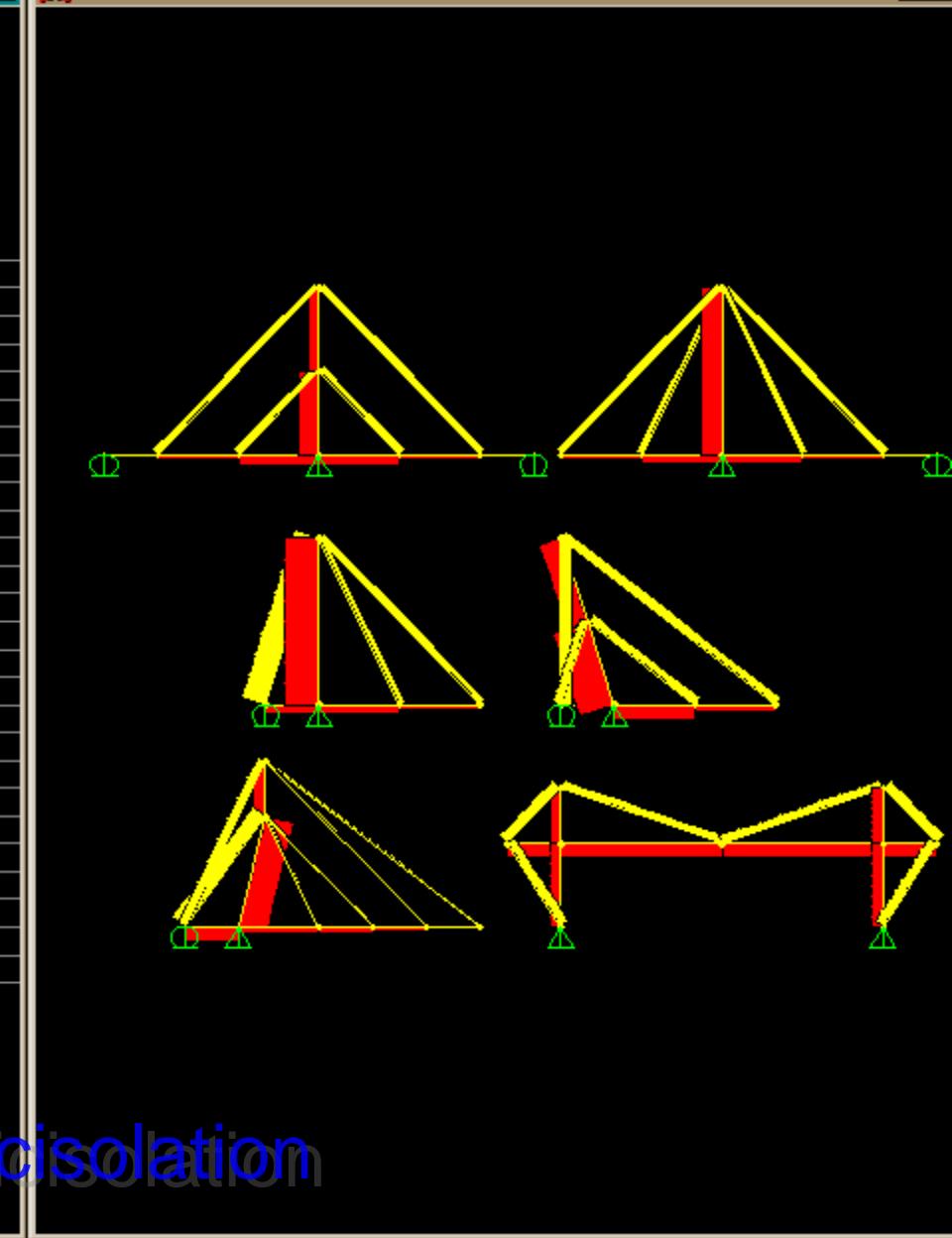
GLOBAL



Joint Loads (LOAD1)



Axial Force Diagram (LOAD1)

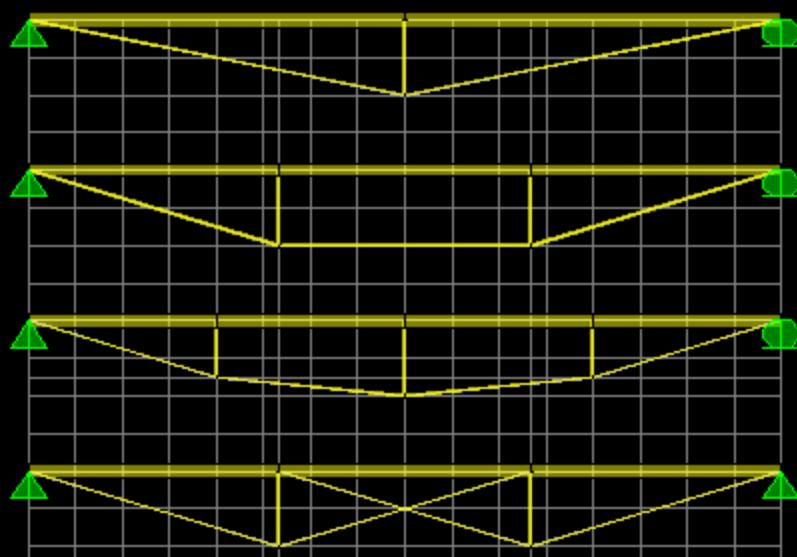


@Seismicisolation

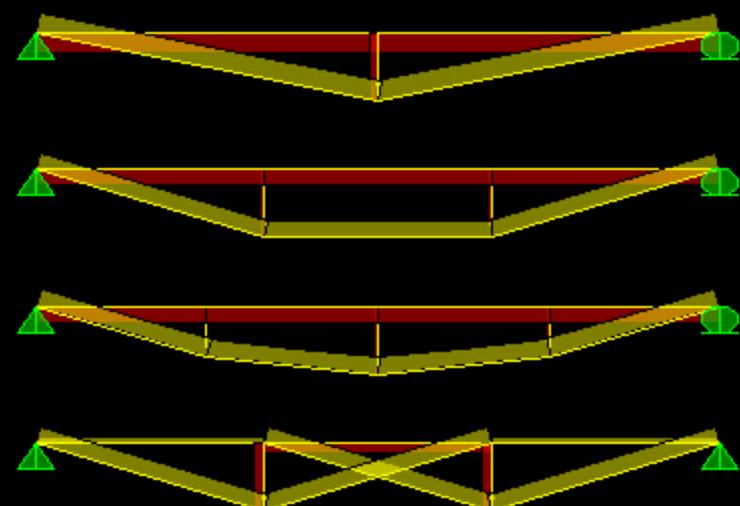
Edit View Define Draw Select Assign Analyze Display Design Options Help



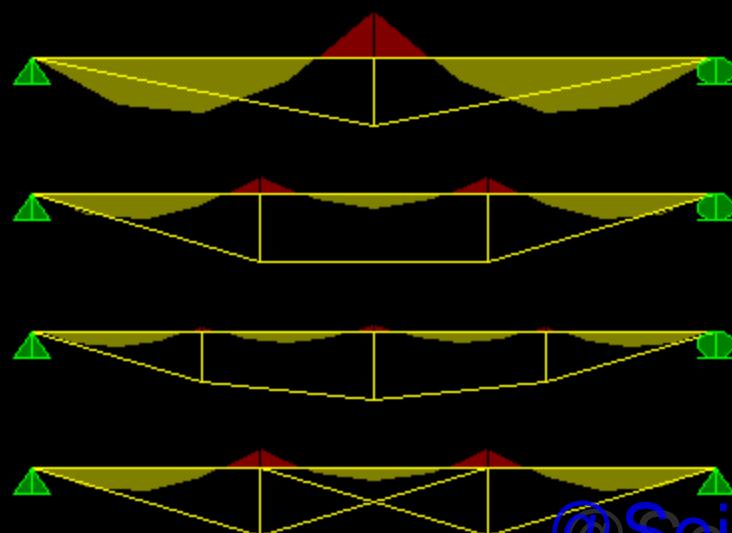
X-Z Plane @ Y=2



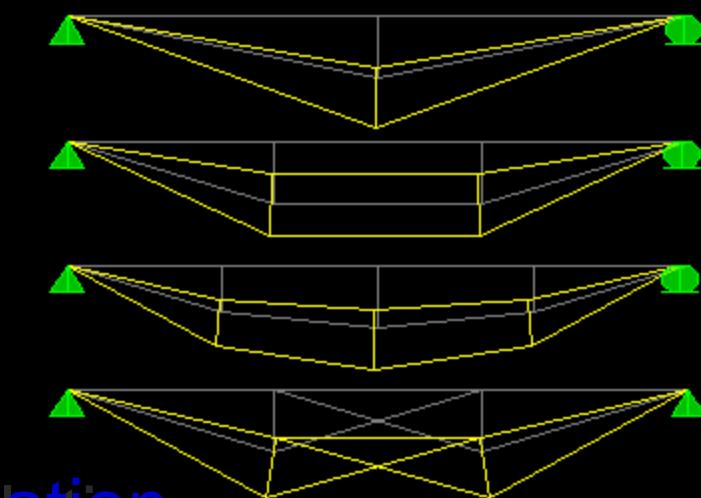
Axial Force Diagram (uniform)



Moment 3-3 Diagram (uniform)



Deformed Shape (uniform)



@Seismicisolation

Click on any Frame Element for detailed diagram

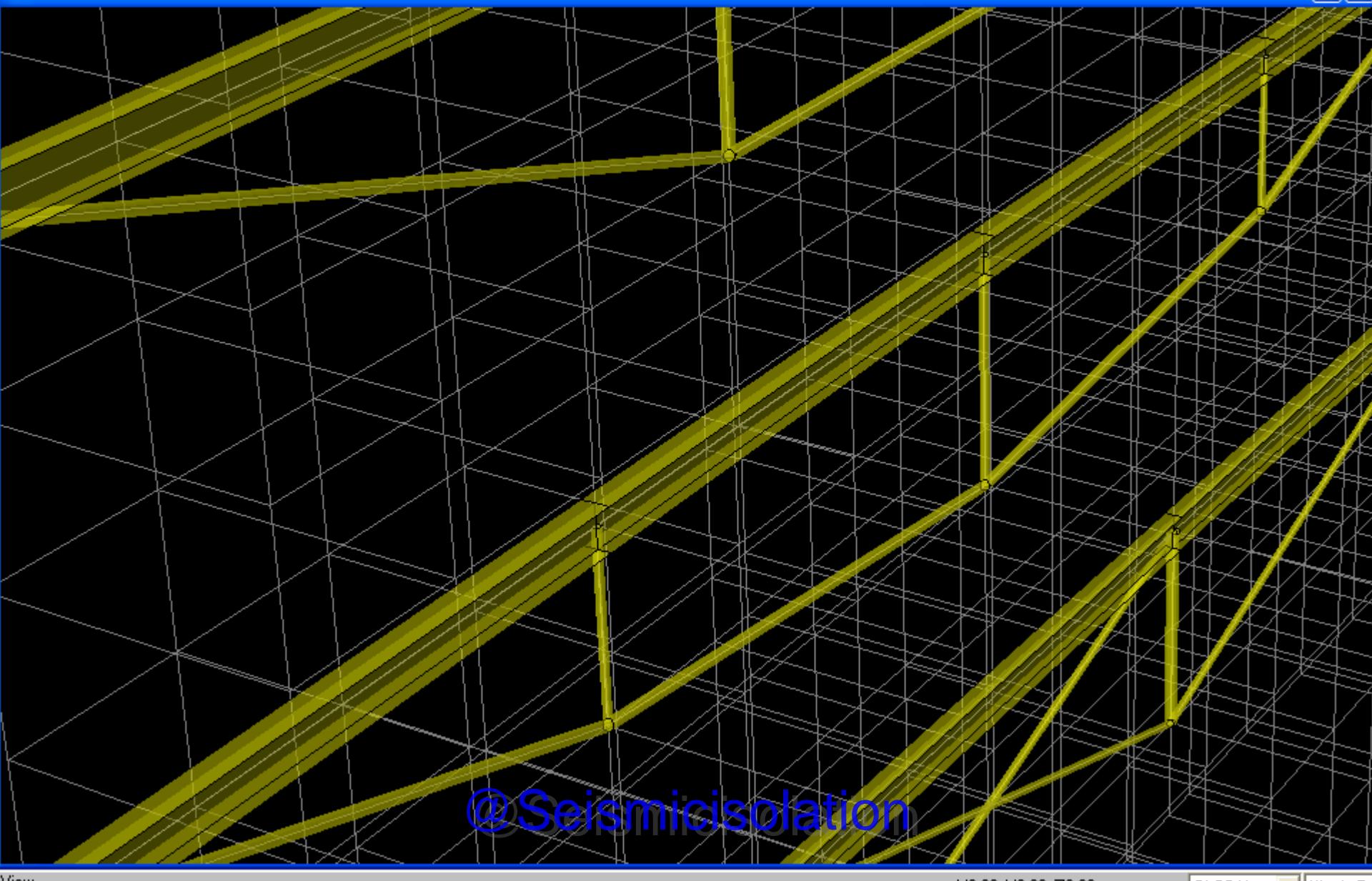
GLOBAL Kip, ft, F



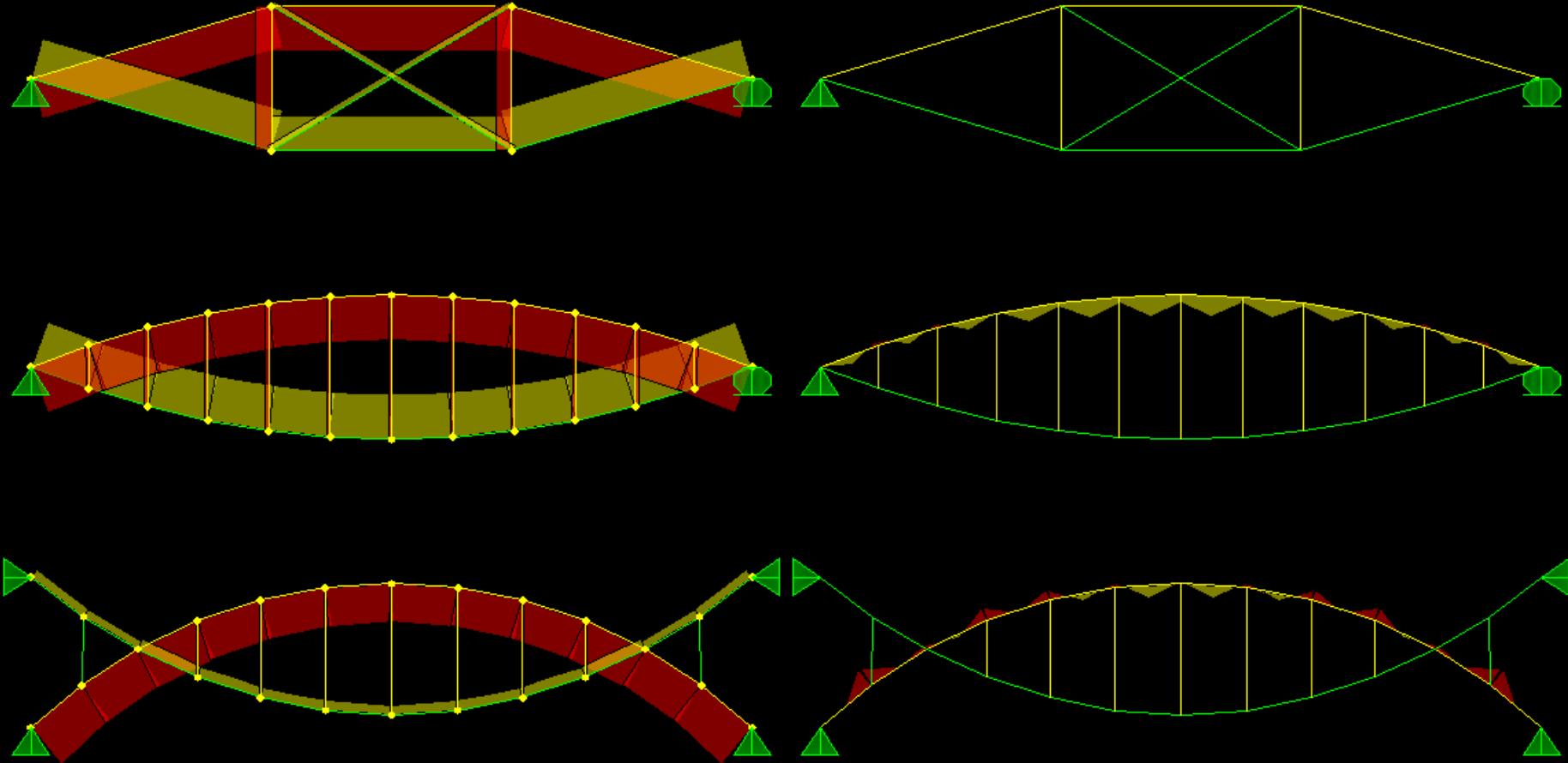
Edit View Define Draw Select Assign Analyze Display Design Options Help



3-D View



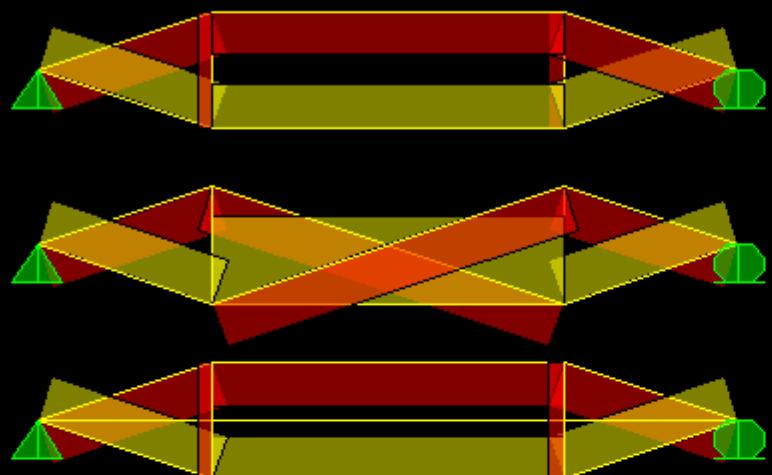
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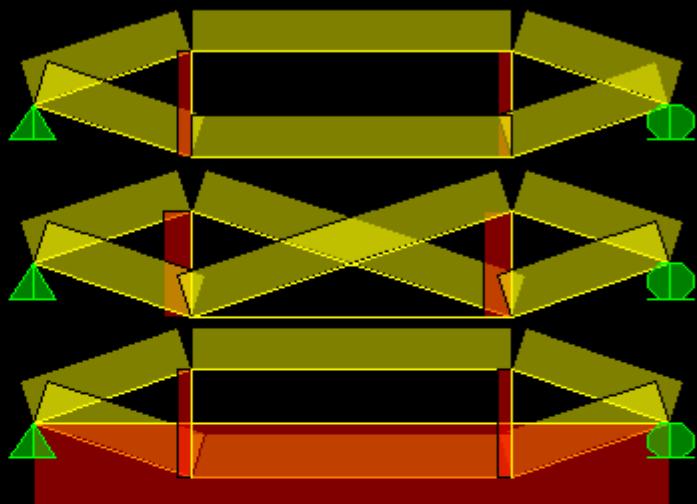
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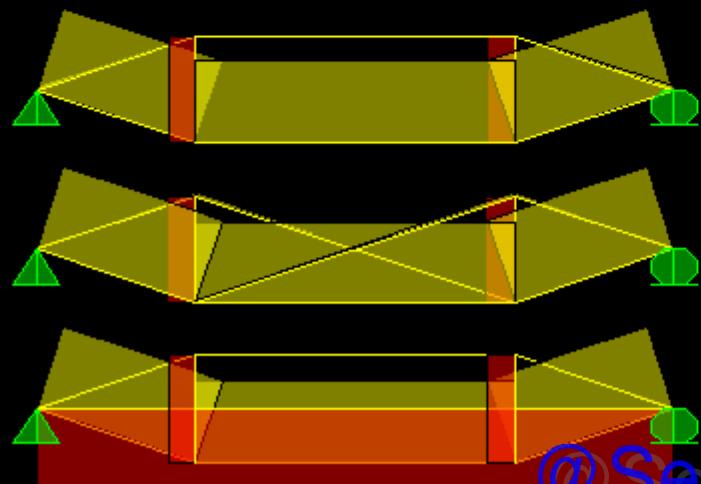
Axial Force Diagram (GRAVITY)



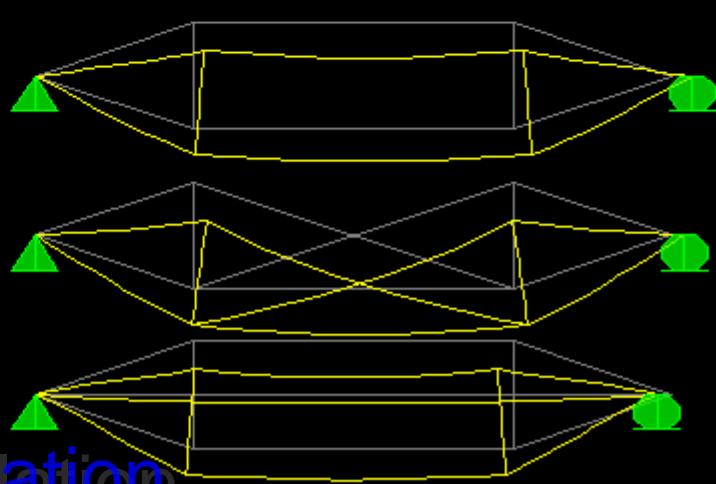
Axial Force Diagram (PRESTRES)



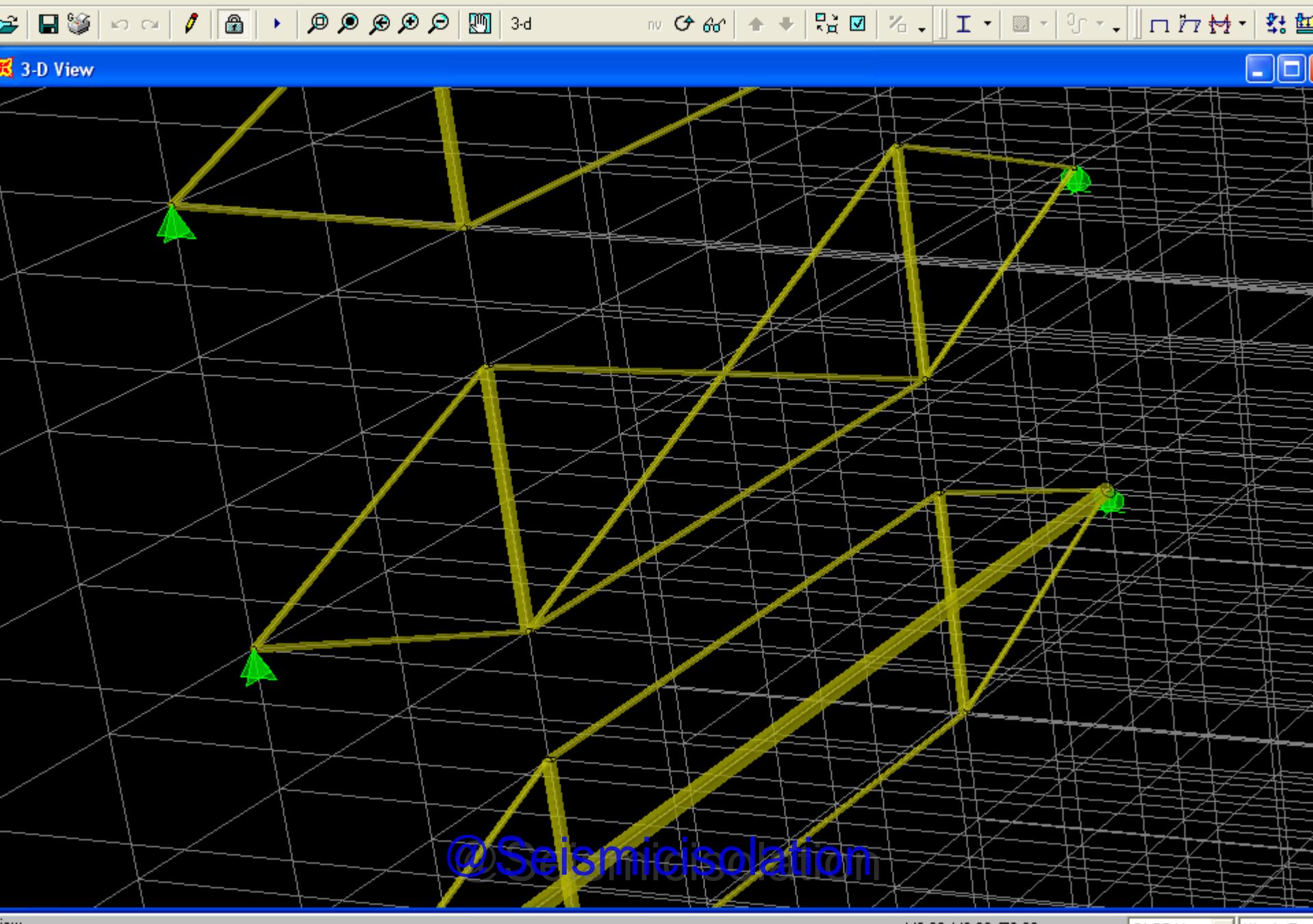
Axial Force Diagram (COMB1)



Deformed Shape (COMB1)



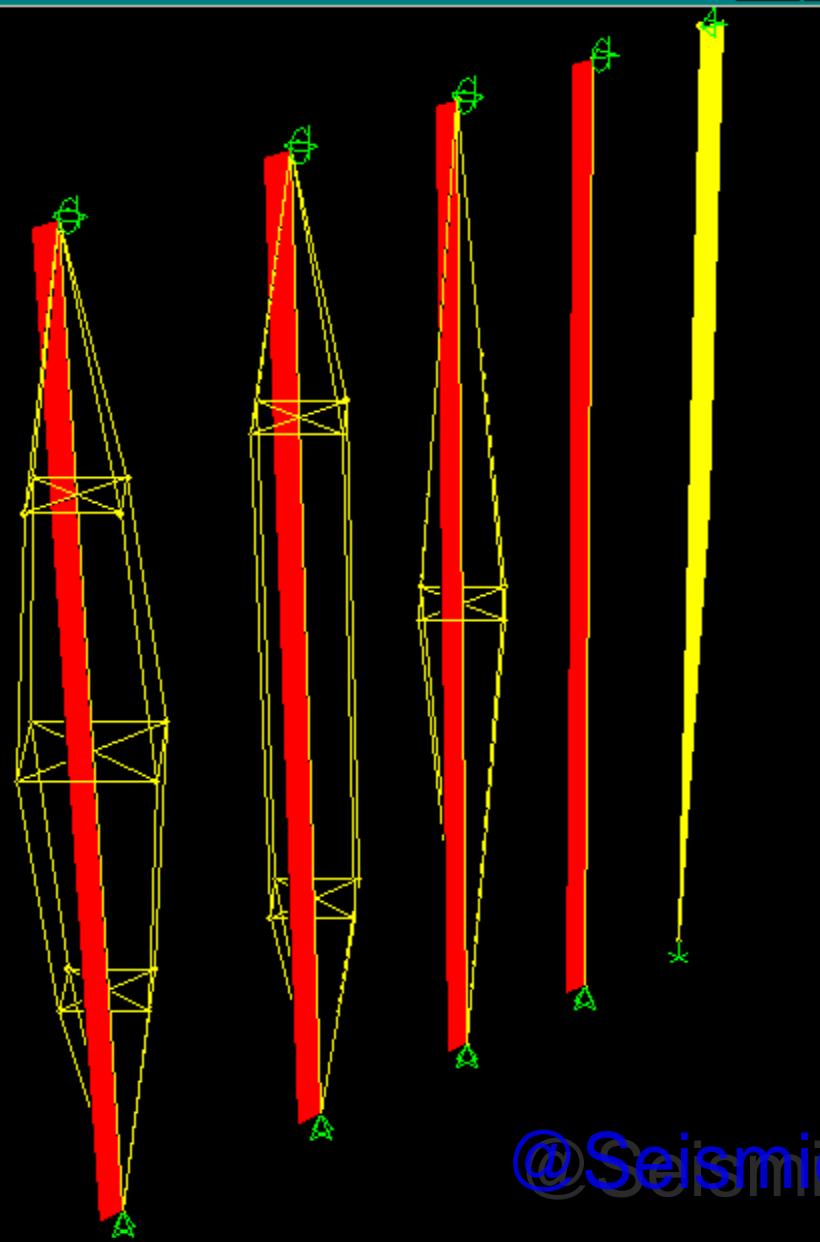
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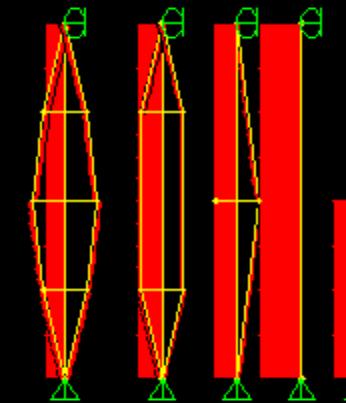
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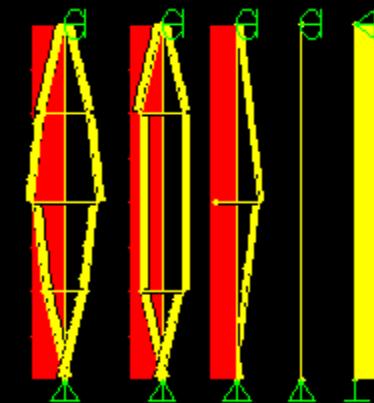
Axial Force Diagram (COMB1)



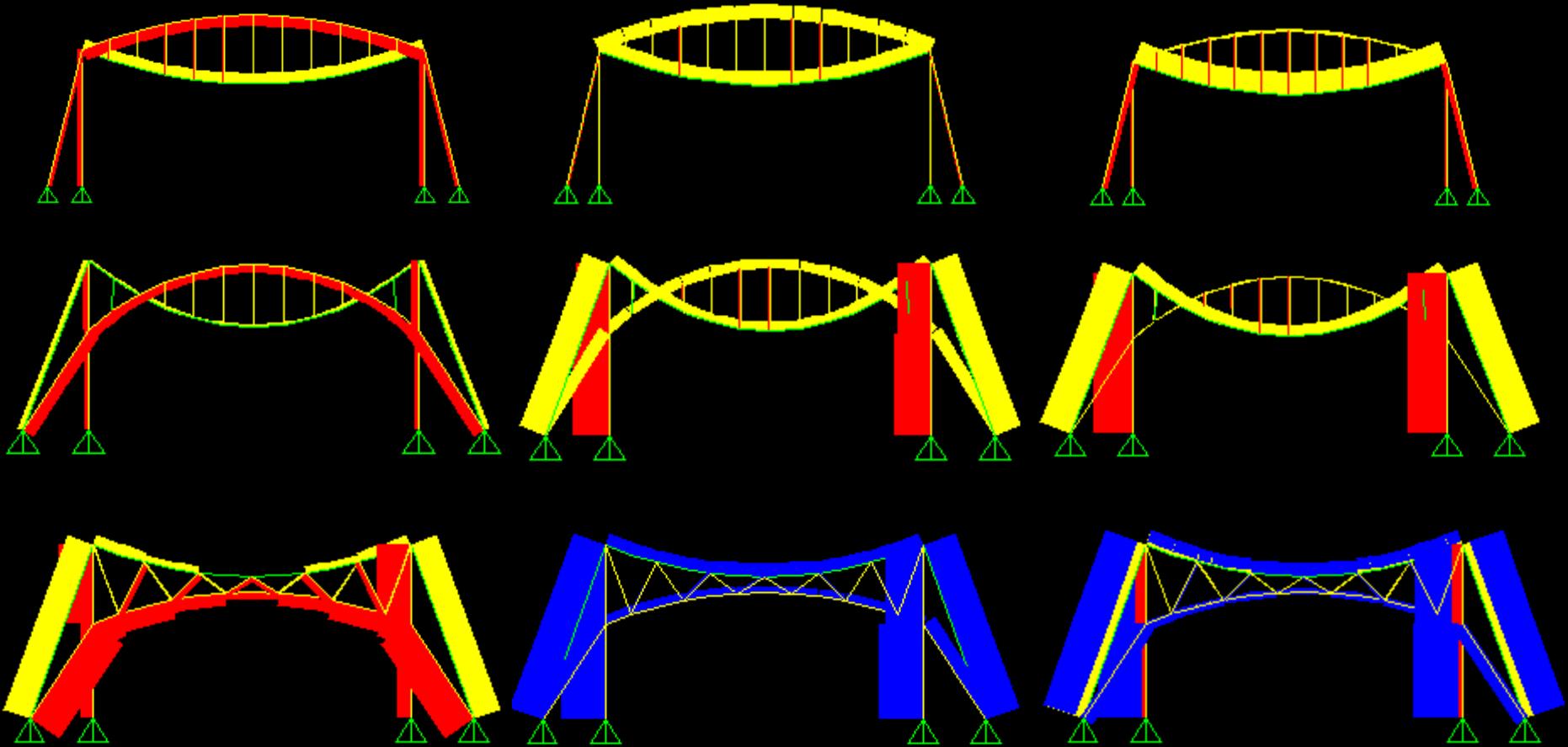
Axial Force Diagram (LOAD1)



Axial Force Diagram (LOAD2)



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Shanghai Pudong International Airport, 2001, Paul Andreu  
principal architect, Coyne et Bellier structural engineers

@Seismicisolation

14 6 2001

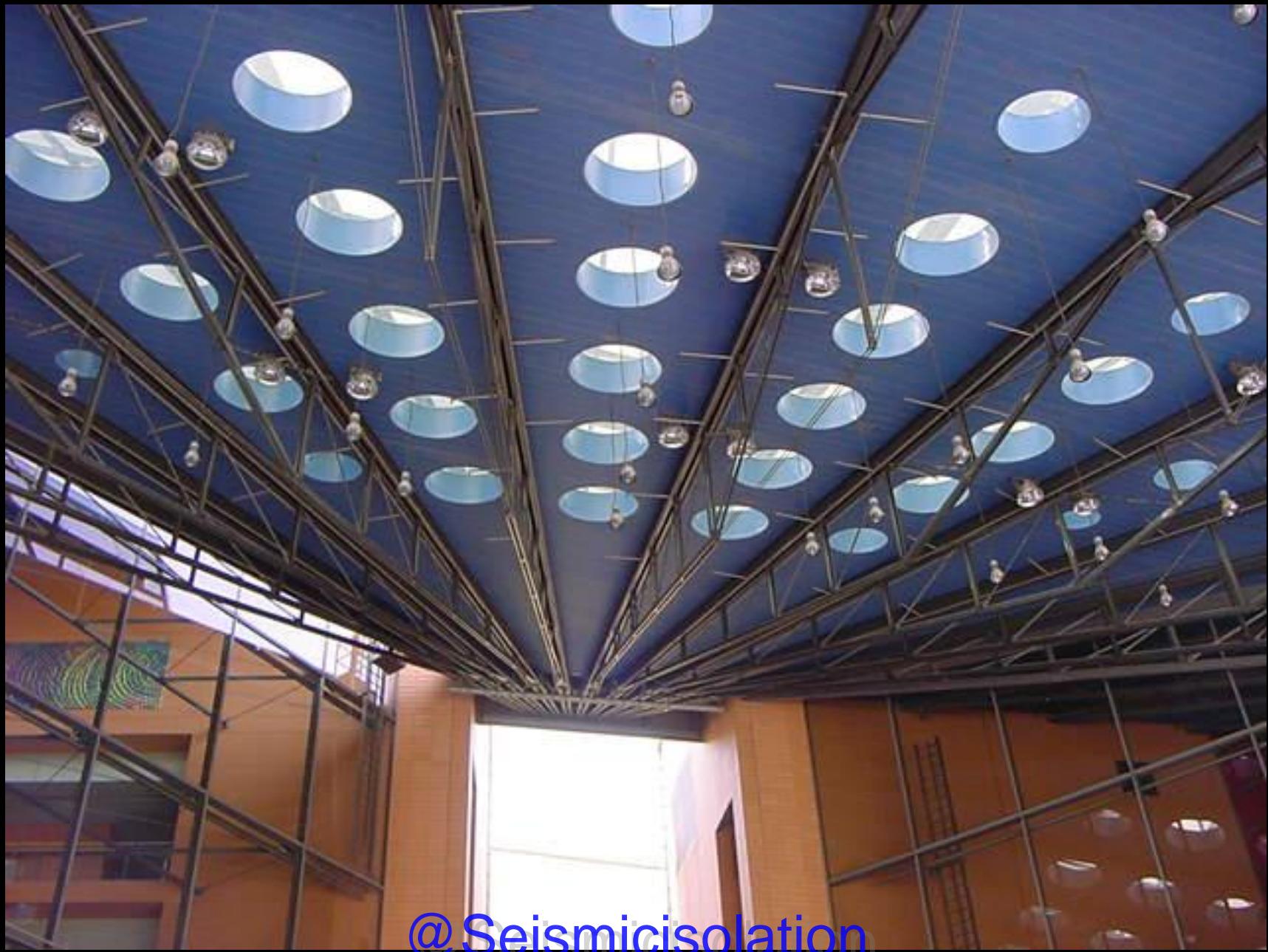


@Seismicisolation

14 6 2001



Debis Theater @ Seismicisolation Berlin, 1998, Renzo Piano



@Seismicisolation

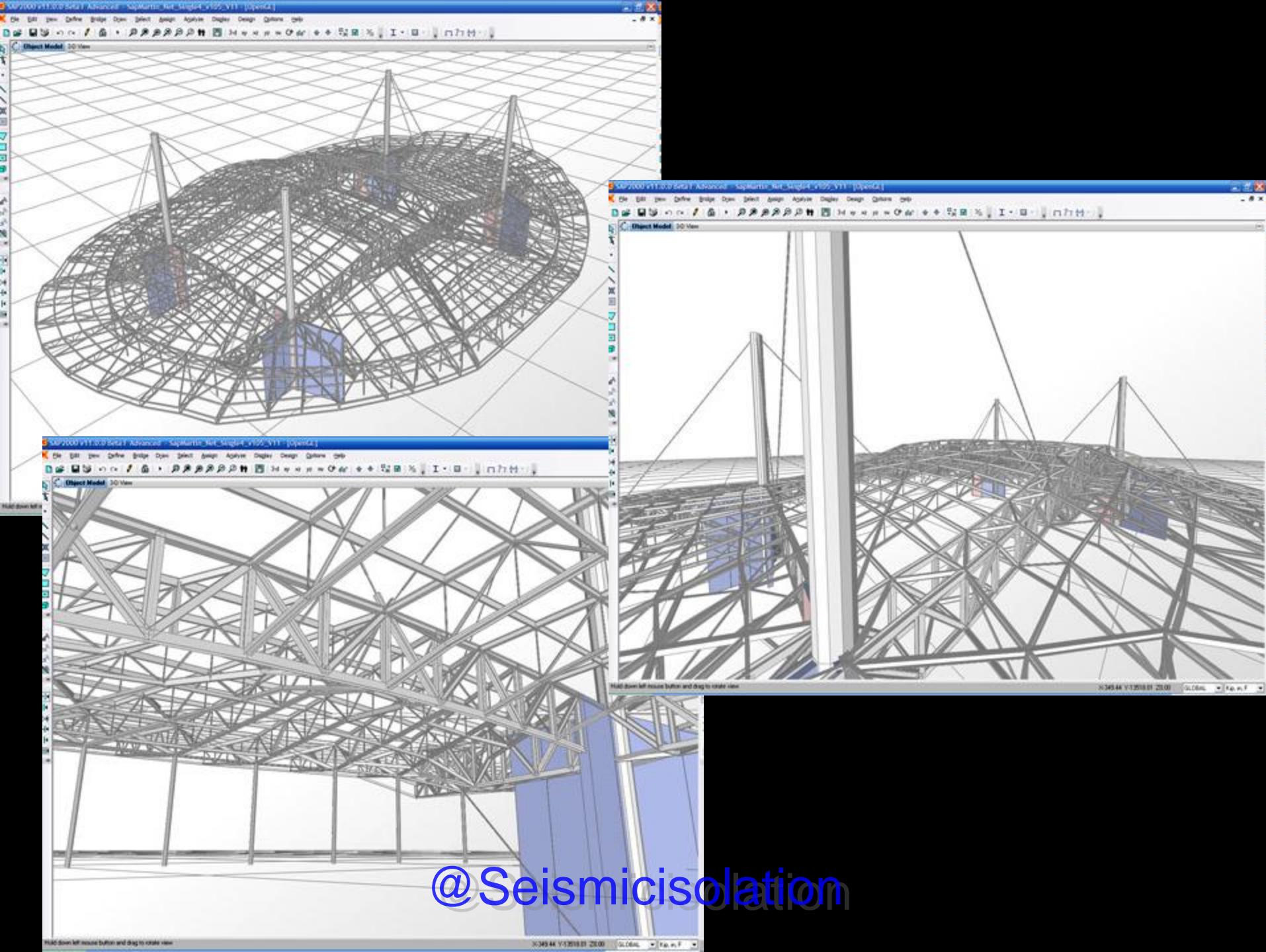


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24 9 2001



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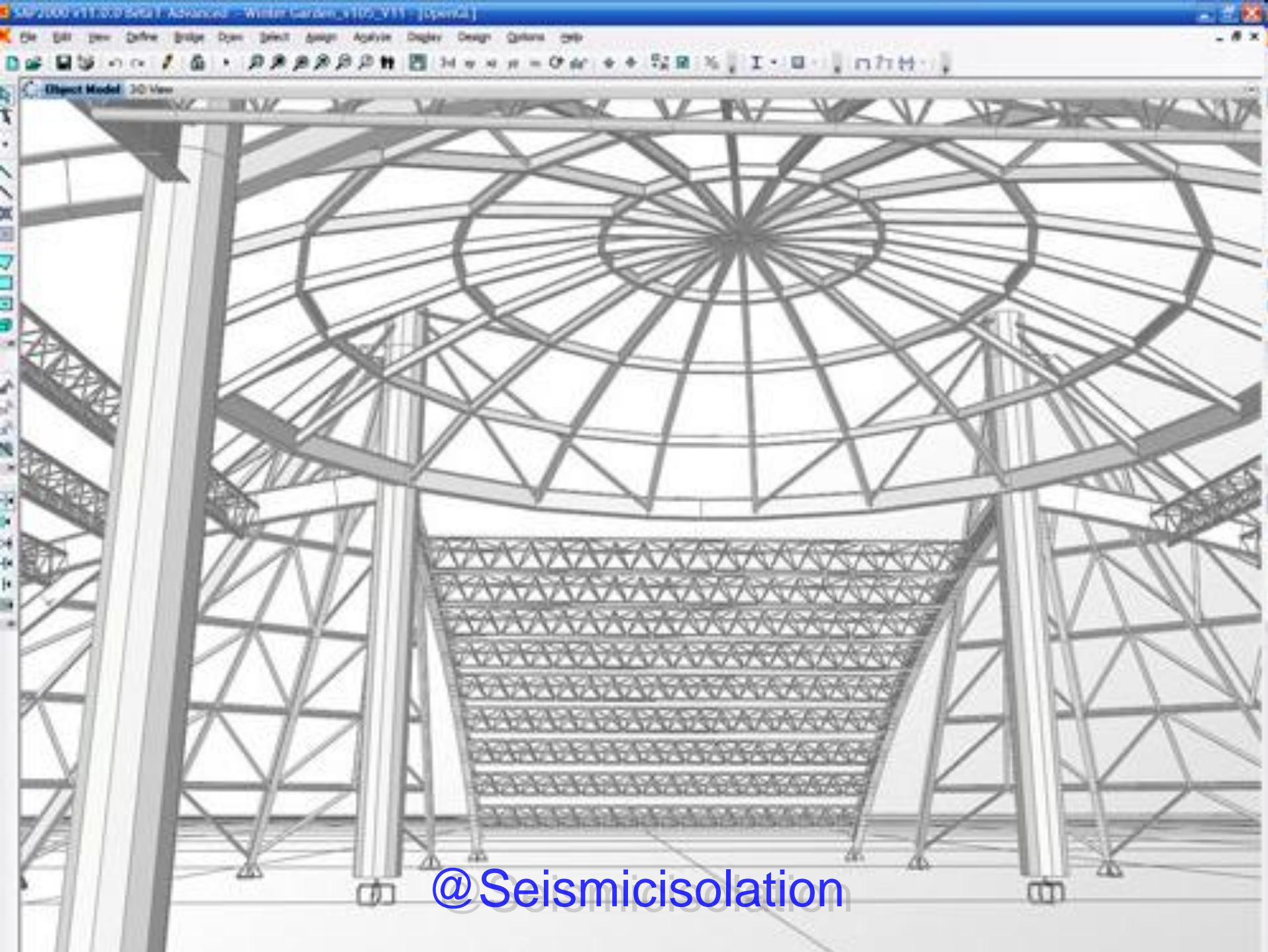


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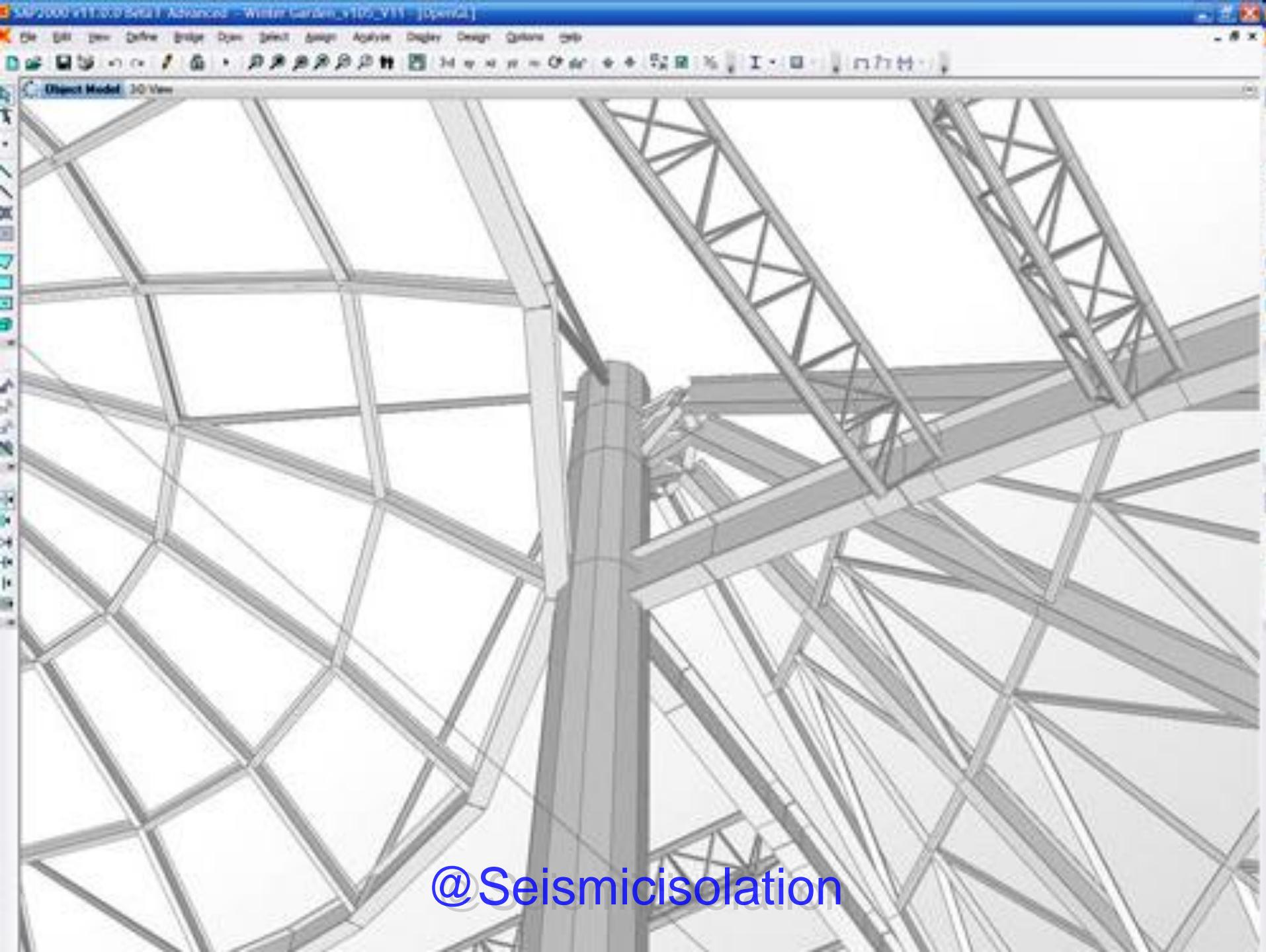


C Object Model 3D View

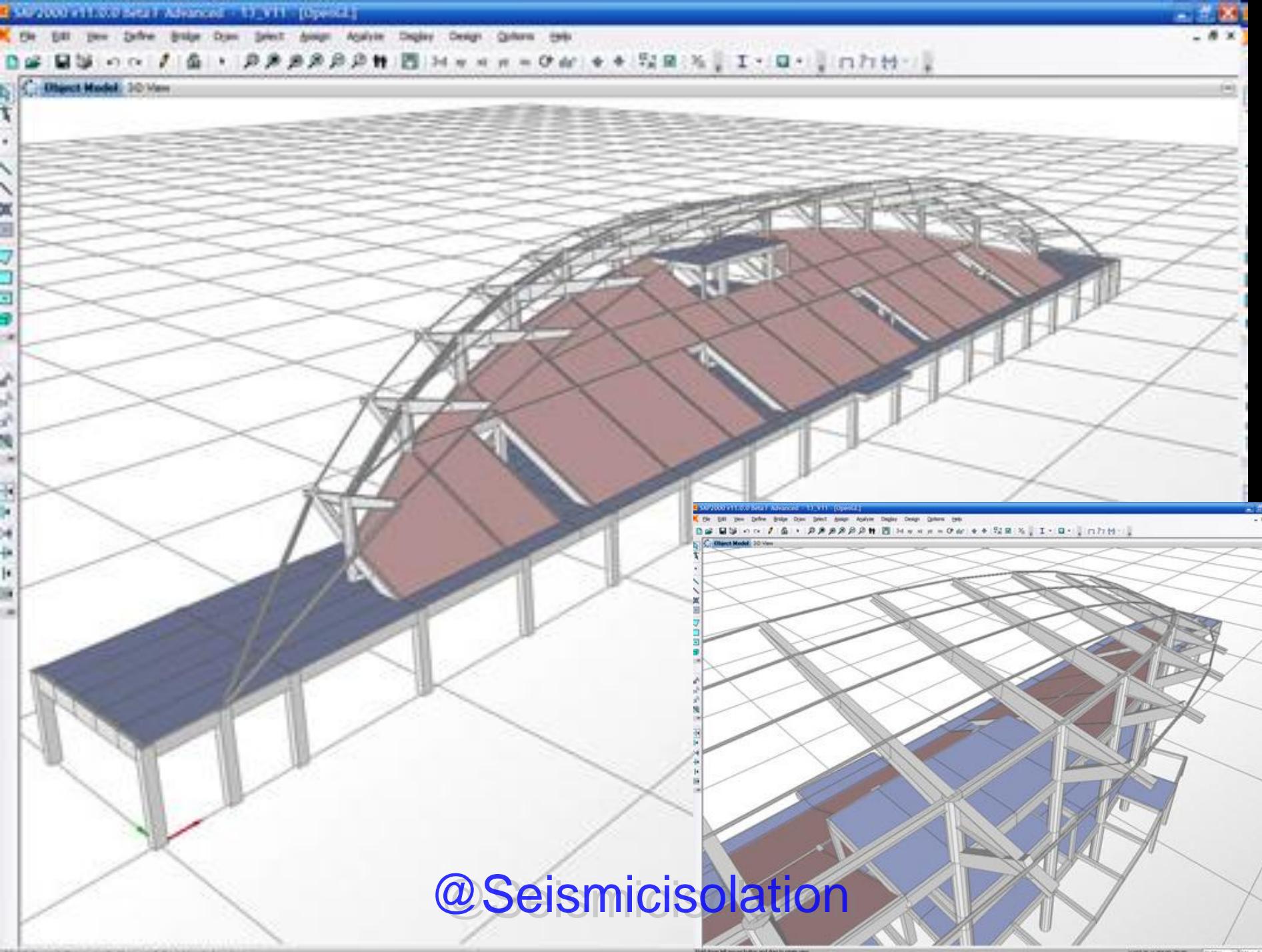
@Seismicisolation



@Seismicisolation



@Seismicisolation



@Seismicisolation



**Sony Center, Potzdamer  
Platz, Berlin, 2000, Helmut  
Jahn Arch., Ove Arup USA  
Struct. Eng**

@Seismicisolation



21.9.2001

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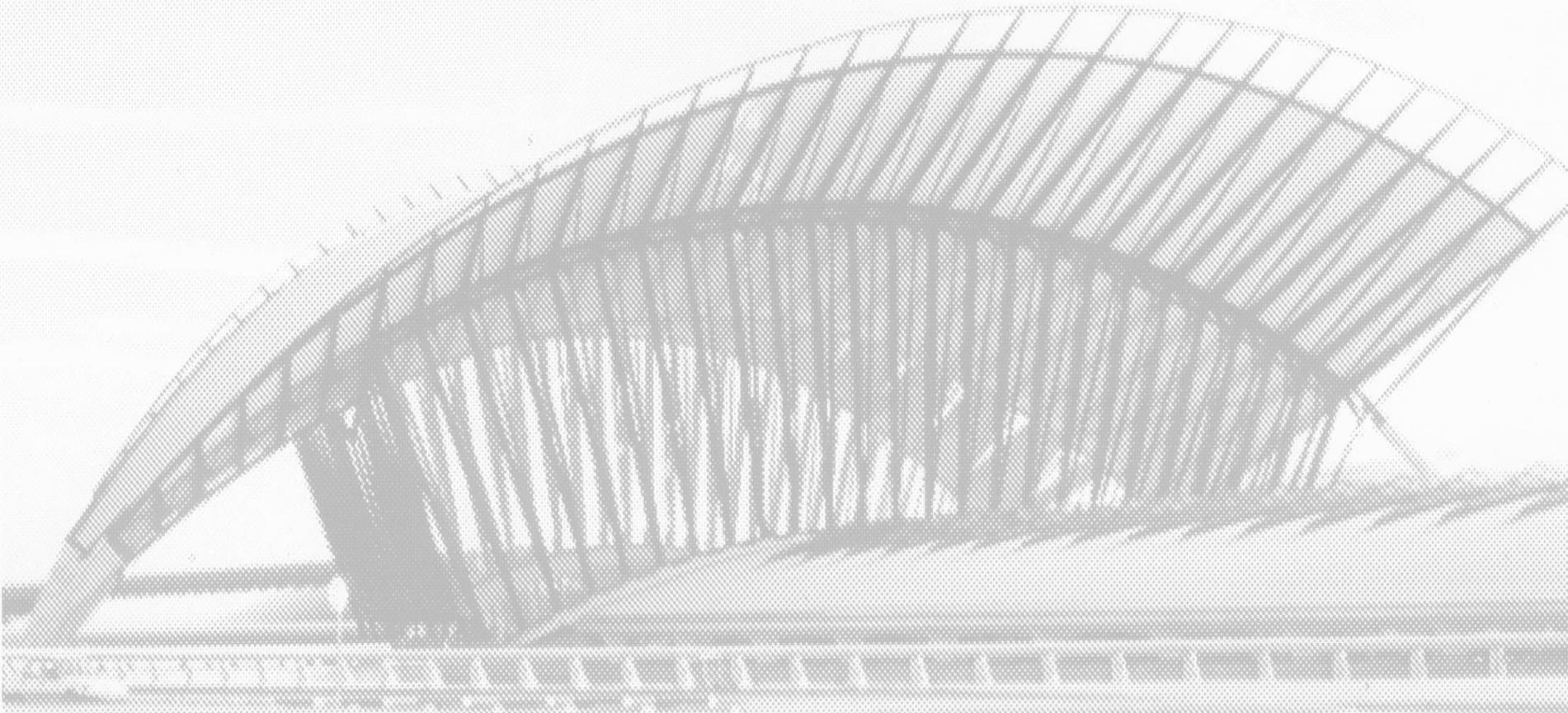


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A photograph looking up at a modern architectural feature, likely a spiral staircase or a large atrium. The structure is composed of a central vertical column surrounded by a series of concentric, curved metal frames that support a translucent or glass roof. The floor below is visible as a dark, curved surface. The overall design is minimalist and geometric.

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© Seismic isolation  
FRAMES / ARCHES



**FRAMES / ARCHES**  
© Seismic isolation

**FRAMES** are **flexural-axial systems** in contrast to **hinged trusses**, which are axial systems, and **beams**, which are flexural systems. Flexural-axial systems are identified by **beam-column behavior** that includes the effects of biaxial bending, torsion, axial deformation, and biaxial shear deformations.

Here, **two-dimensional skeleton structures composed of linear elements** are briefly investigated. The most common group of planar structure systems includes

- post-beam structures,**
- bent and folded beams,**
- rectangular portal frames,**
- cantilever frames,**
- braced frames,**
- pitched frames,**
- arches,**
- and so on.

These structures may form

**short-span or long-span,  
single-story or multi-story,  
single-bay or multi-bay systems.**

They range from low-rise to

**single, open, large volume buildings  
cellular massive buildings  
skyscrapers.**

Primary emphasis here is on the investigation of simple, but common single-story enclosure systems to develop a feeling for the behavior of structures under gravity and lateral loads. Investigated are the

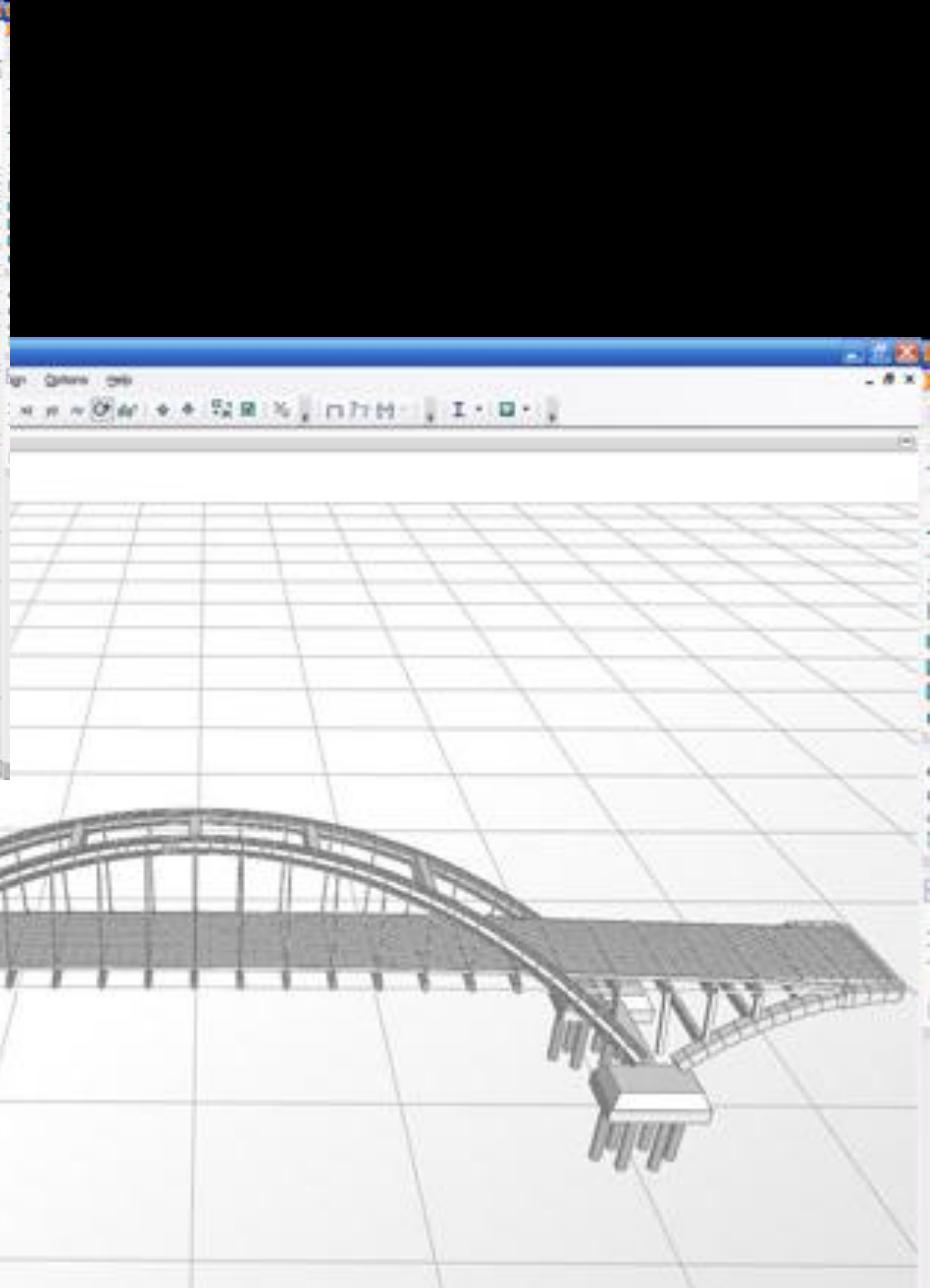
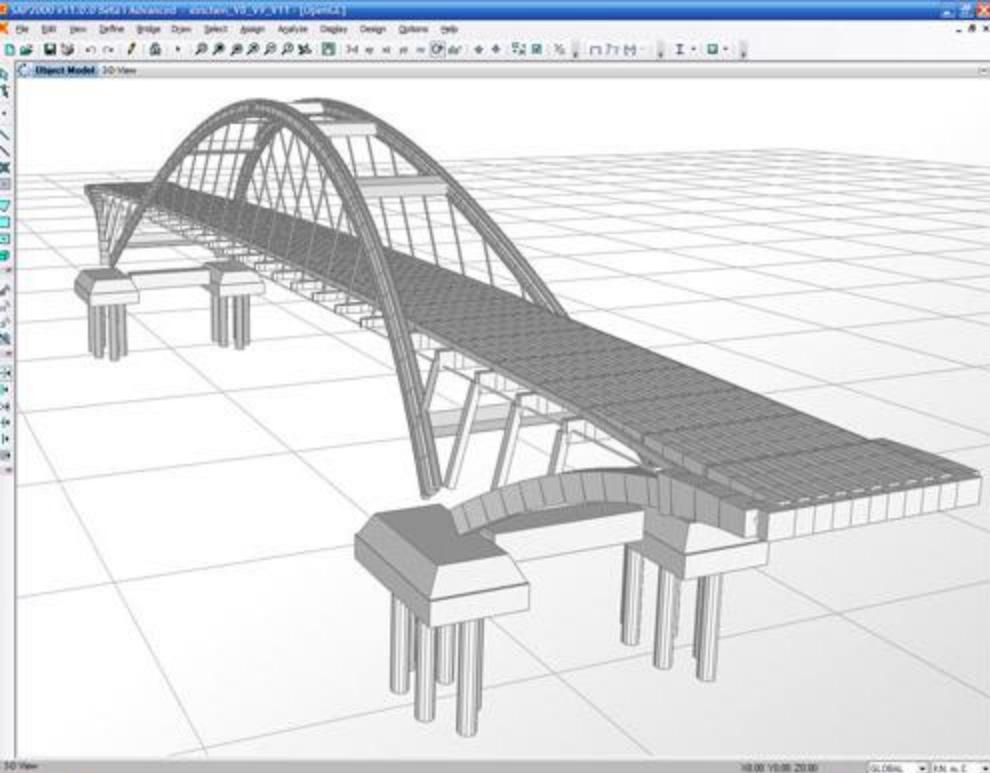
- response and effect of the frame profile on **uniform gravity action**  
and on **lateral loading**.
- the magnitude of the internal member forces is determined so that the computer results can be checked.



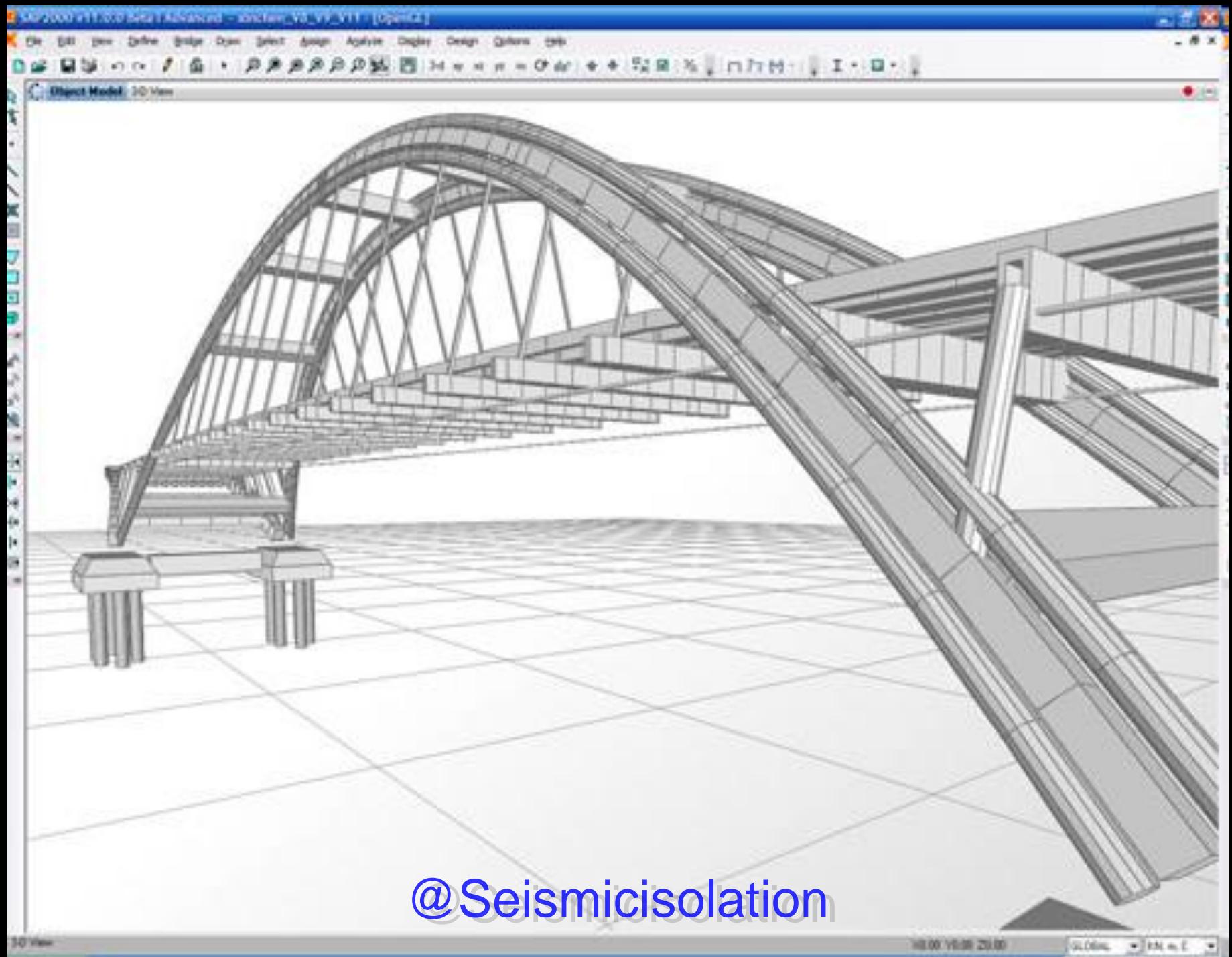
@Seismicisolation  
**Glass House**, New Caanan, 1949, Philip Johnson



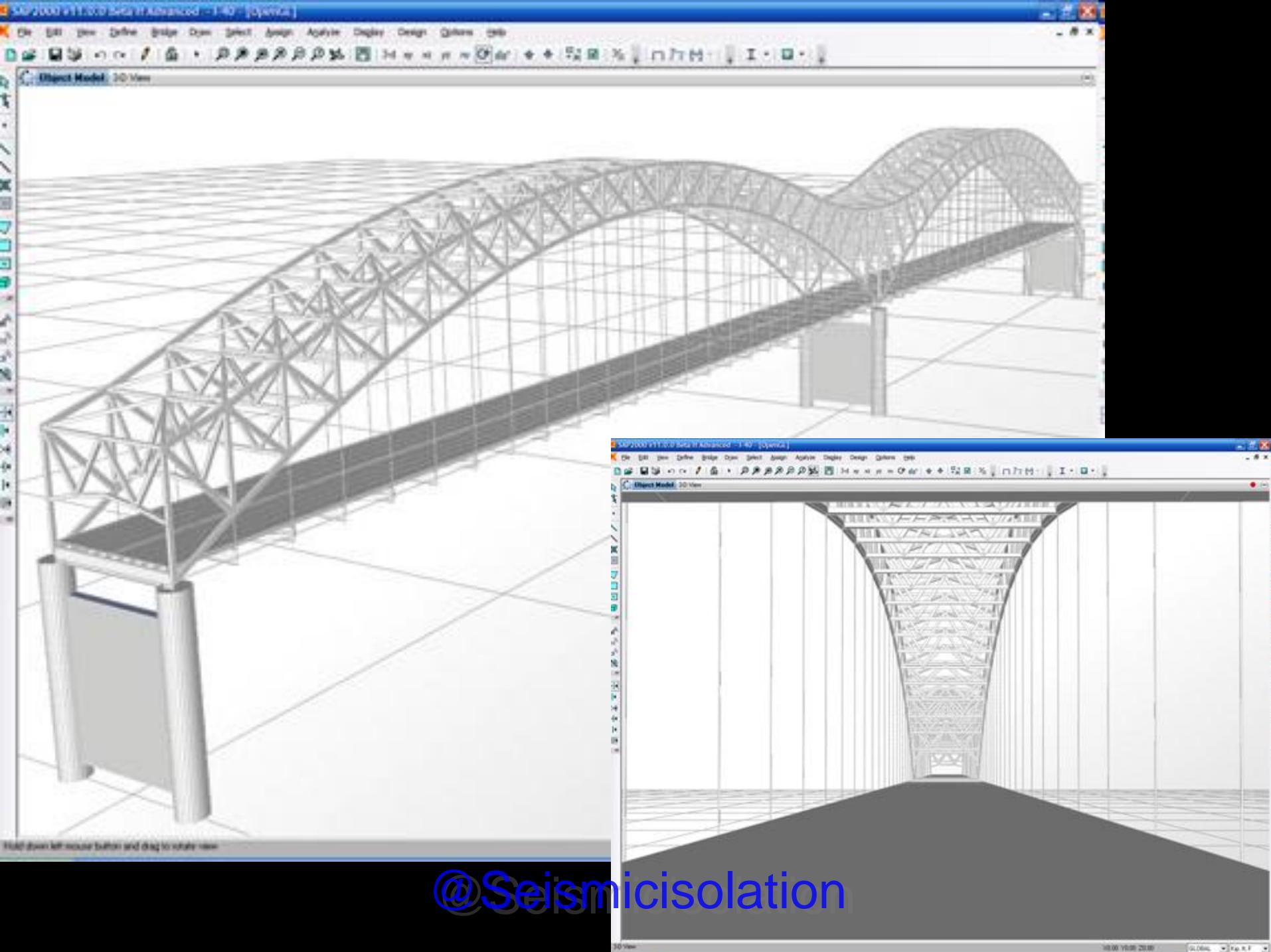
Crown Hall, IIT, Chicago, 1956. Mies van der Rohe  
© Seismic Isolation



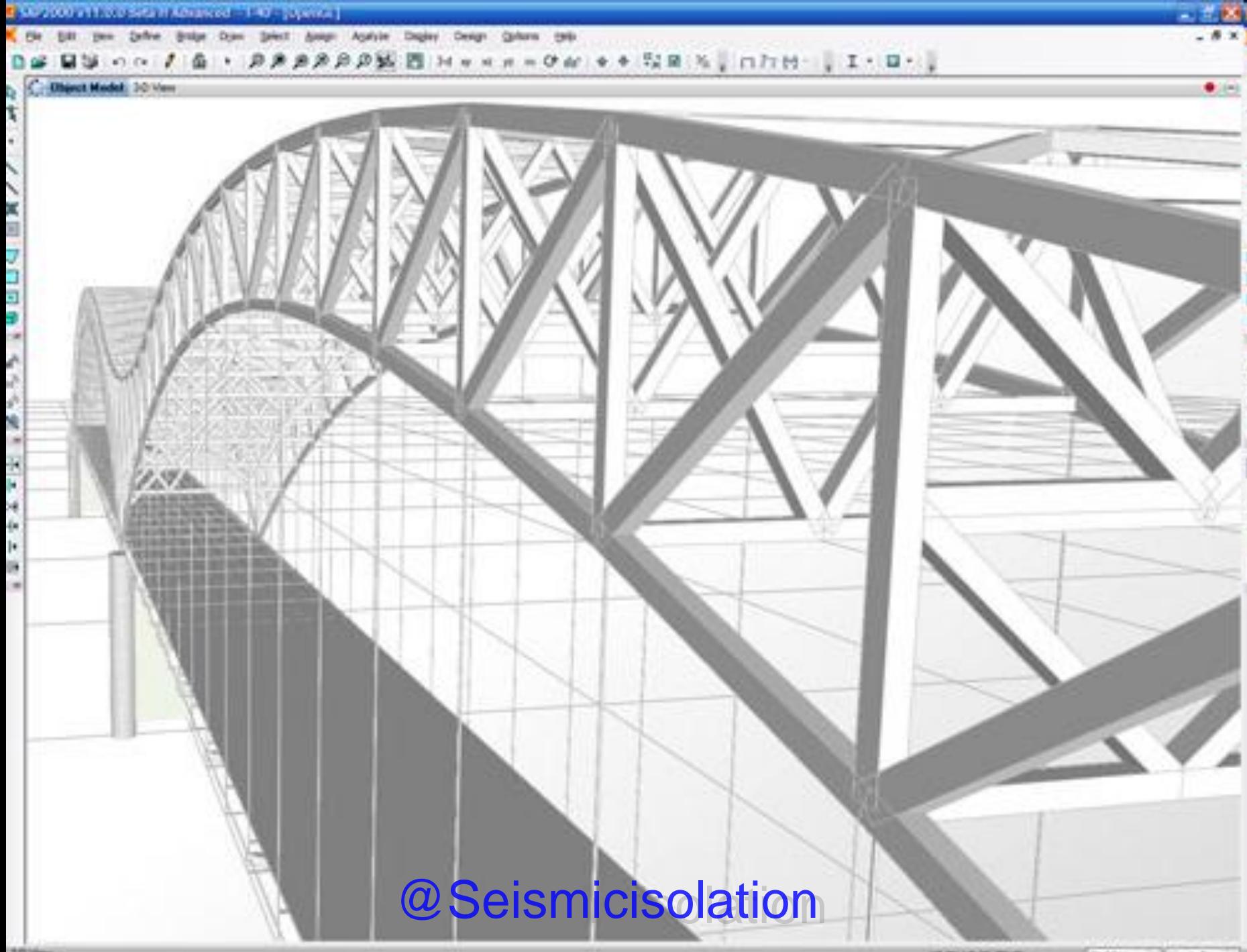
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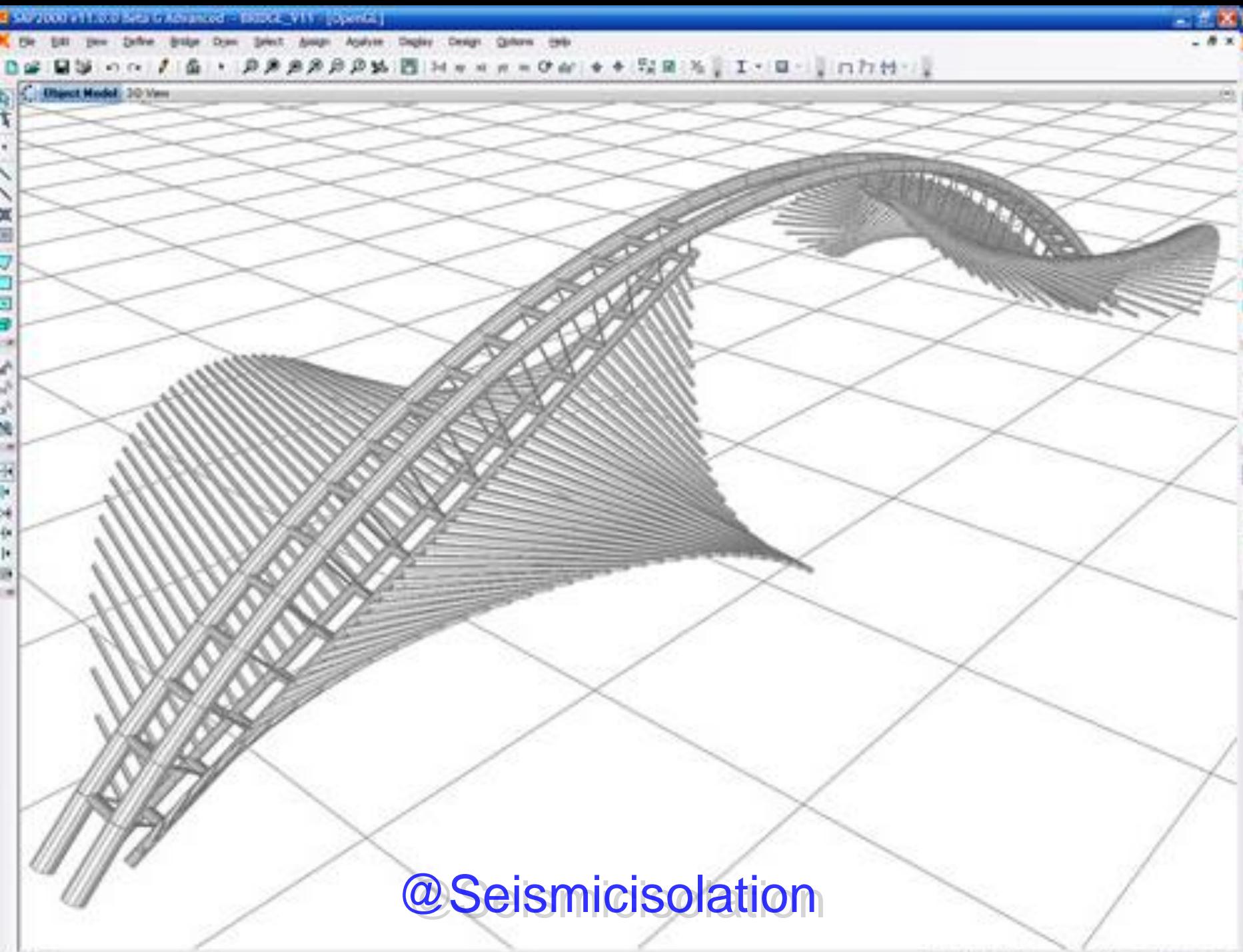
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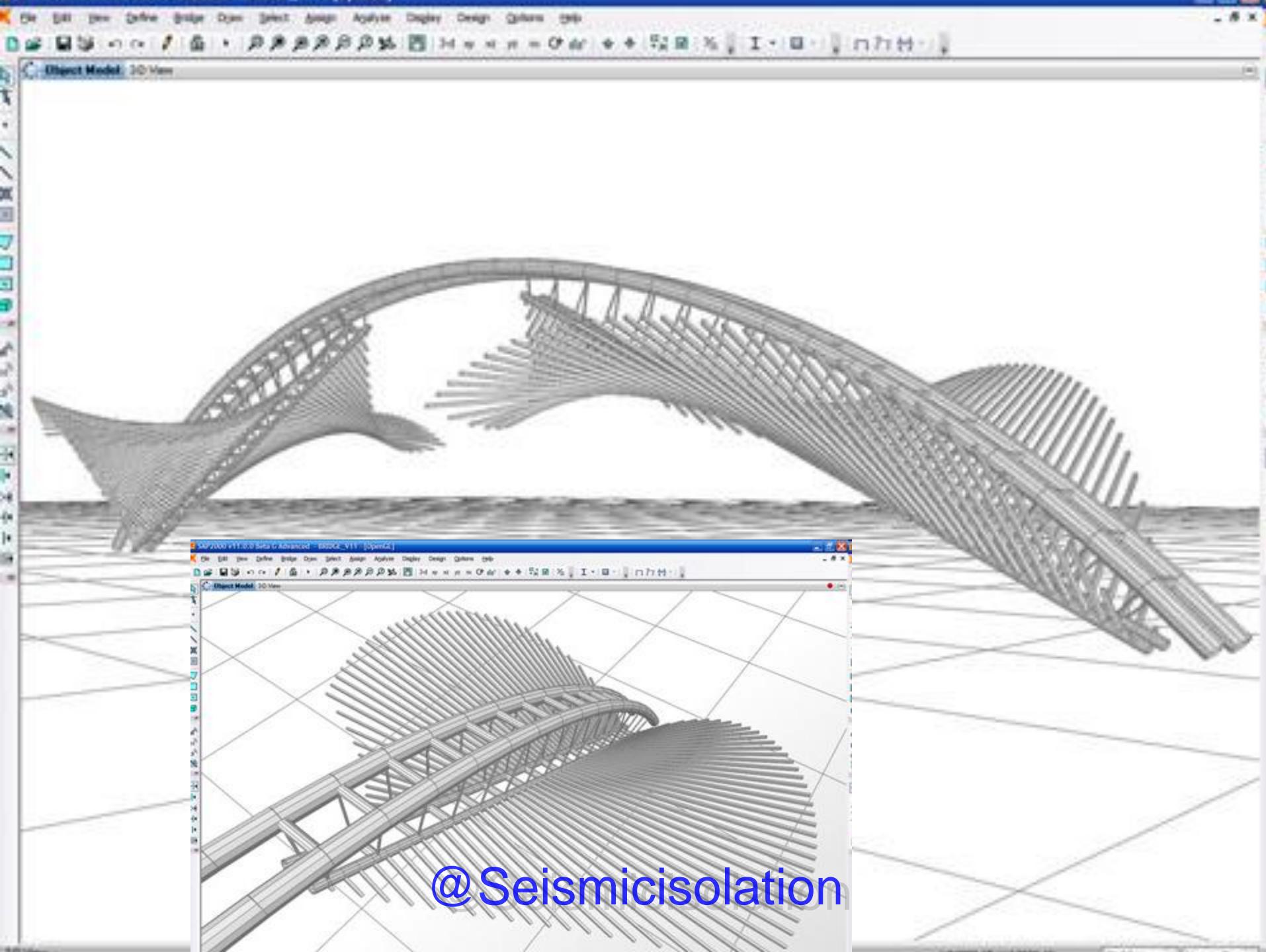
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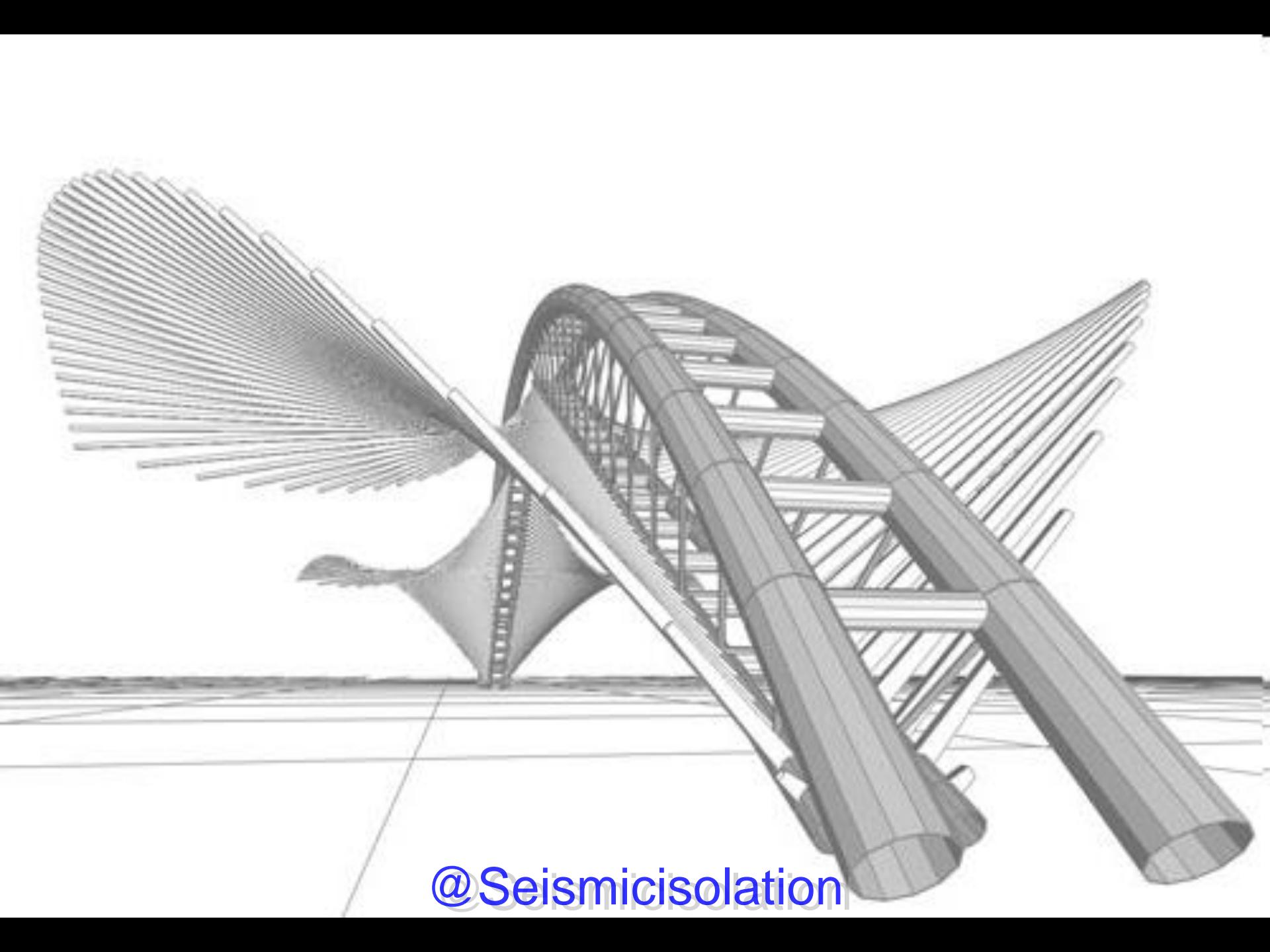
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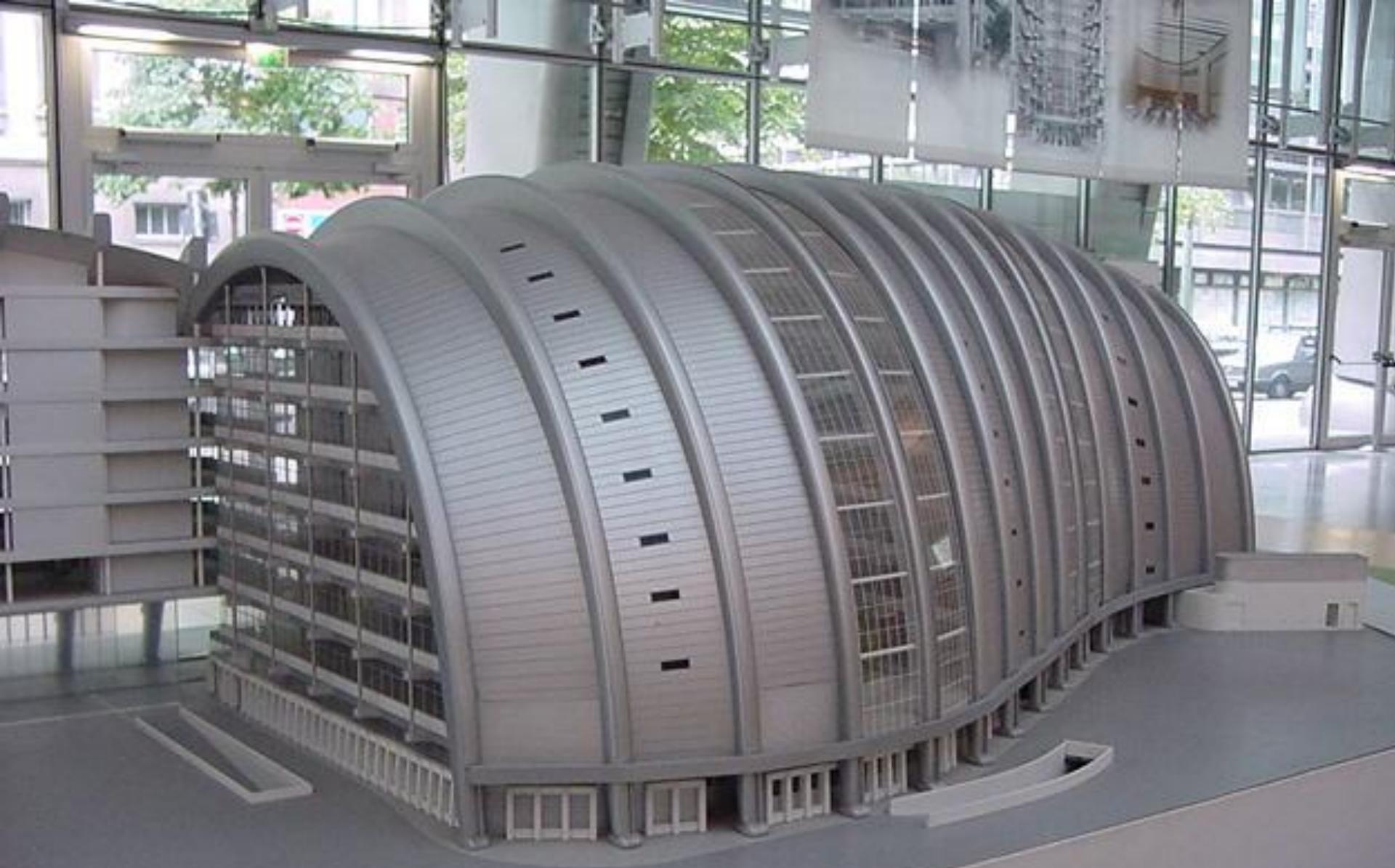


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**Berlin Stock Exchange,  
Berlin, Germany, 1999,  
Nick Grimshaw**

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25 9 2001



25 9 2001

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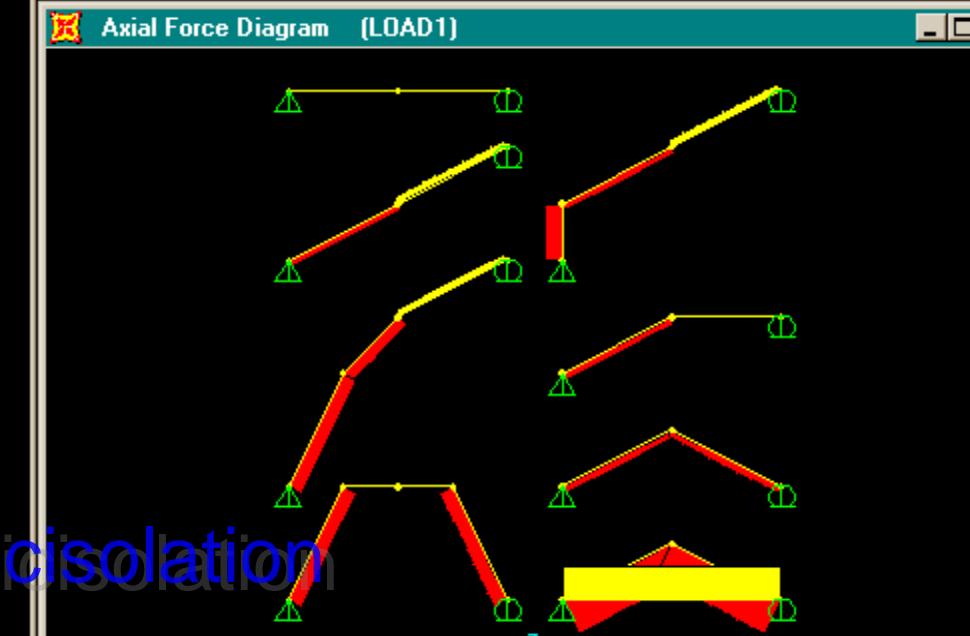
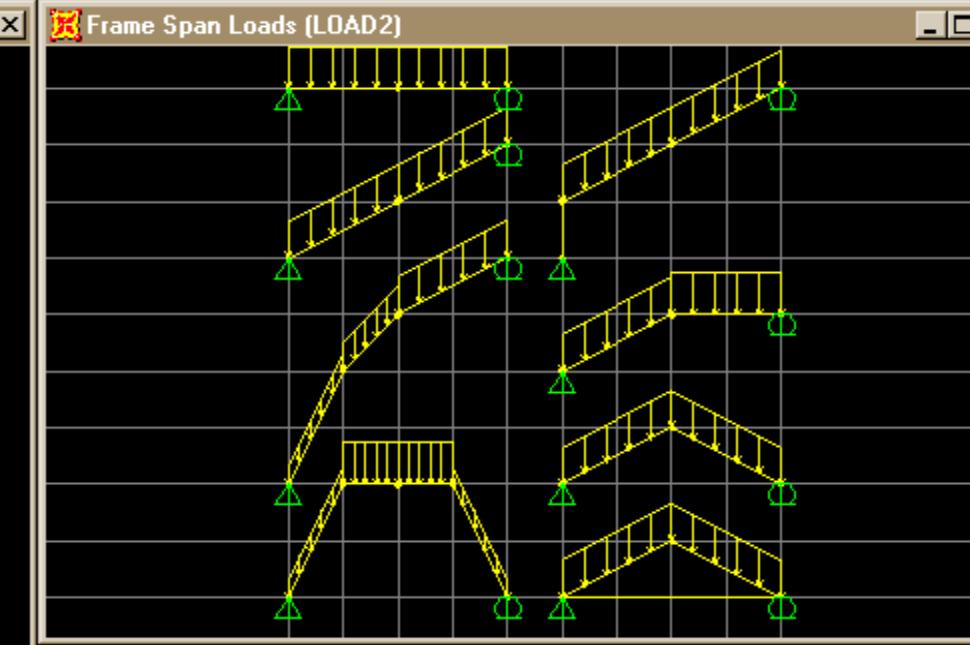
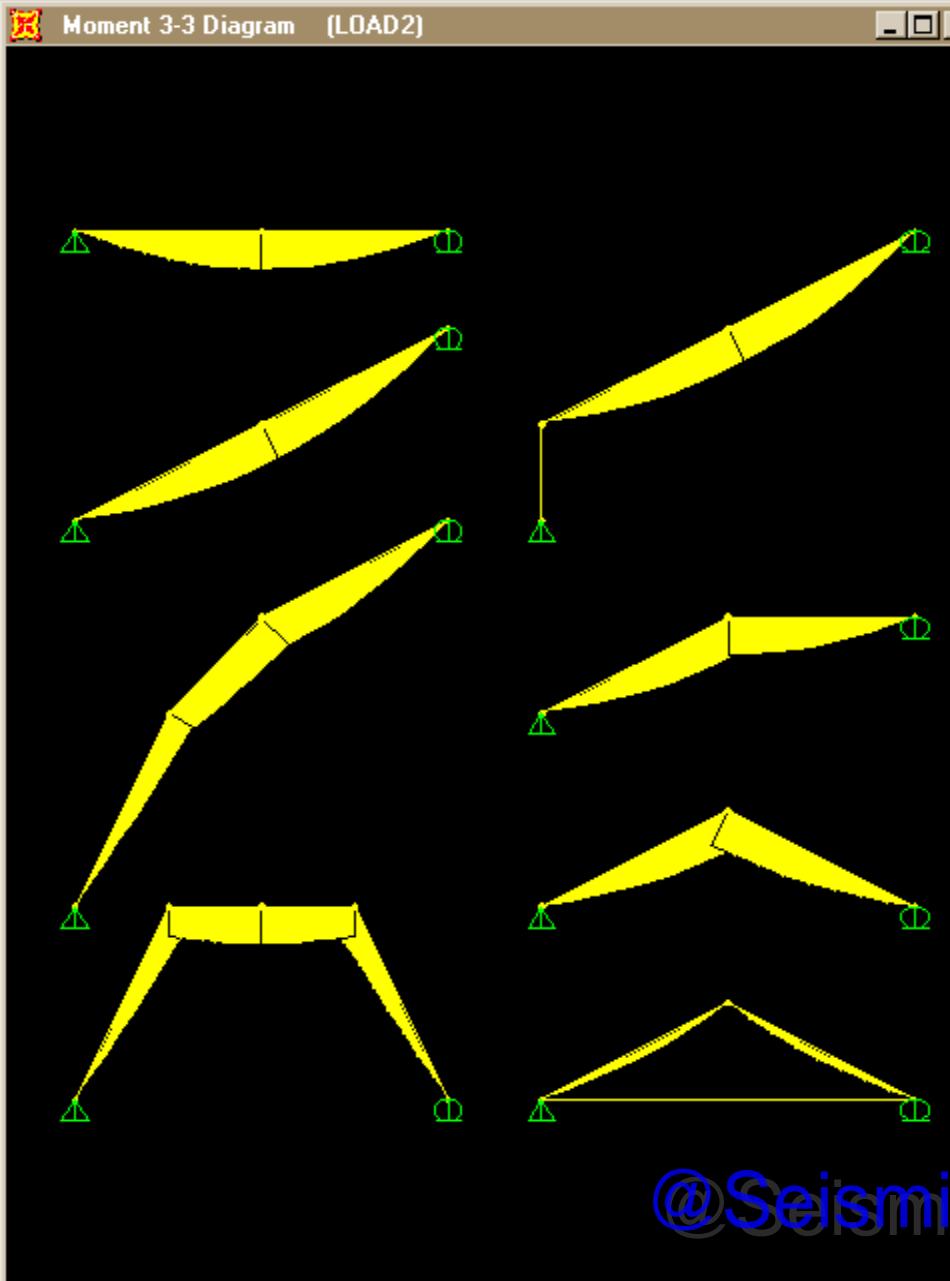
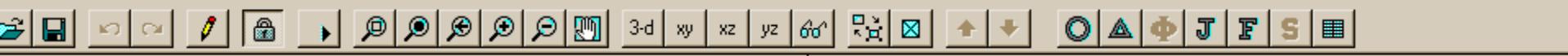
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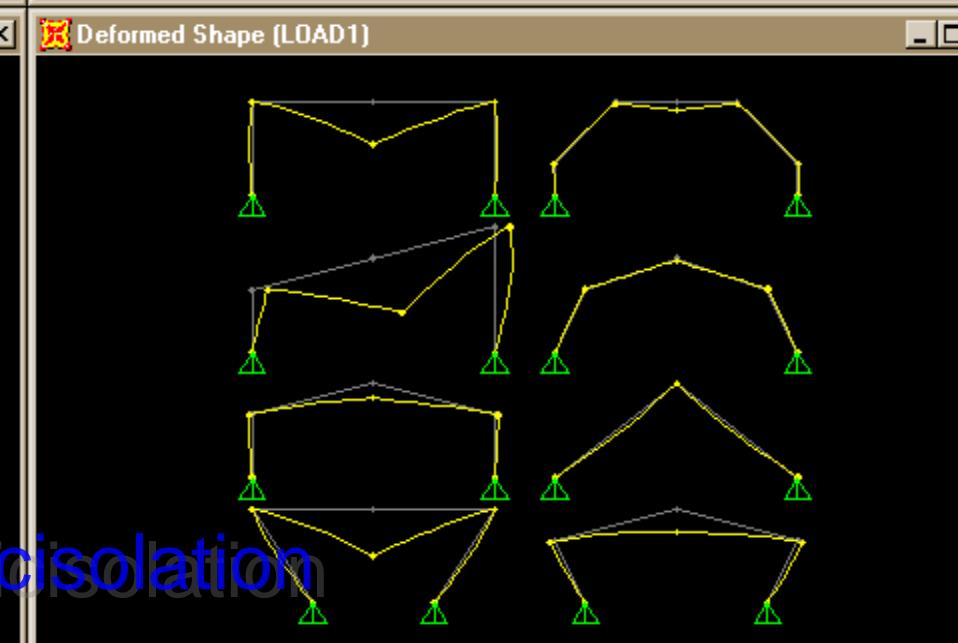
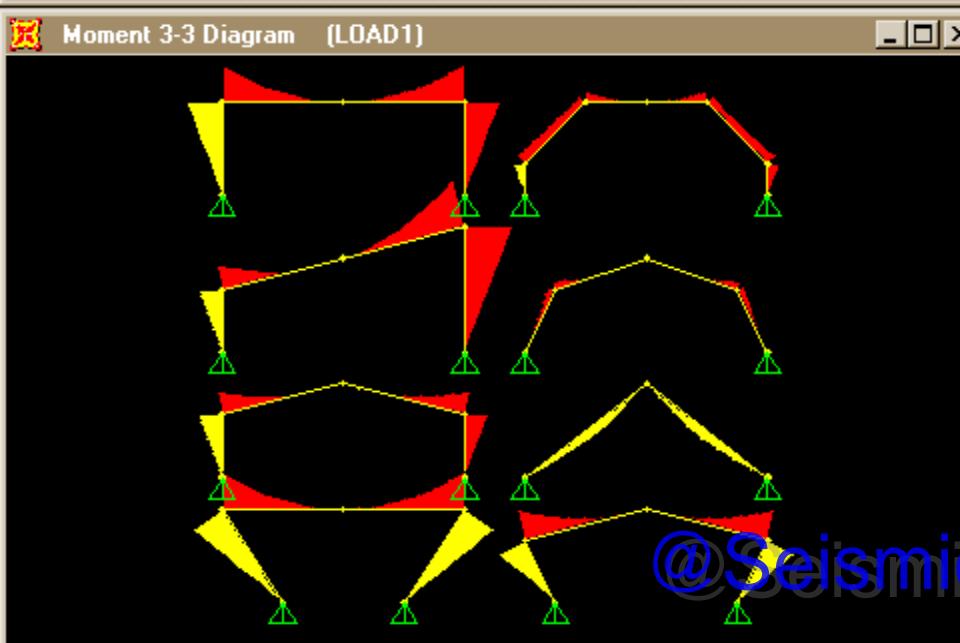
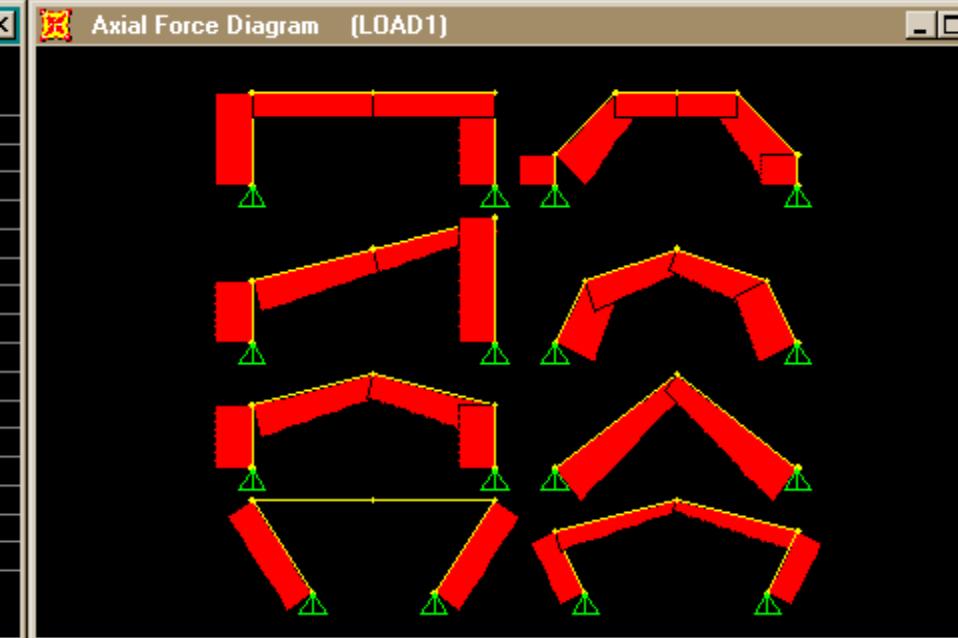
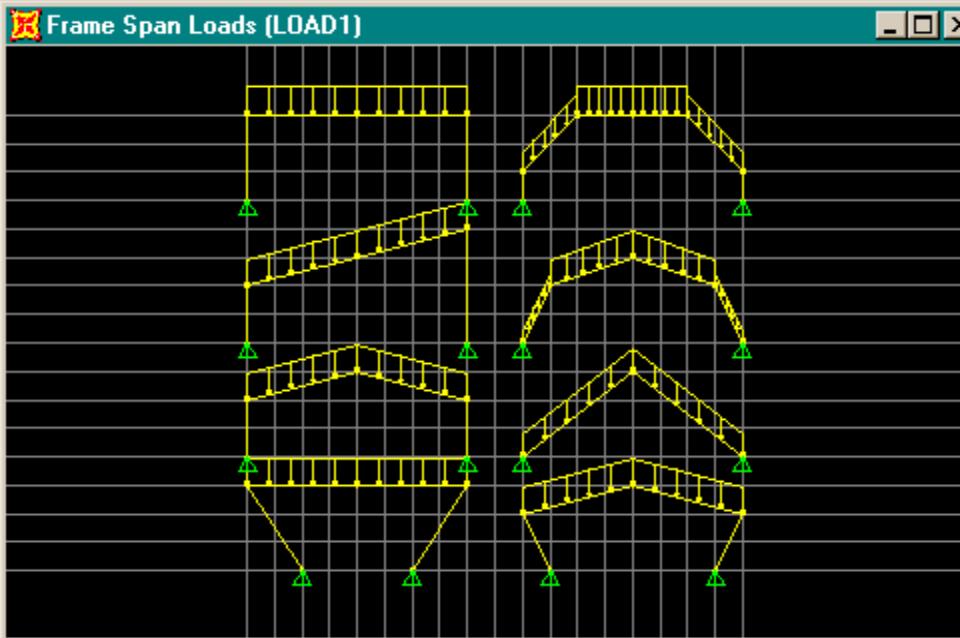
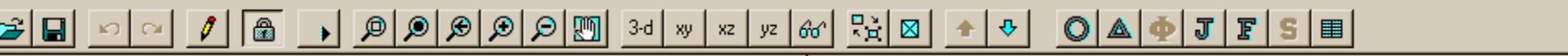
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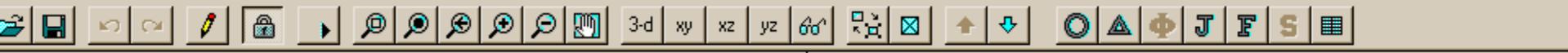
San Diego Natural History Museum, 1970, William Pereira



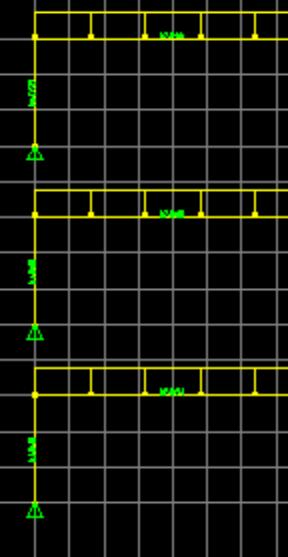
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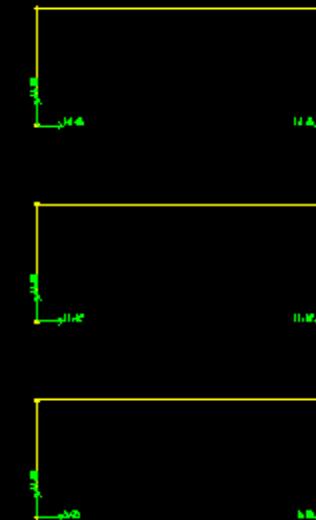
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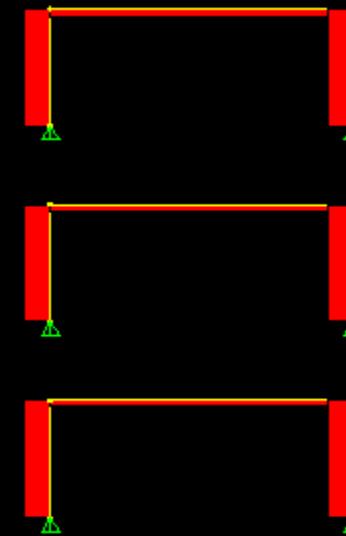
Frame Span Loads (LOAD1)



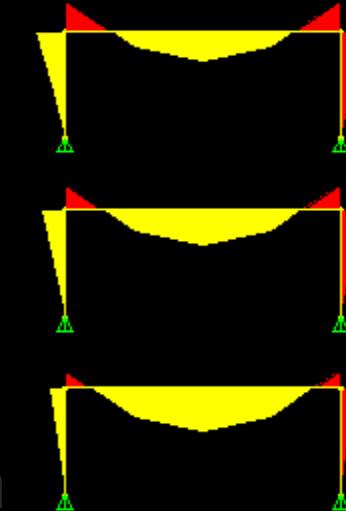
Restraint Reactions (COMB1)



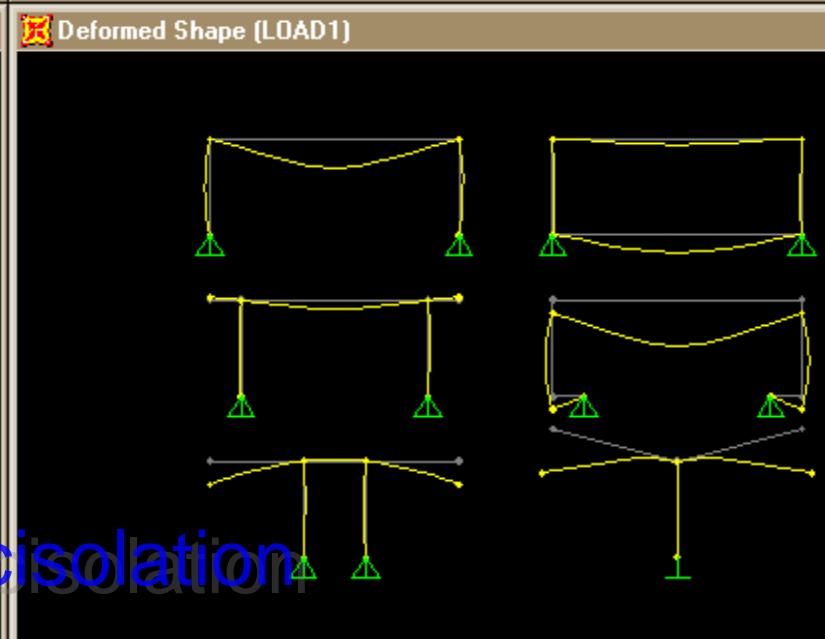
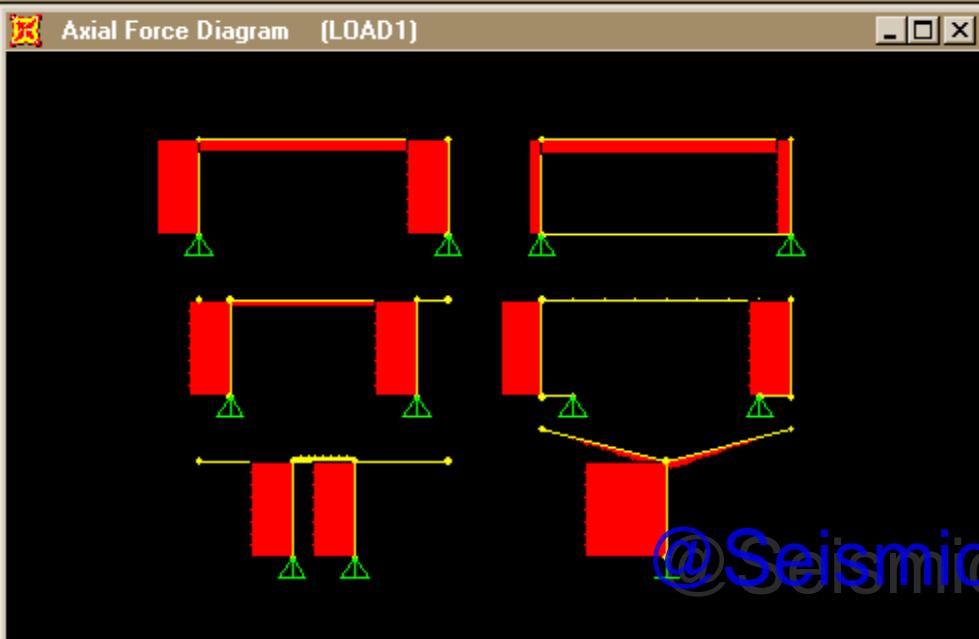
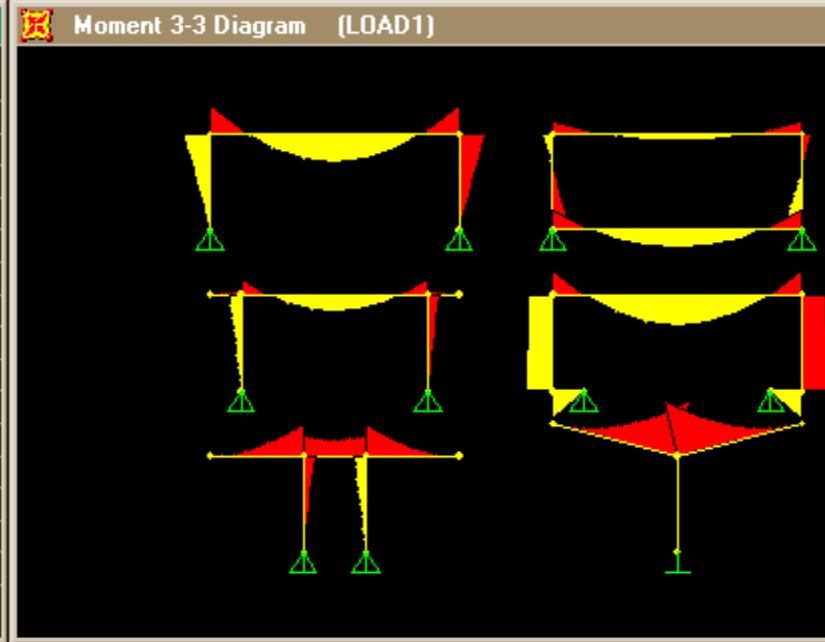
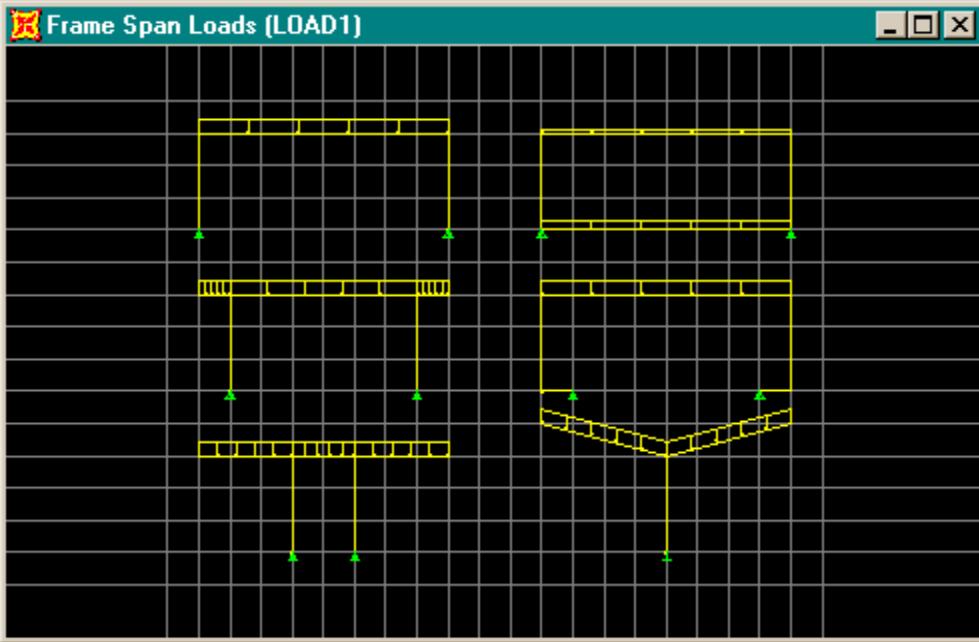
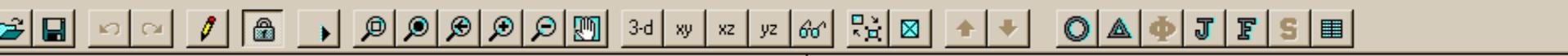
Axial Force Diagram (COMB1)

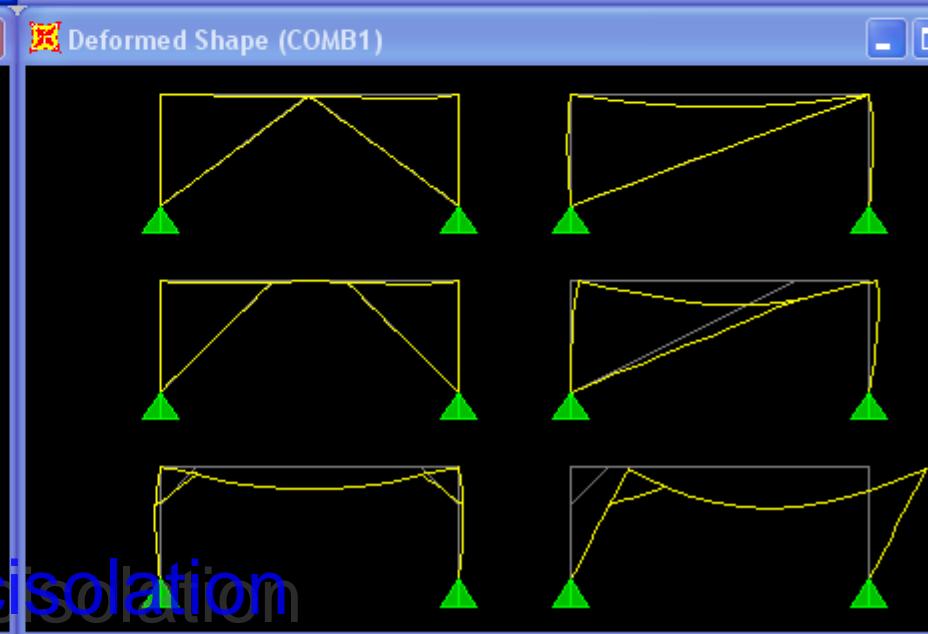
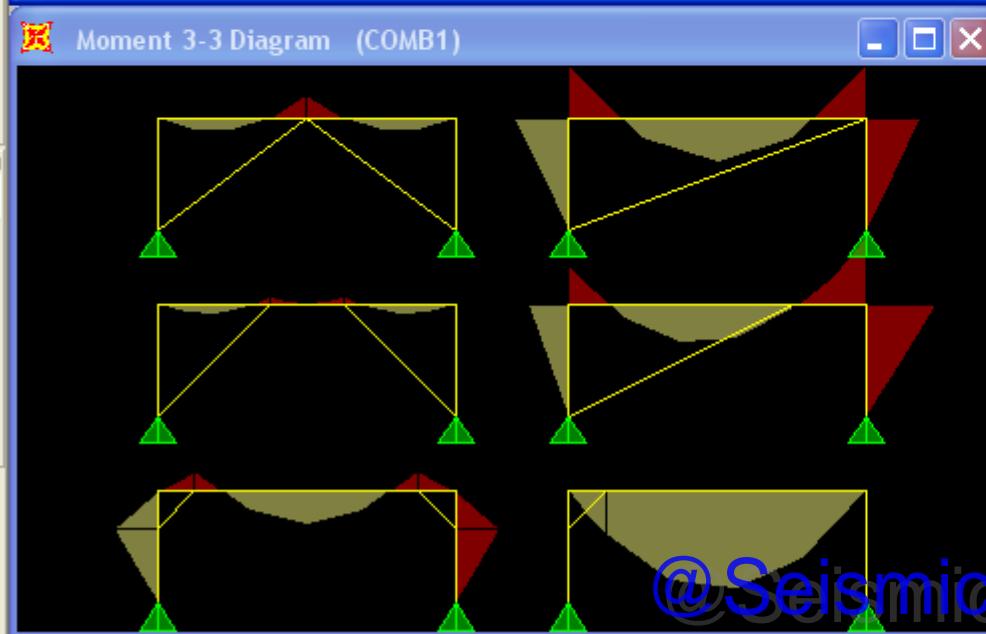
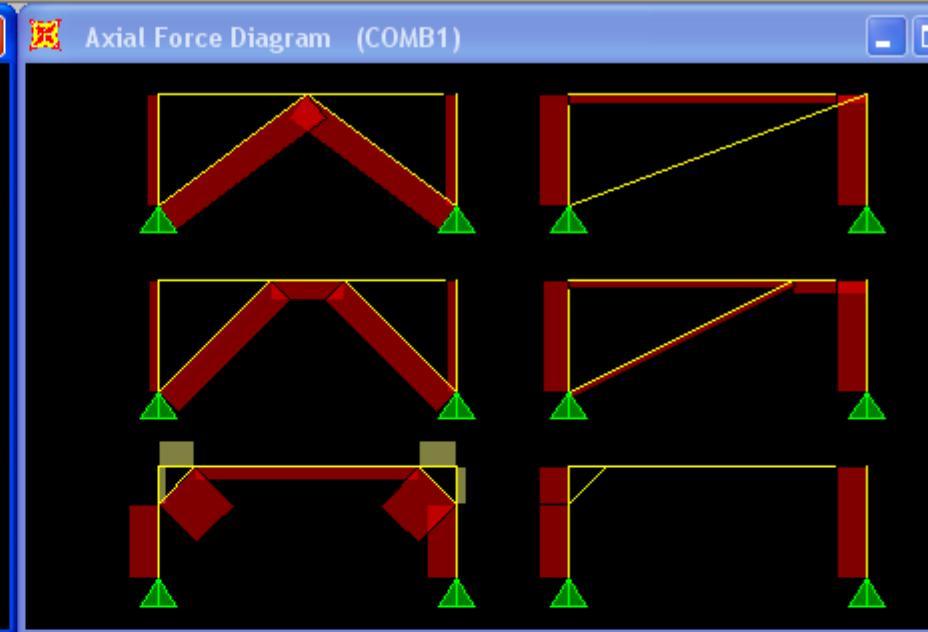
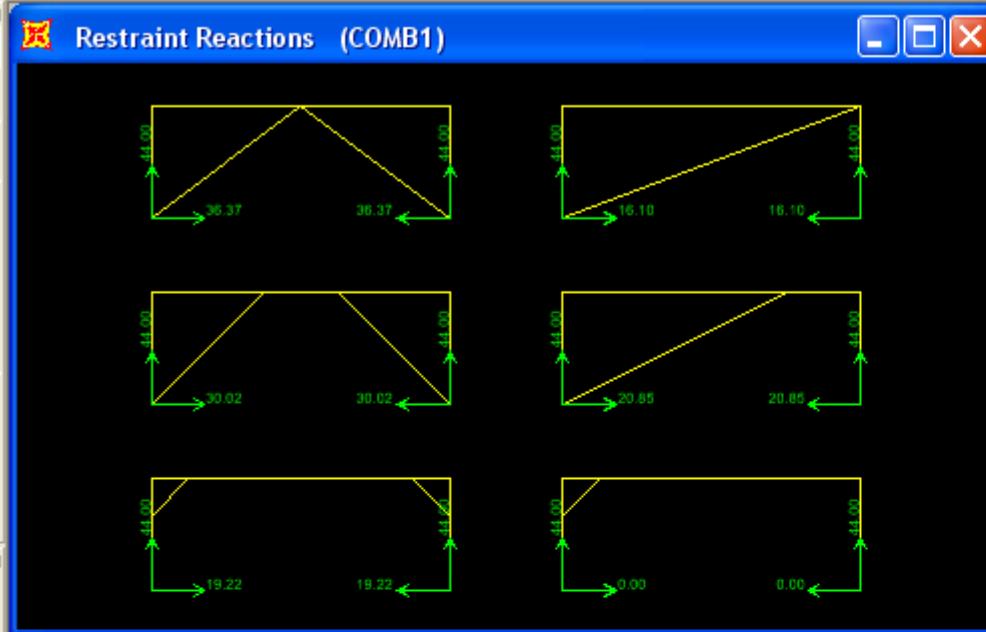


Moment 3-3 Diagram (COMB1)



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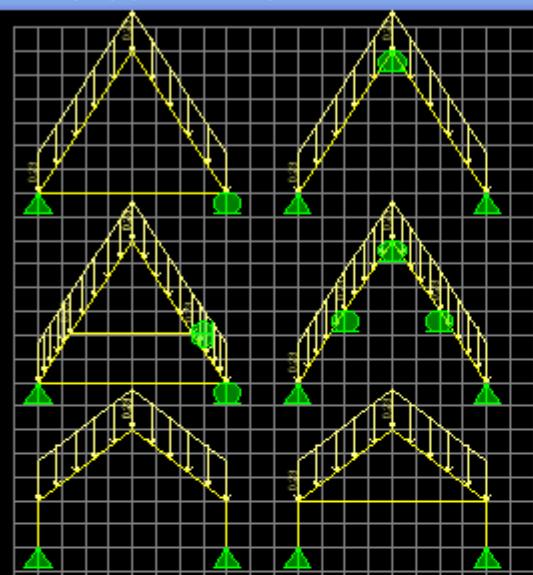
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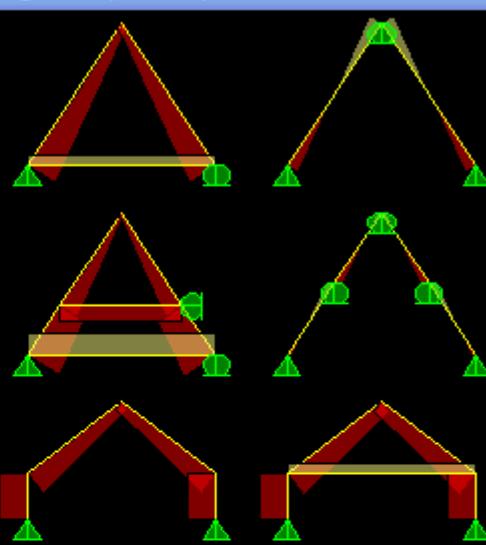
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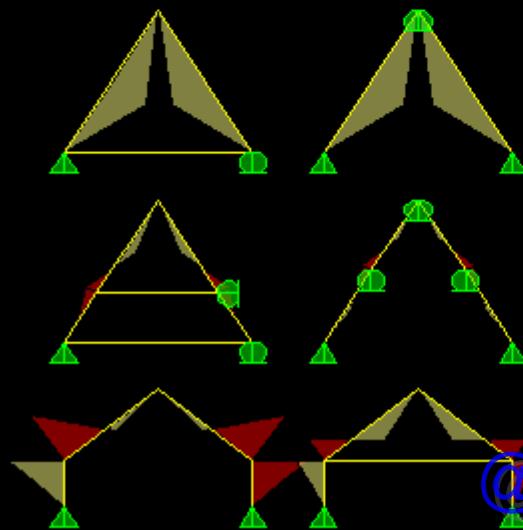
Frame Span Loads (DL) (As Defined)



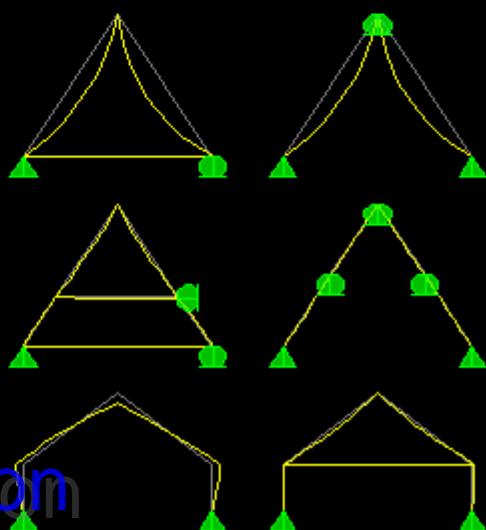
Axial Force Diagram (COMB1)



Moment 3-3 Diagram (COMB1)



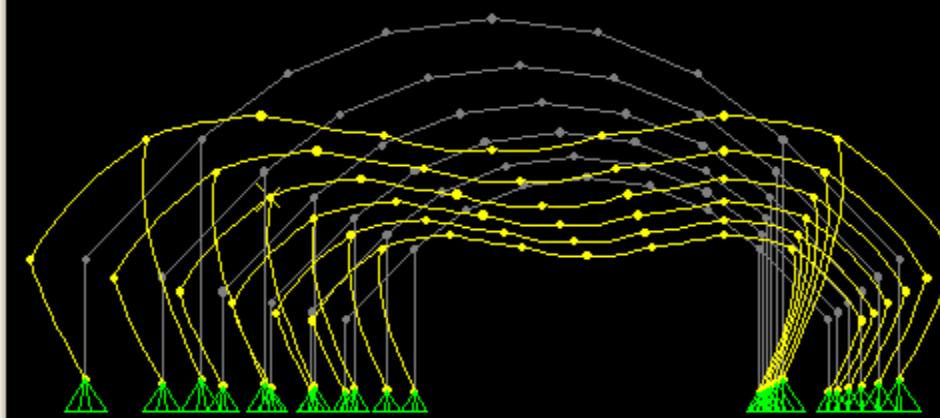
Deformed Shape (COMB1)



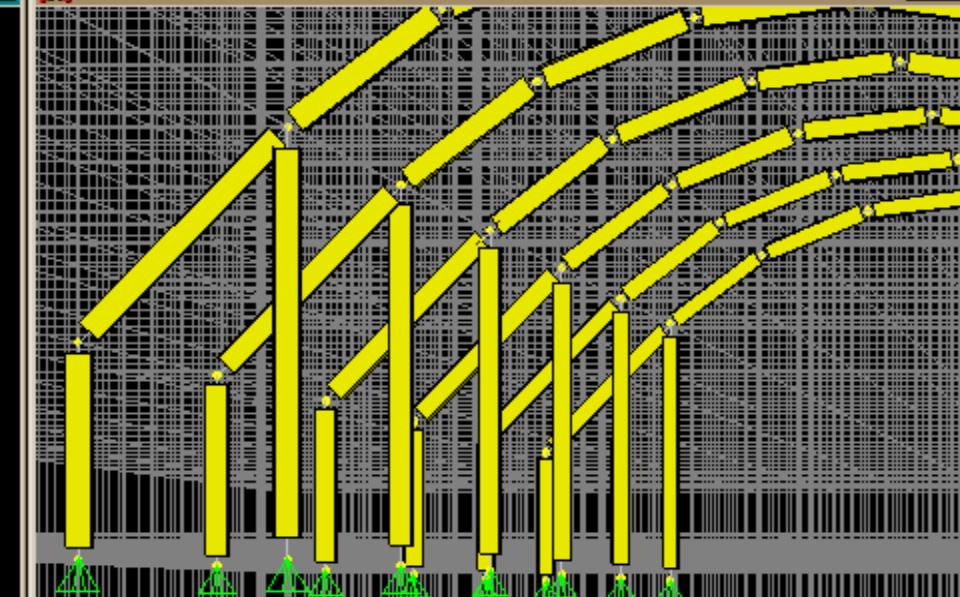
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Deformed Shape (LOAD1)



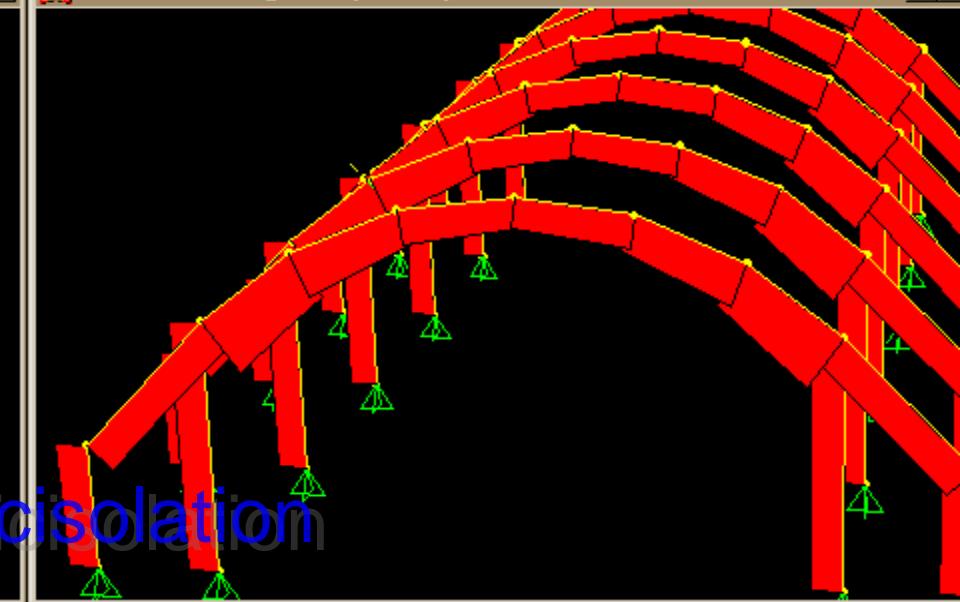
3-D View



Moment 3-3 Diagram (COMB1)



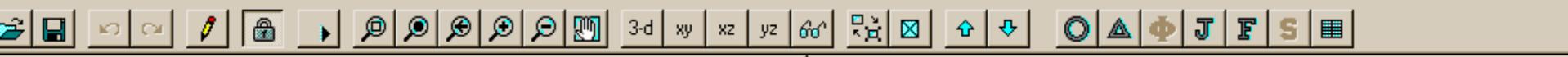
Axial Force Diagram (COMB1)



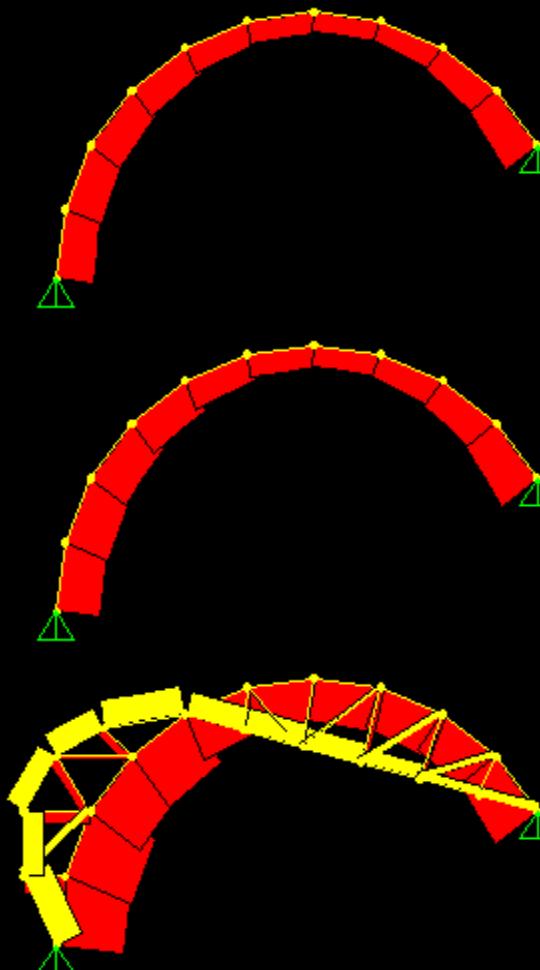
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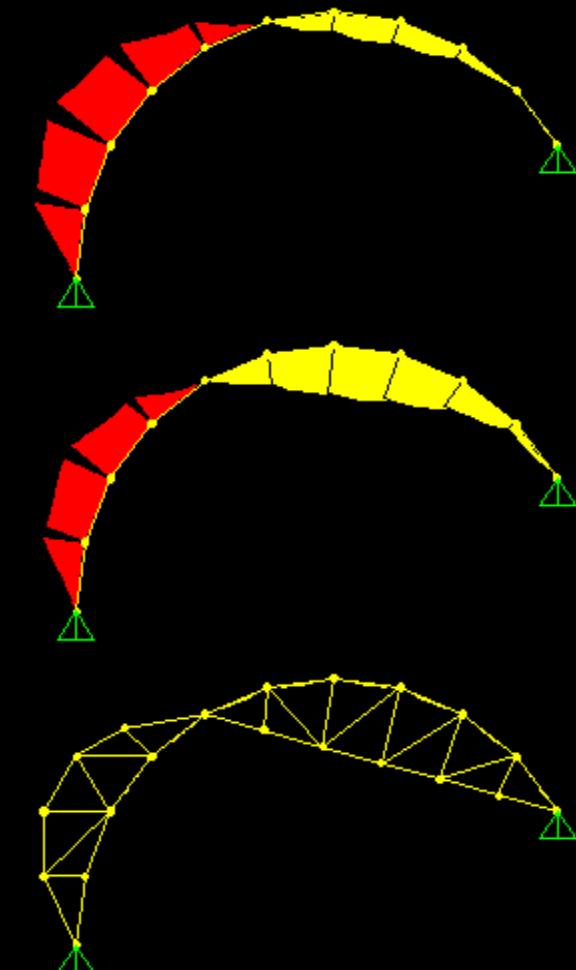
Waterloo Terminal, London, 1993, Nicholas Grimshaw +  
Anthony Hunt



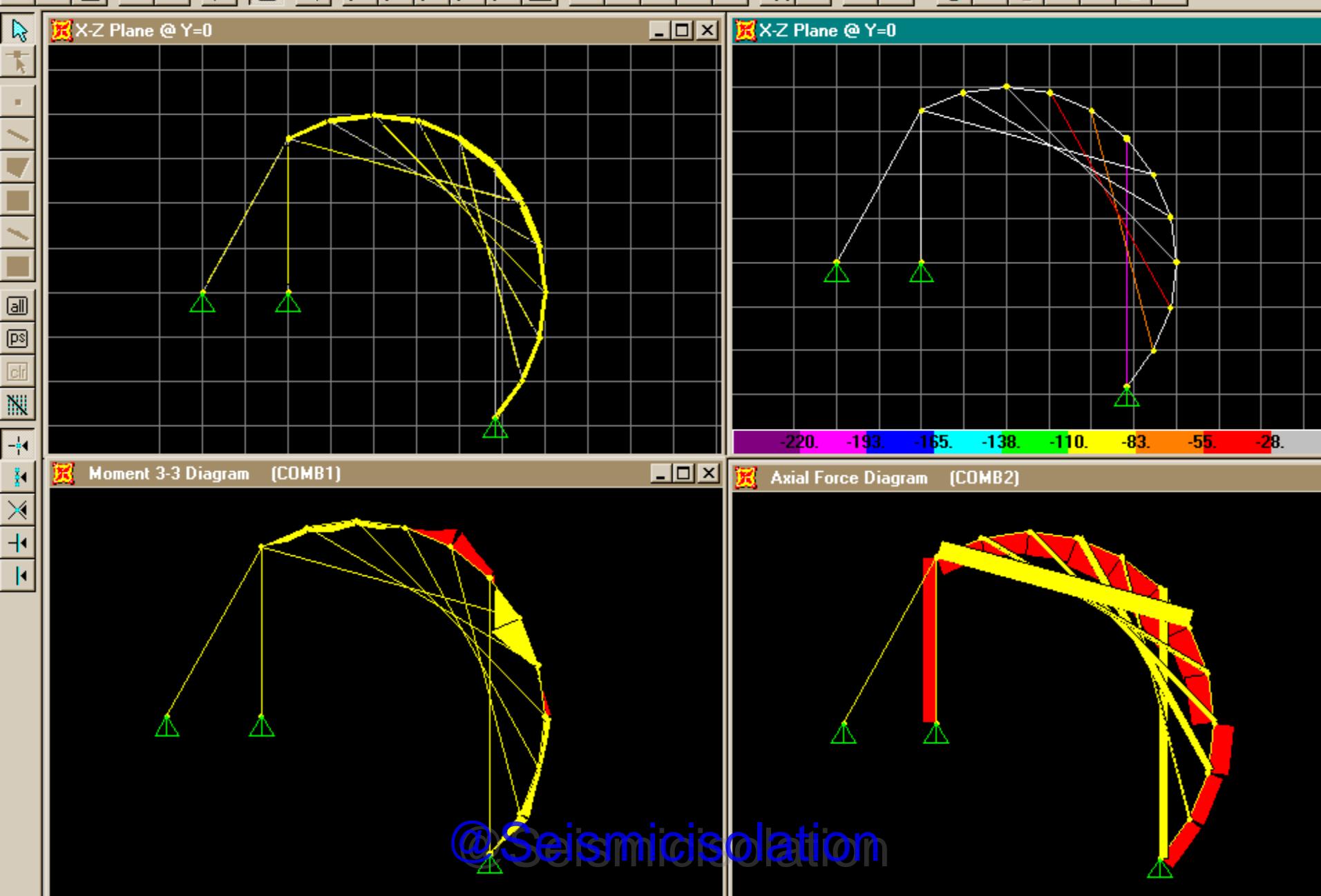
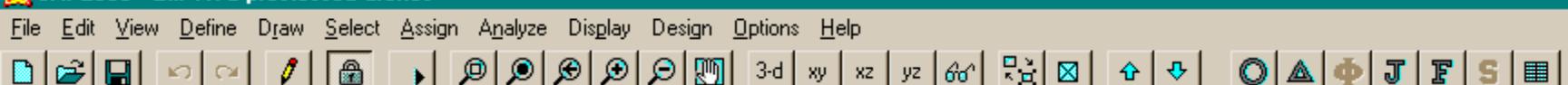
Axial Force Diagram (COMB2)



Moment 3-3 Diagram (LOAD1)

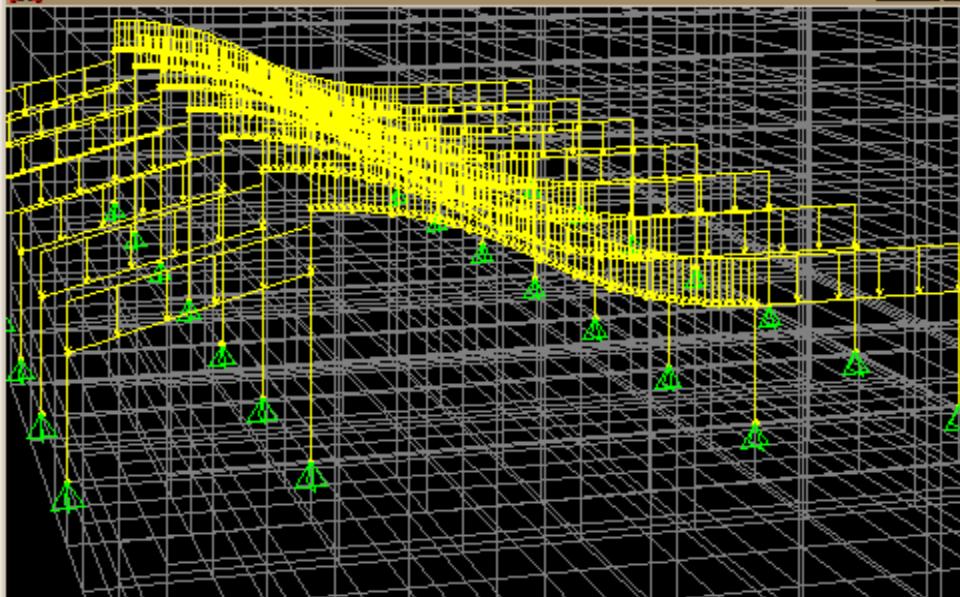


@Seismicisolation

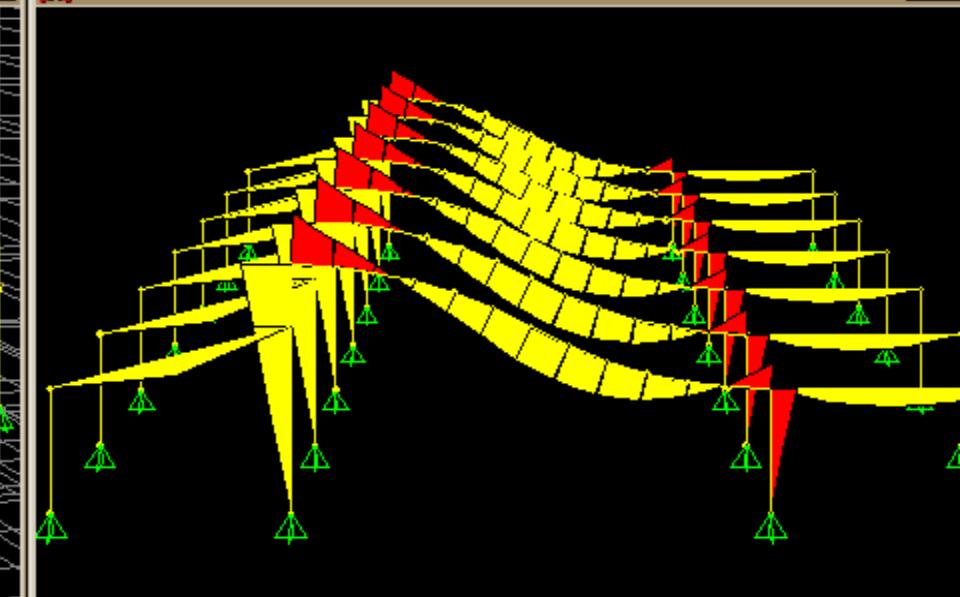




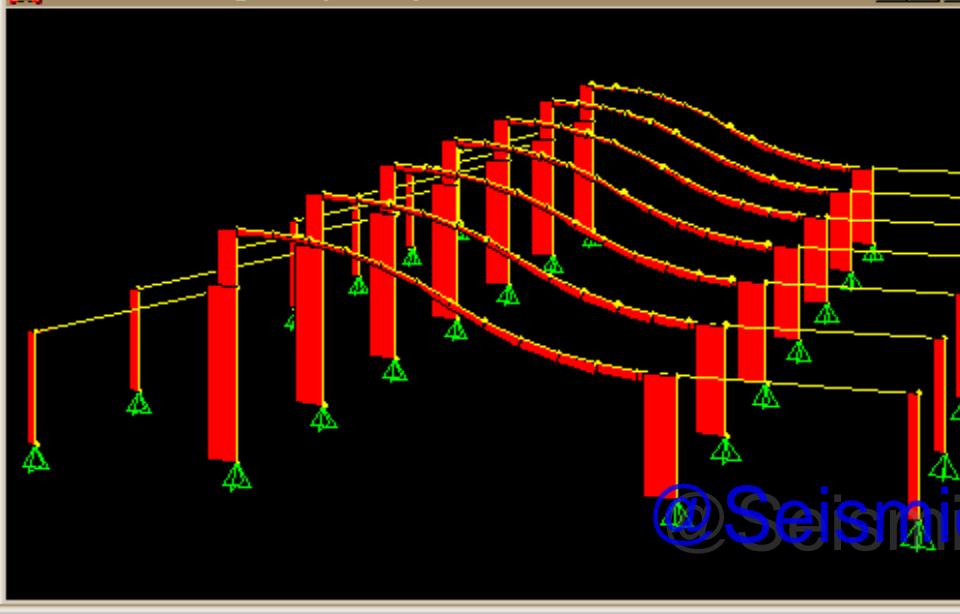
Frame Span Loads (LOAD1)



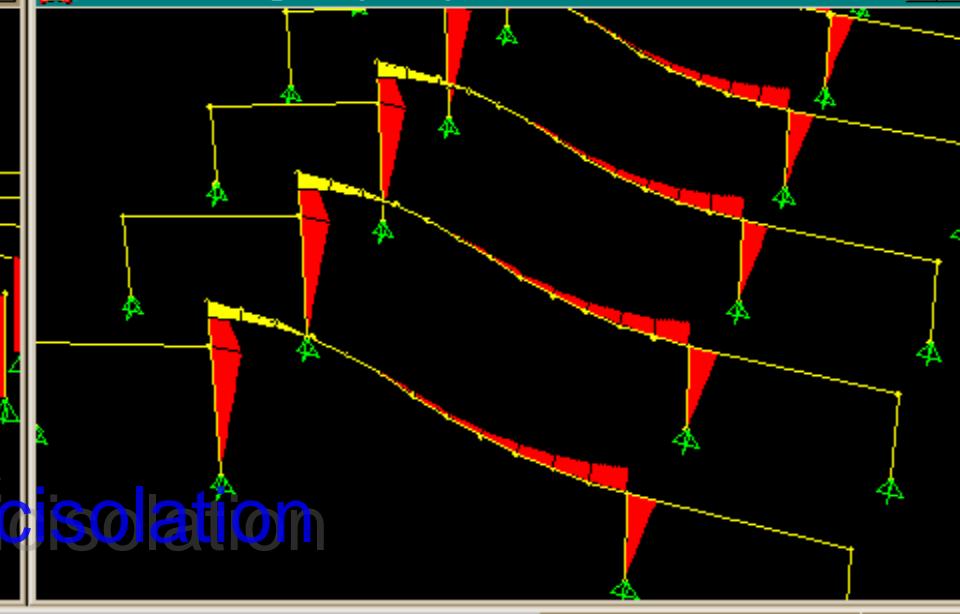
Moment 3-3 Diagram (COMB1)



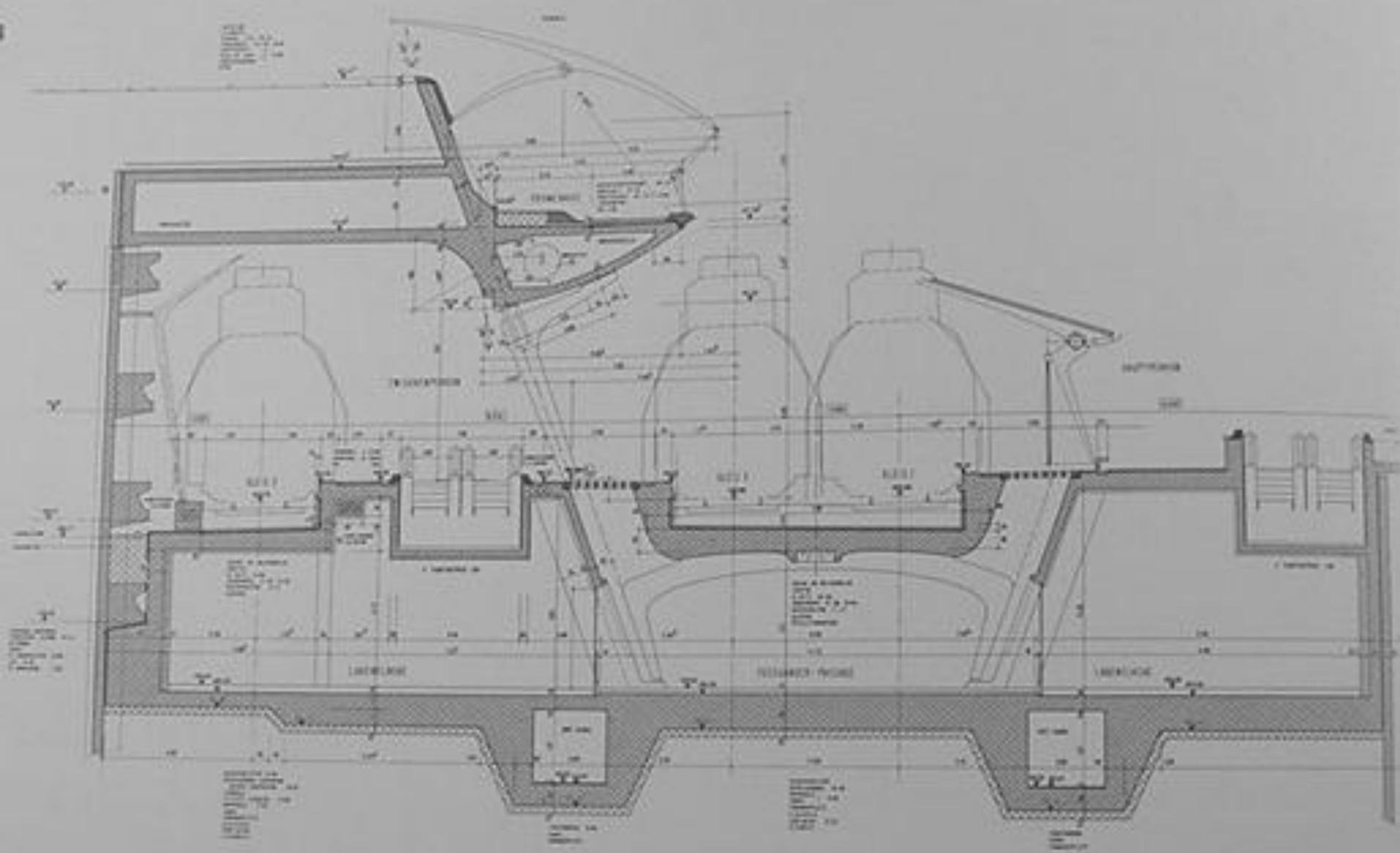
Axial Force Diagram (COMB1)



Moment 3-3 Diagram (LOAD3)

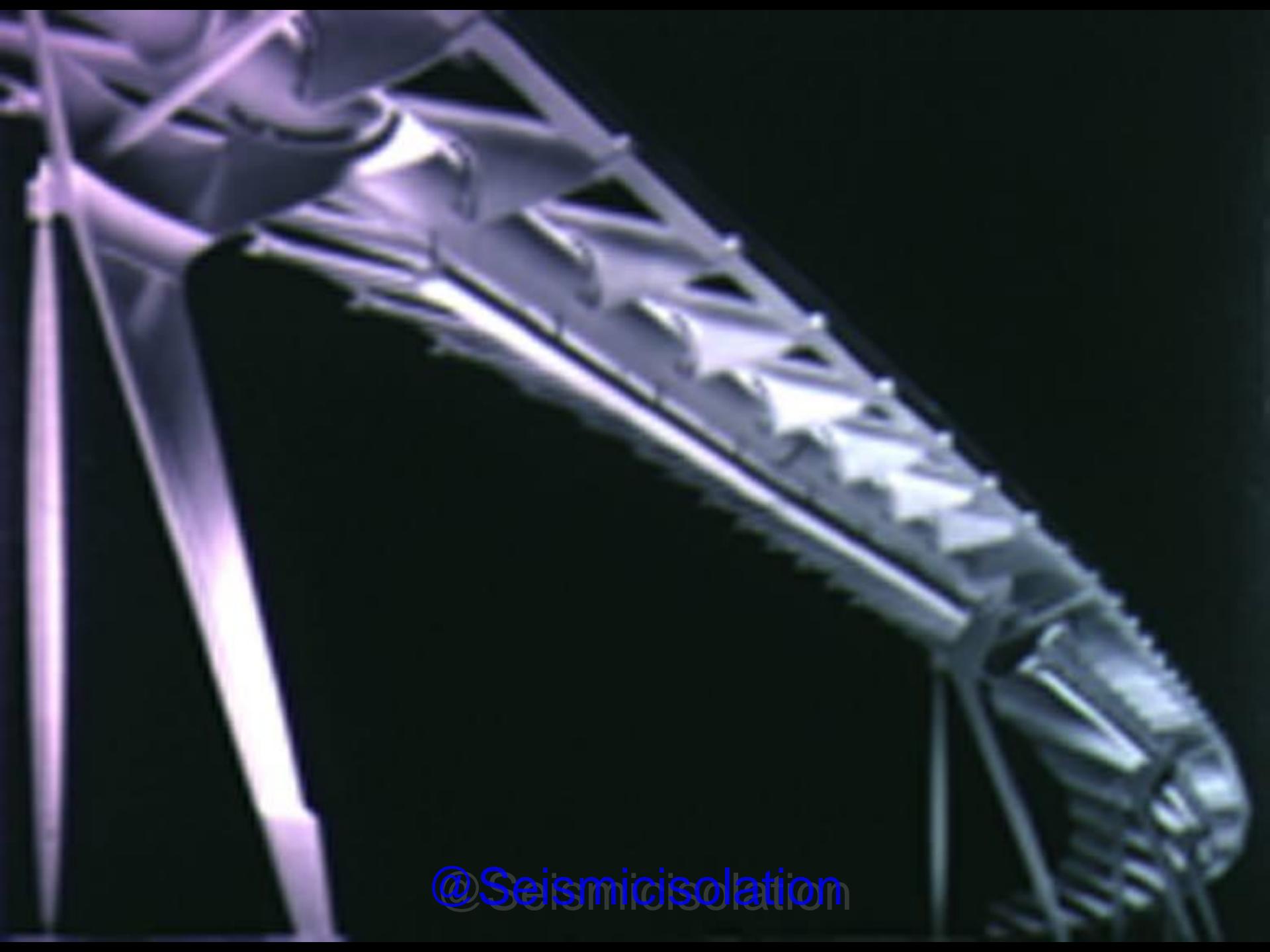


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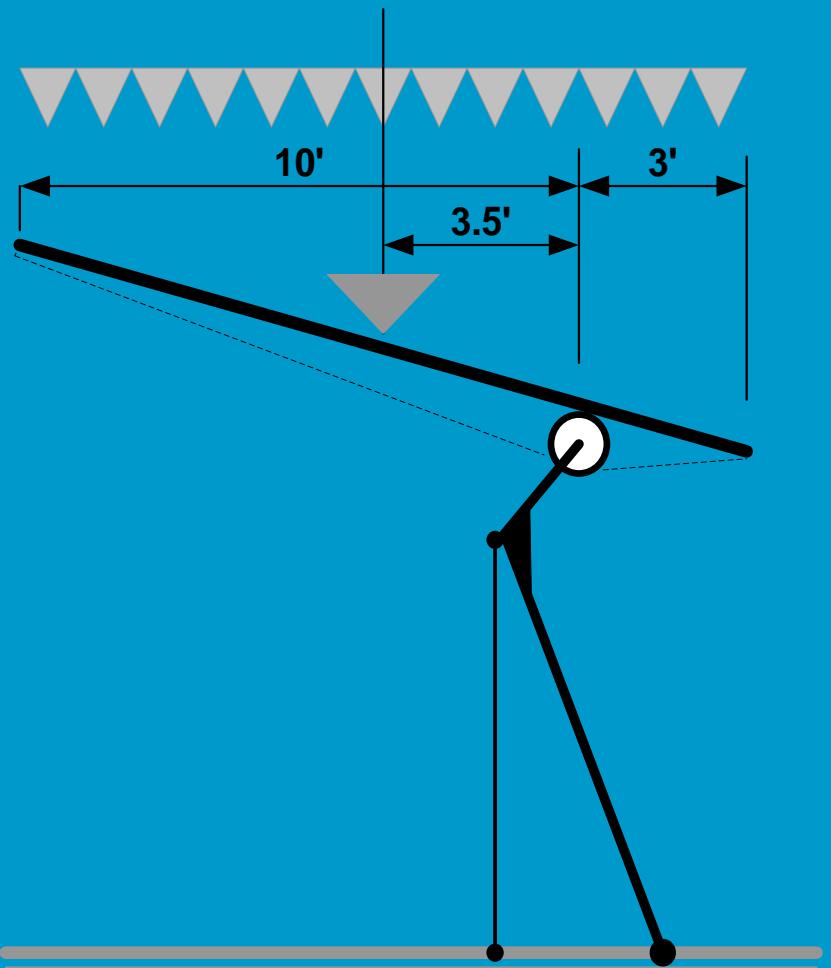
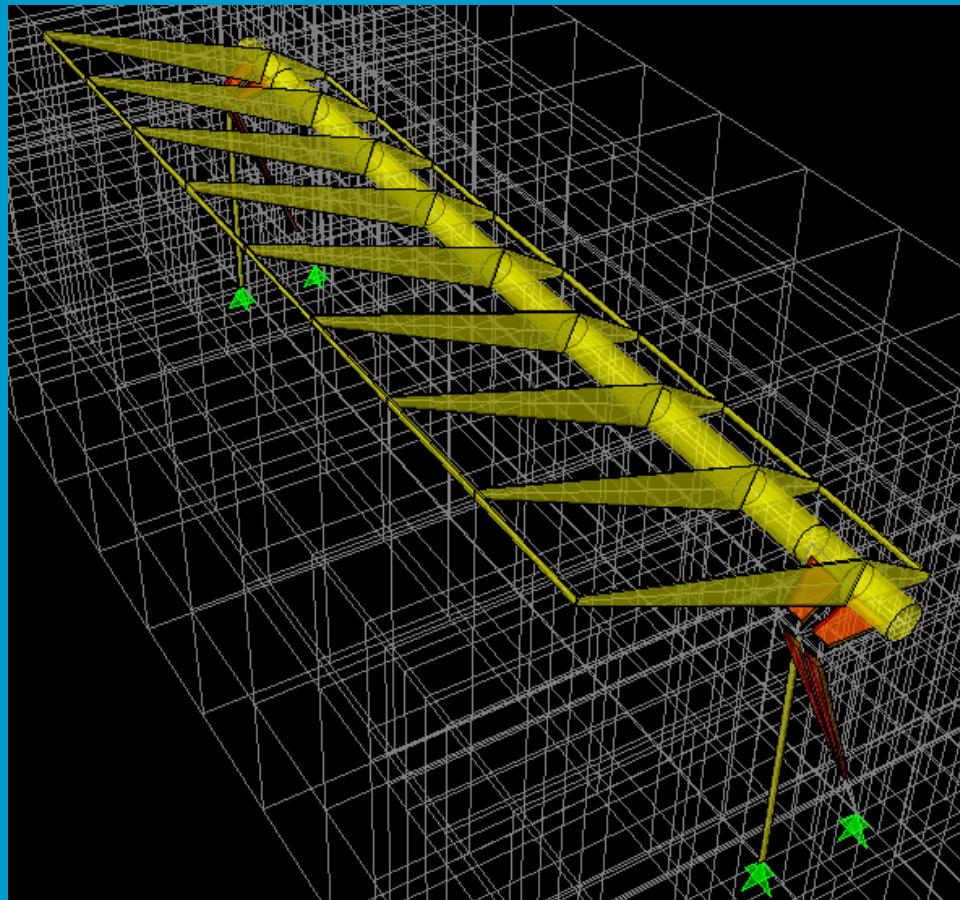


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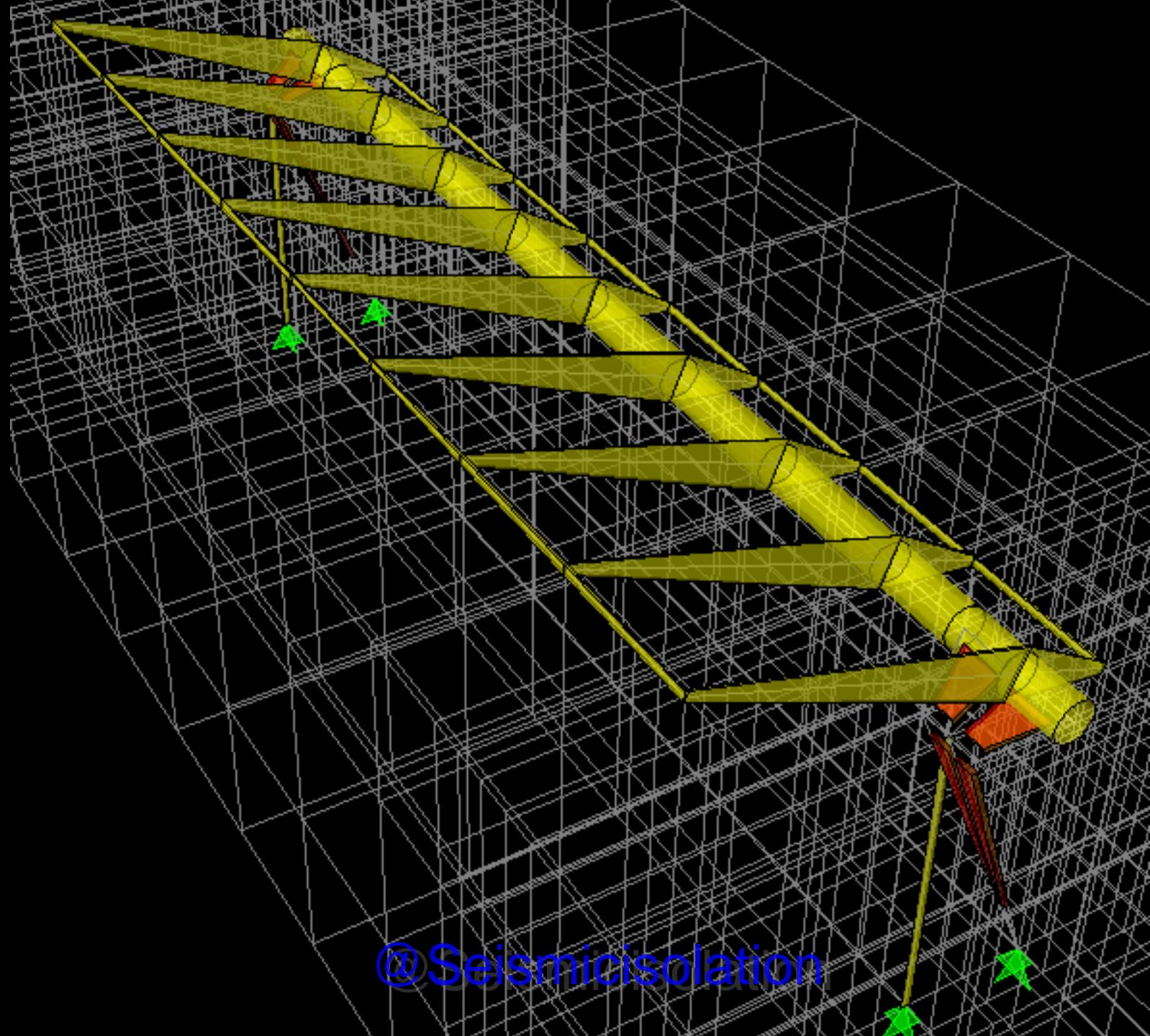
Stadelhofen Railway Station, Zurich, 1990, Santiago Calatrava, cross section

A blurred, high-angle view of a bridge or elevated roadway structure at night. The structure is illuminated by artificial lights, creating streaks of light against a dark background. The perspective is looking down the length of the bridge.

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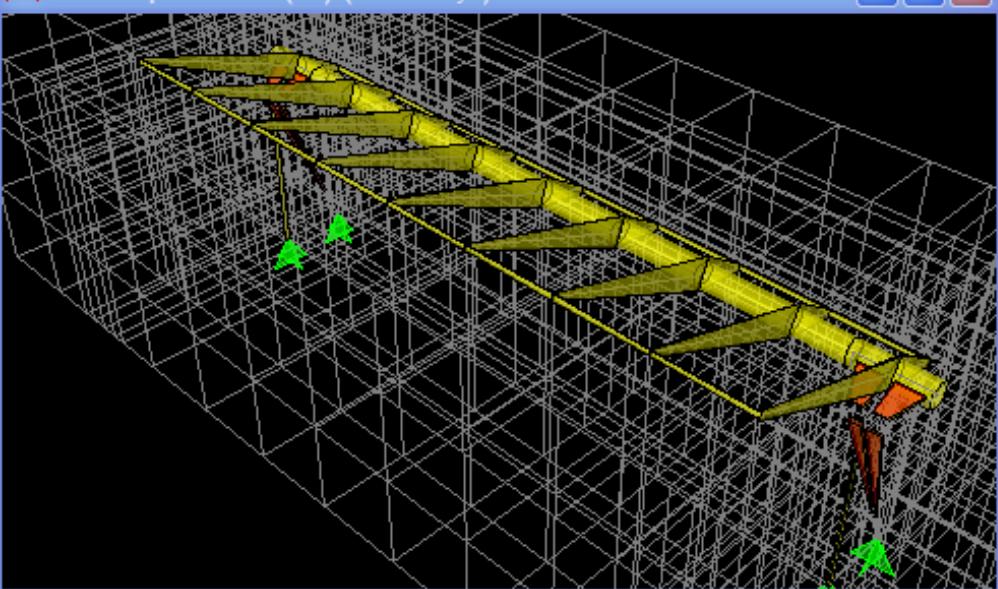


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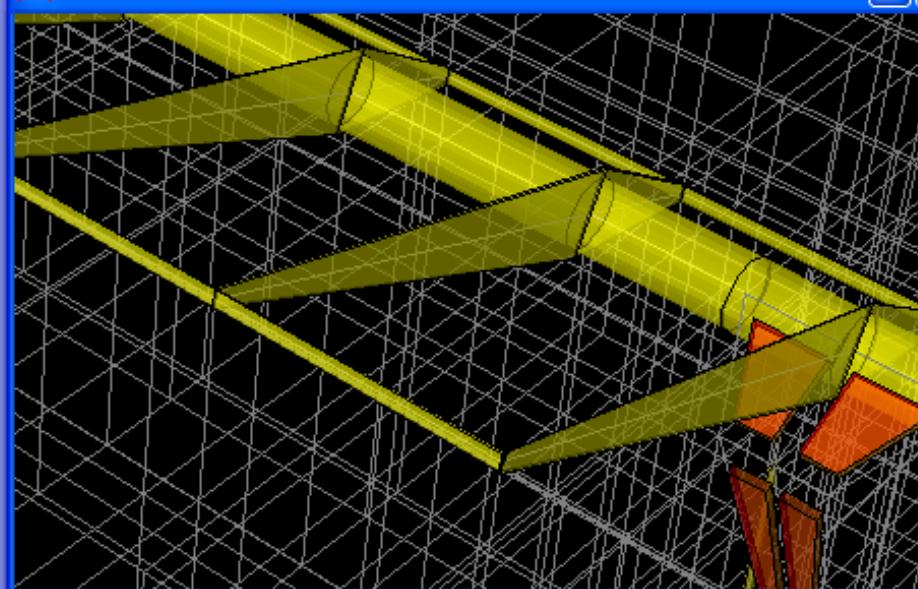
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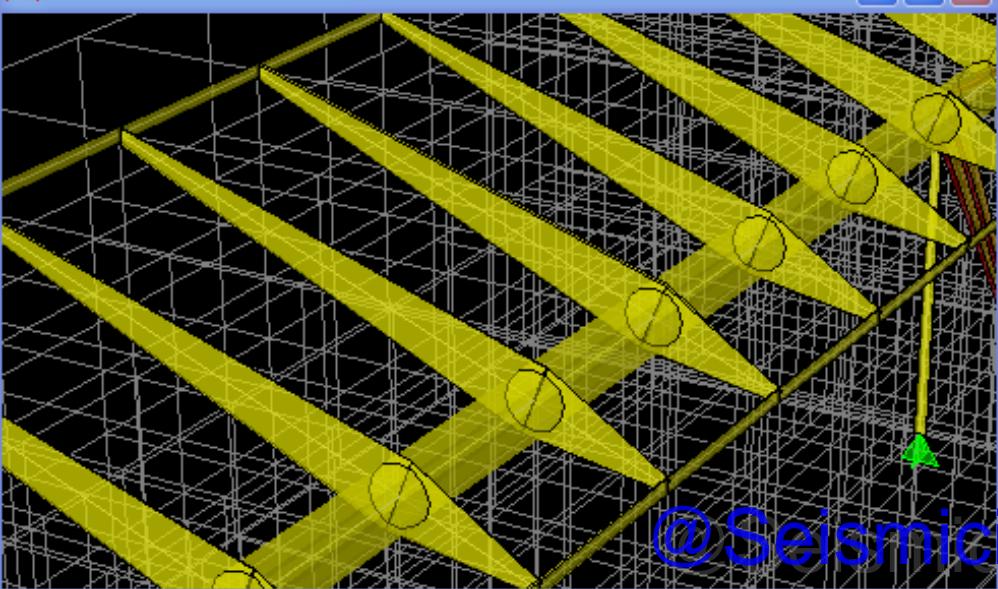
Frame Span Loads (DL) (Local CSys)



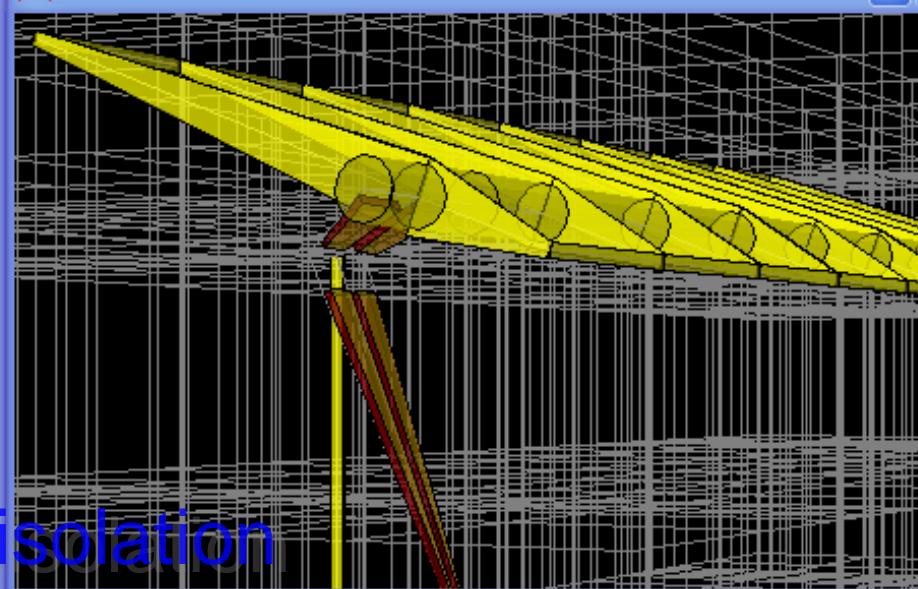
3-D View



3-D View



3-D View

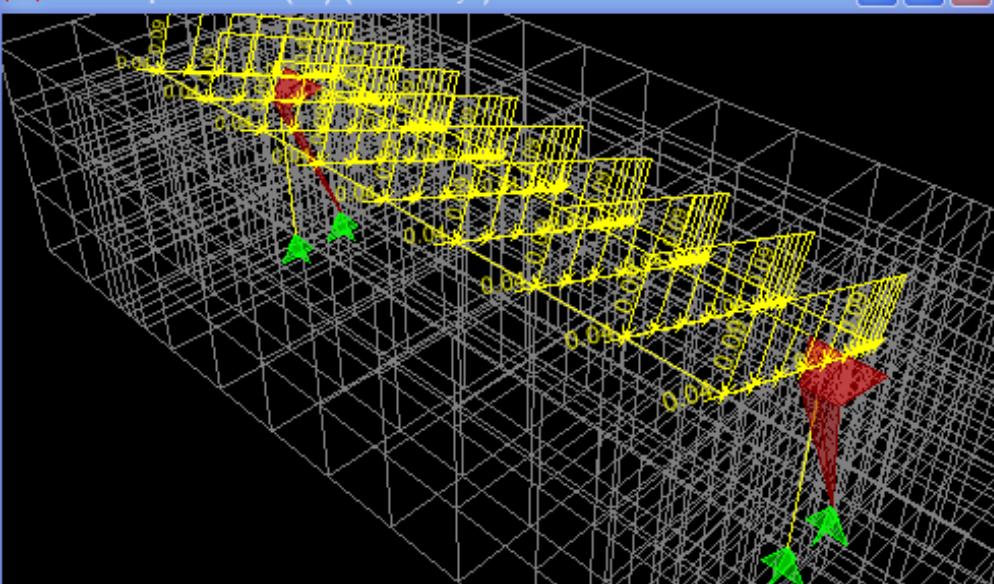


@Seismicisolation

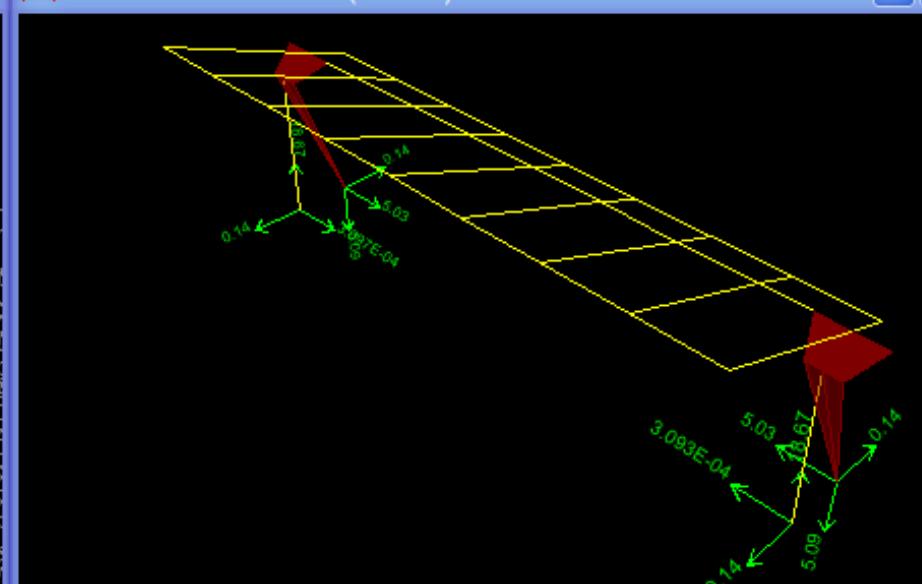
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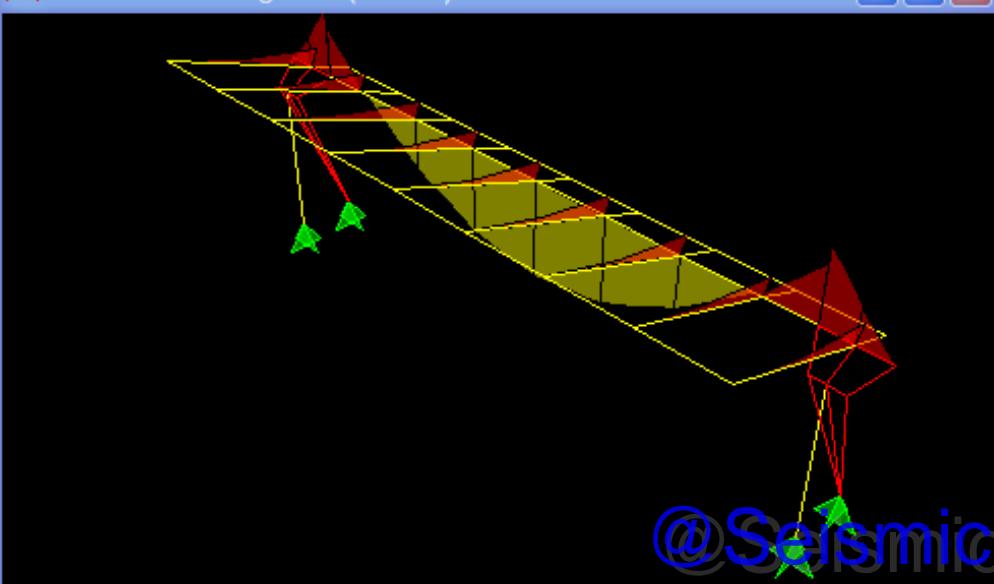
Frame Span Loads (DL) (Local CSys)



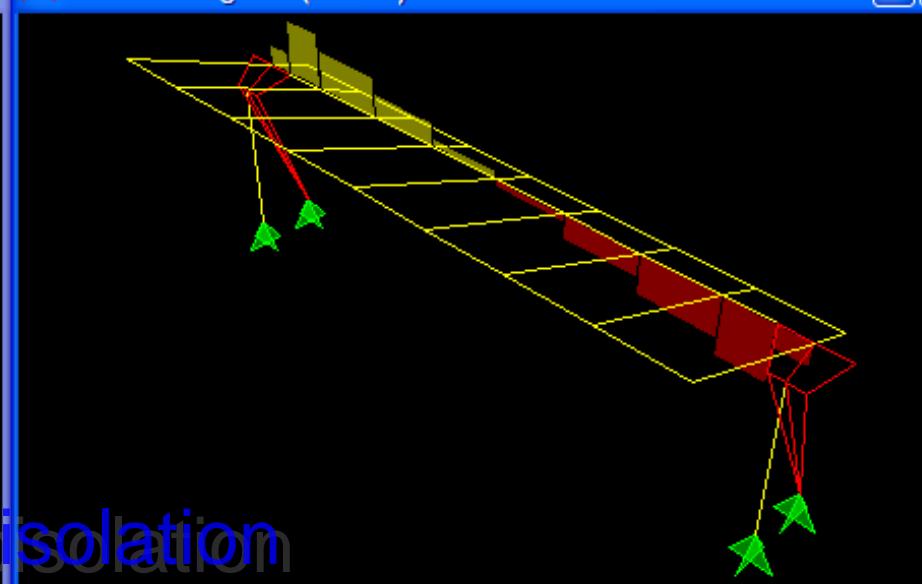
Restraint Reactions (COMB1)



Moment 3-3 Diagram (COMB1)



Torsion Diagram (COMB1)

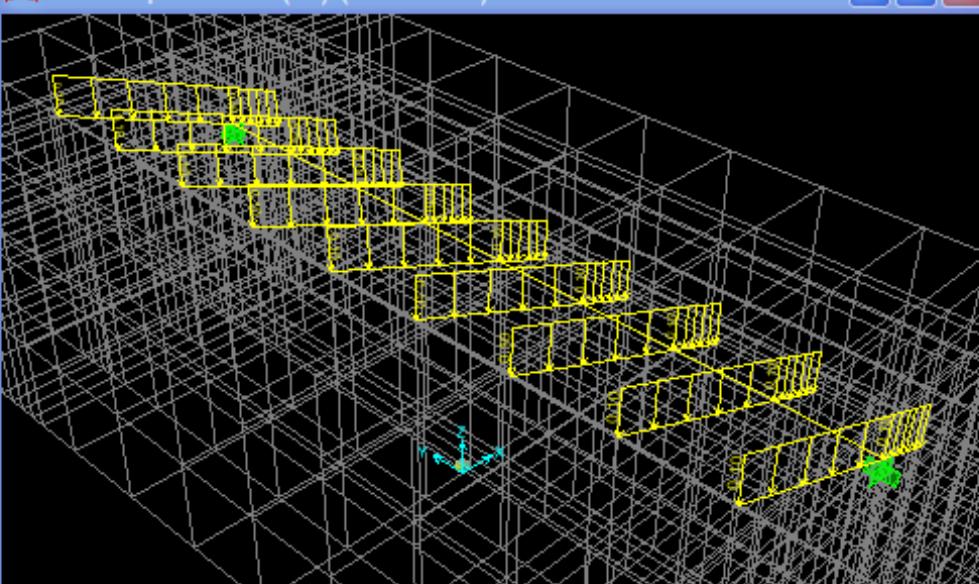


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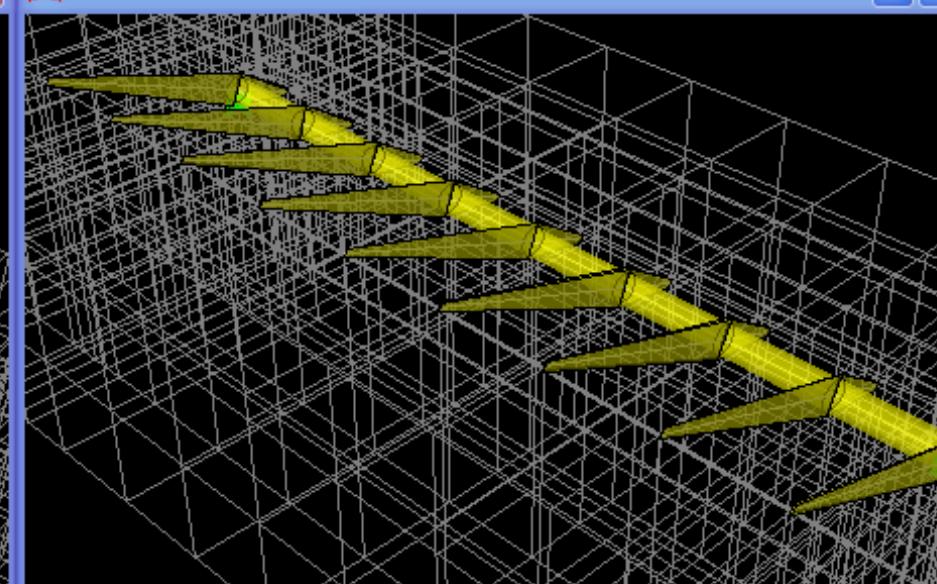
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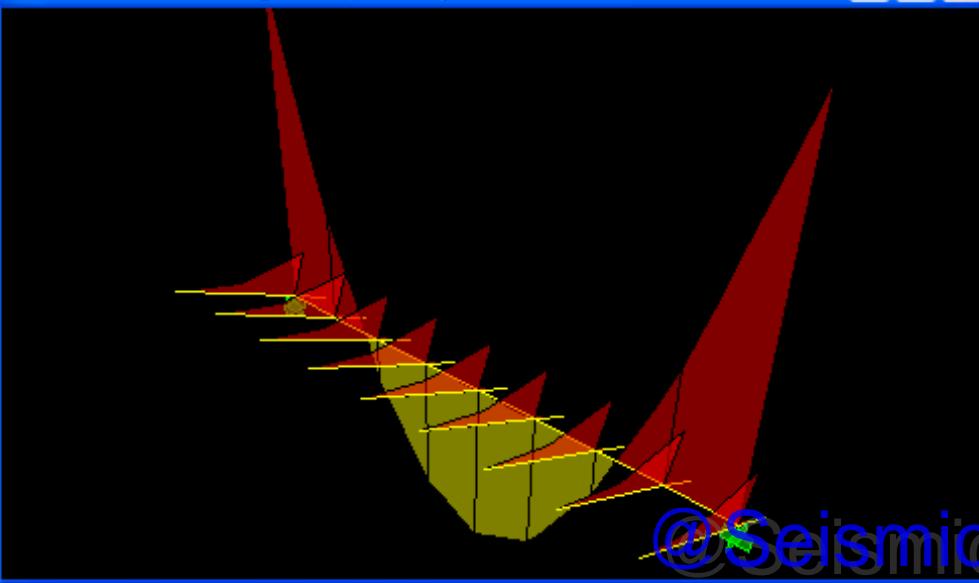
Frame Span Loads (DL) (As Defined)



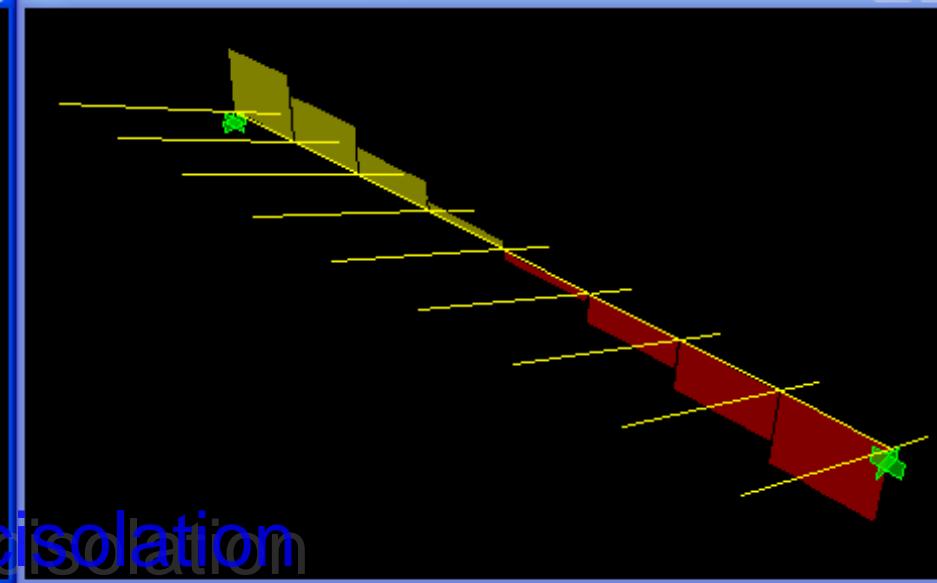
3-D View



Moment 3-3 Diagram (COMB1)



Torsion Diagram (COMB1)



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Click on any Frame Element for detailed diagram

GLOBAL Kip, ft, F



Dresdner Bank, Verwaltungszentrum, Leipzig, 1997, Engel und Zimmermann Arch  
**@Seismicisolation**



Theater Erfurt, 2003, Germany, Joerg Friedrich Arch, glass house



Ningbo downtown, 2002,  
Qingyun Ma

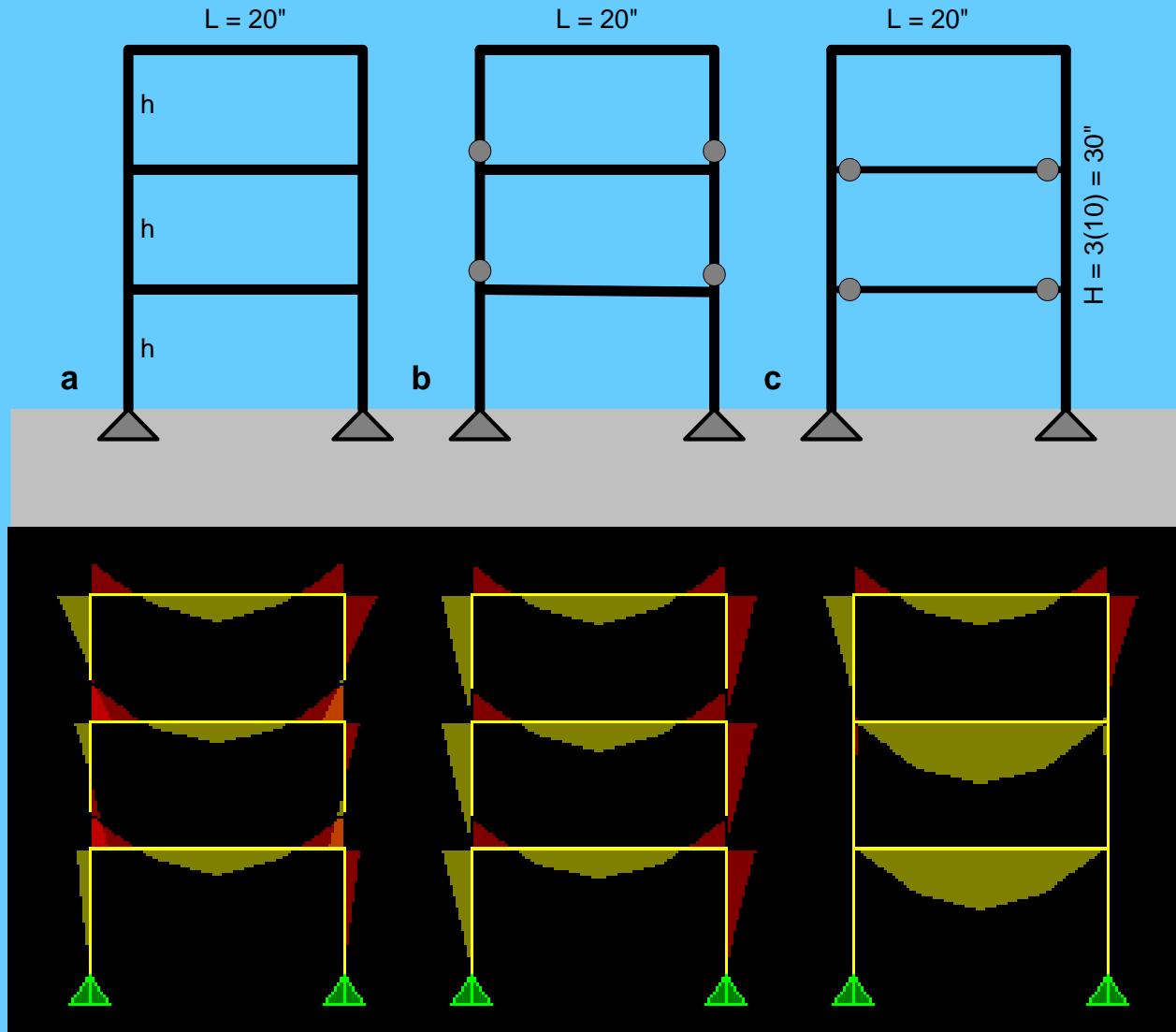
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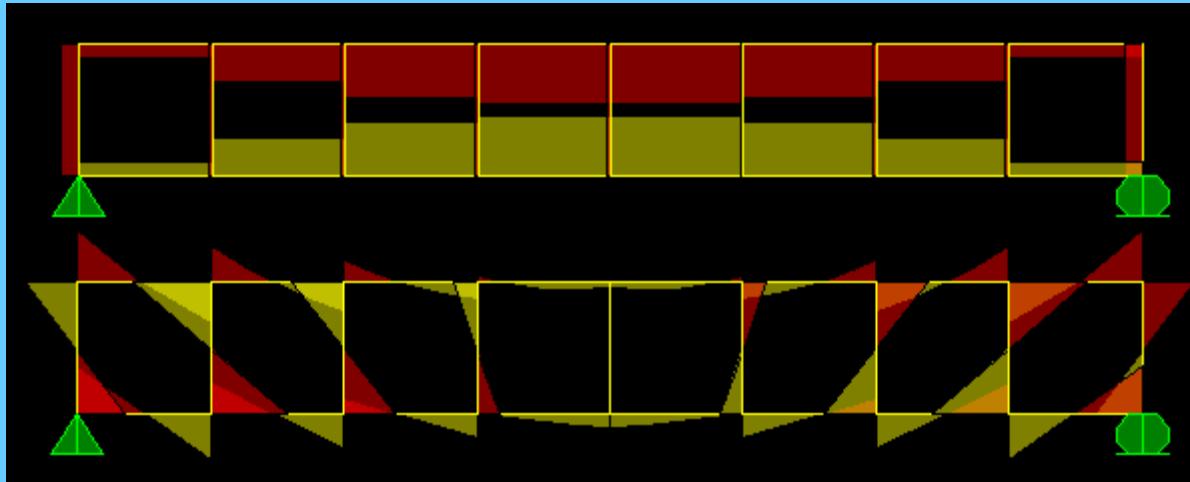
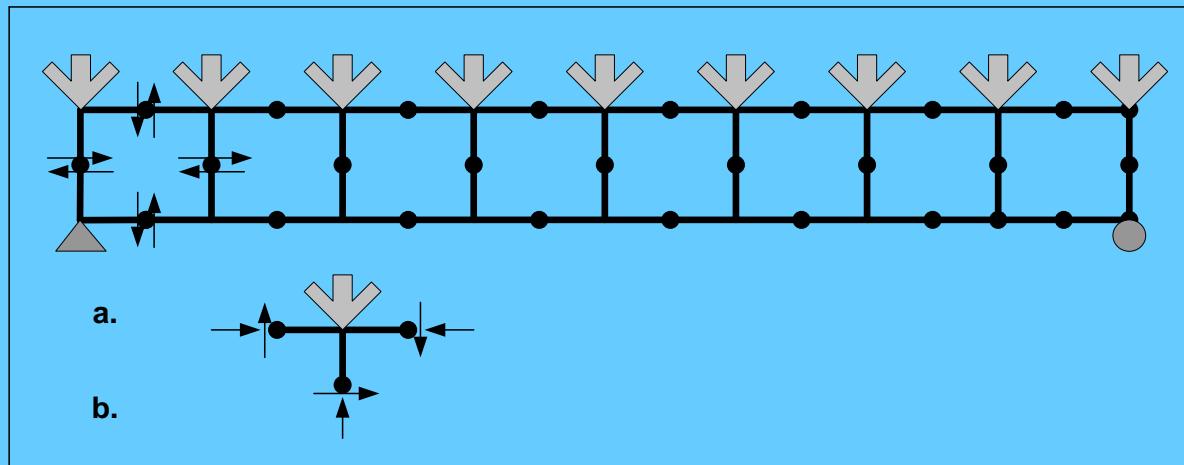
Dokumenta Urbana, Kassel, 1984, Otto Seidle  
@Seismicisolation



@Seismicisolation Residential complex, Berlin, Herman Herzberger, 1997



@Seismicisolation



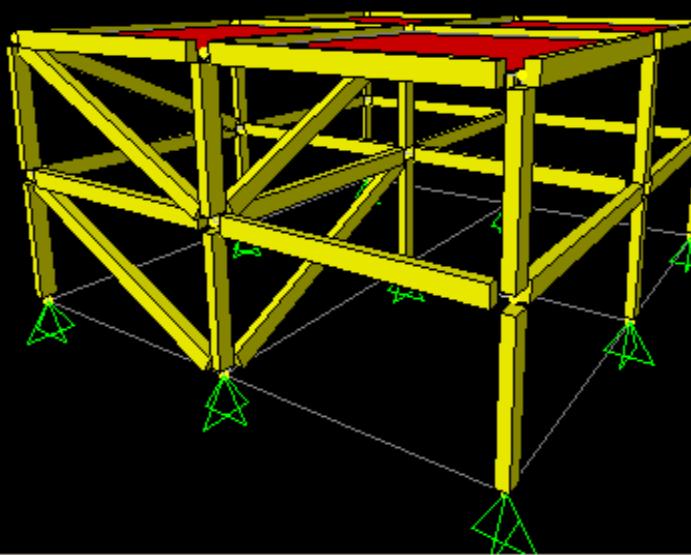
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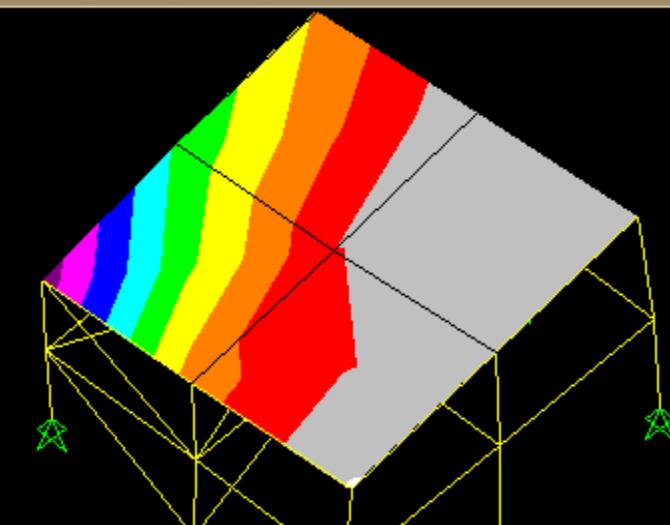


# LATERAL STABILITY OF BUILDINGS

3-D View

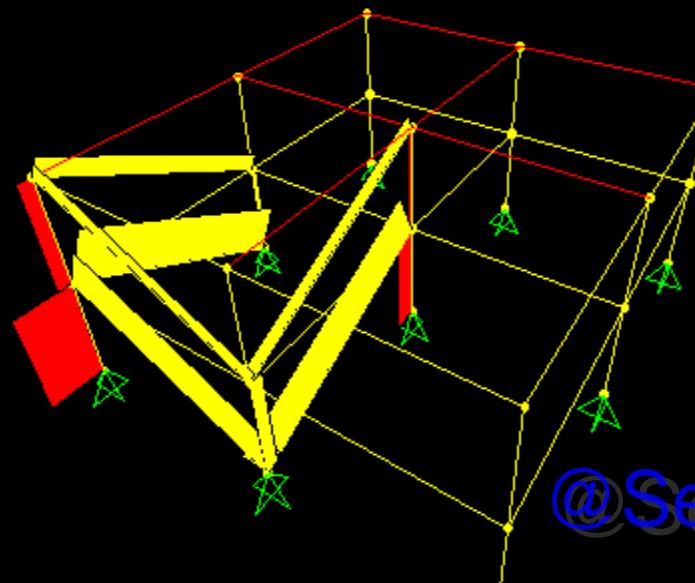


Stress S11 Diagram (COMB1)

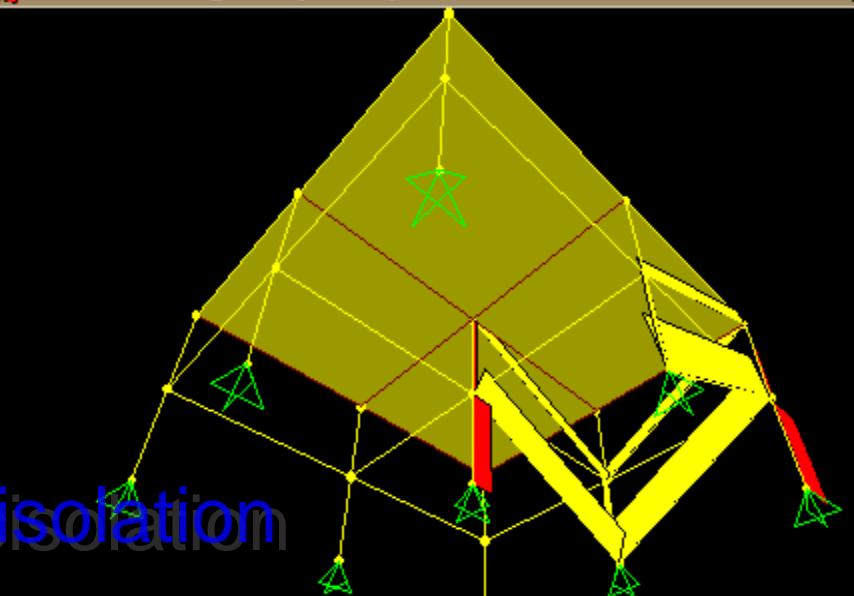


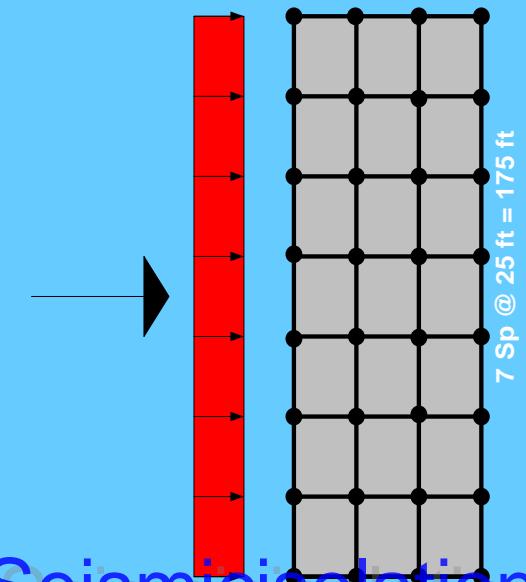
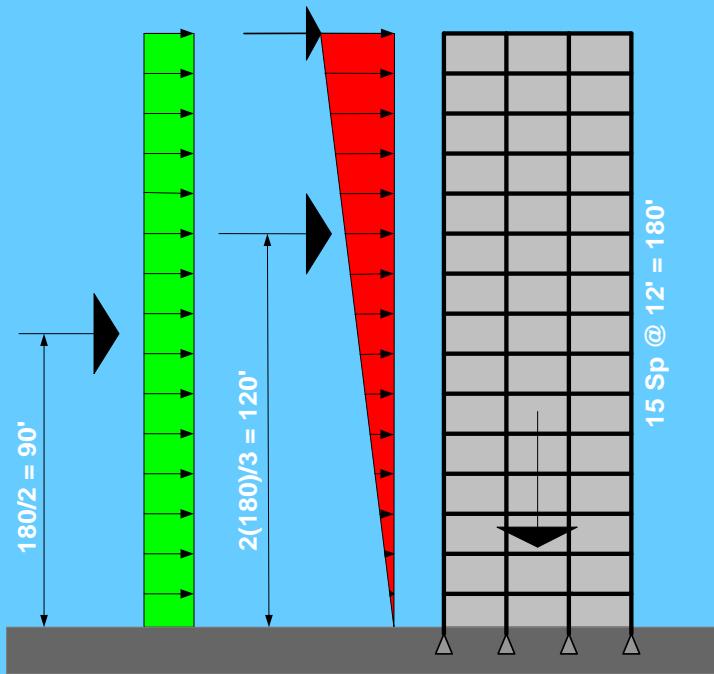
Axial Force Diagram (COMB1)

Axial Force Diagram (COMB1)

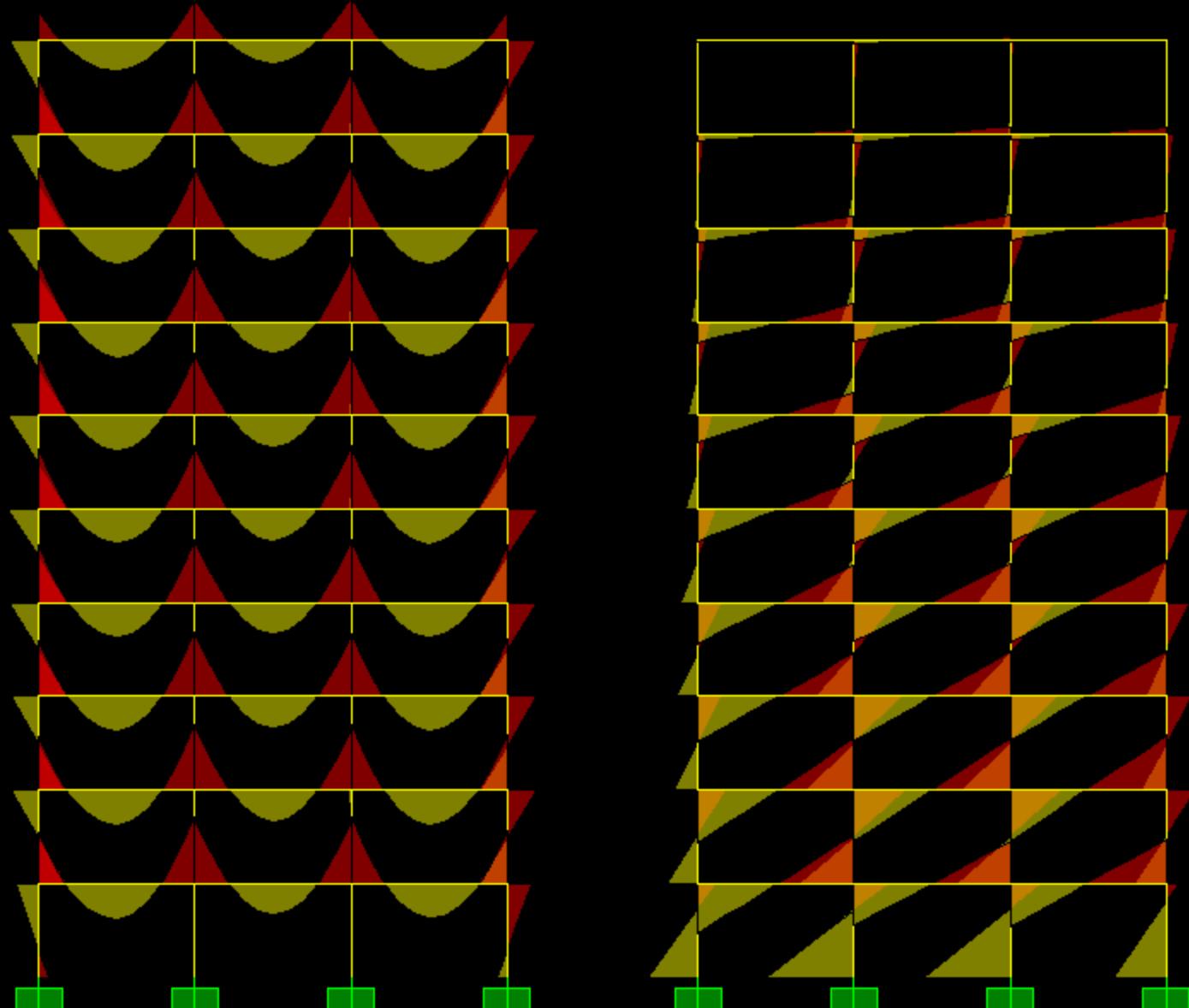


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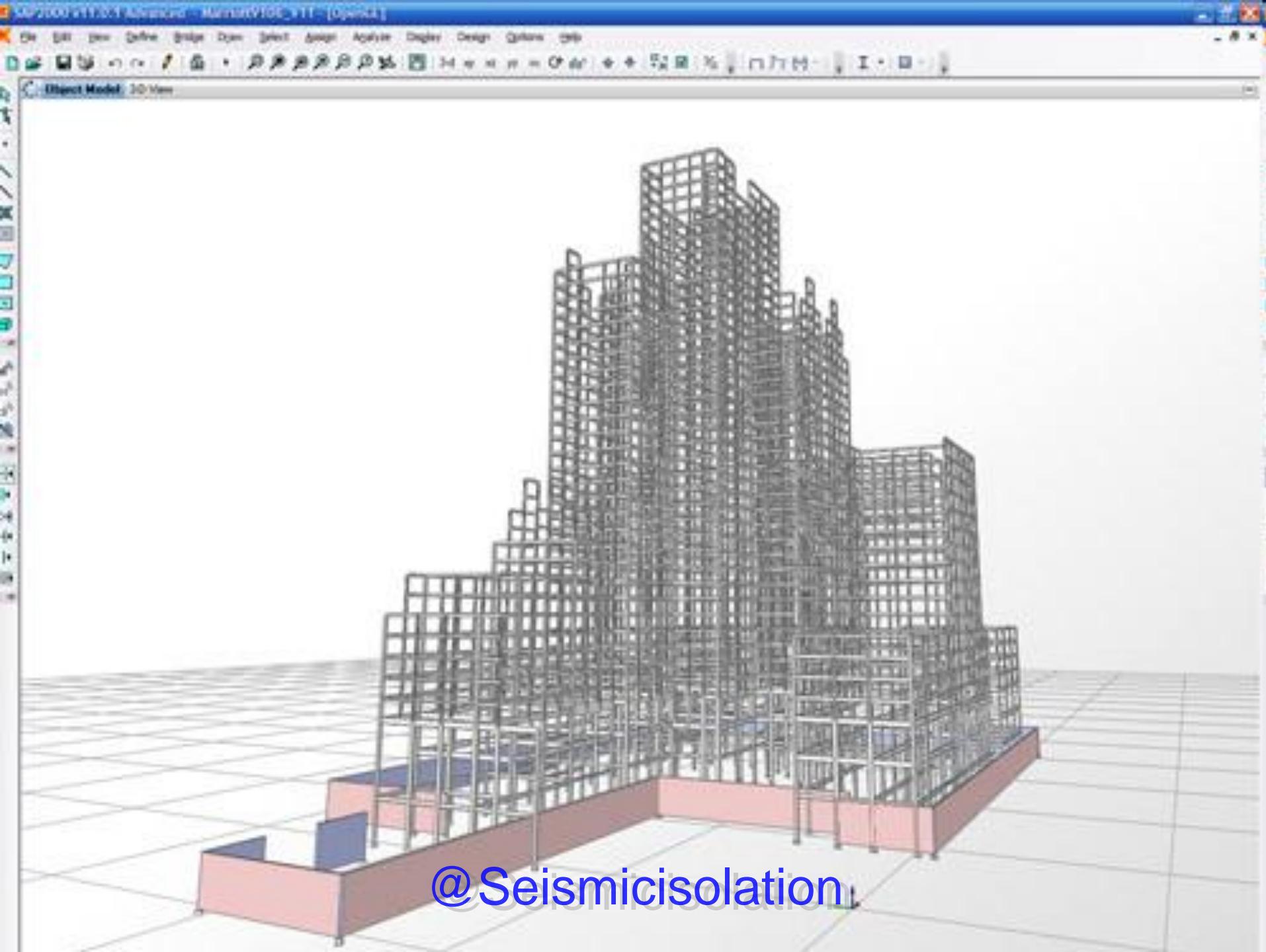


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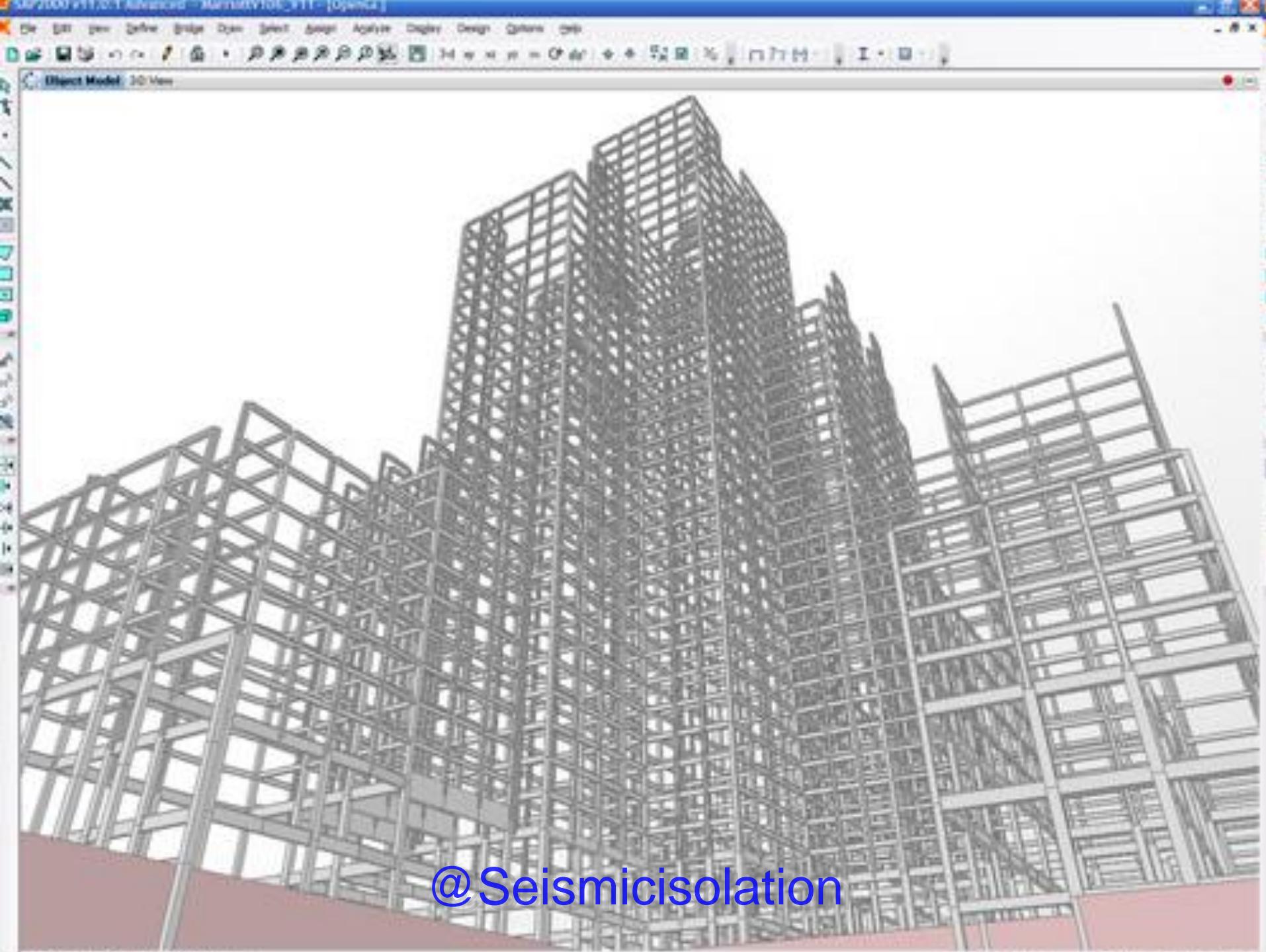


Beijing Jian Wai SOHO, Beijing, 2004, Riken Yamamoto  
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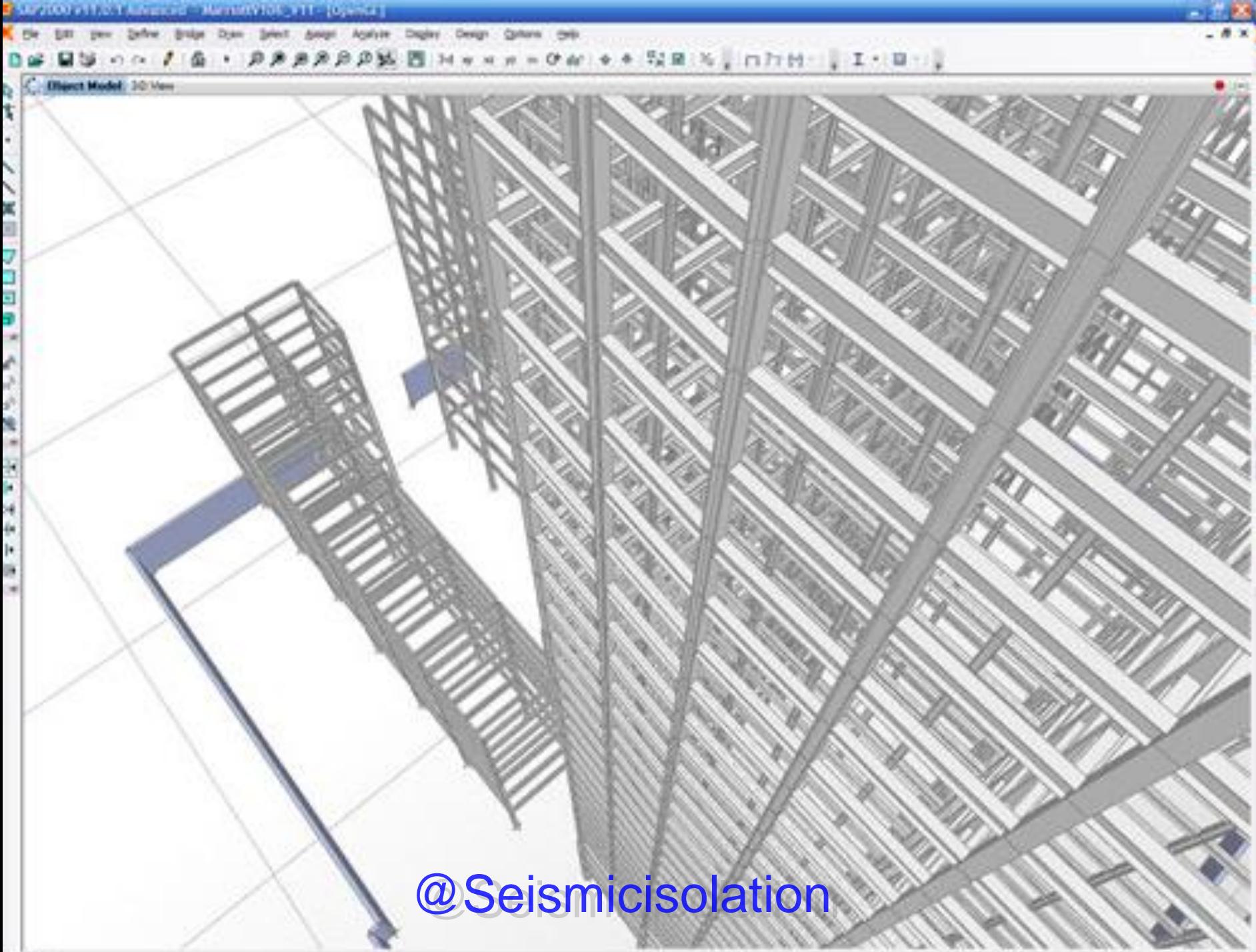




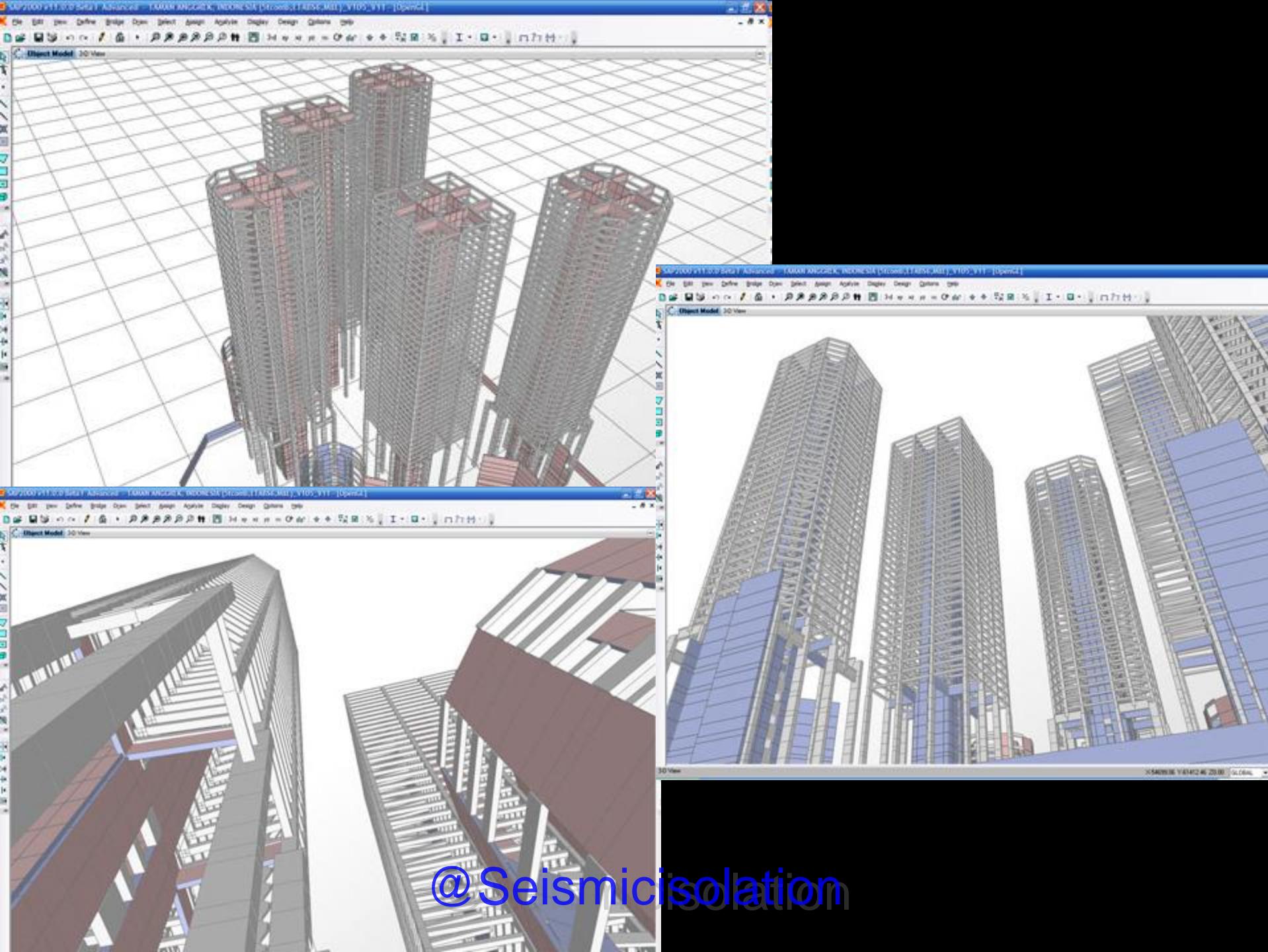
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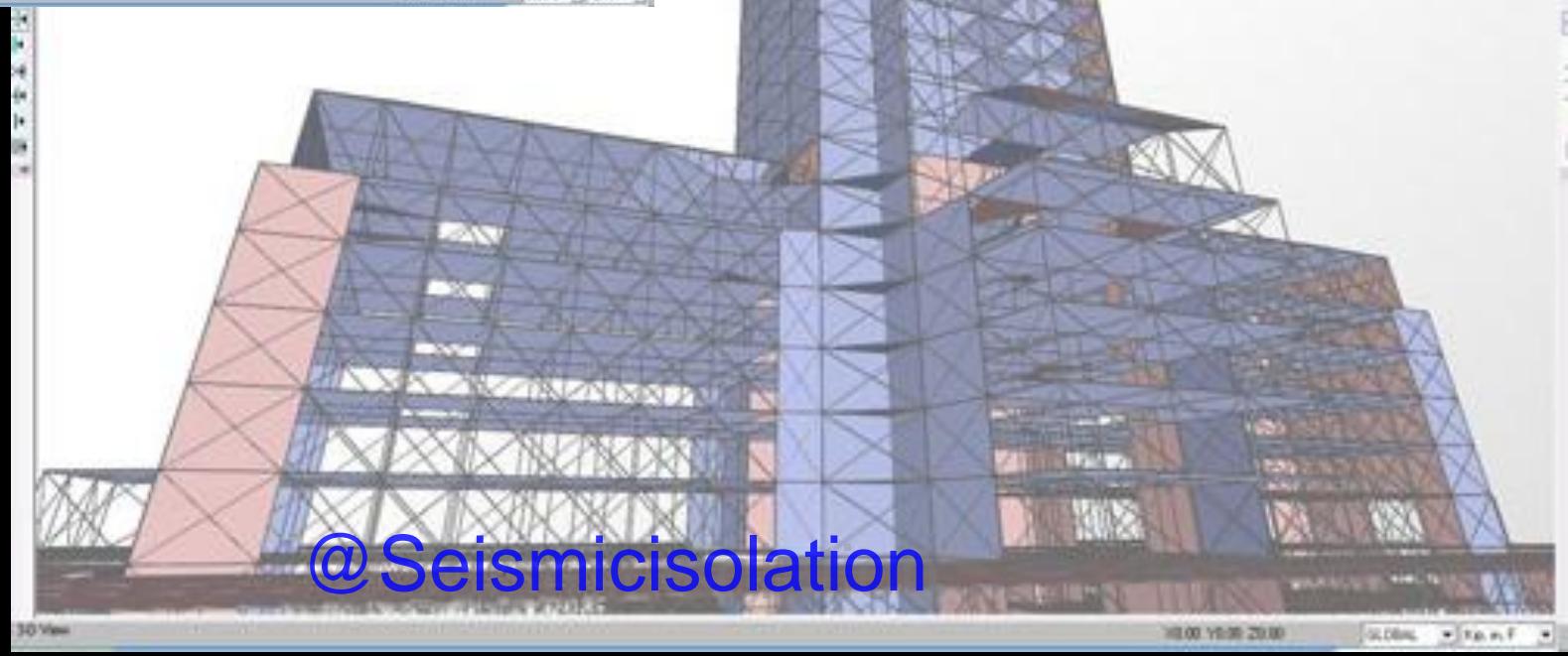
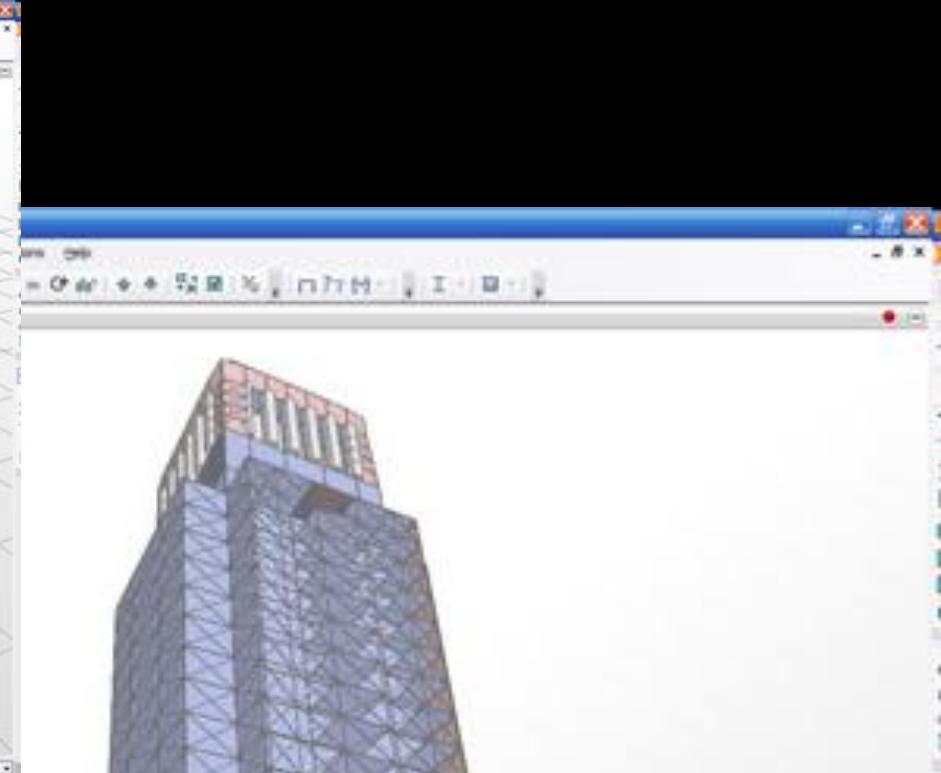
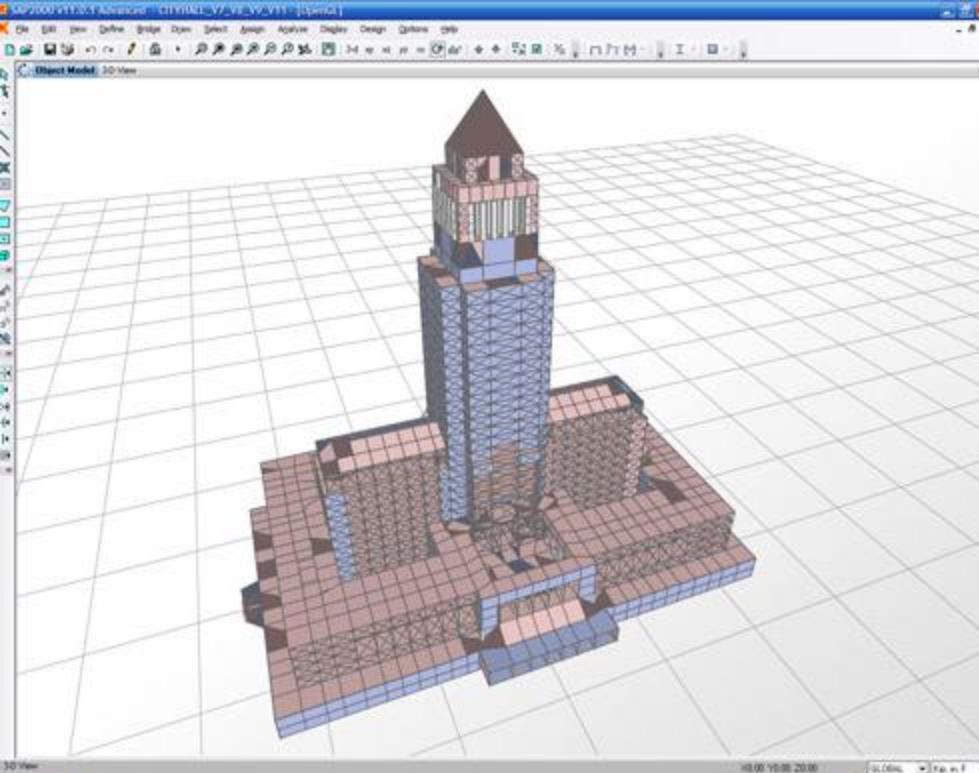
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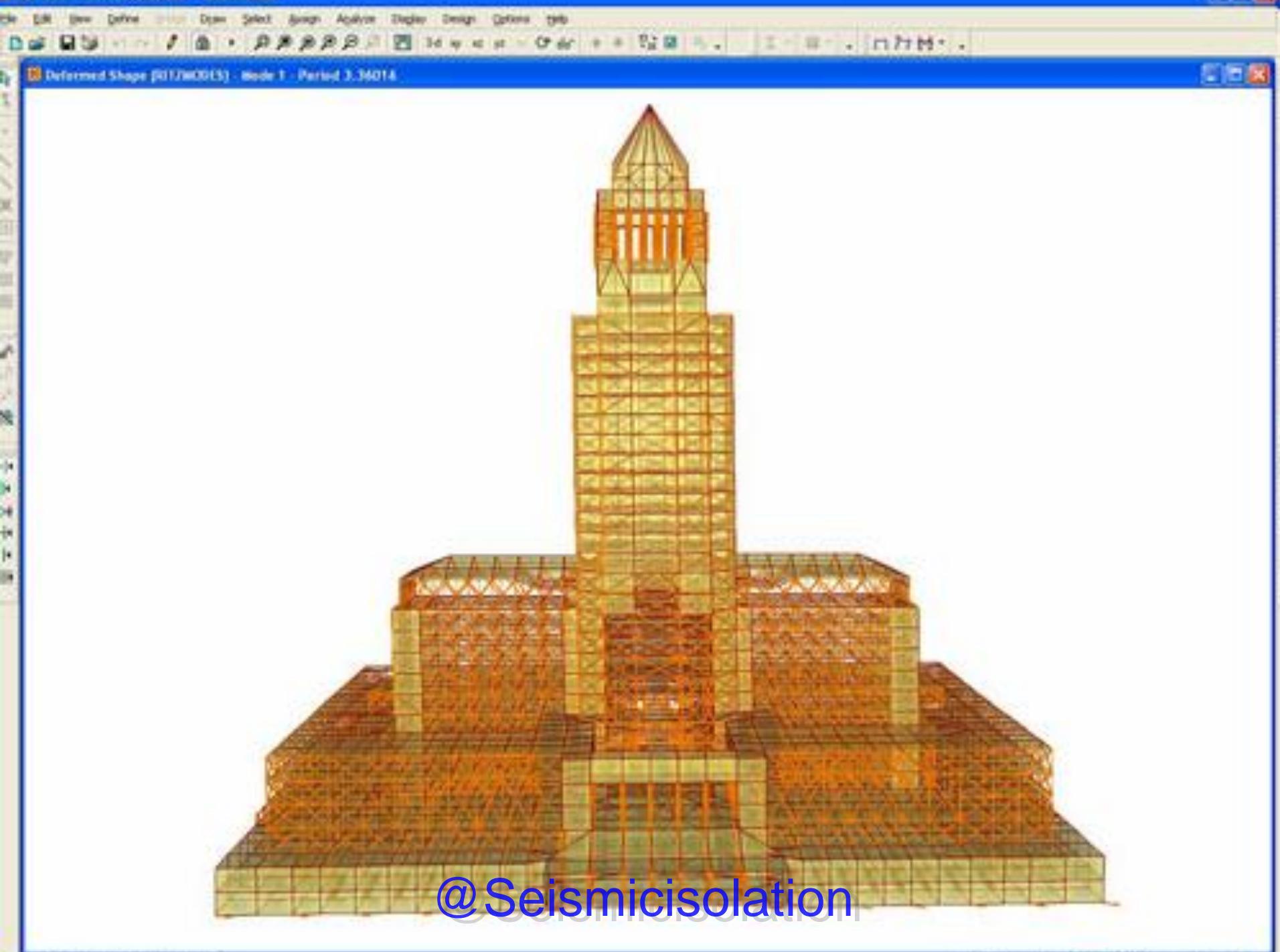
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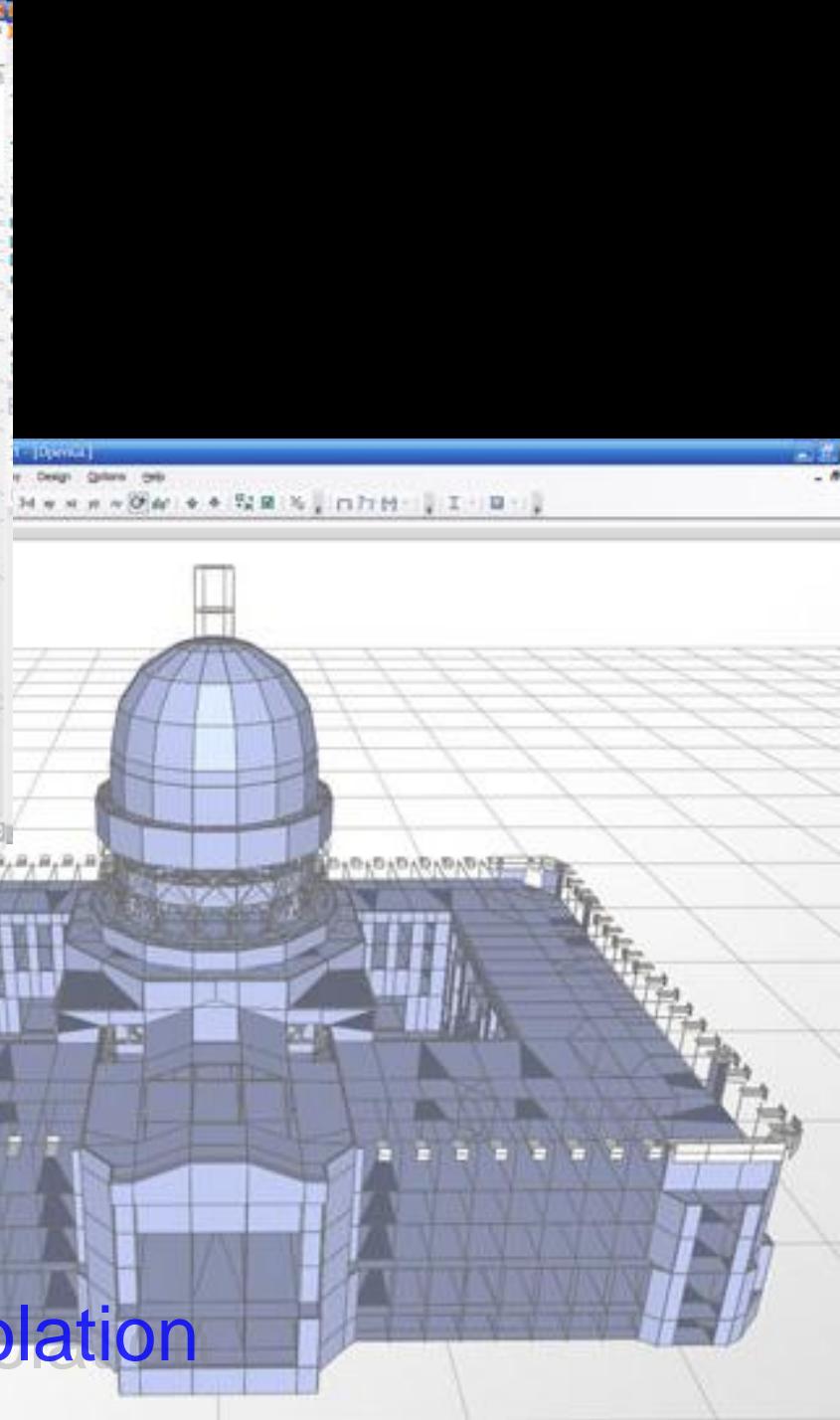
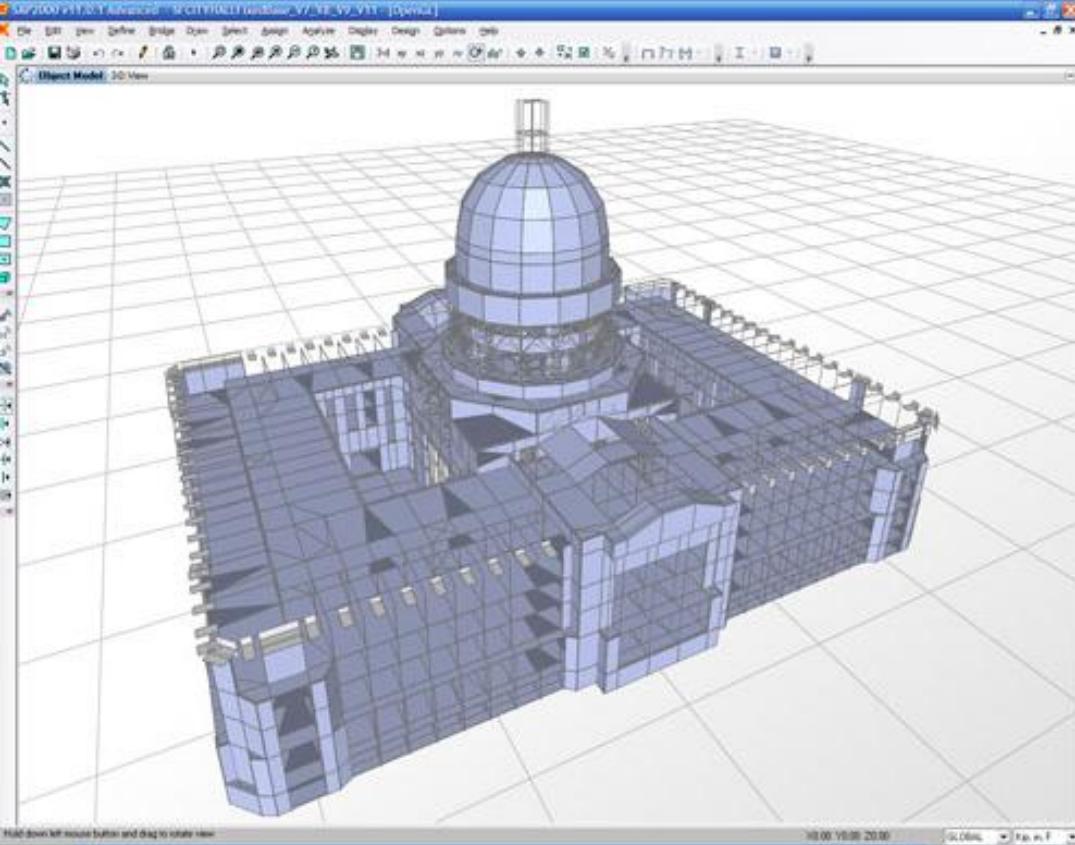
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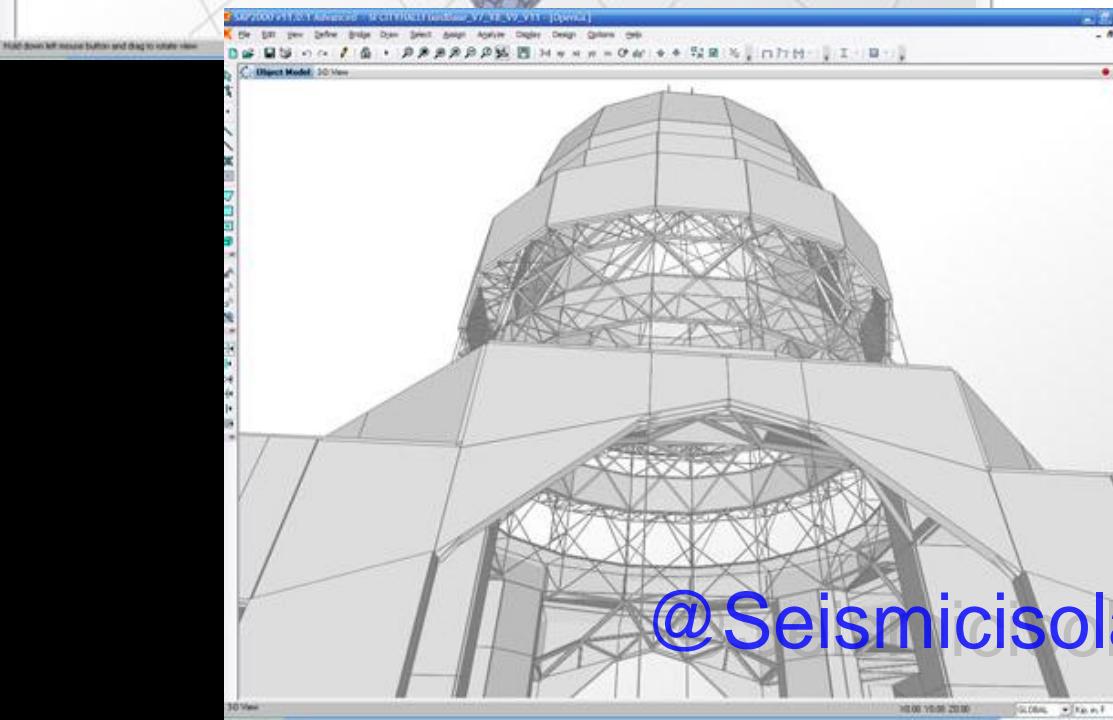
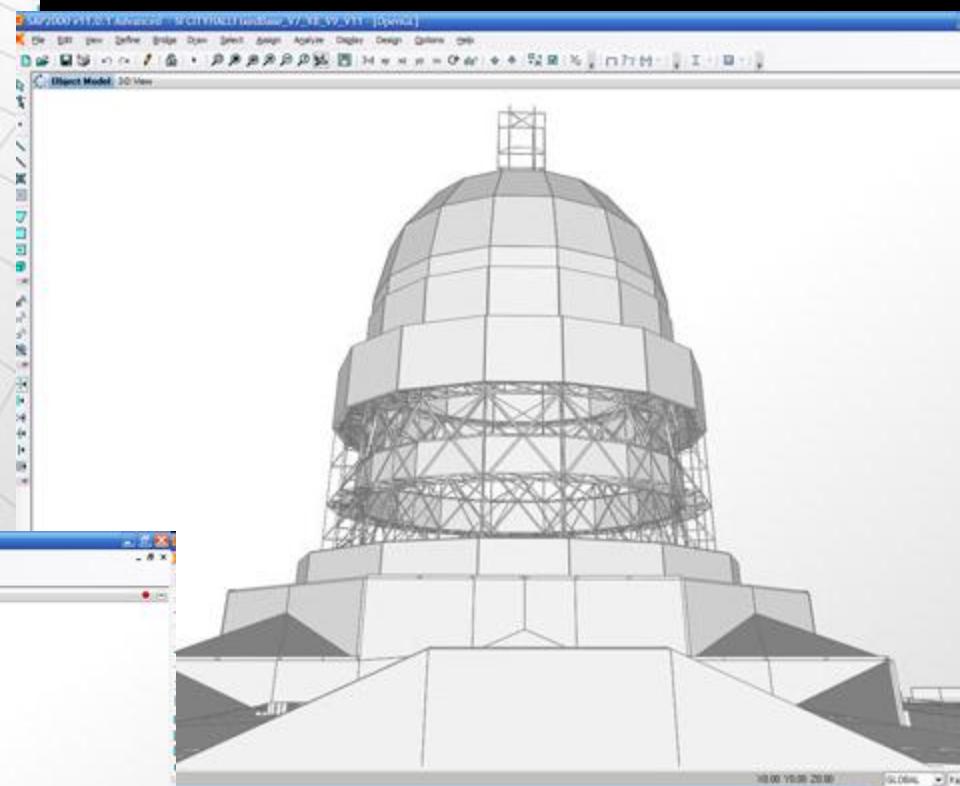
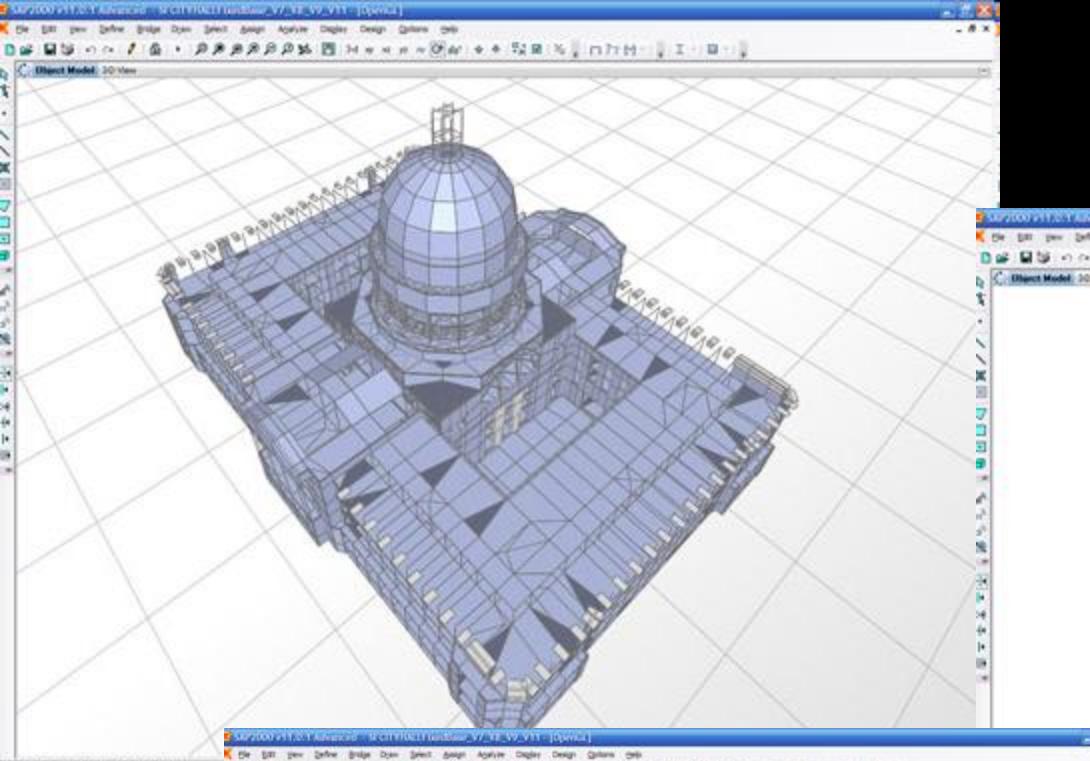
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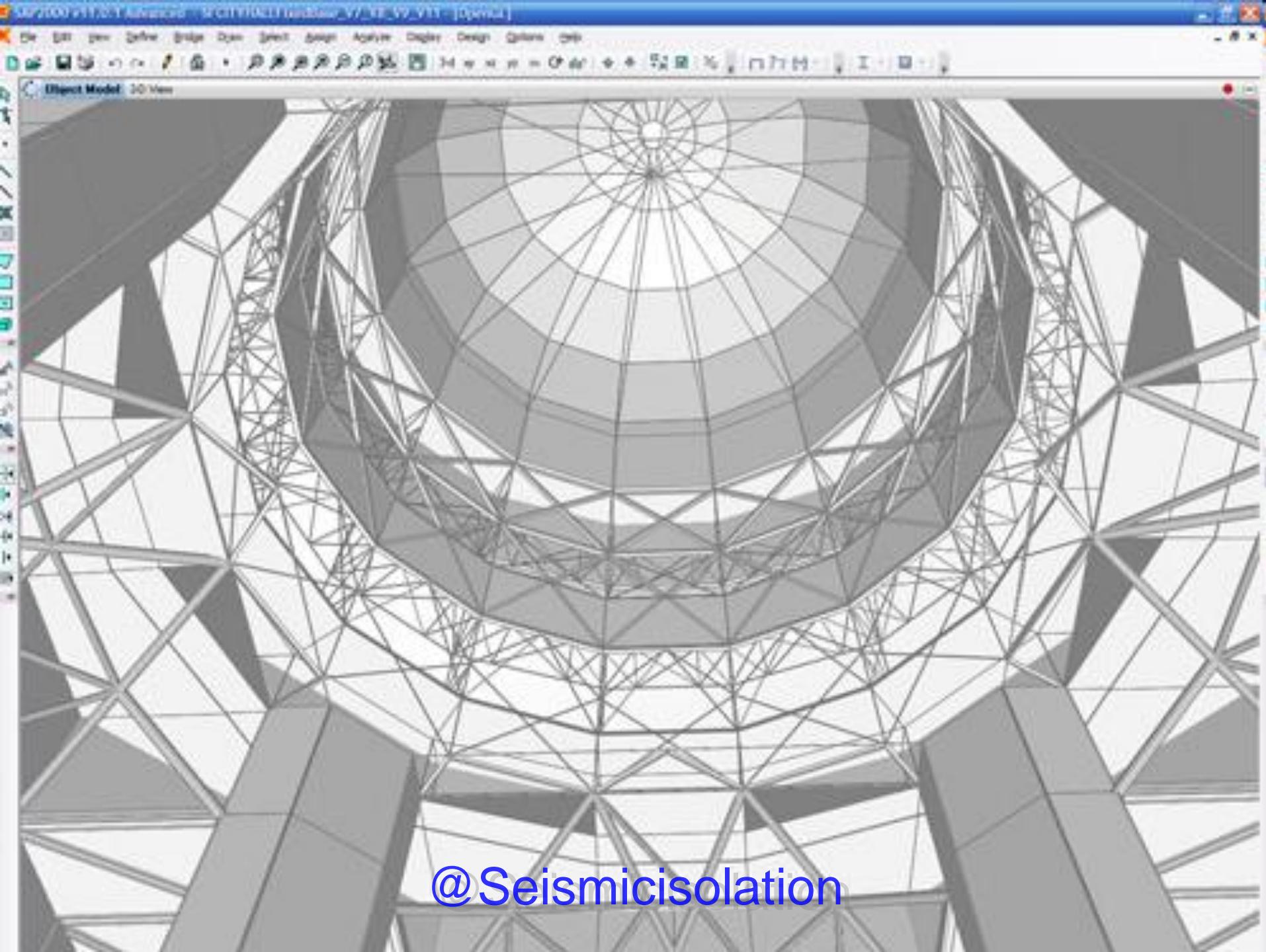
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@Seismicisolation



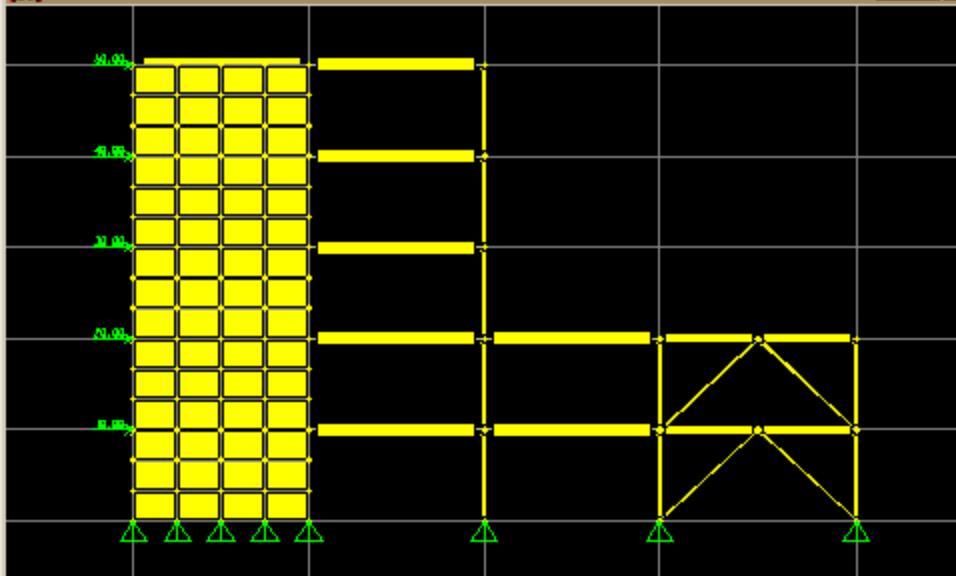
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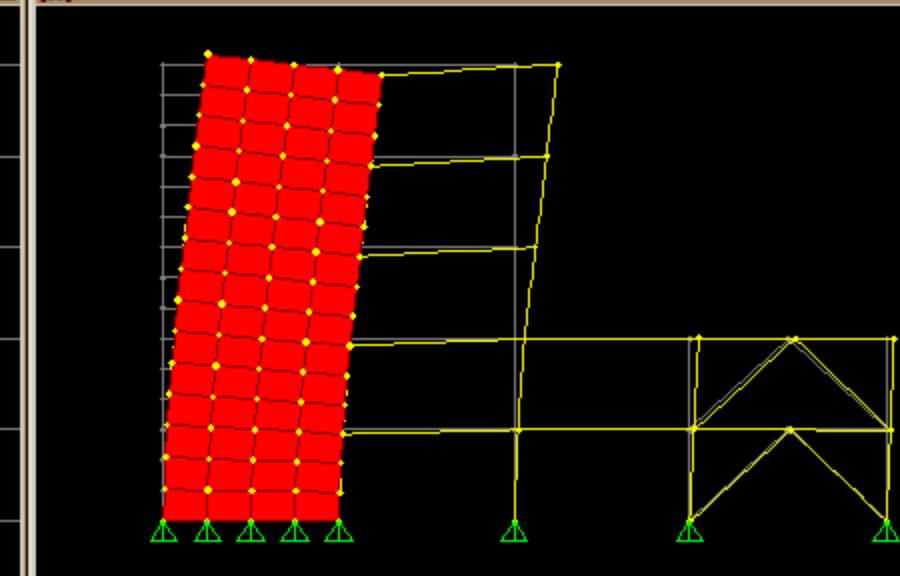
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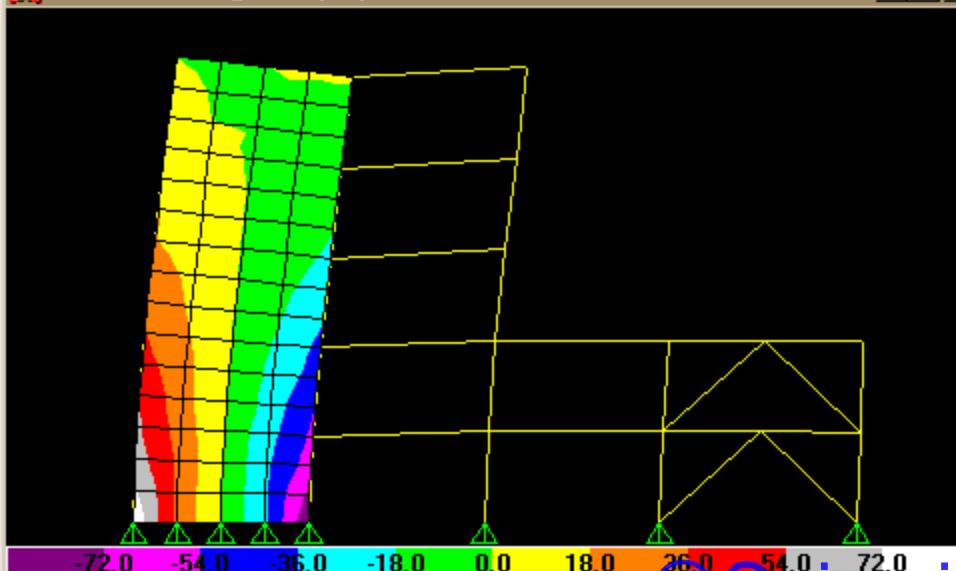
Joint Loads (EQ)



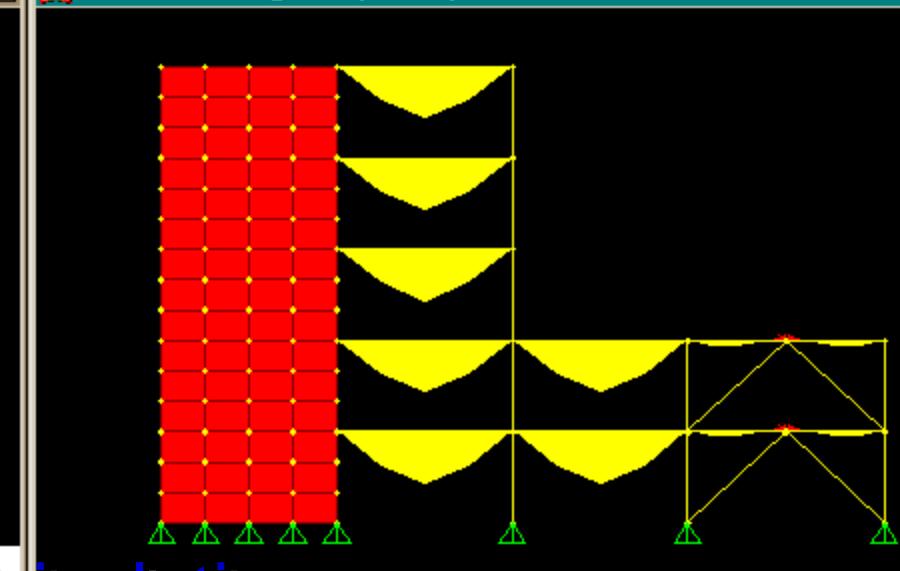
Deformed Shape (EQ)



Stress S22 Diagram (EQ)



Moment 3-3 Diagram (COMB1)

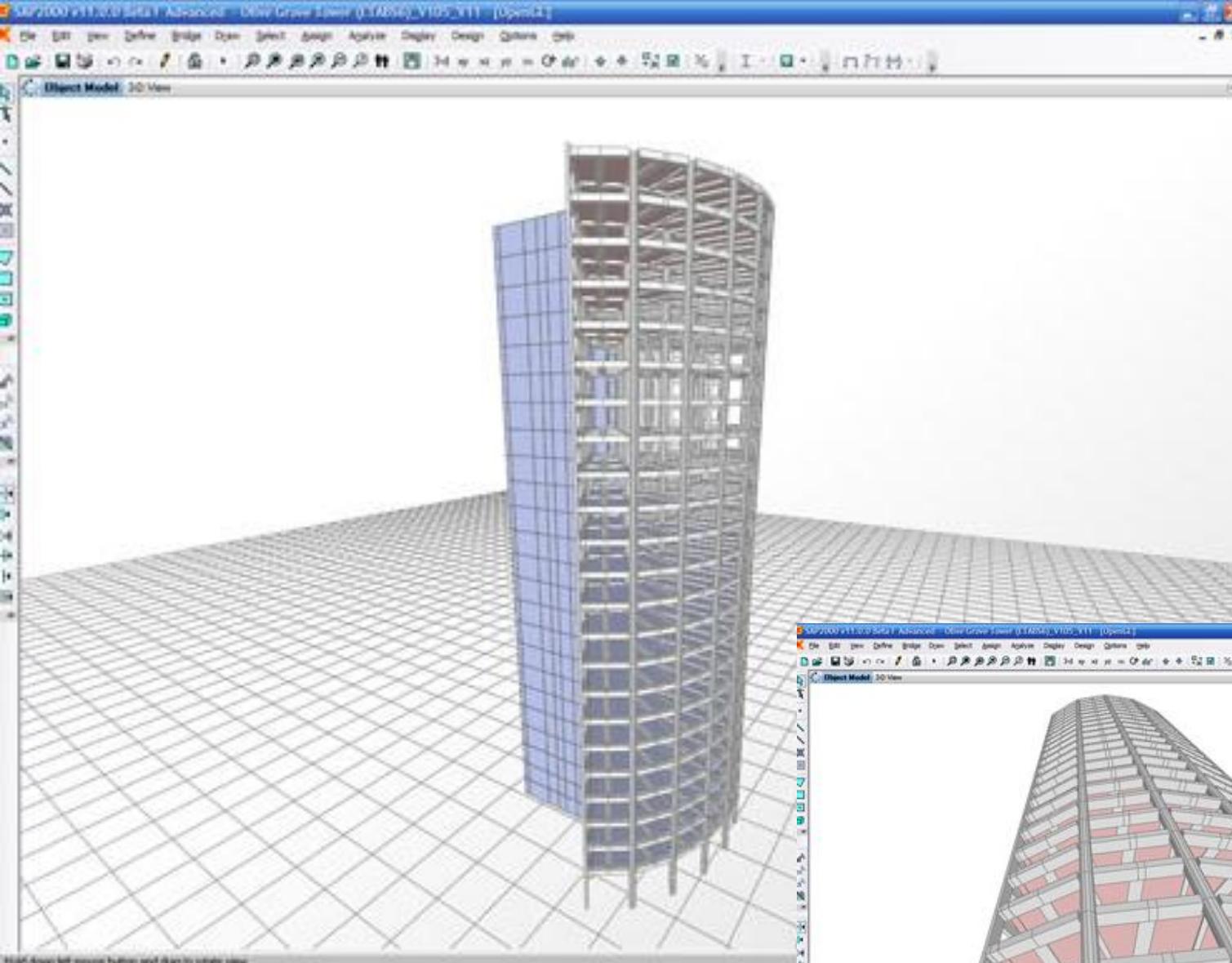


Click on any Frame Element for detailed diagram

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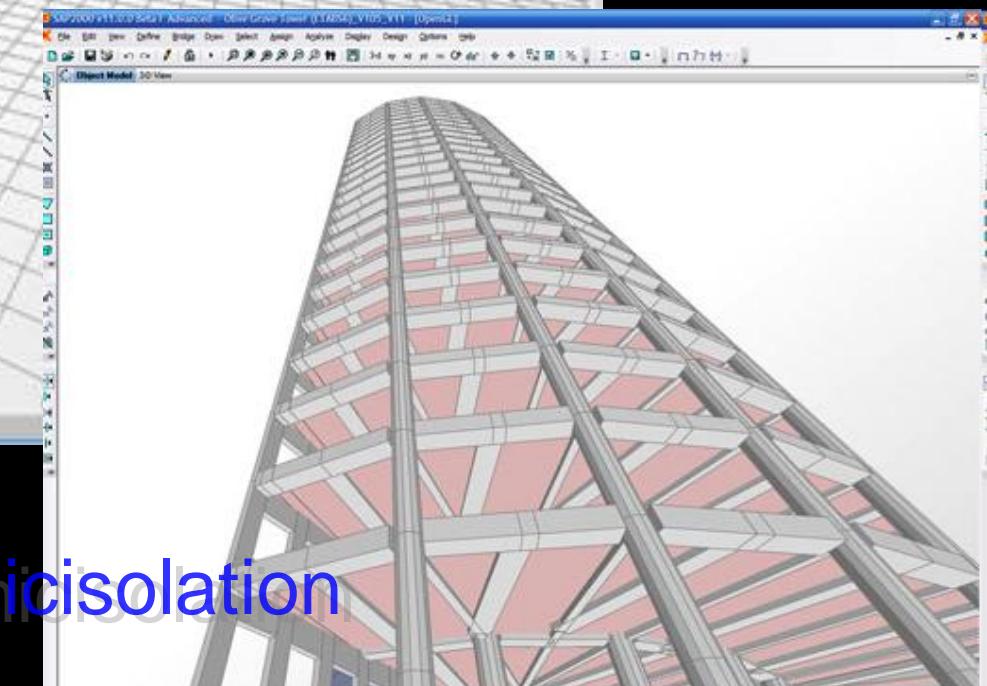


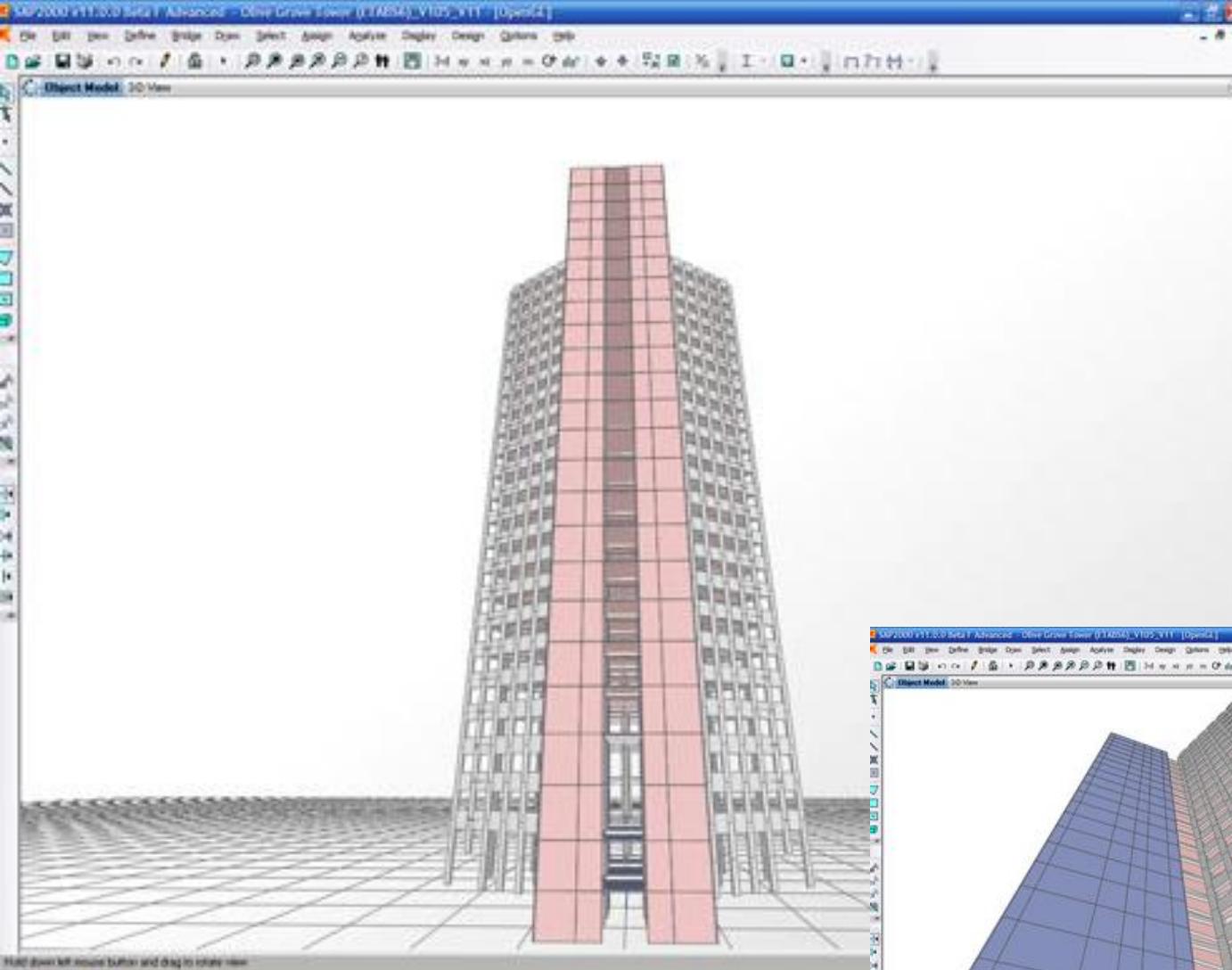
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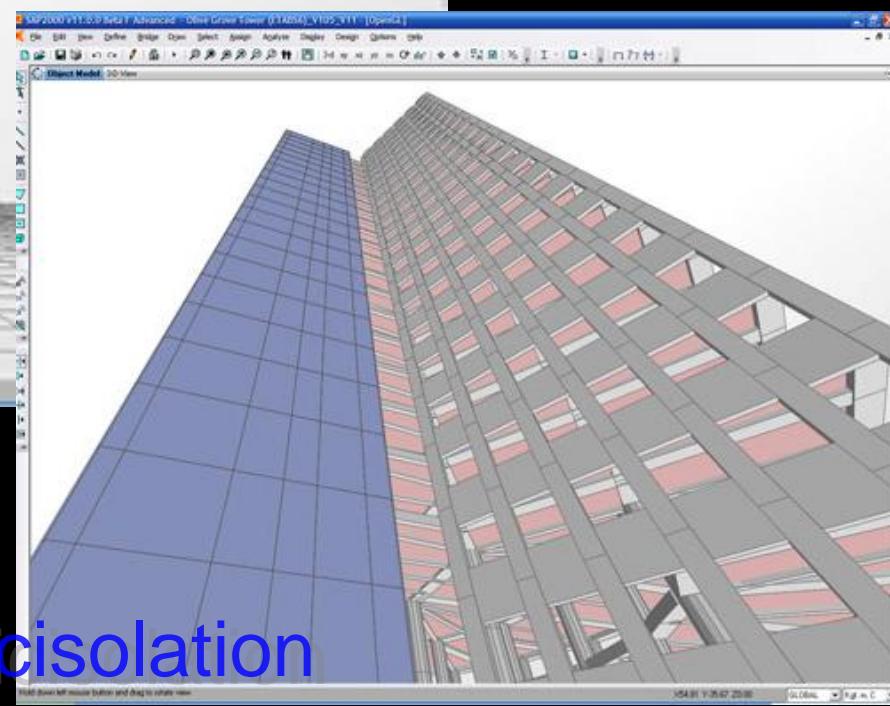
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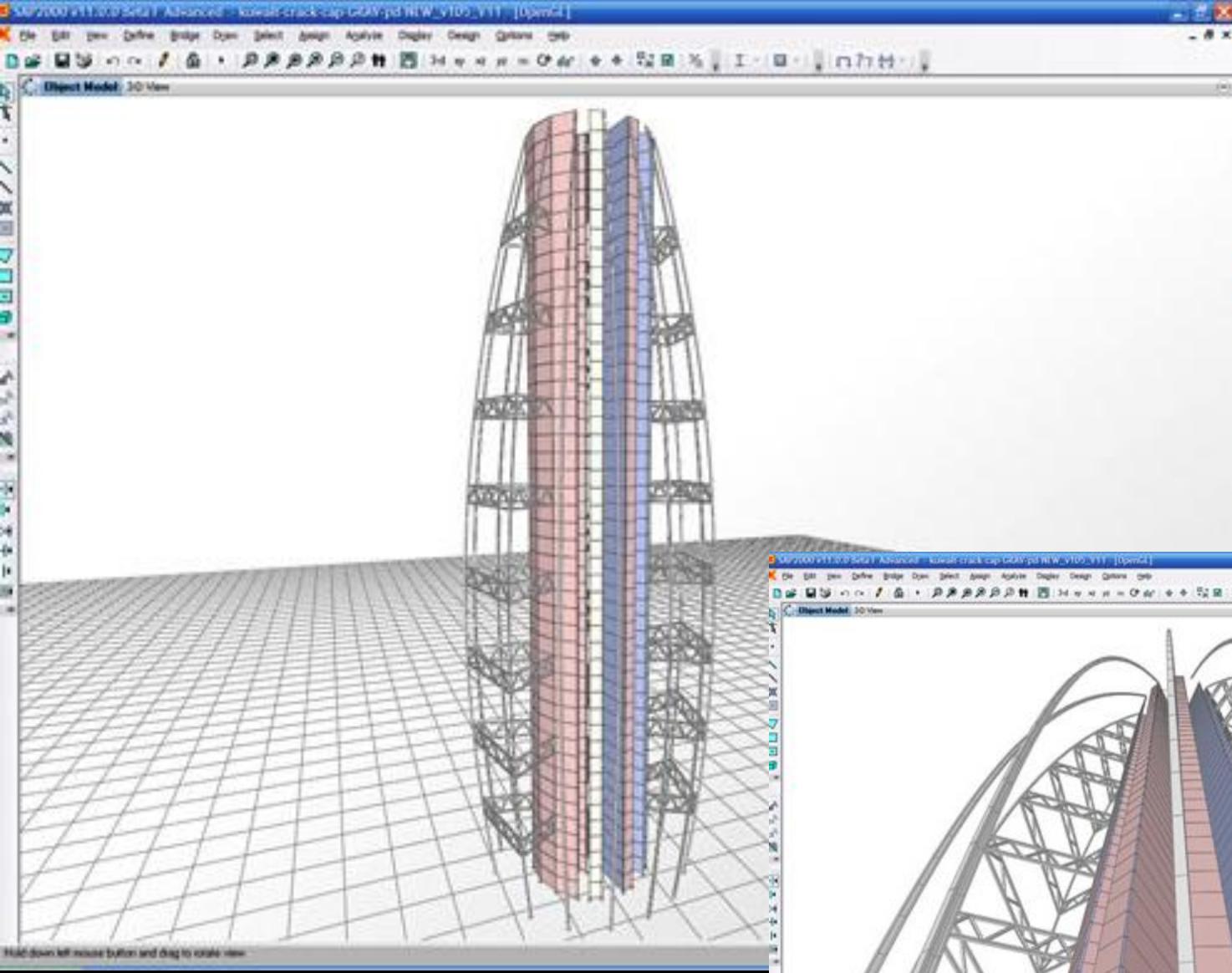
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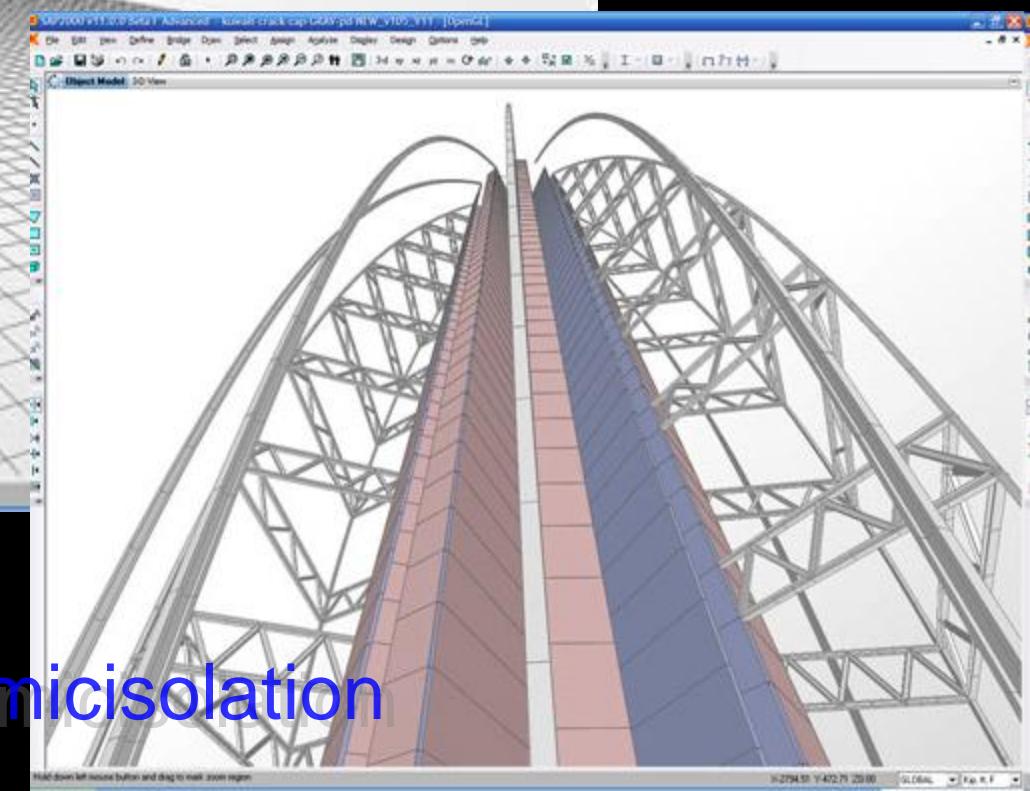
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164.91 V.25.67 20.00 GLOBAL Eng.m.C



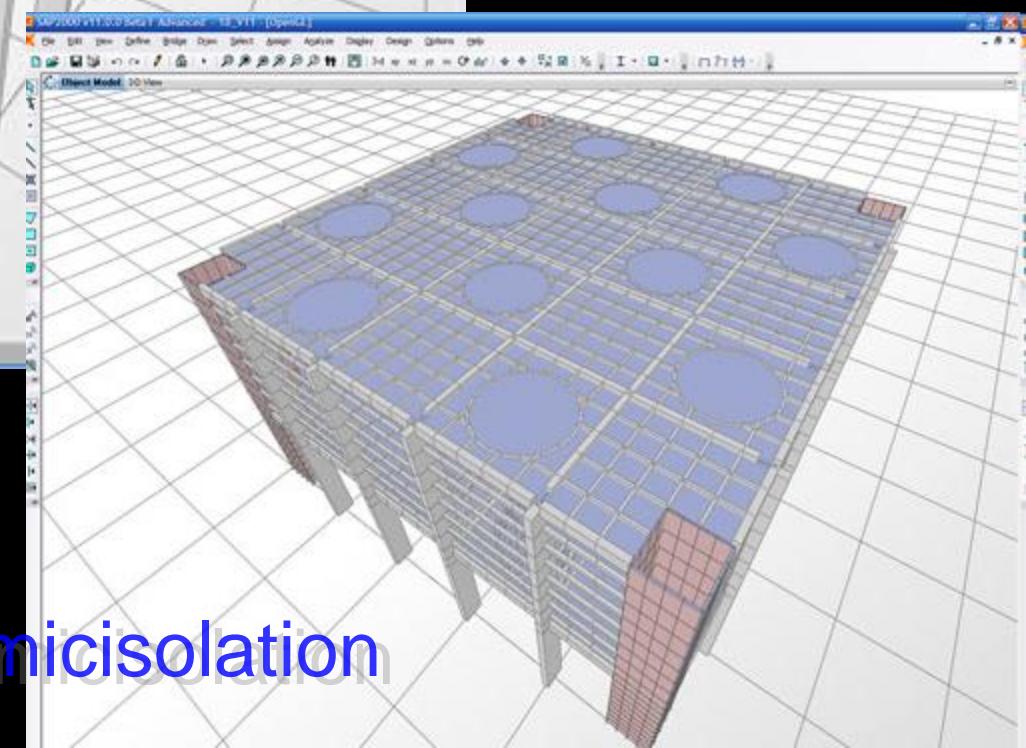
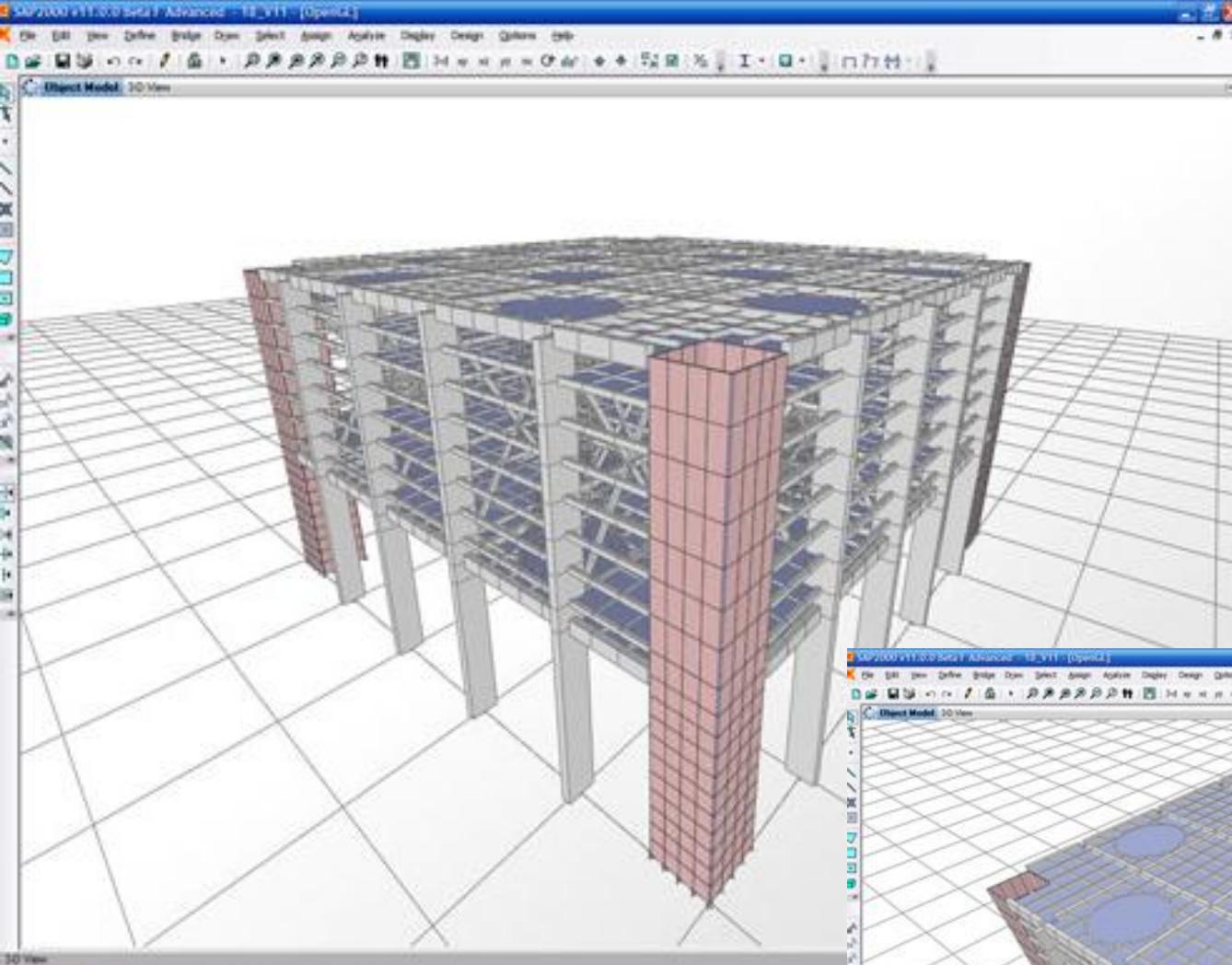
Hold down left mouse button and drag to rotate view

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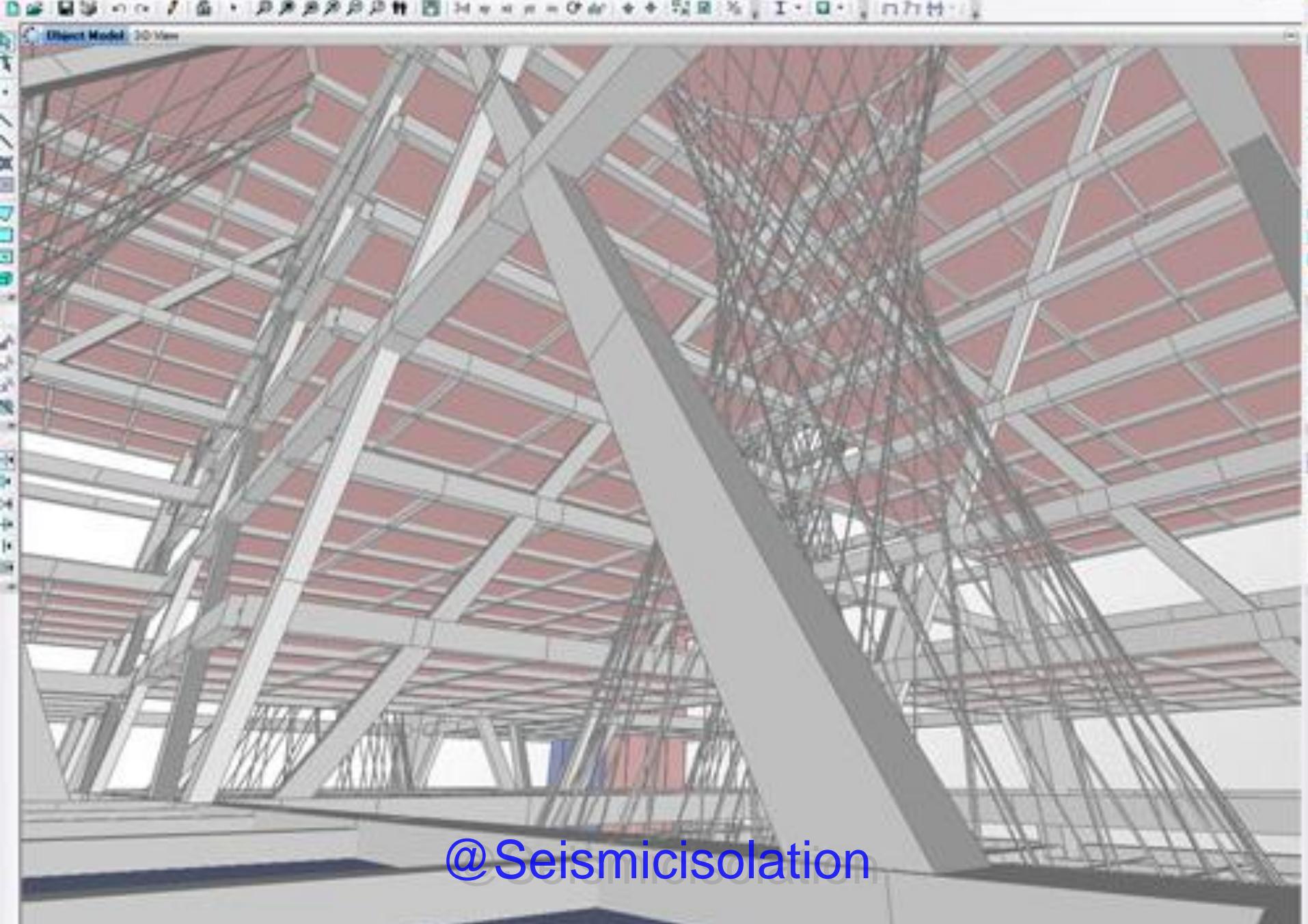


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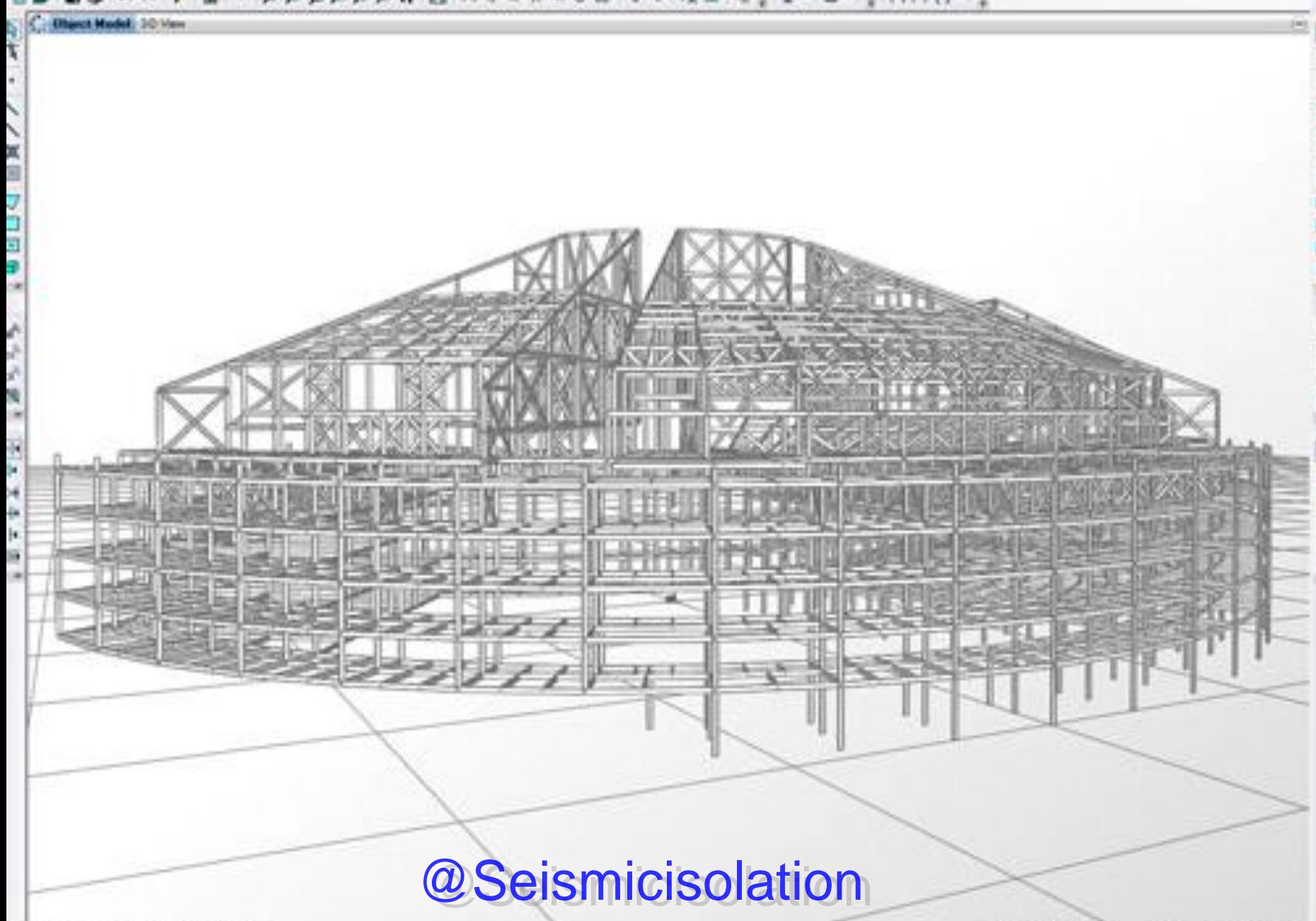
32794.51 Y40279 20.00 GLOBAL Fap. R. F



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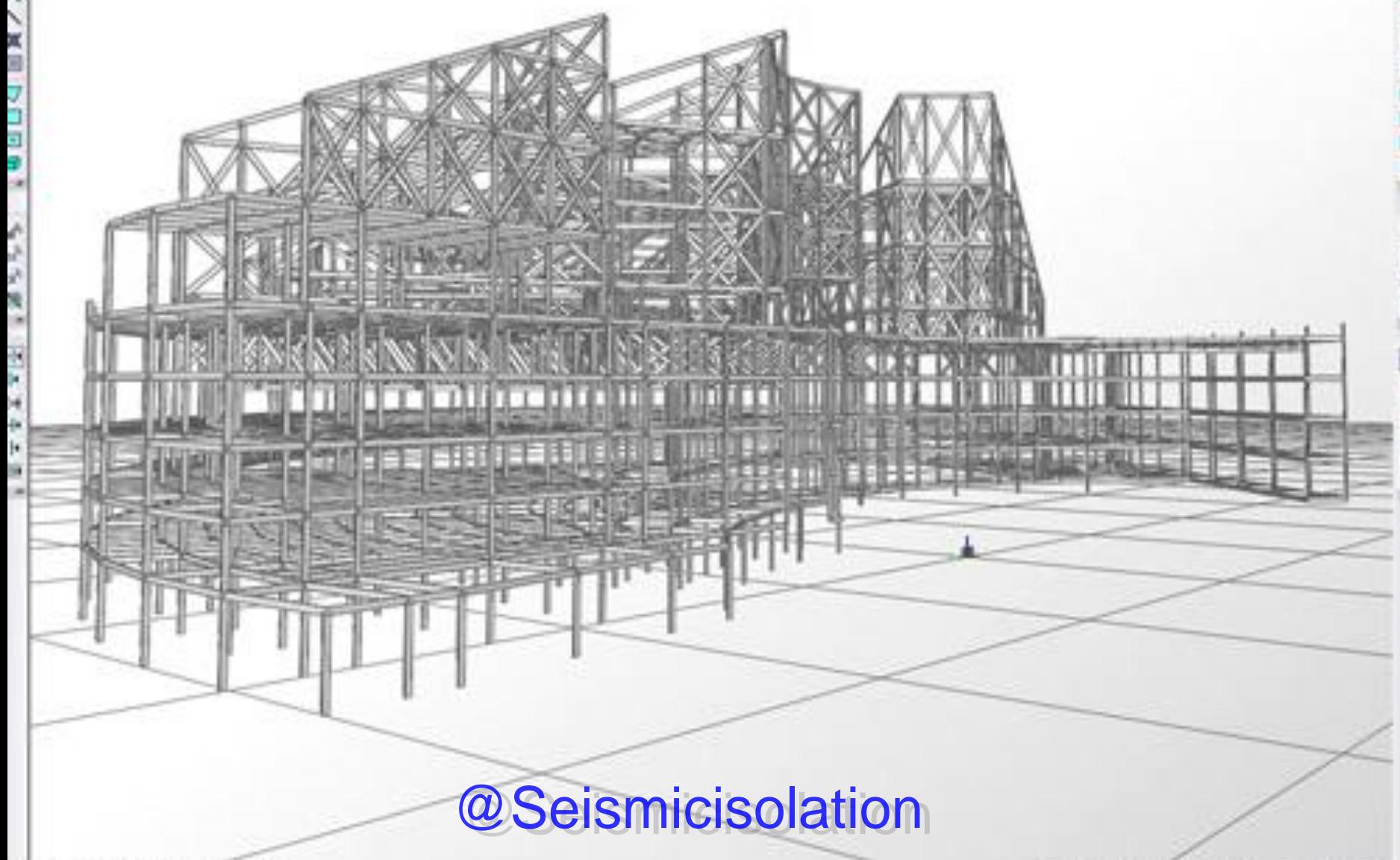
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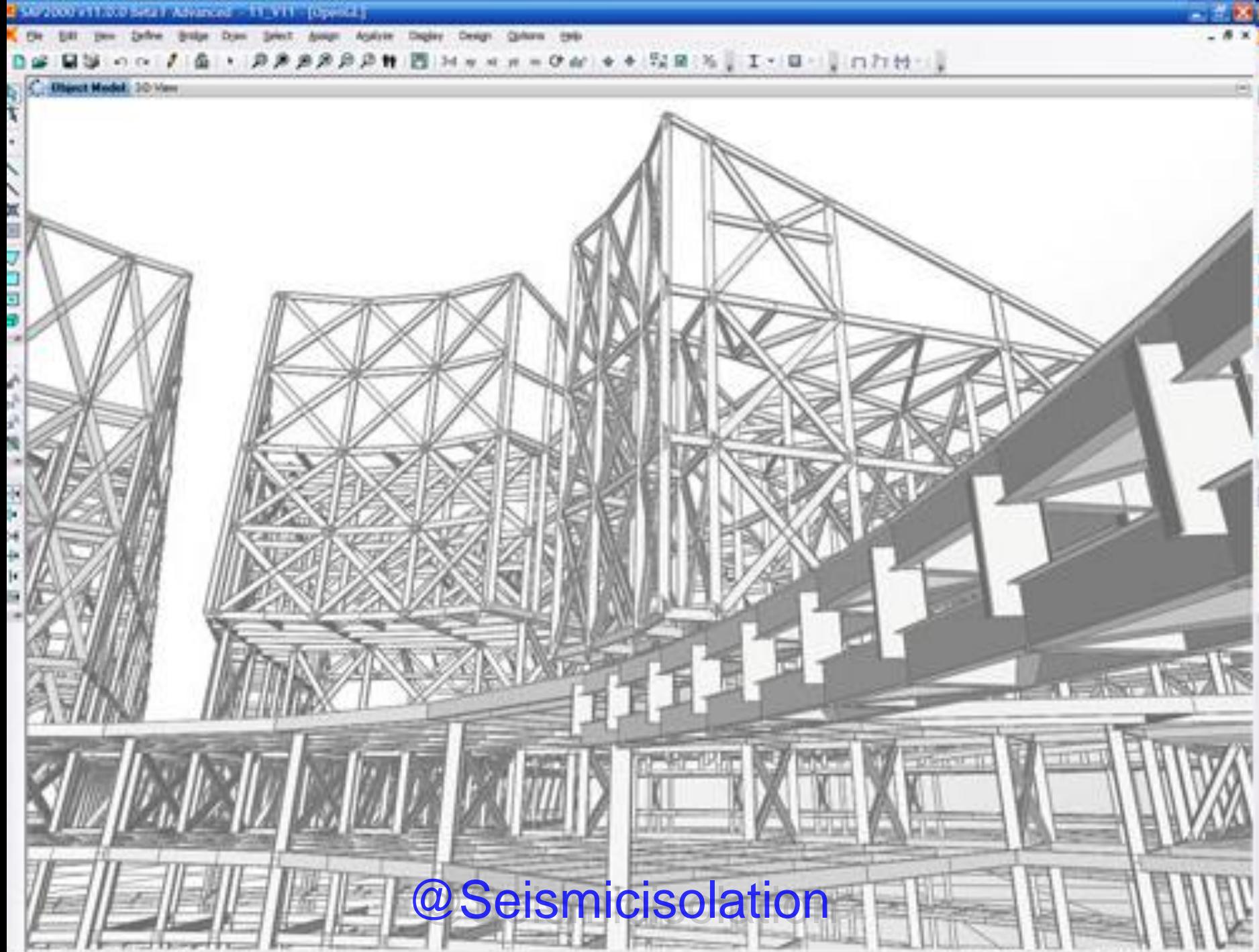
Hold down left mouse button and drag to rotate view

R229454 Rev Y-231309 32

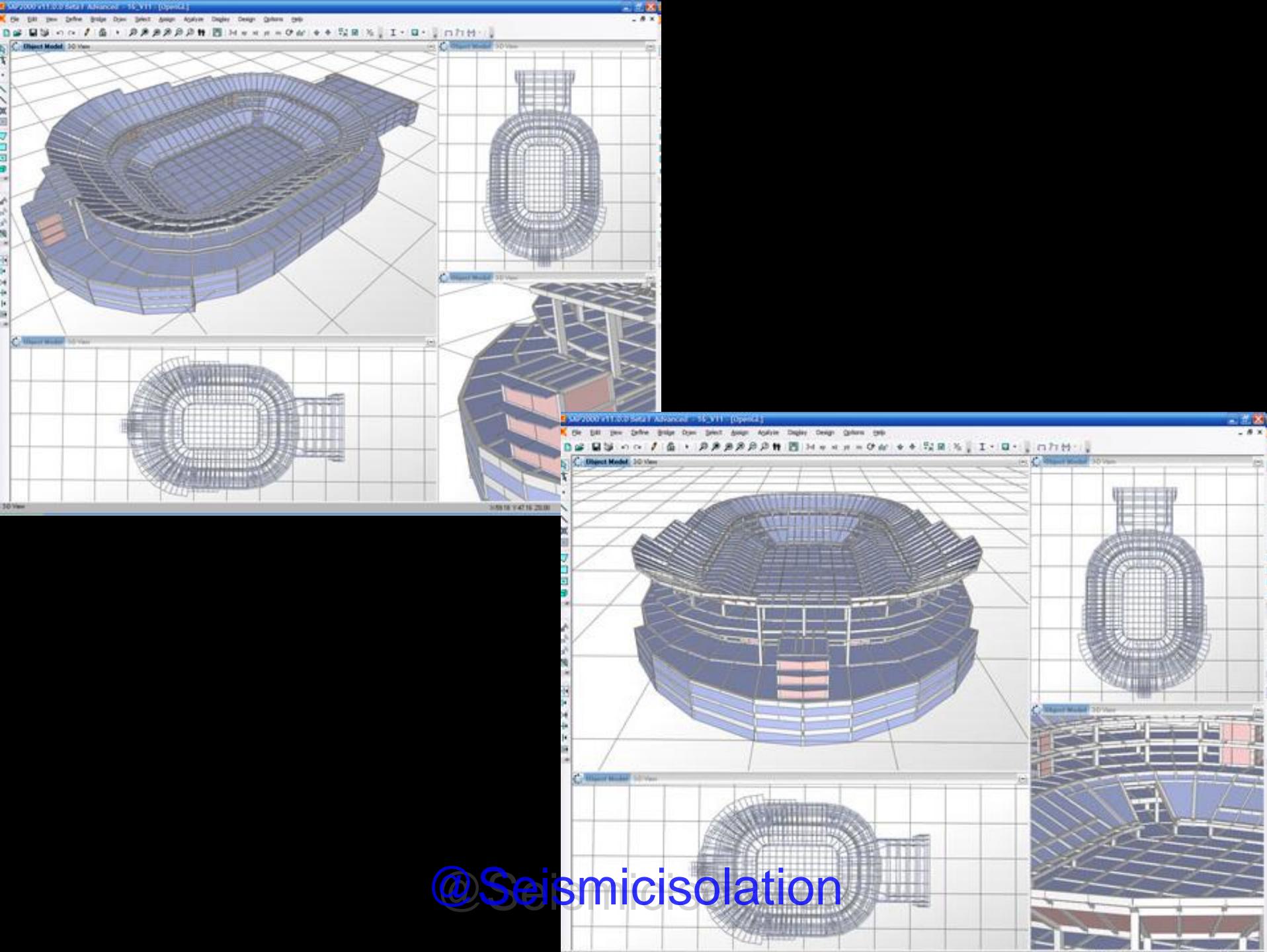
SL0600 N.m, C



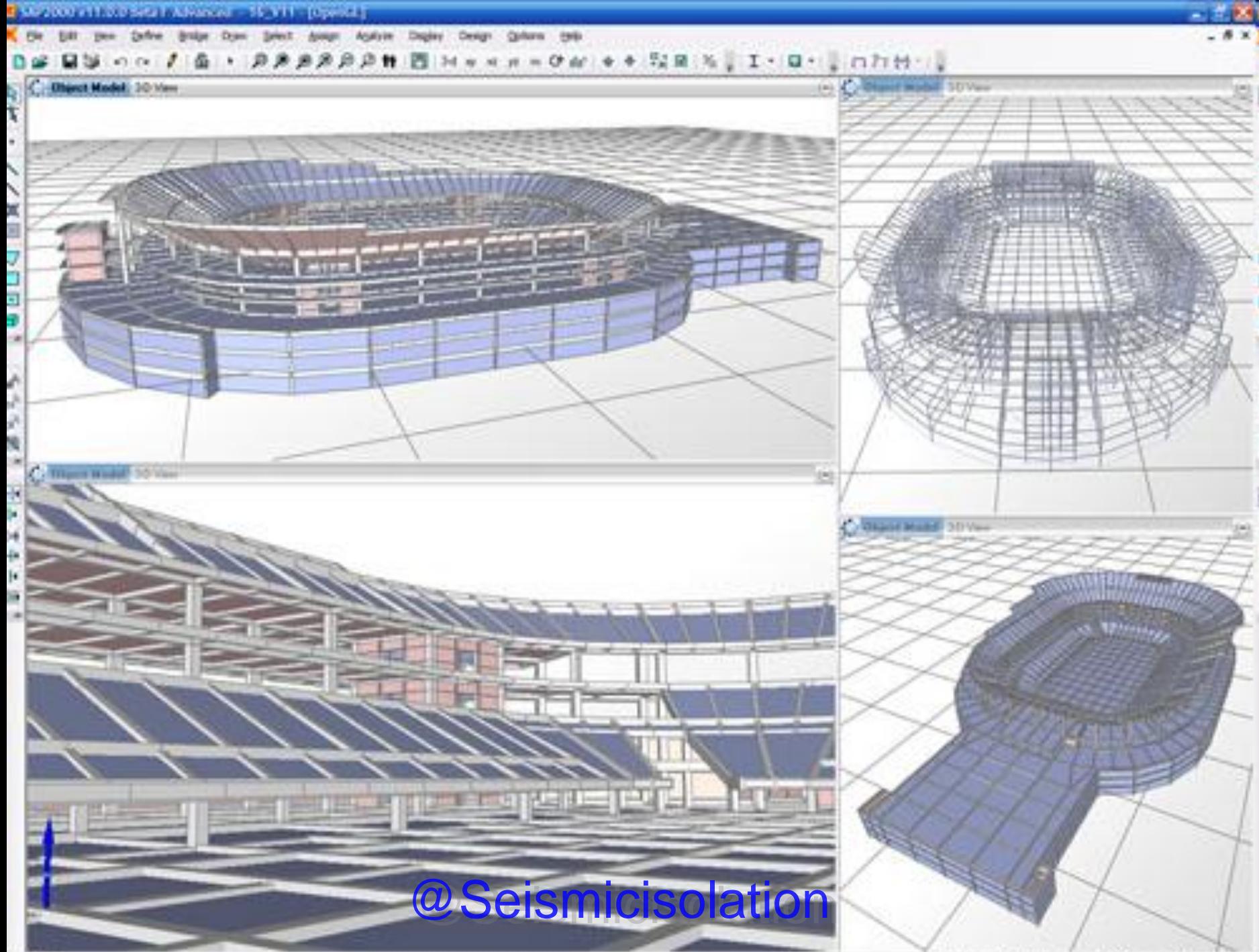
@Seismicisolation



@Seismicisolation



@Seismicisolation



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A grayscale photograph of a massive industrial space frame structure. The image shows multiple levels of steel beams forming a grid-like pattern, supported by thick vertical columns. The structure appears to be a large hangar or a factory under construction, with no internal partitions or equipment visible.

# SPACE FRAMES

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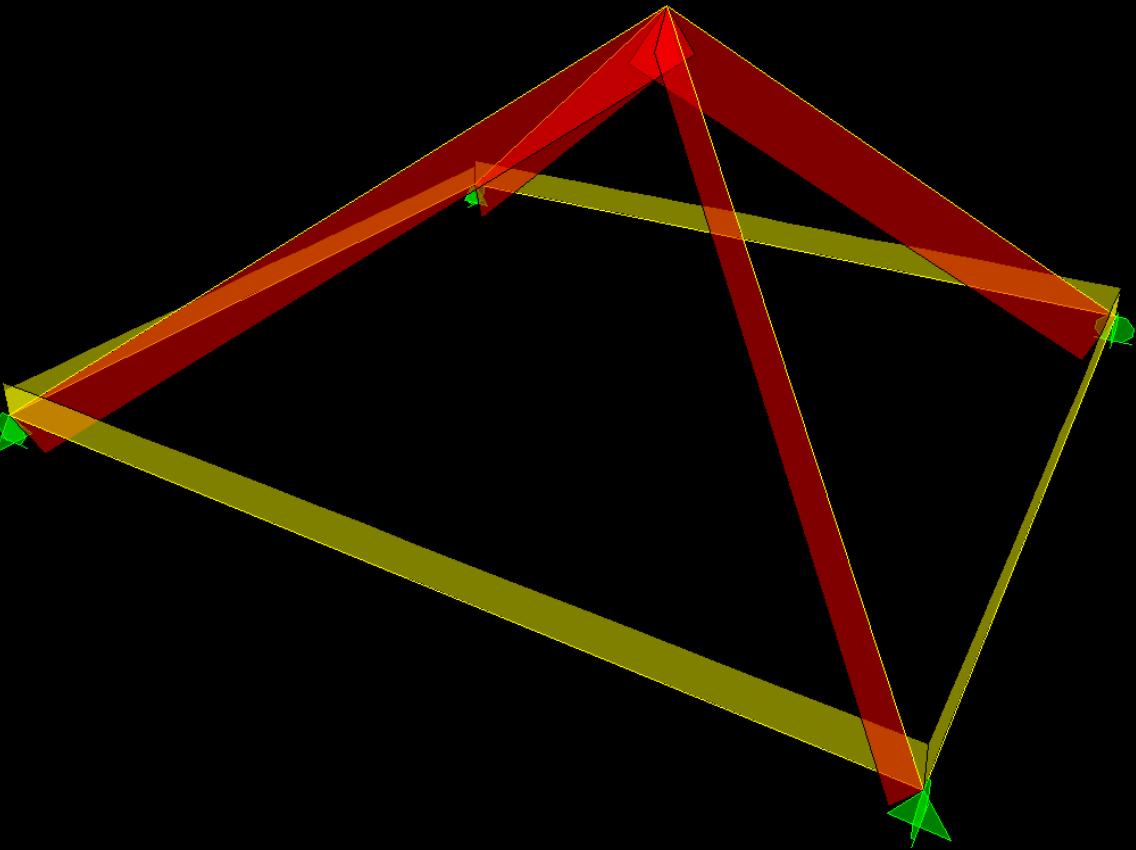
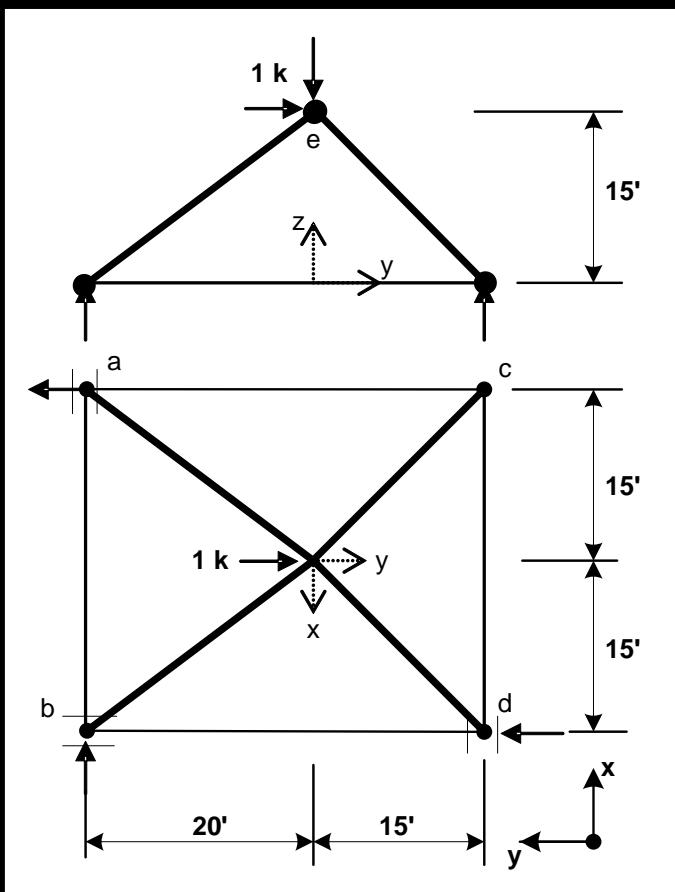


**Cologne/Bonn Airport,  
Germany, 2000, Helmut Jahn  
Arch., Ove Arup USA Struct.  
Eng.**

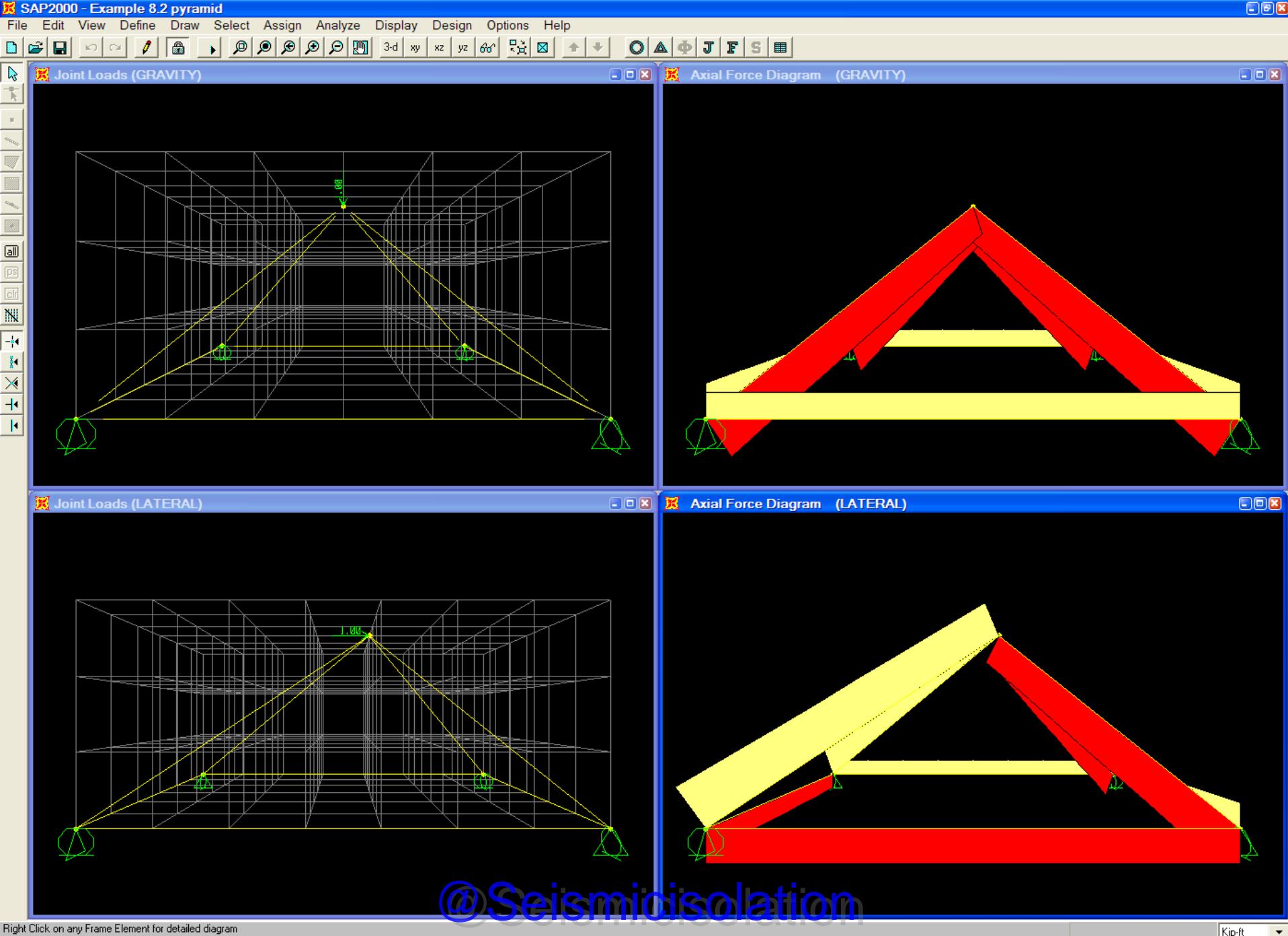
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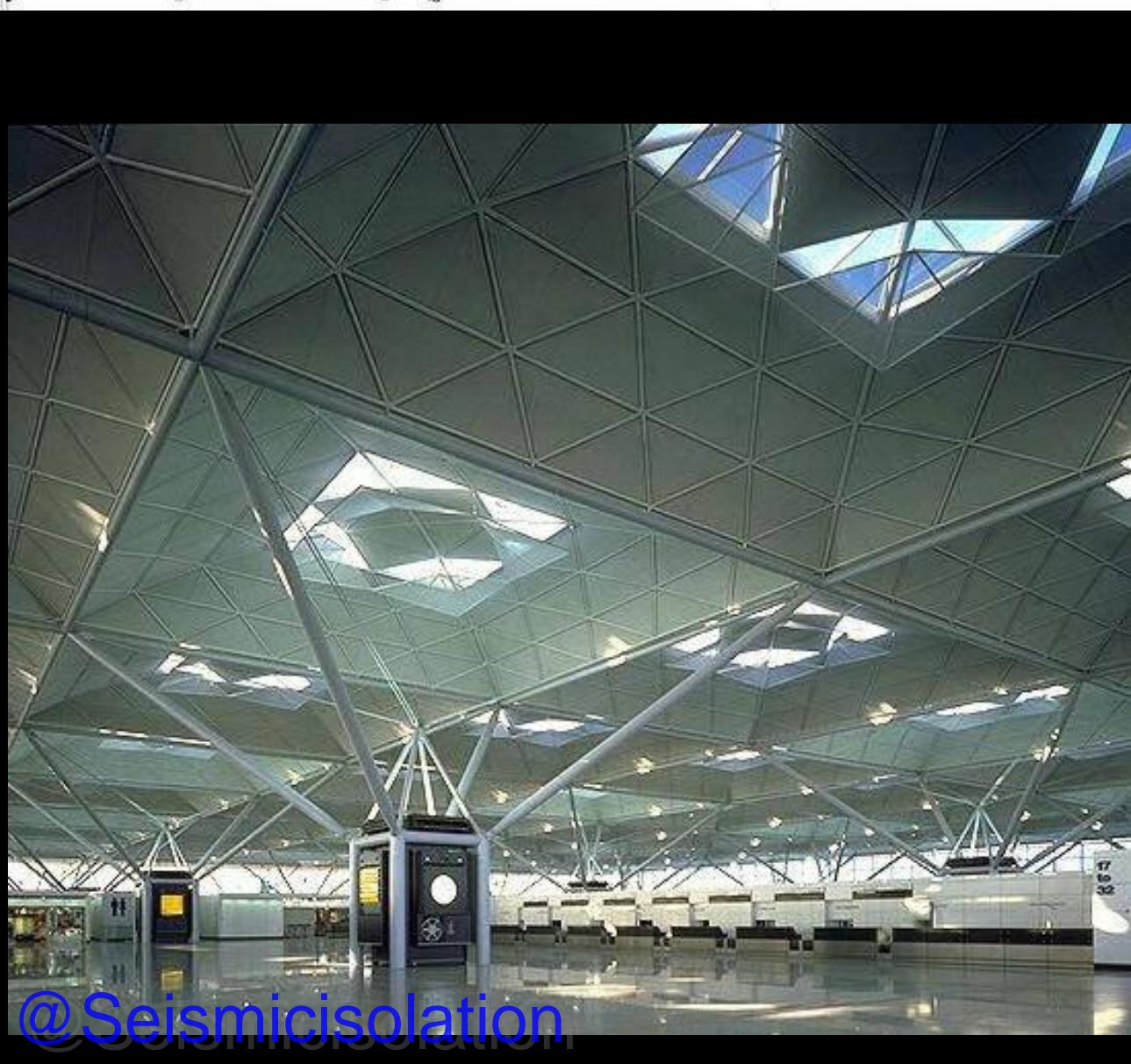
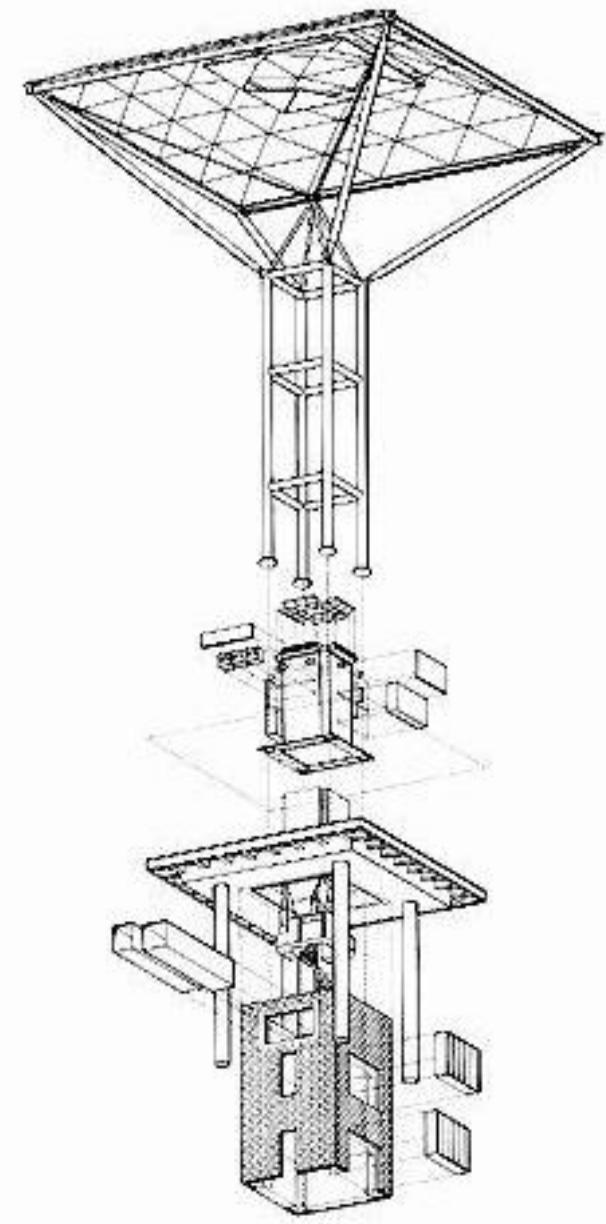
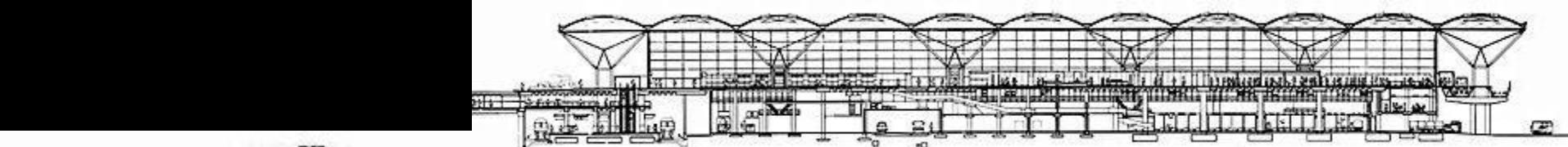


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Stansted Airport, London, UK, 1991, Foster/Arup  
© Seismic Isolation

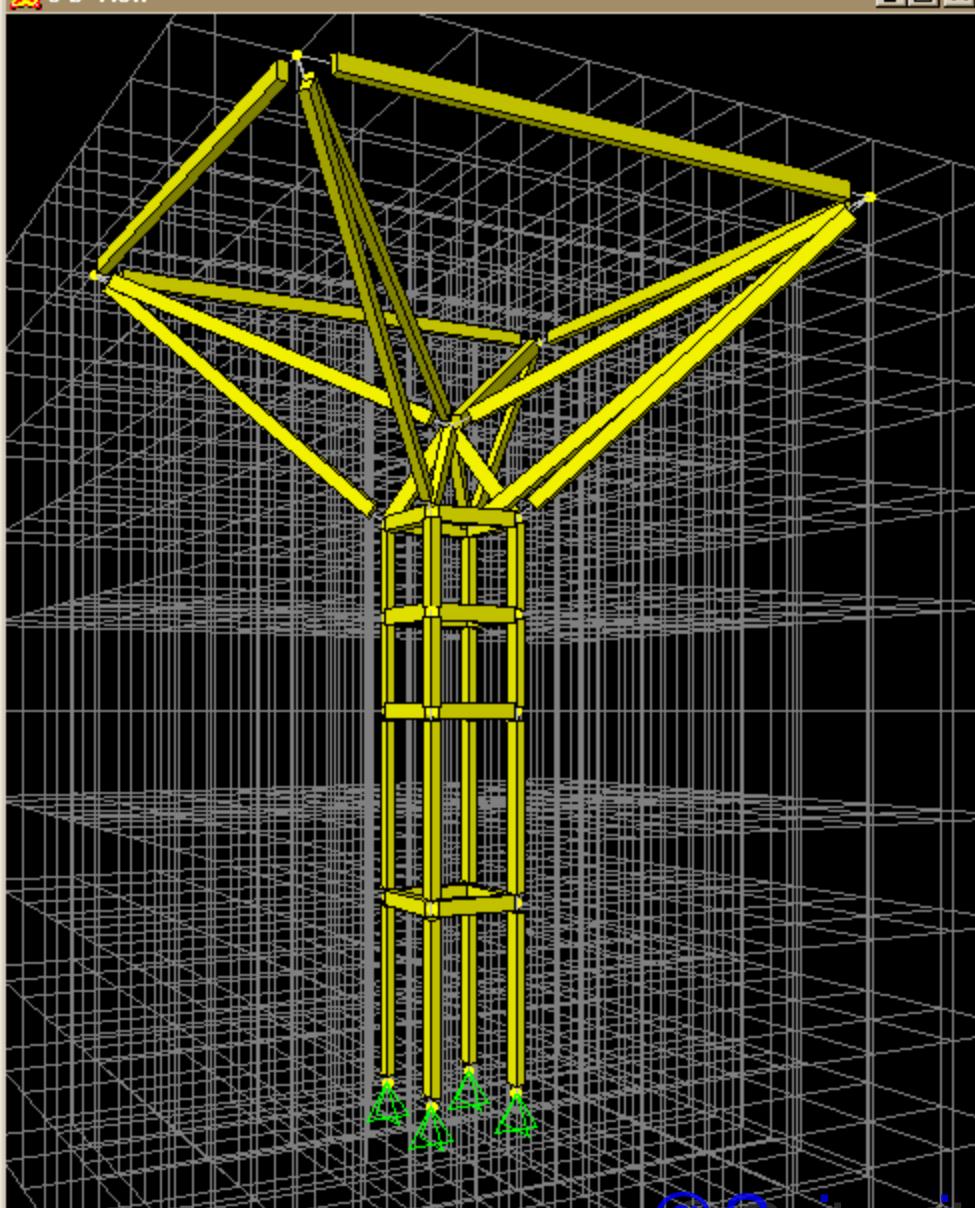


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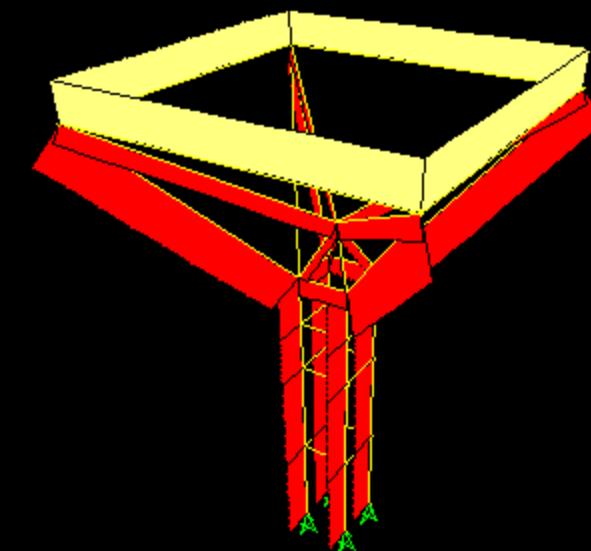
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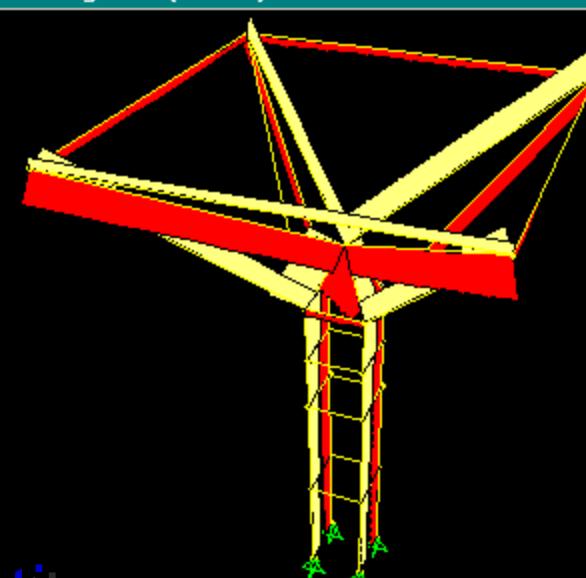
3-D View



Axial Force Diagram (LOAD1)



Axial Force Diagram (LOAD2)



Click on any Frame Element for detailed diagram

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KN-m

Start

SAP2000 - Ex.spacef...

Microsoft PowerPoint - Vis...



10:03 AM



Cologne/Bonn Airport, Germany 2000, Helmut Jahn Arch, Ove Arup USA Struct. Eng.  
©Seismicisolation

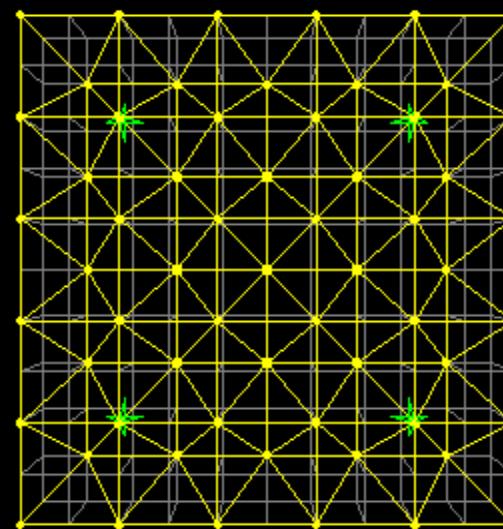


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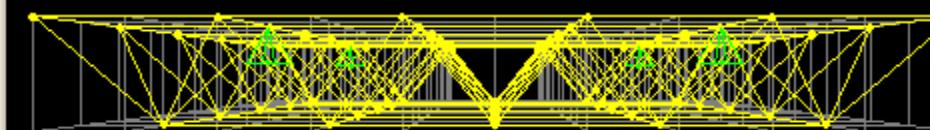
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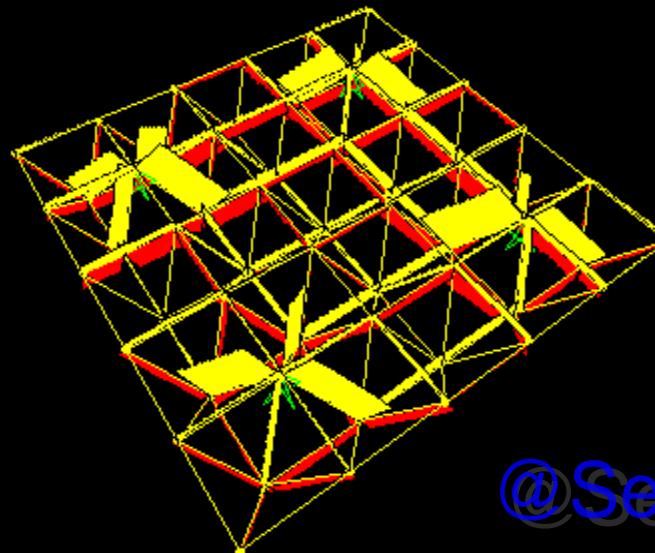
3-D View



3-D View



Axial Force Diagram (LOAD1)



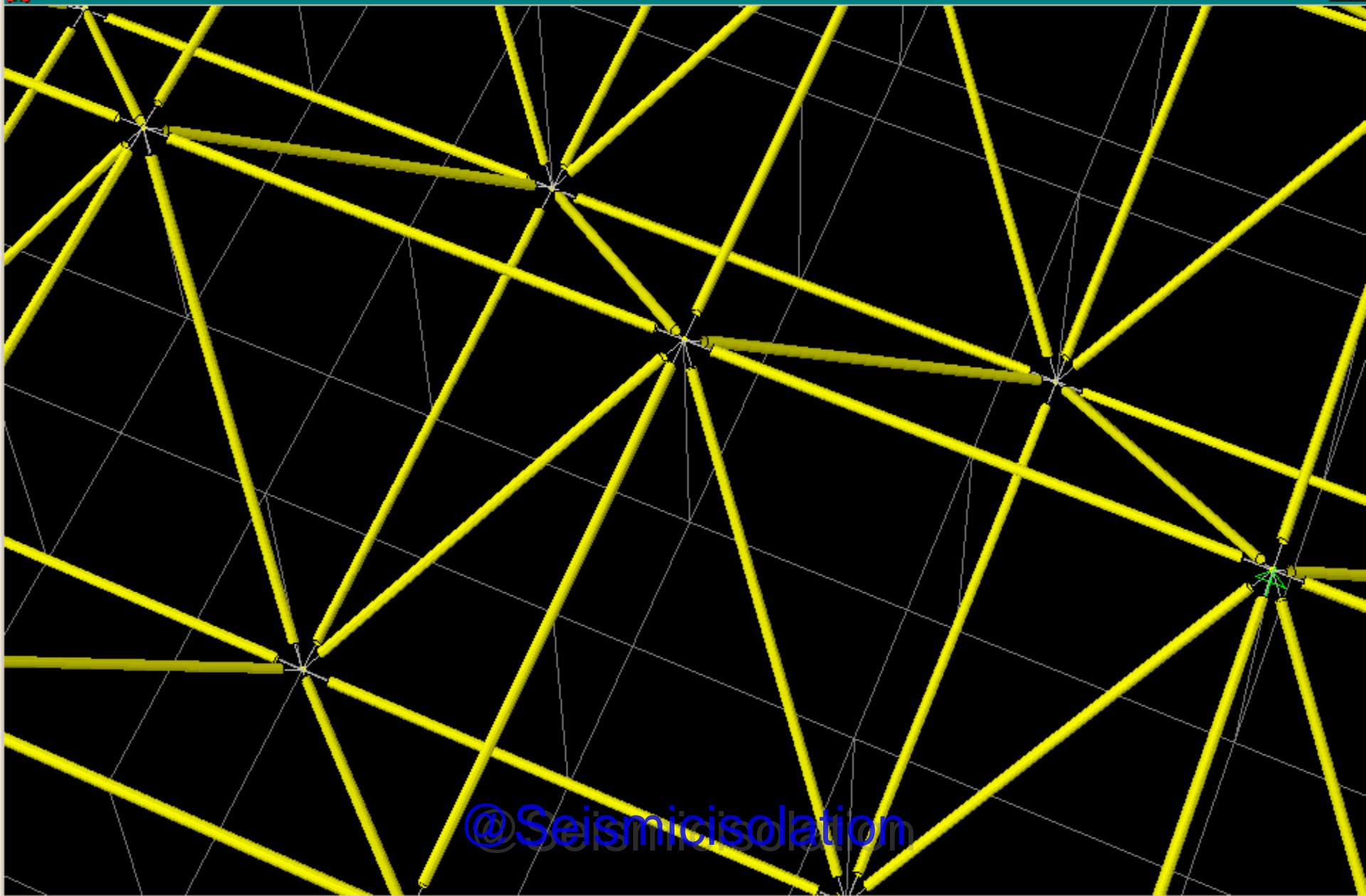
Axial Force Diagram (LOAD1)



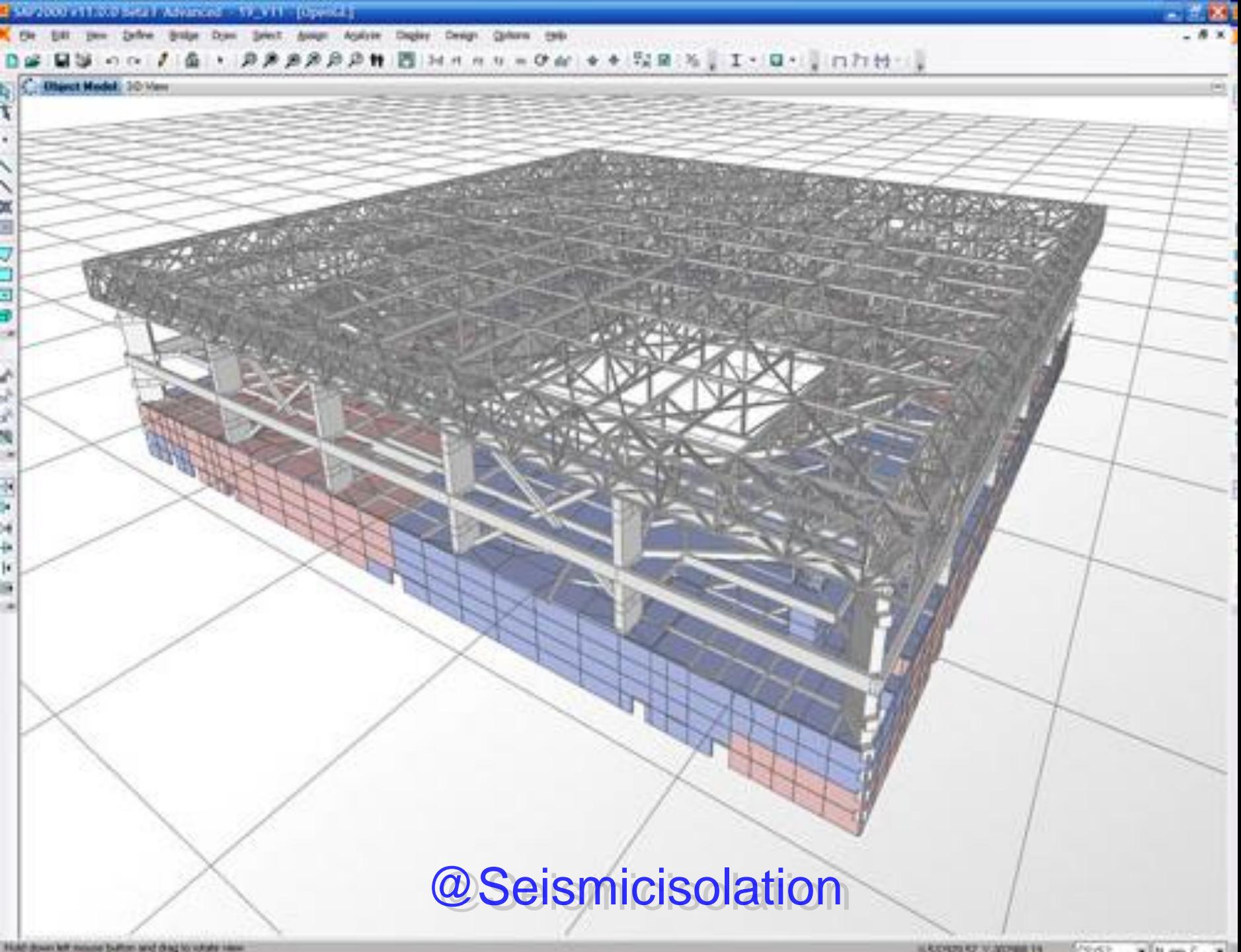
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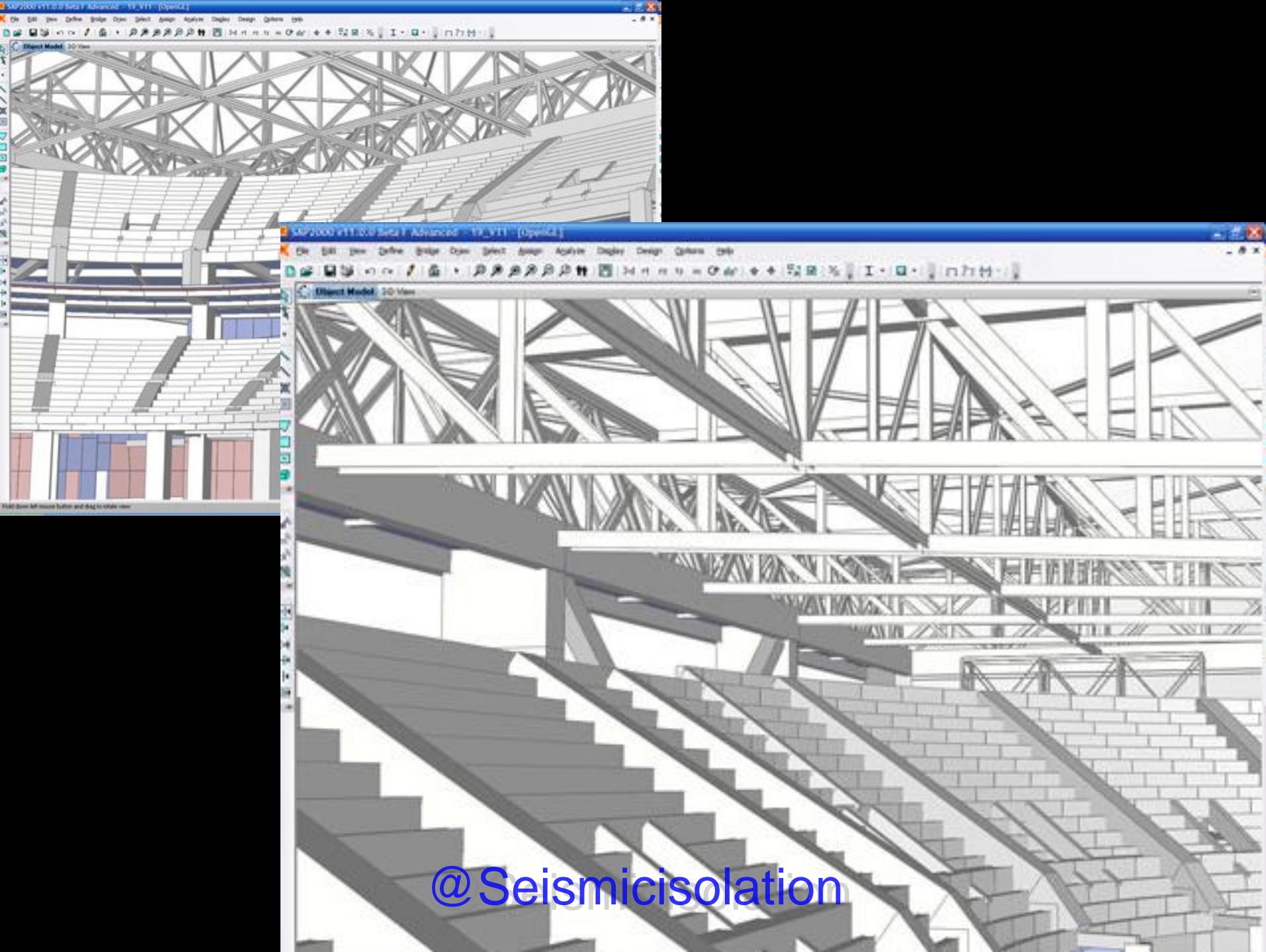
3-D View



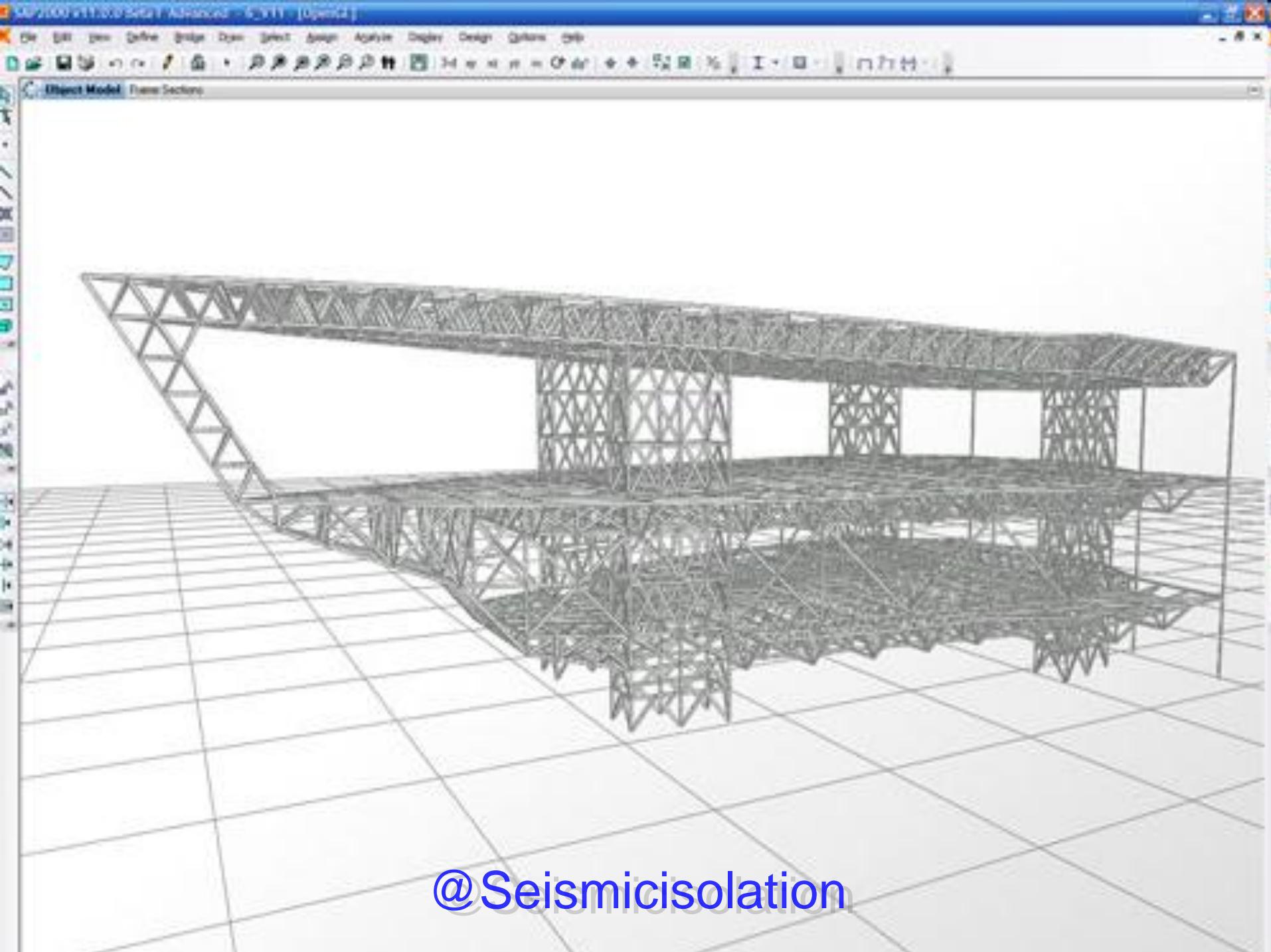
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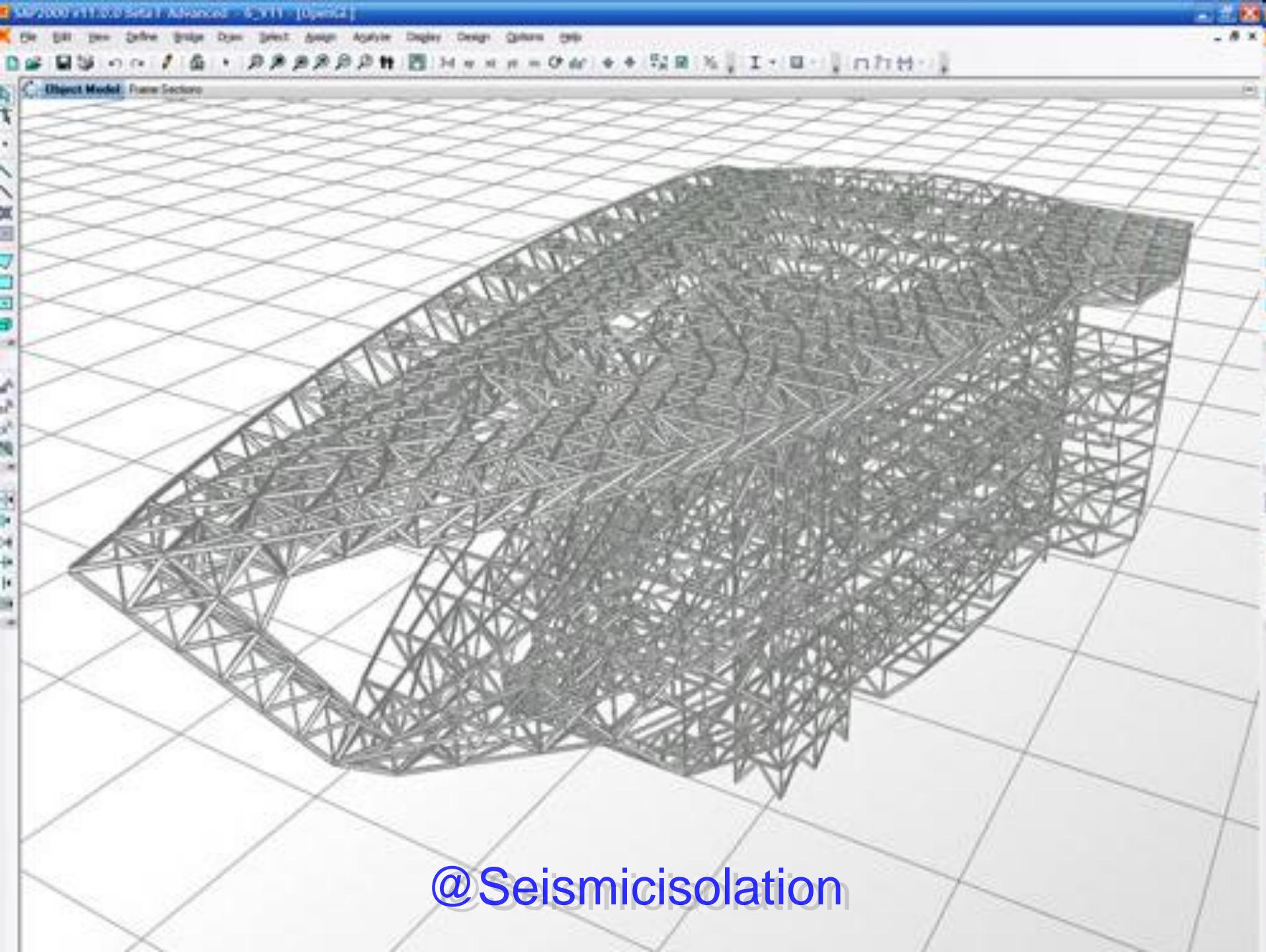
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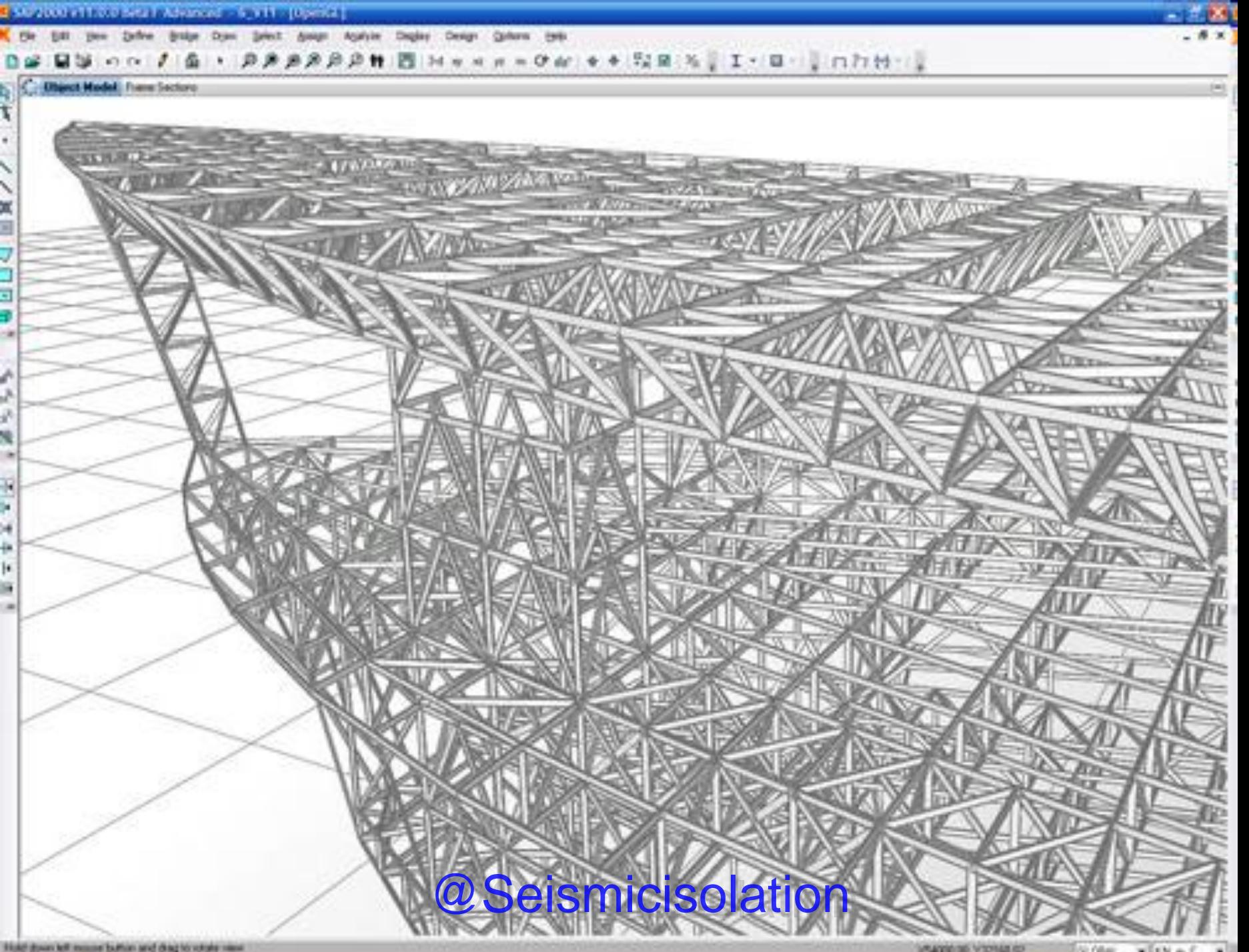
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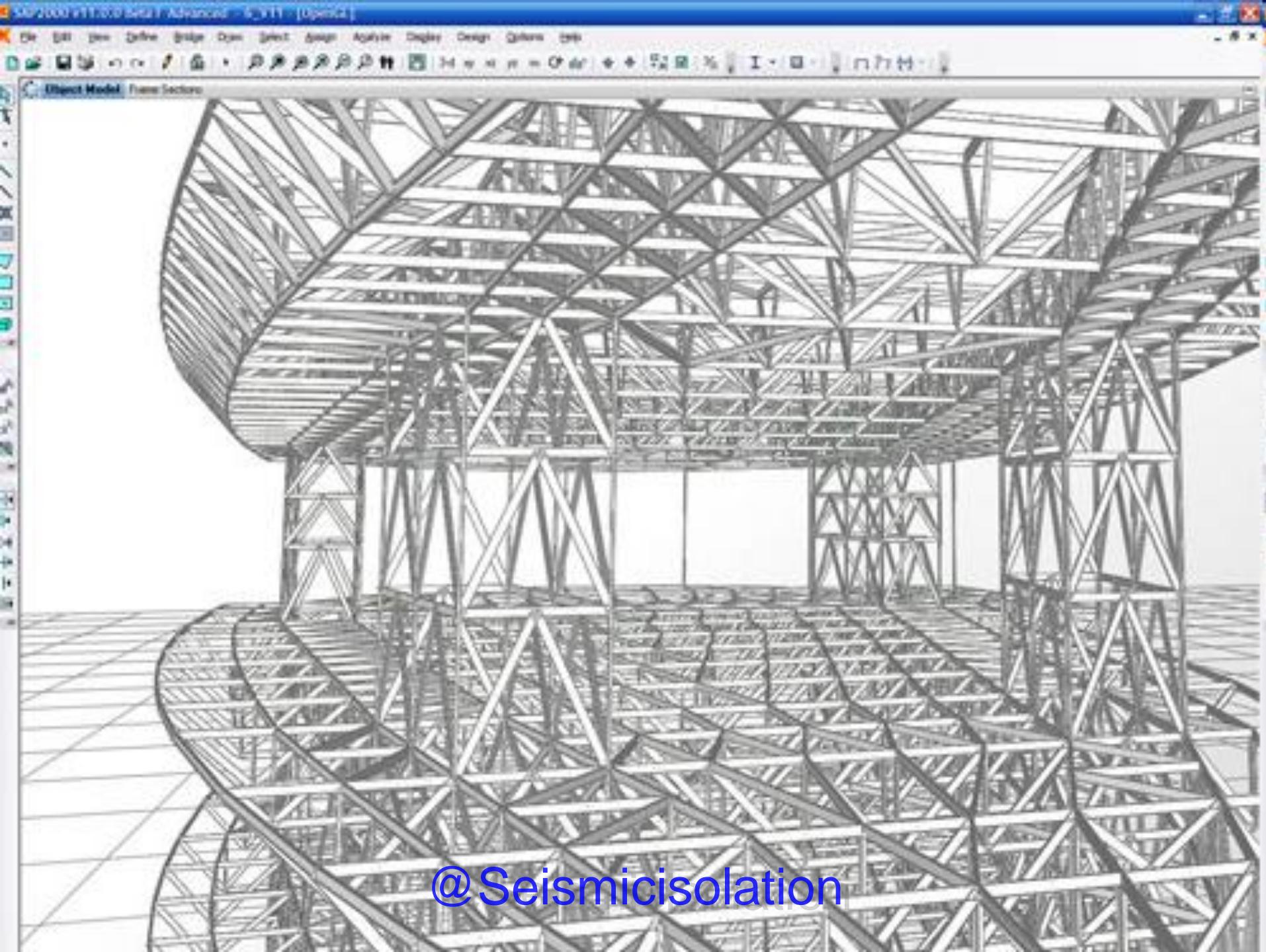
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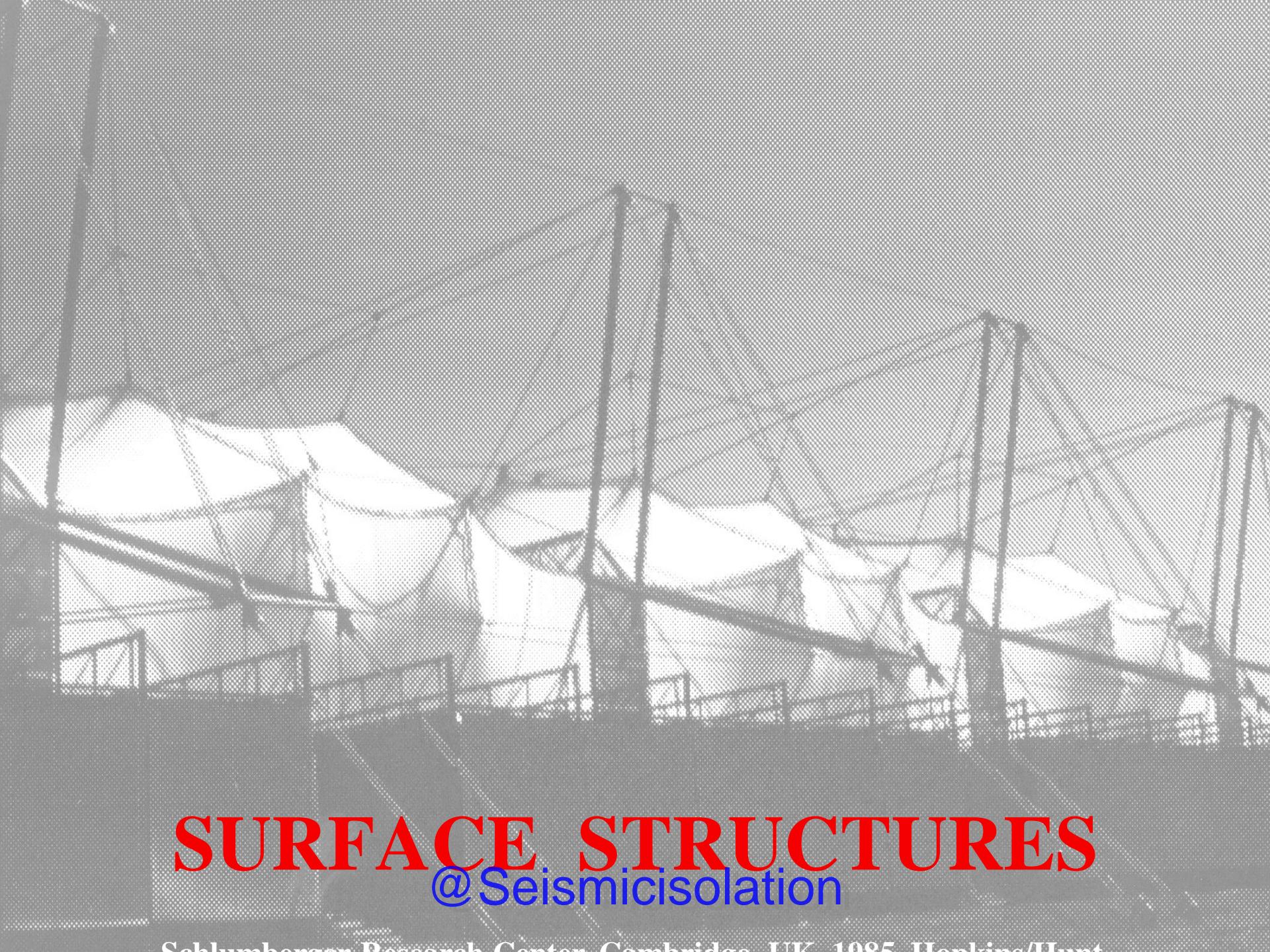
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# **SURFACE STRUCTURES**

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Schlumberger Research Center, Cambridge, UK 1985 Hopkins/Hunt

**MEMBRANES**

**BEAMS**

**BEARING WALLS and SHEAR WALLS**

**FOLDED SURFACES**

RIBBED VAULTING

LINEAR and RADIAL ADDITIONS

parallel, triangular, and tapered folds

CURVILINEAR FOLDS

**SHELLS: solid shells, grid shells**

CYLINDRICAL SHELLS

THIN SHELL DOMES

HYPERBOLIC PARABOLOIDS

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# TENSILE MEMBRANE STRUCTURES

## Pneumatic structures

Air-supported structures

Air-inflated structures (i.e. air members)

Hybrid air structures

## Anticlastic prestressed membrane structures

Edge-supported saddle roofs

Mast-supported conical saddle roofs

Arch-supported saddle roofs

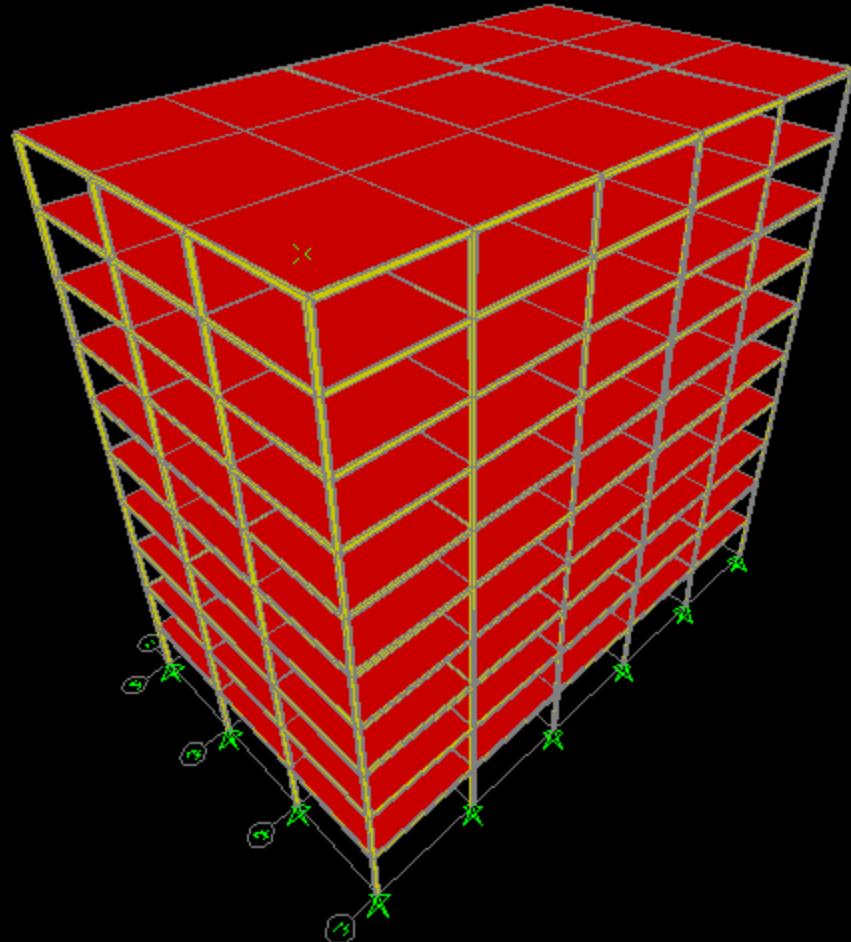
## Hybrid tensile surface structures

Edit View Define BrIM Draw Select Assign Analyze Display Design Options Help



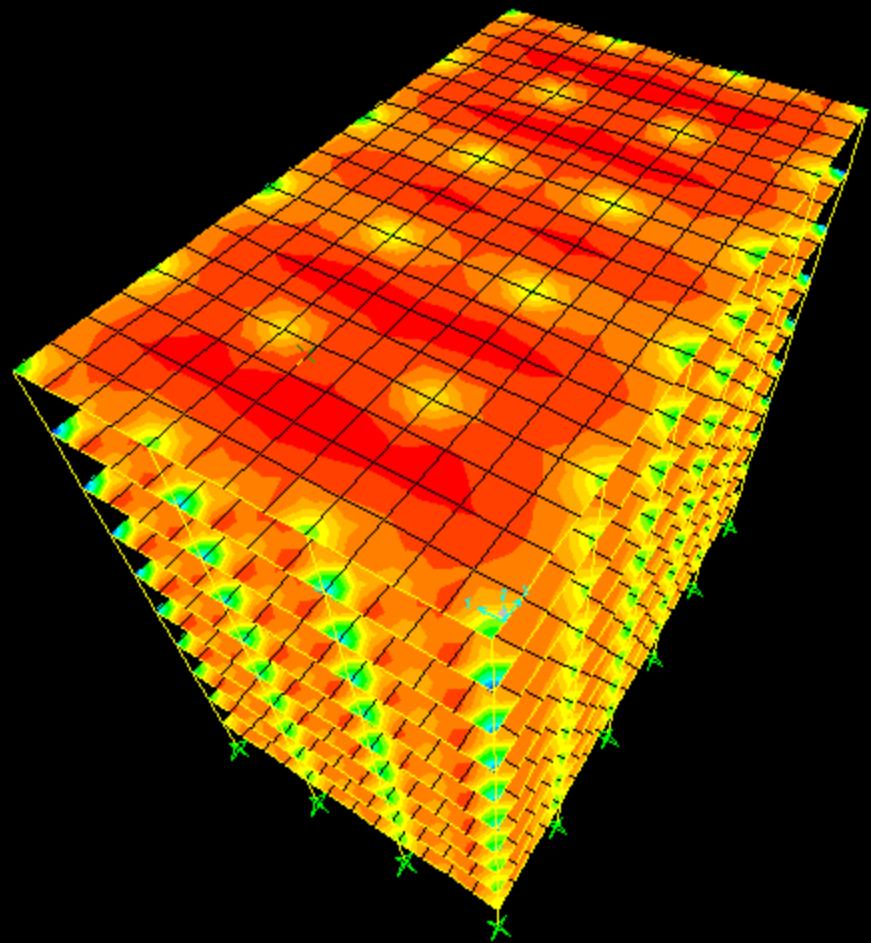
3-D View

Object Model



Stress SMAX Diagram - Visible Face (DEAD)

Analysis Model



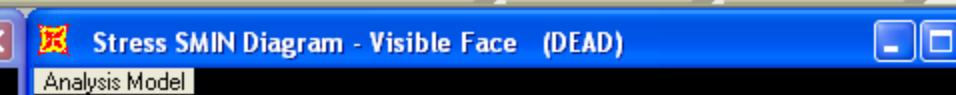
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points Selected

1.60 1.45 1.30 1.15 1.00 0.85 0.70 0.55 0.40 0.25 0.10 0.05 0.00 Kip, in, F

GLOBAL Kip, in, F

Edit View Define BrIM Draw Select Assign Analyze Display Design Options Help

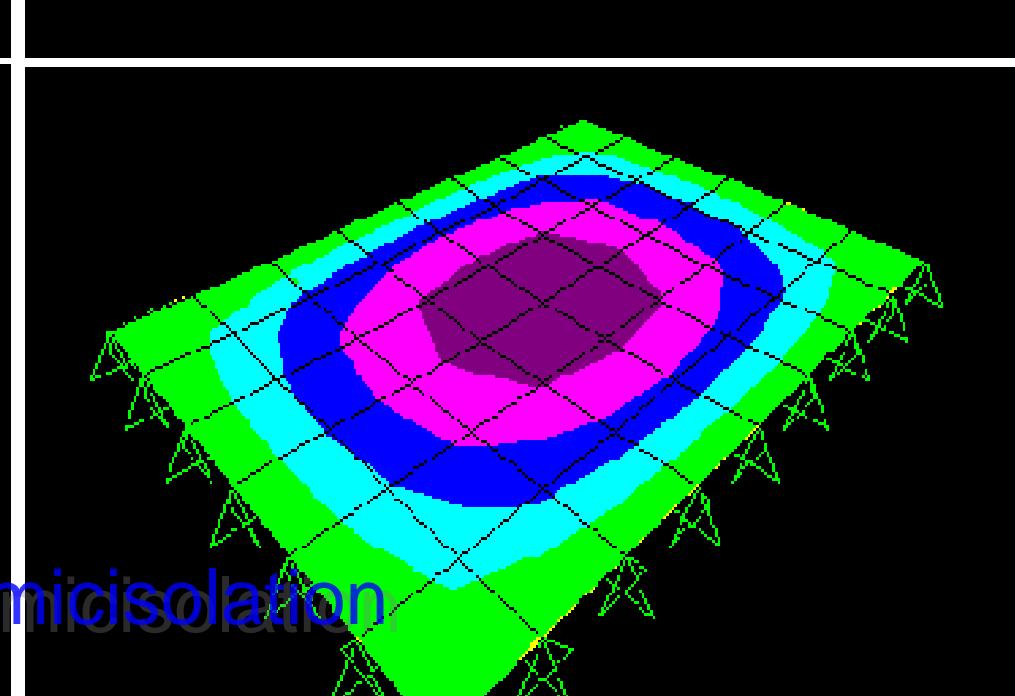
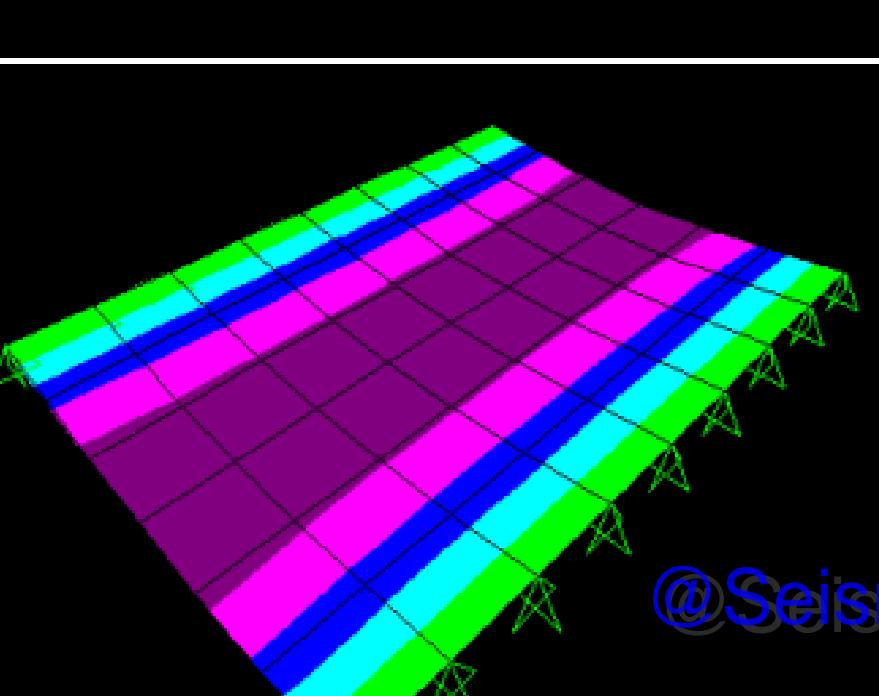
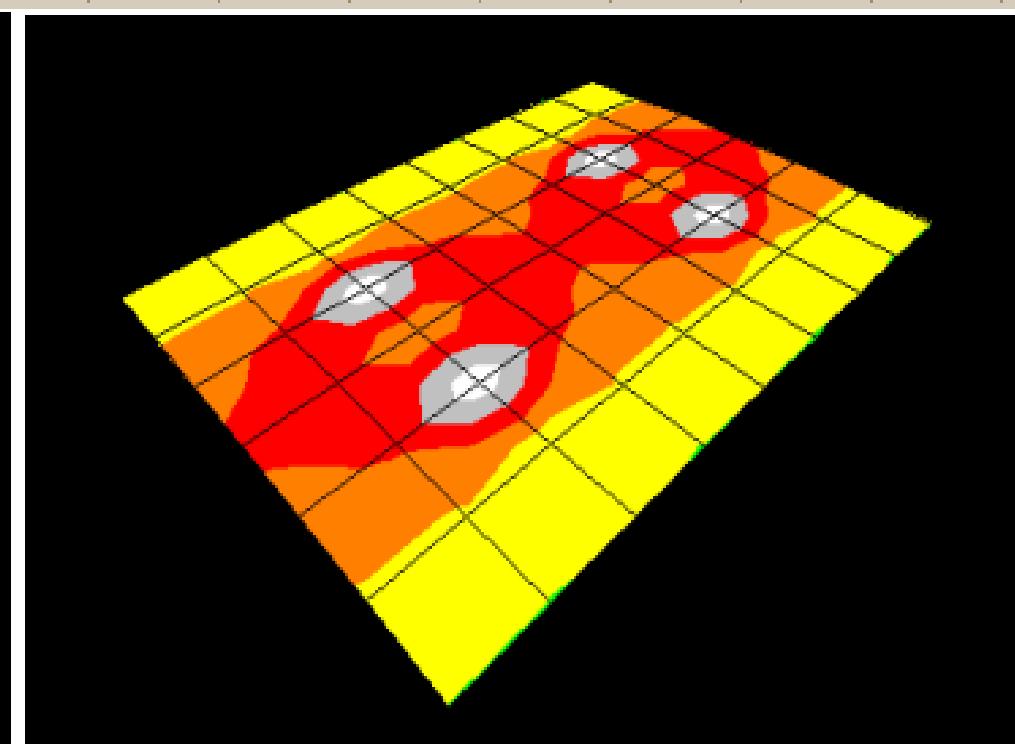
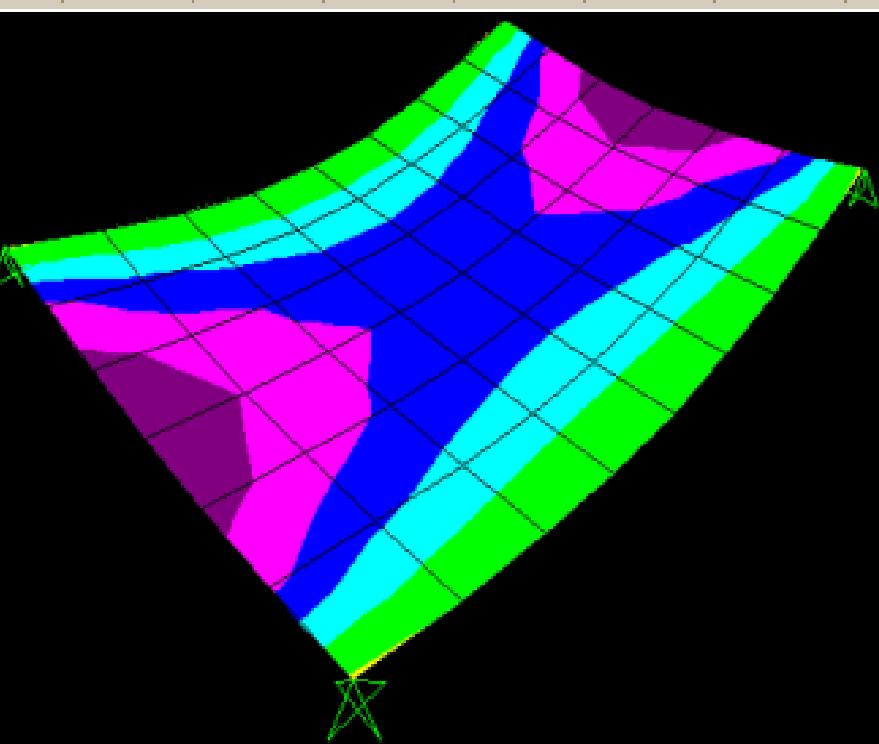


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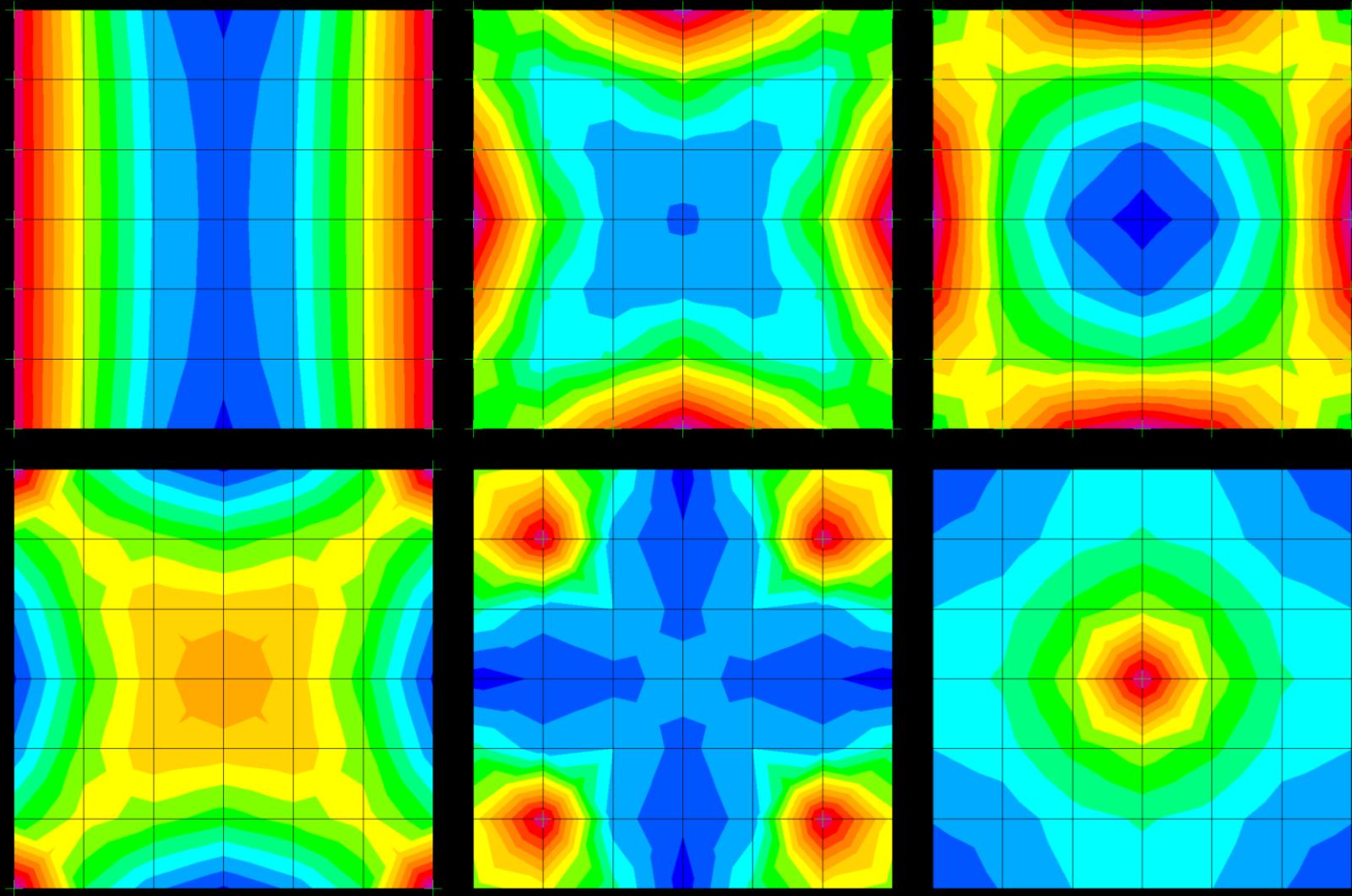
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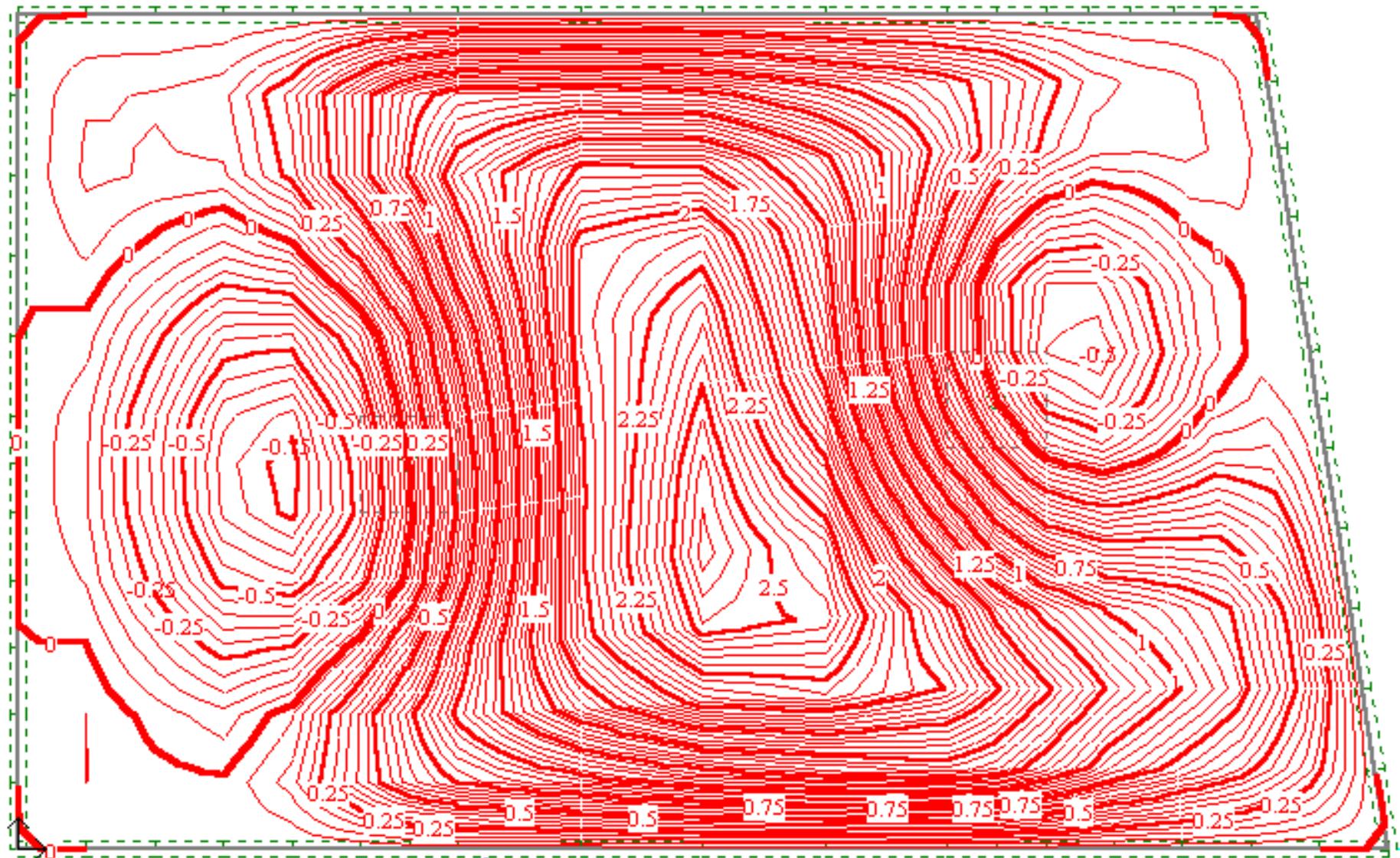
GLOBAL KN, m, C



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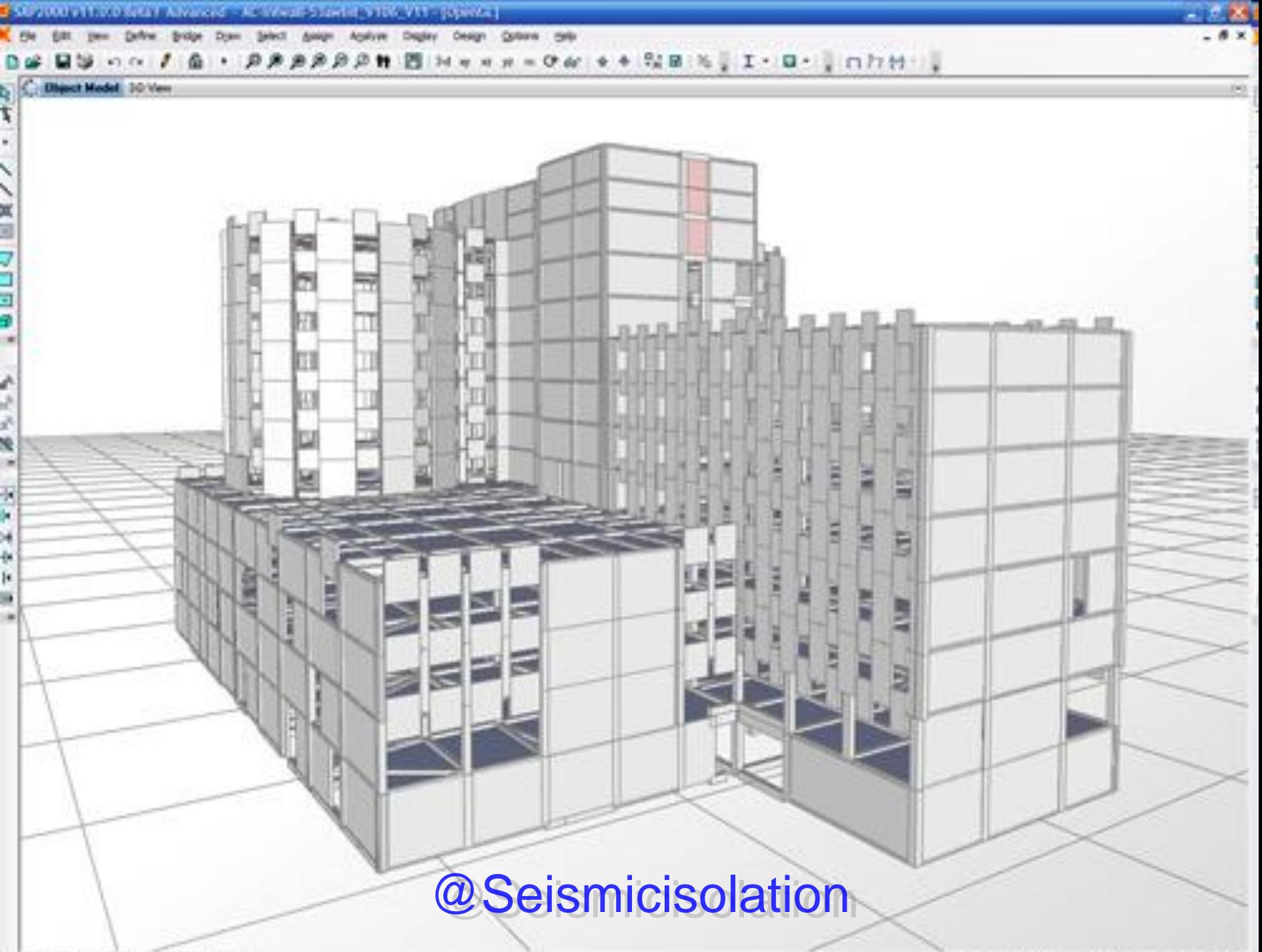


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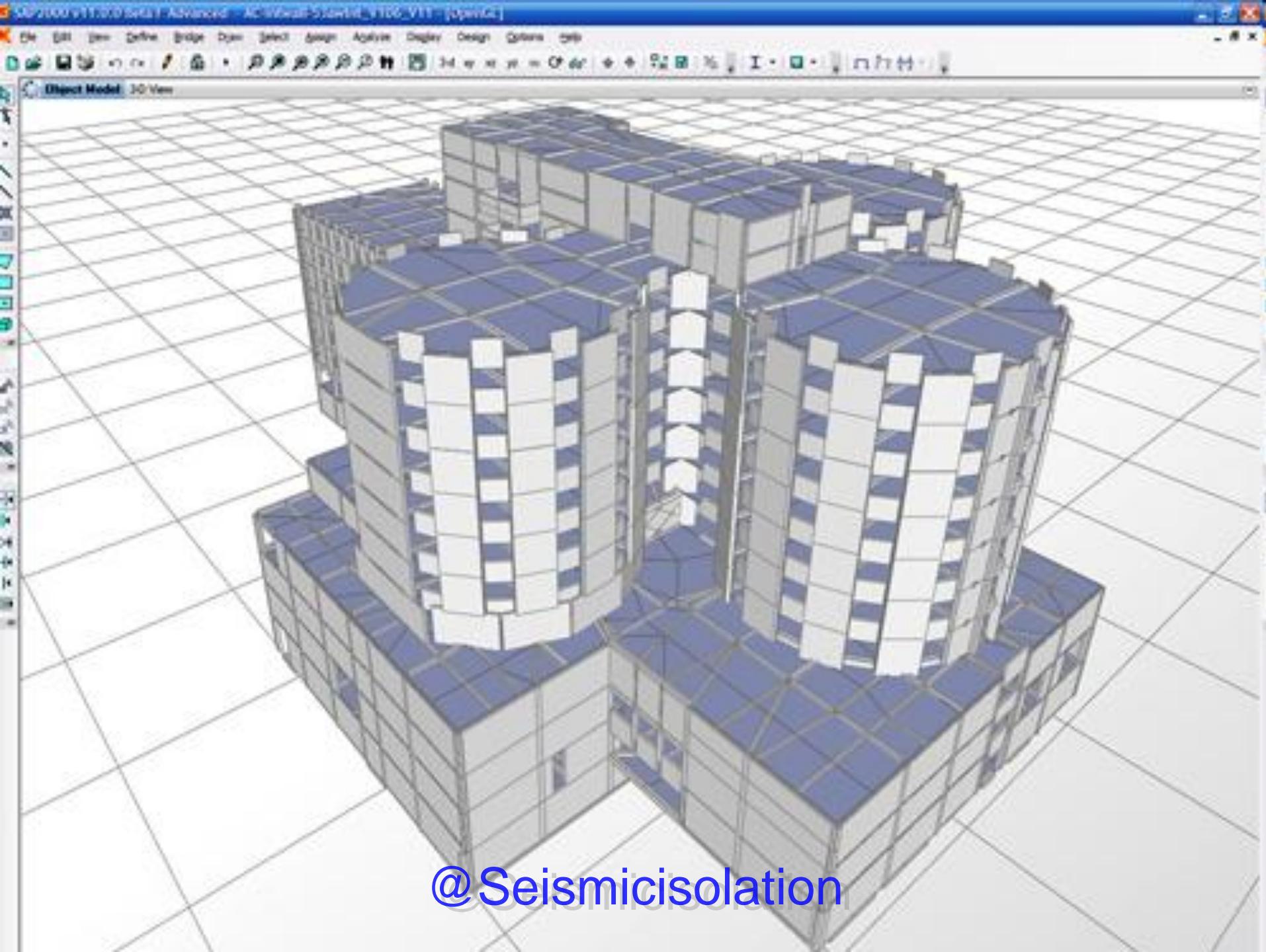
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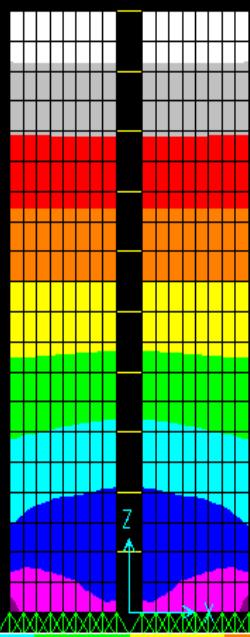
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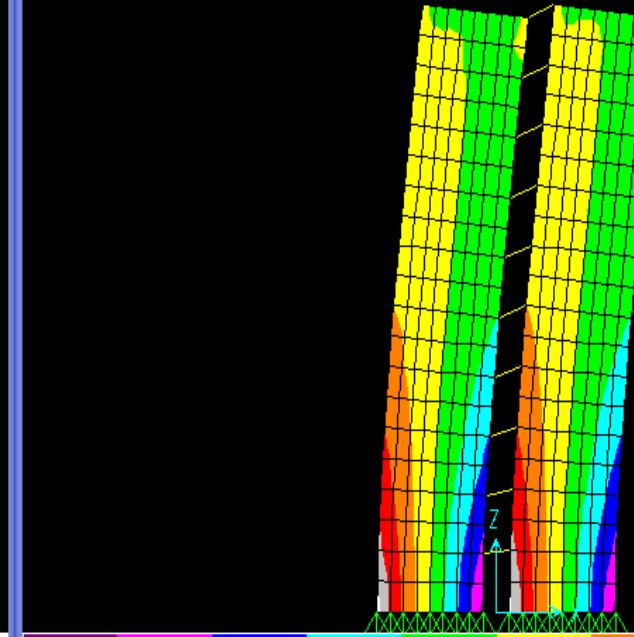


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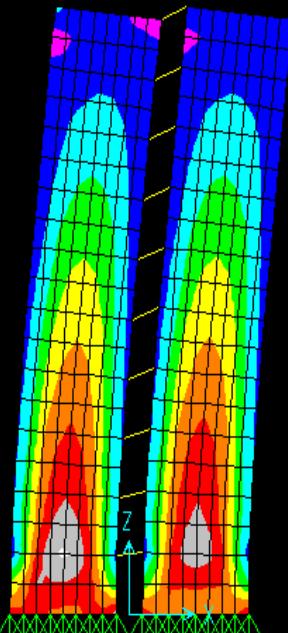
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Stress S12 Diagram (W)



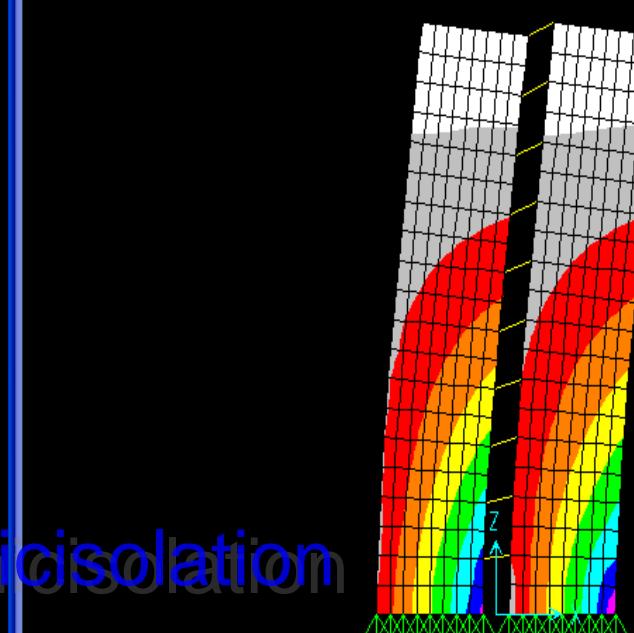
-160. -120. -80. -40. 0. 40. 80. 120. 160.

Stress S22 Diagram (COMB3)

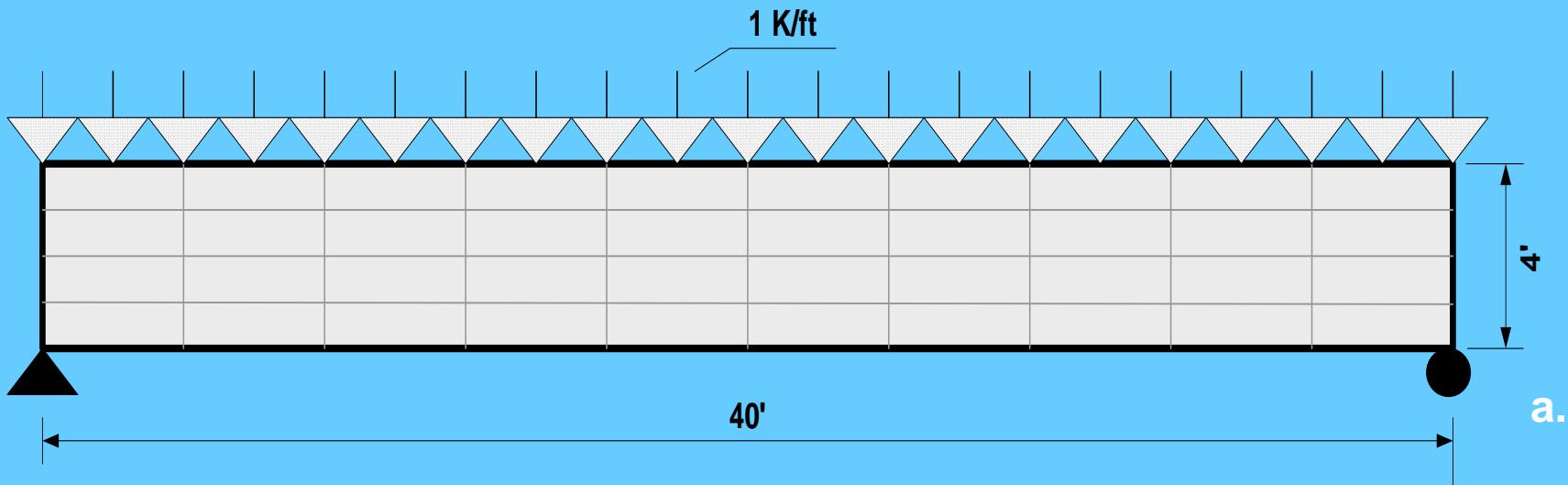


-2.2 0.0 2.2 4.4 6.6 8.8 11.0 13.2 15.4

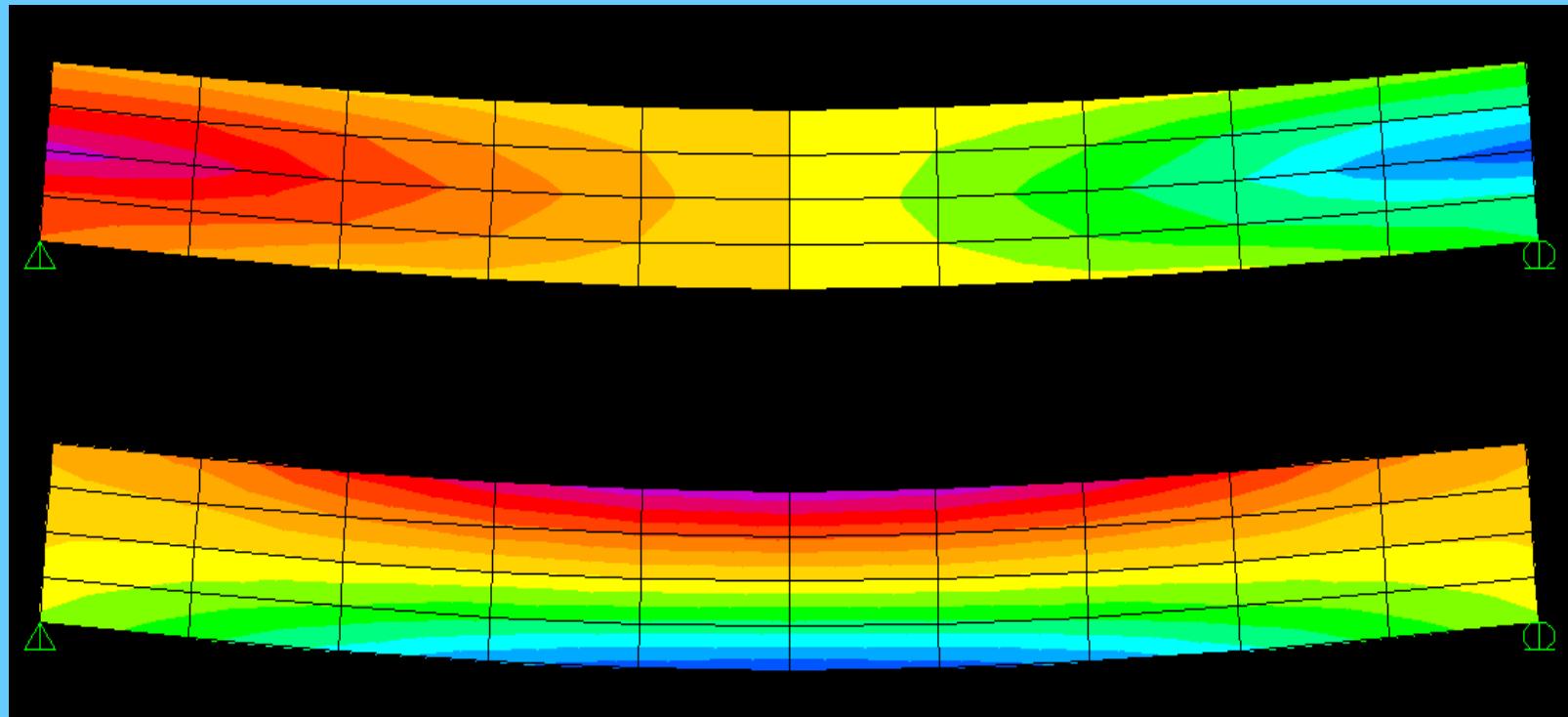
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-450. -400. -350. -300. -250. -200. -150. -100. -50.



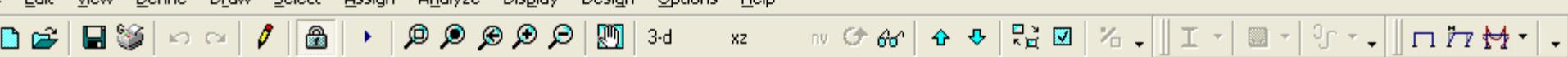
a.



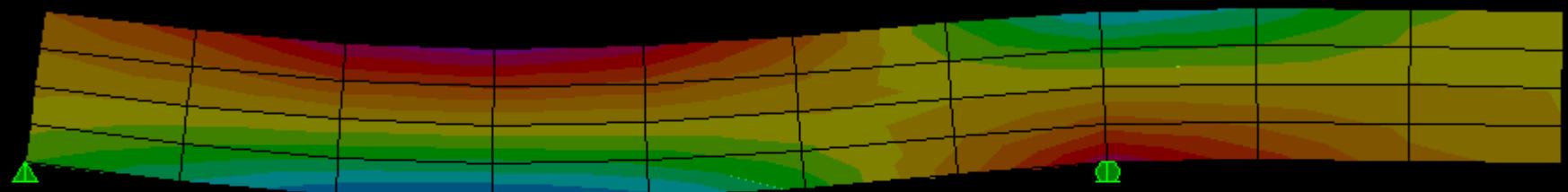
b.

c.

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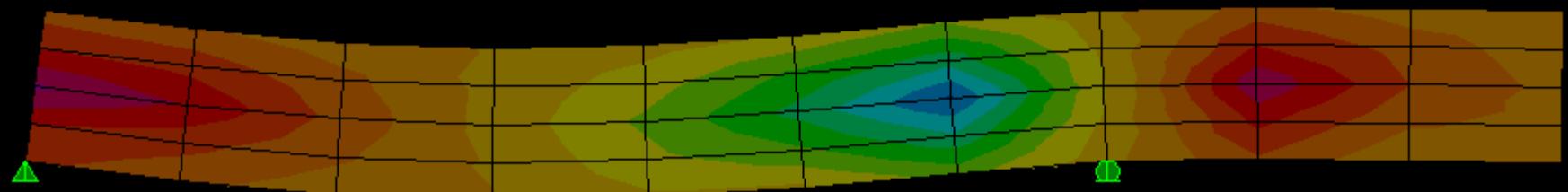


Stress S11 Diagram (DEAD)



-42.0 -35.0 -28.0 -21.0 -14.0 -7.0 0.0 7.0 14.0 21.0 28.0 35.0 42.0 49.0

Stress S12 Diagram (DEAD)



-7.5 -6.0 -4.5 -3.0 0.0 1.5 3.0 4.5 6.0 7.5 9.0 10.5 12.0

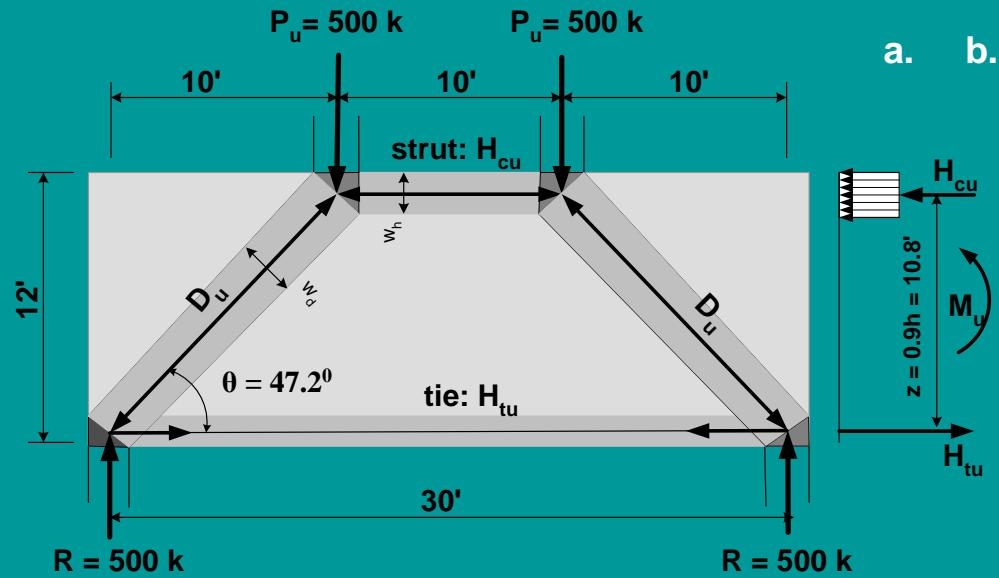
-8.856, MAX=12.251, Right Click on any Shell Element for detailed diagram

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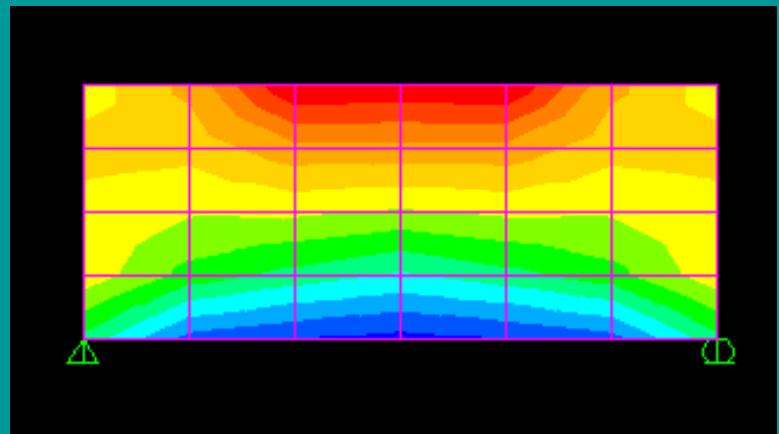
Start Animation

GLOBAL

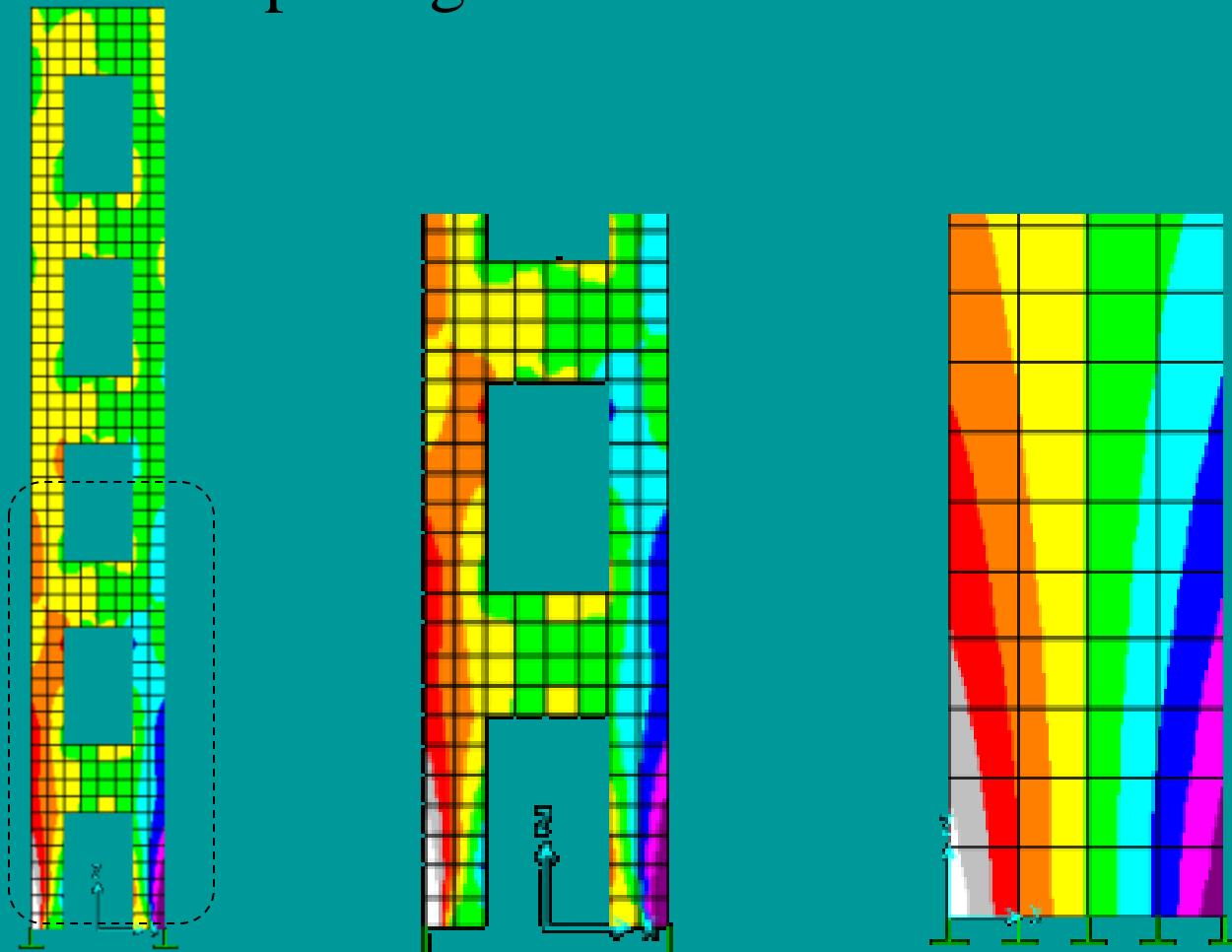
Kip, ft, F



a. b.



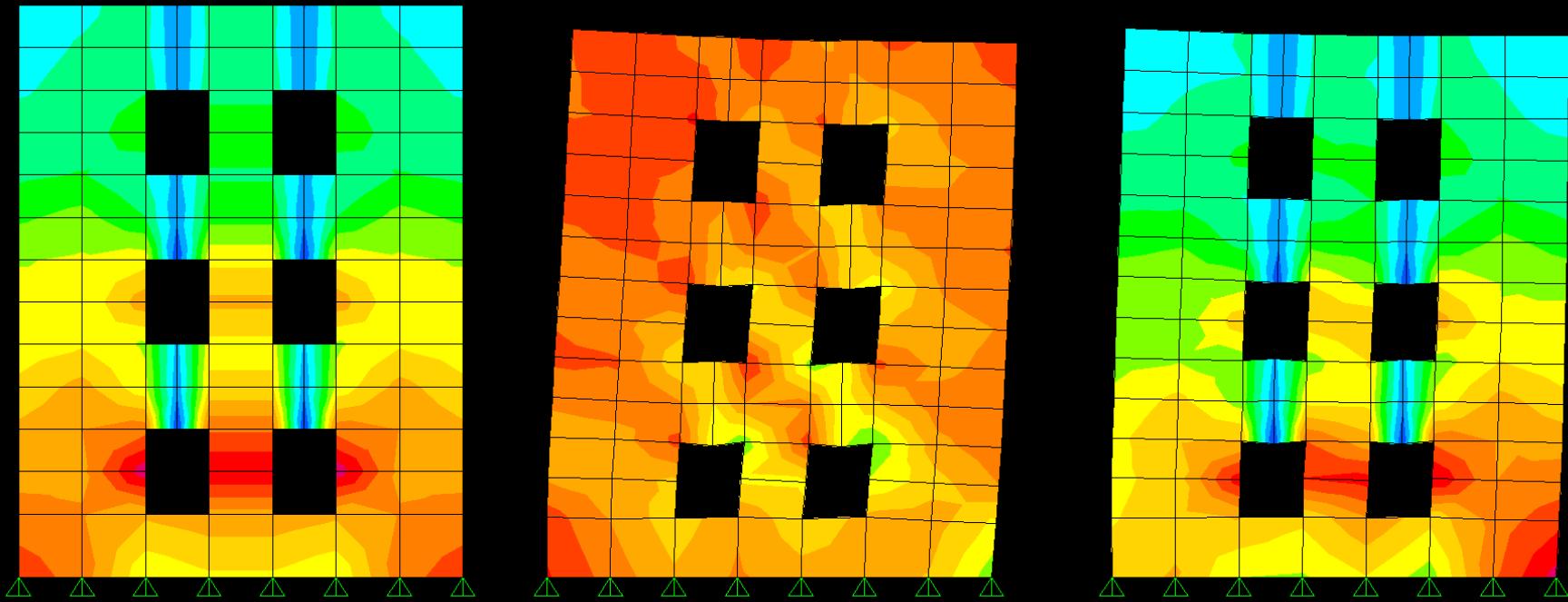
# Openings in Shear Walls - Planer



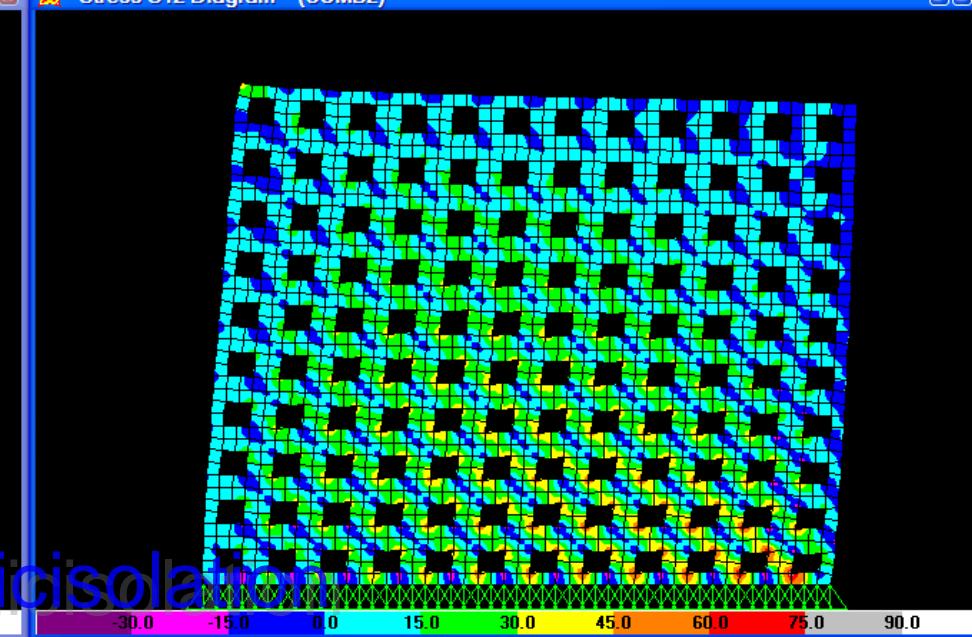
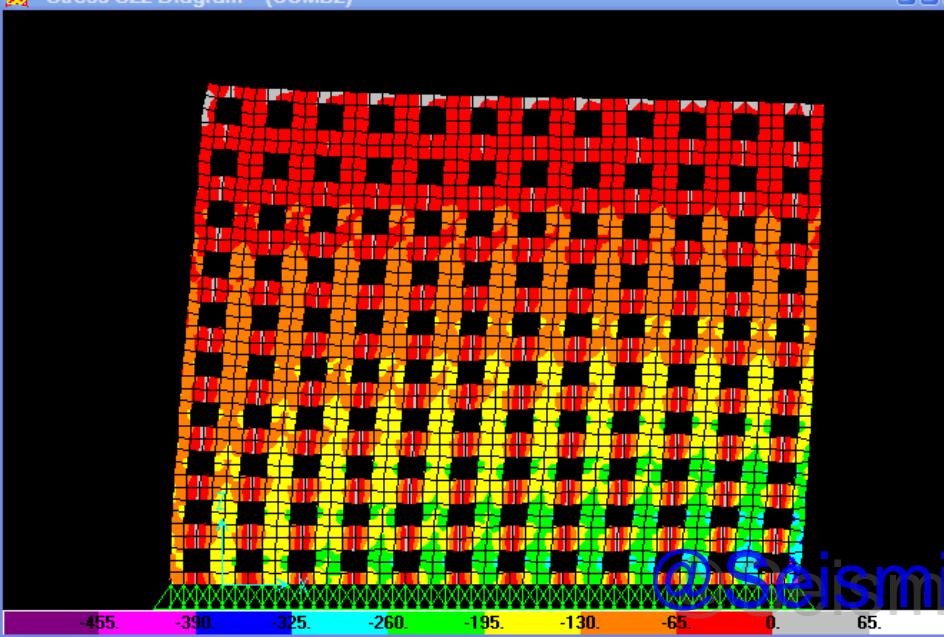
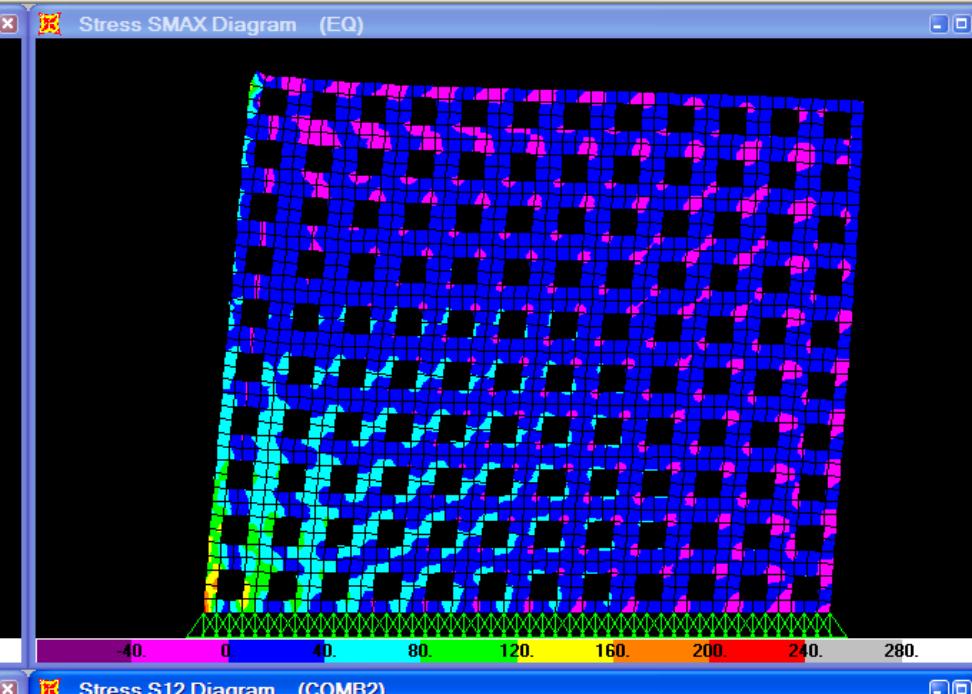
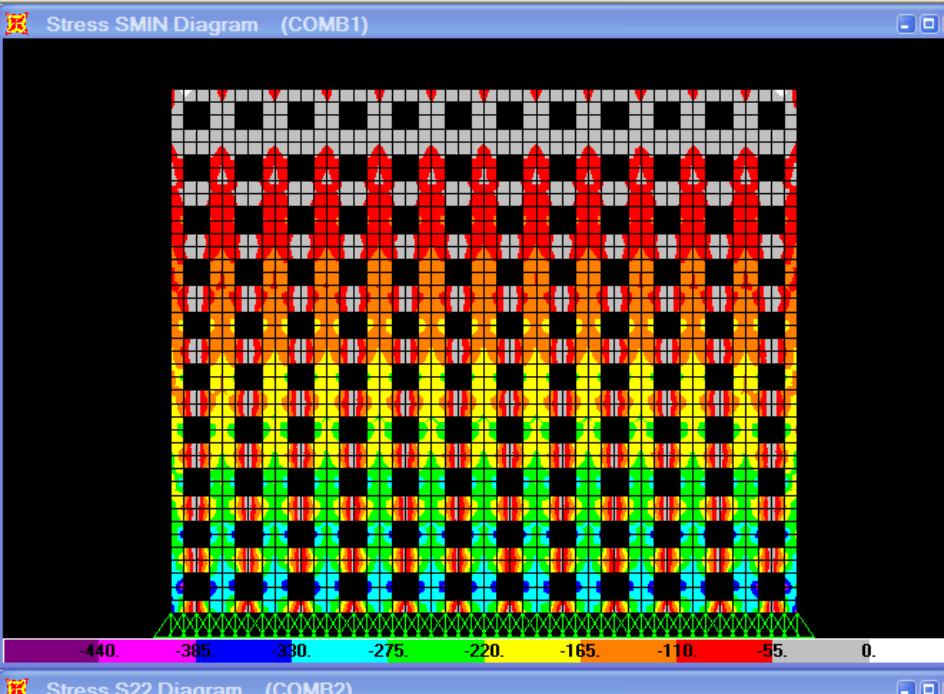
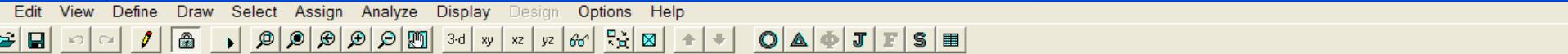
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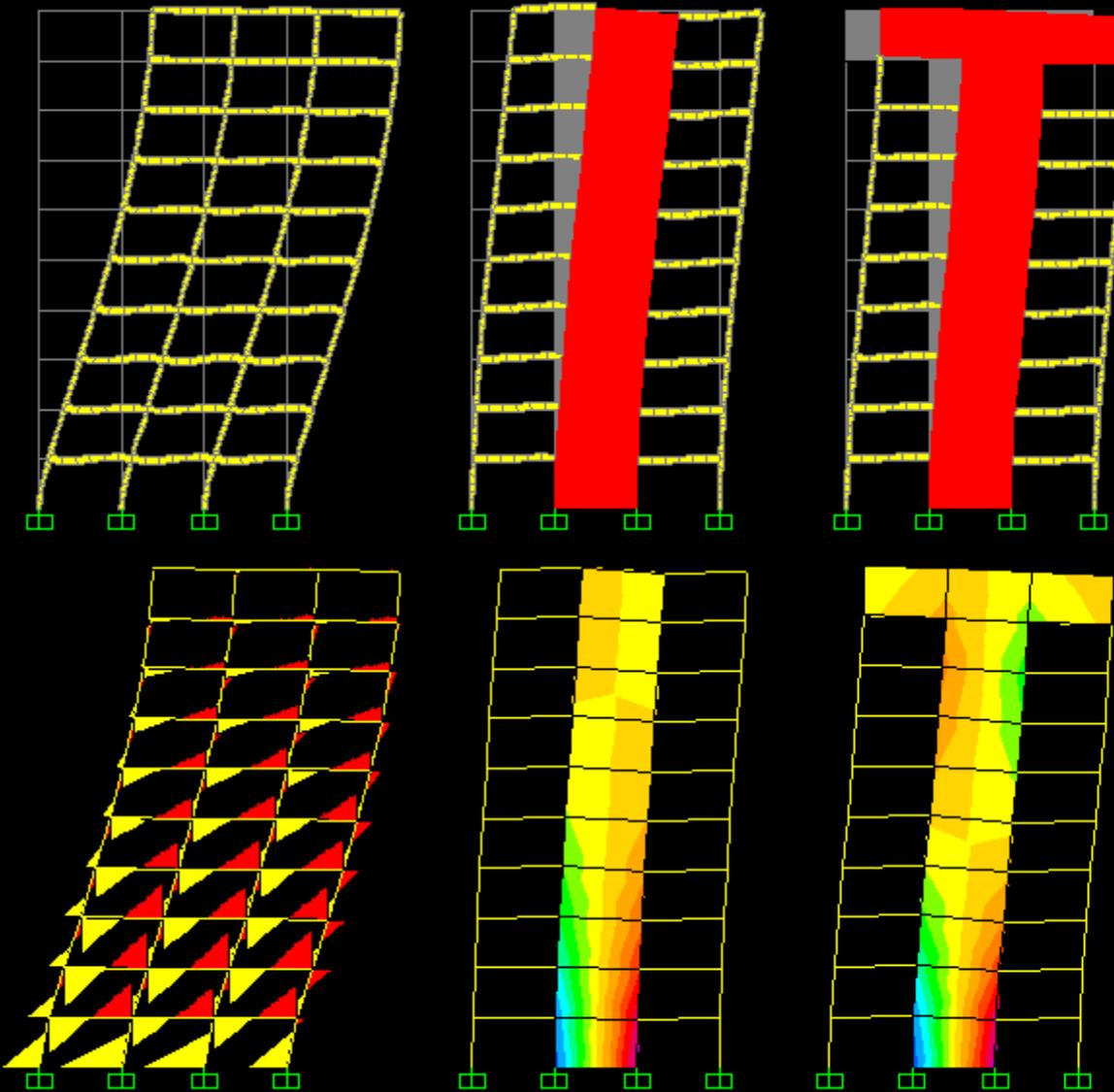


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Click on any Shell Element for detailed diagram

Start Animation lb-in



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@Seismicisolation Shanghai Grand Theatre, 1998, Jean-Marie Charpentier



24 9 2001

**Reichstag, Berlin, Germany,  
1999, Norman Foster Arch.  
Leonhardt & Andrae Struct.  
Eng**

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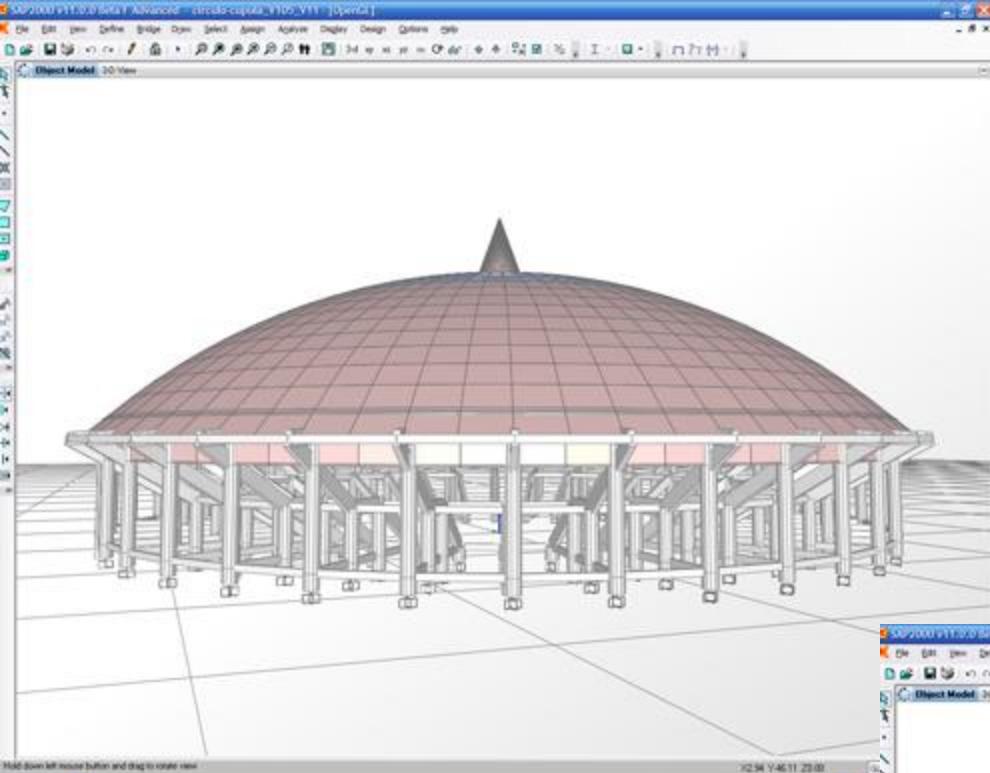
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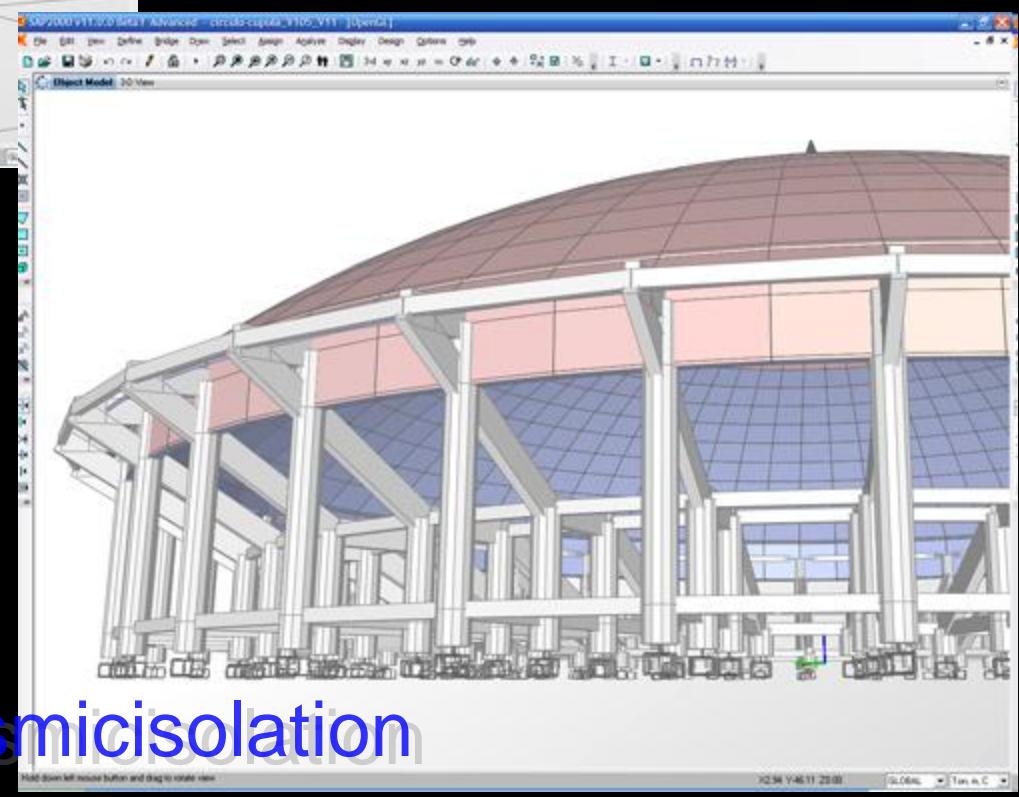


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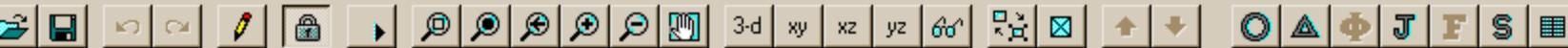
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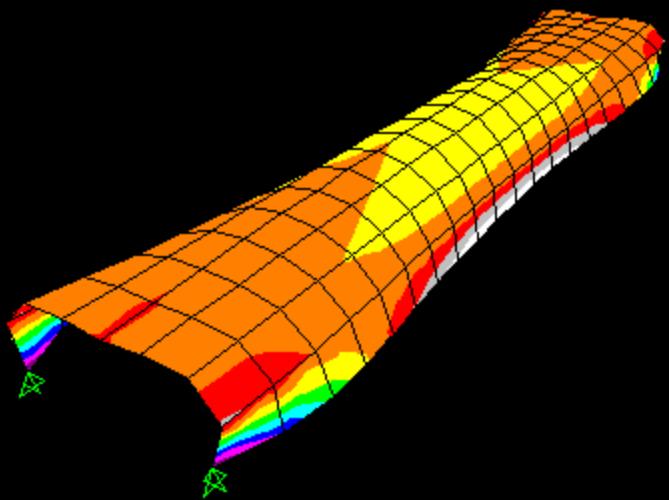
Hold down left mouse button and drag to rotate view



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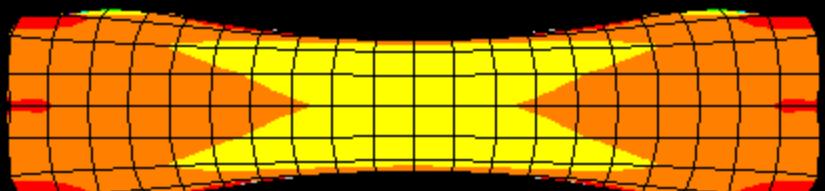


Resultant F11 Diagram (COMB1)



-66.0 -55.0 -44.0 -33.0 -22.0 -11.0 0.0 11.0 22.0

Resultant F11 Diagram (COMB1)



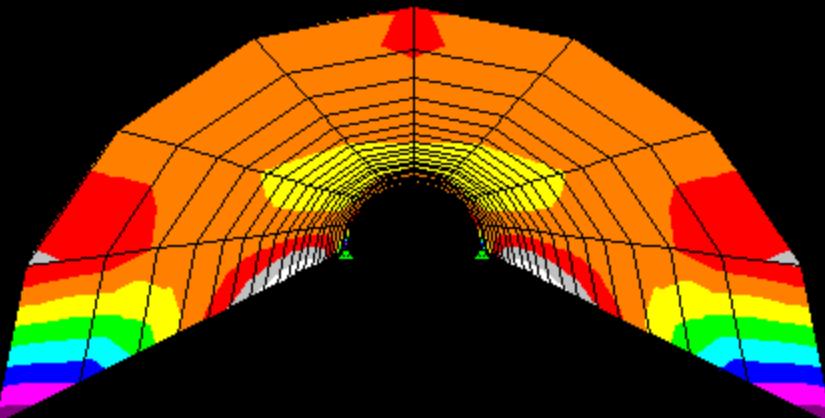
-66.0 -55.0 -44.0 -33.0 -22.0 -11.0 0.0 11.0 22.0

Resultant F11 Diagram (COMB1)



-66.0 -55.0 -44.0 -33.0 -22.0 -11.0 0.0 11.0 22.0

Resultant F11 Diagram (COMB1)



-66.0 -55.0 -44.0 -33.0 -22.0 -11.0 0.0 11.0 22.0

Click on any Shell Element for detailed diagram

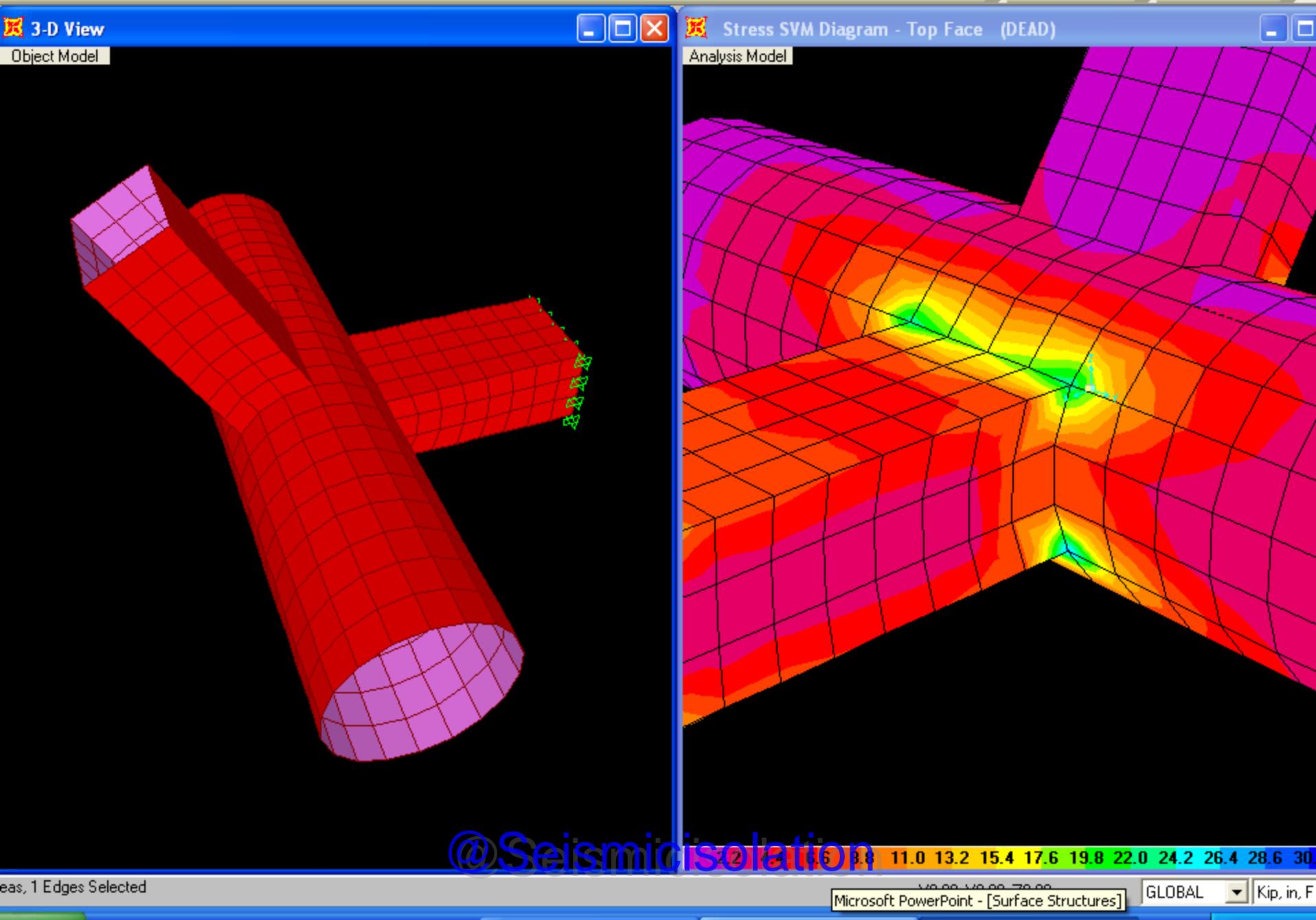
@Seismicisolation

Start Animation

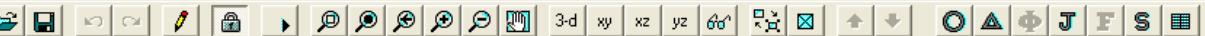


Kip-It

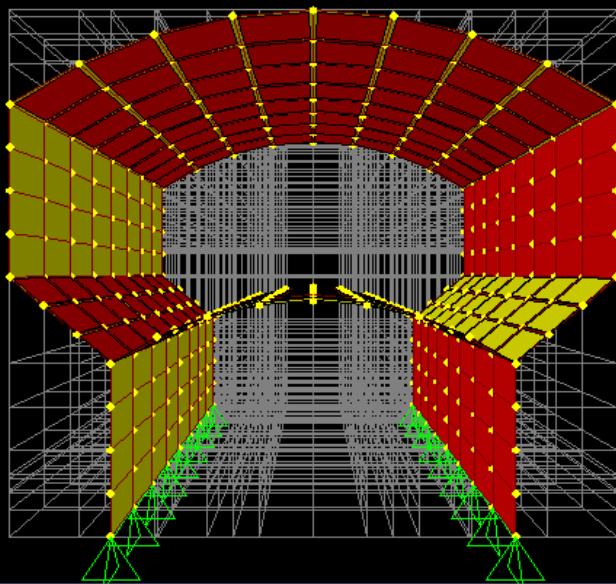
Edit View Define BrIM Draw Select Assign Analyze Display Design Options Help



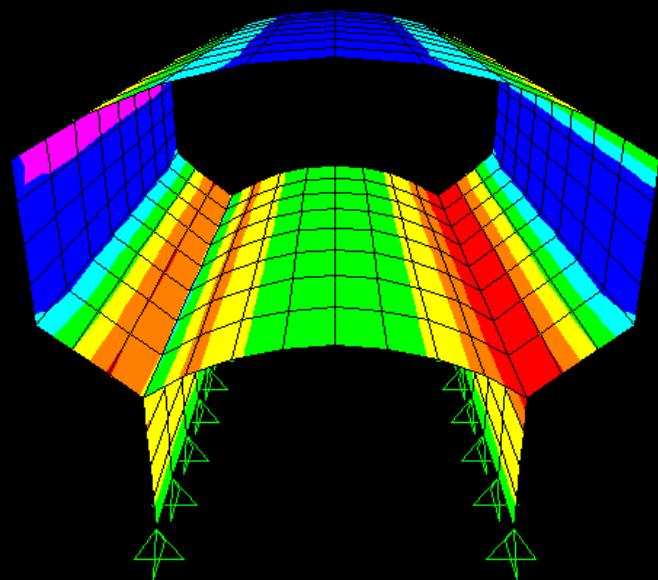
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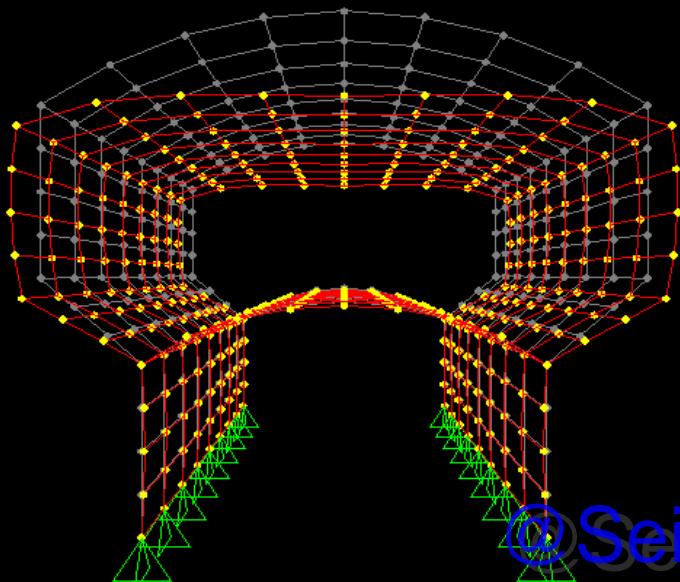
Shell Uniform GLOBAL-Z (LL)



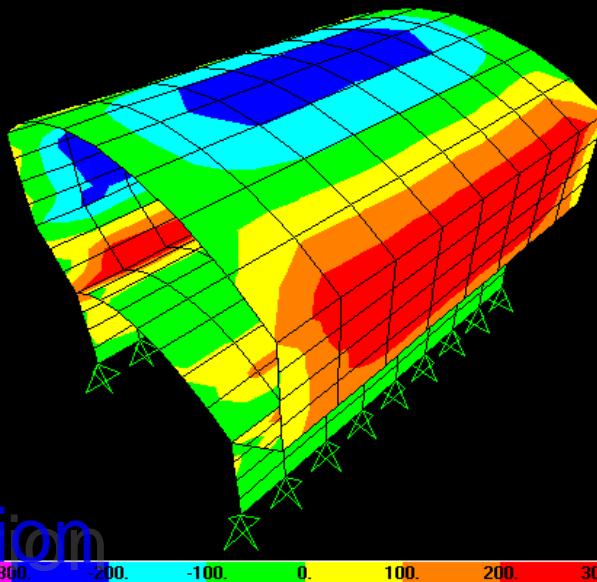
Stress S22 Diagram (COMB1)



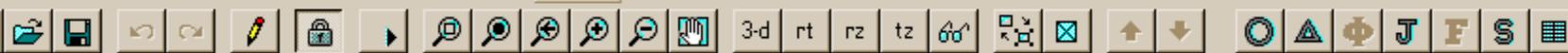
Deformed Shape (COMB1)



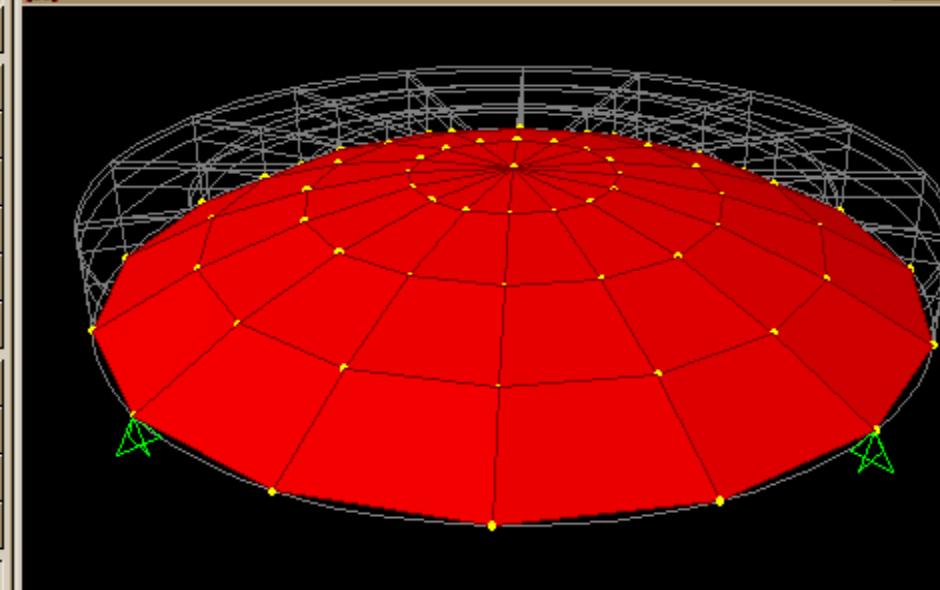
Stress S11 Diagram (COMB1)



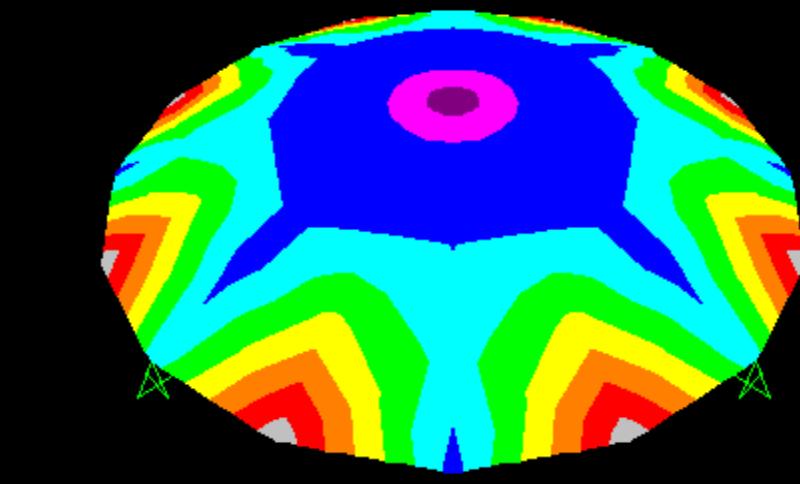
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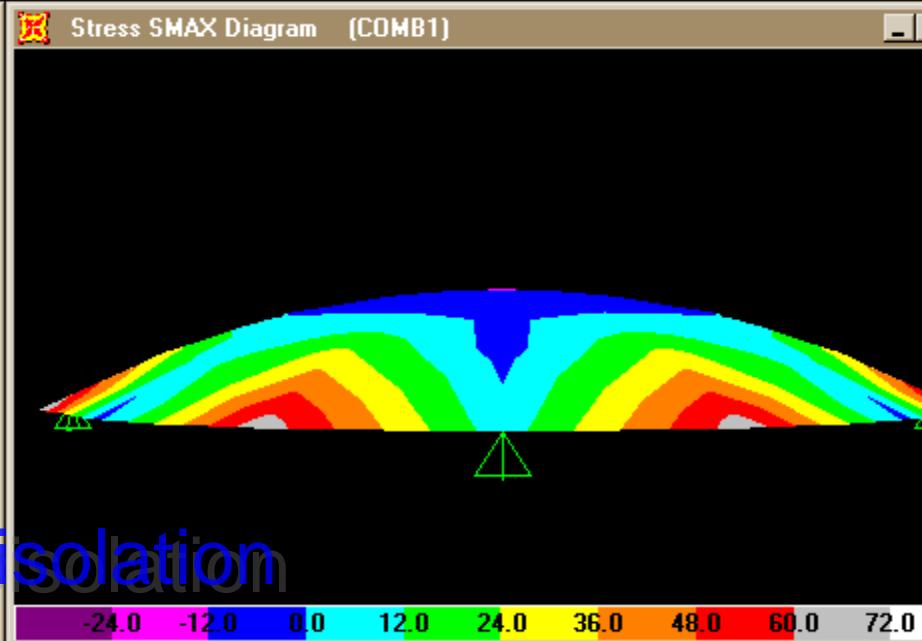
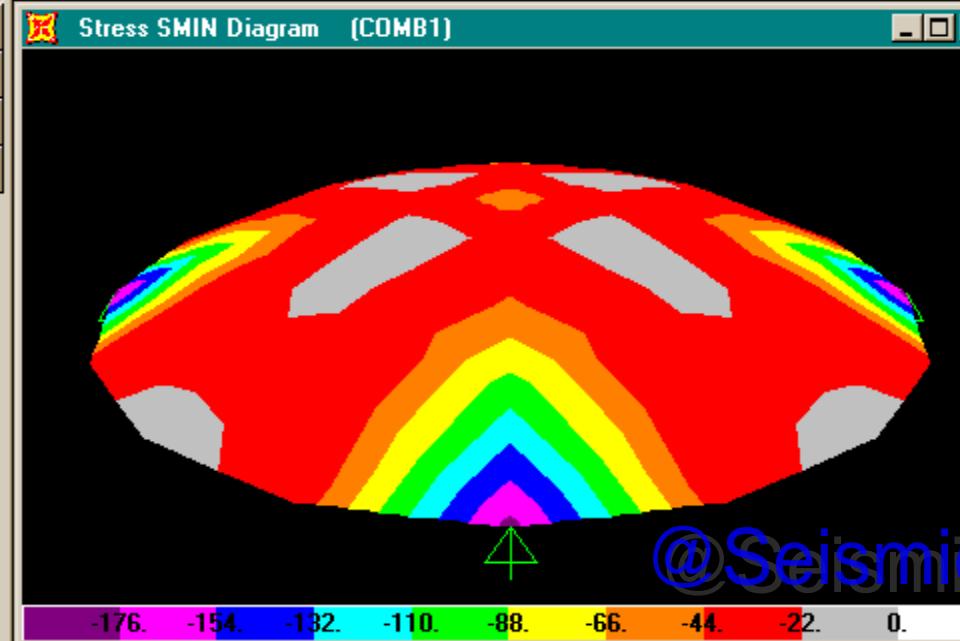
3-D View



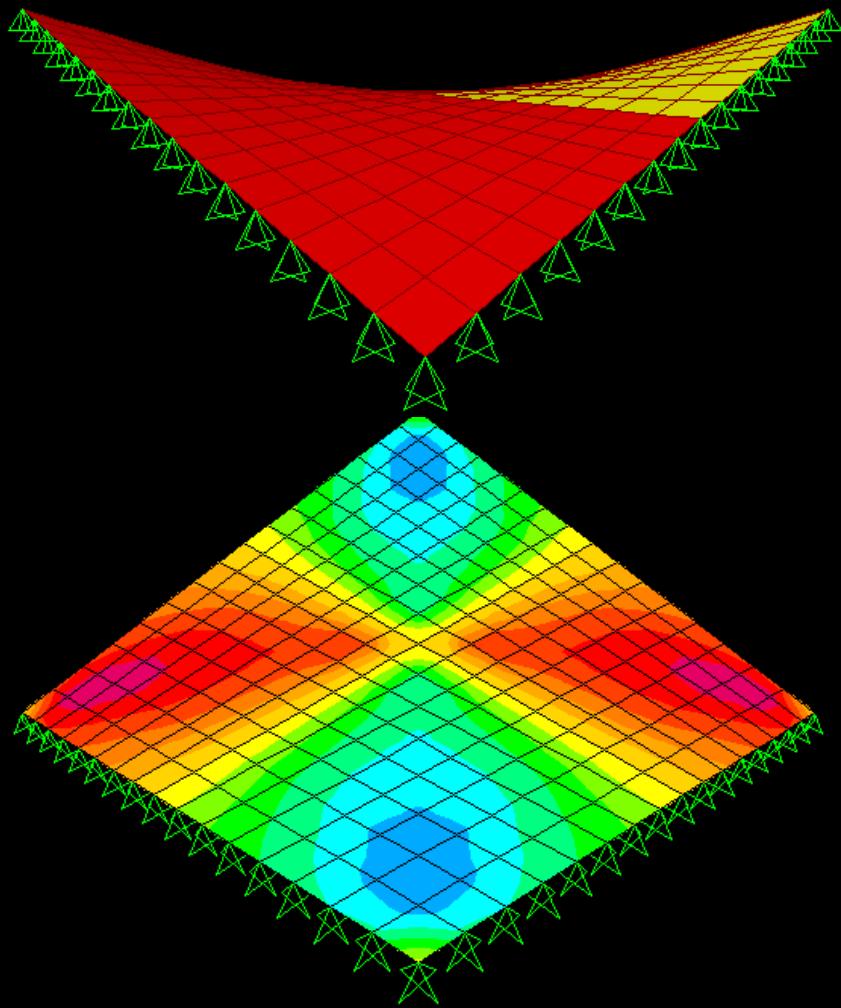
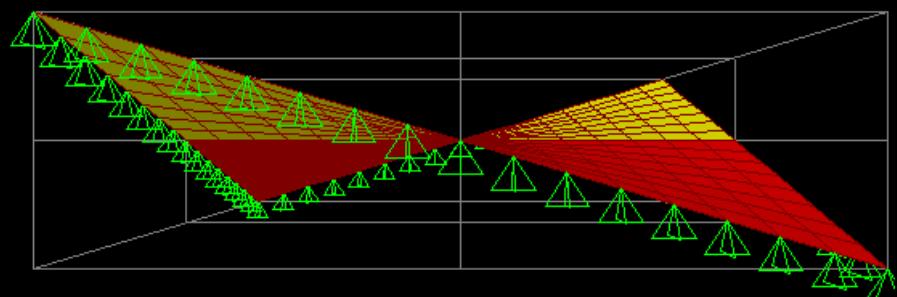
Stress SMAX Diagram (COMB1)



Stress SMIN Diagram (COMB1)



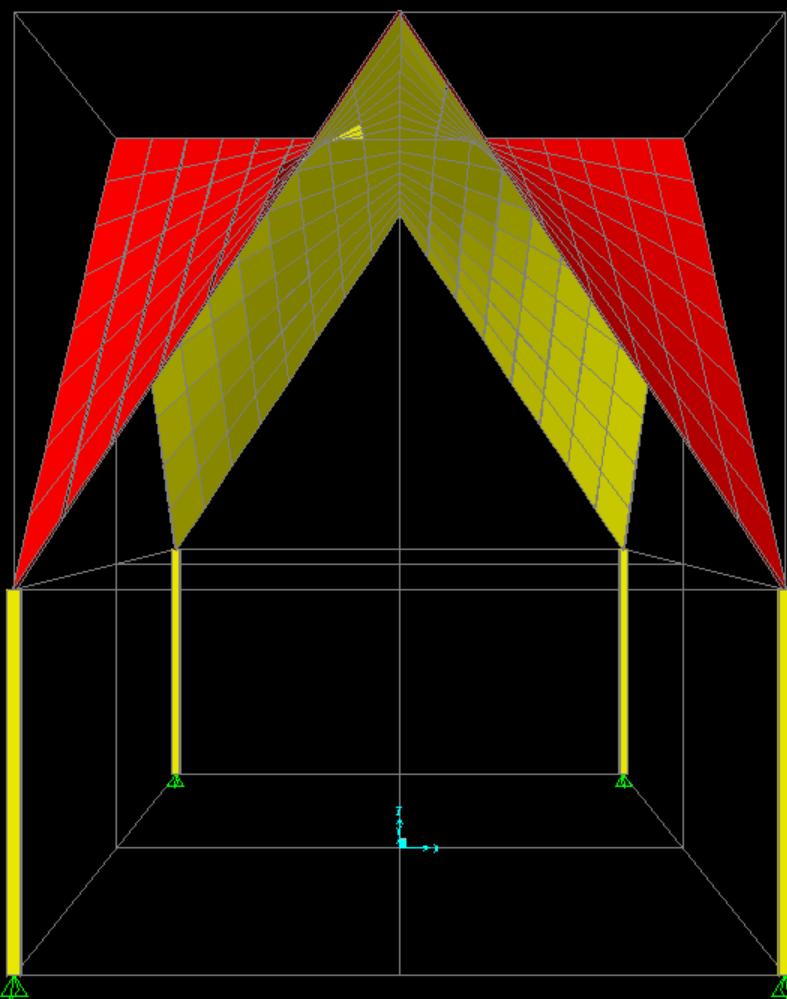
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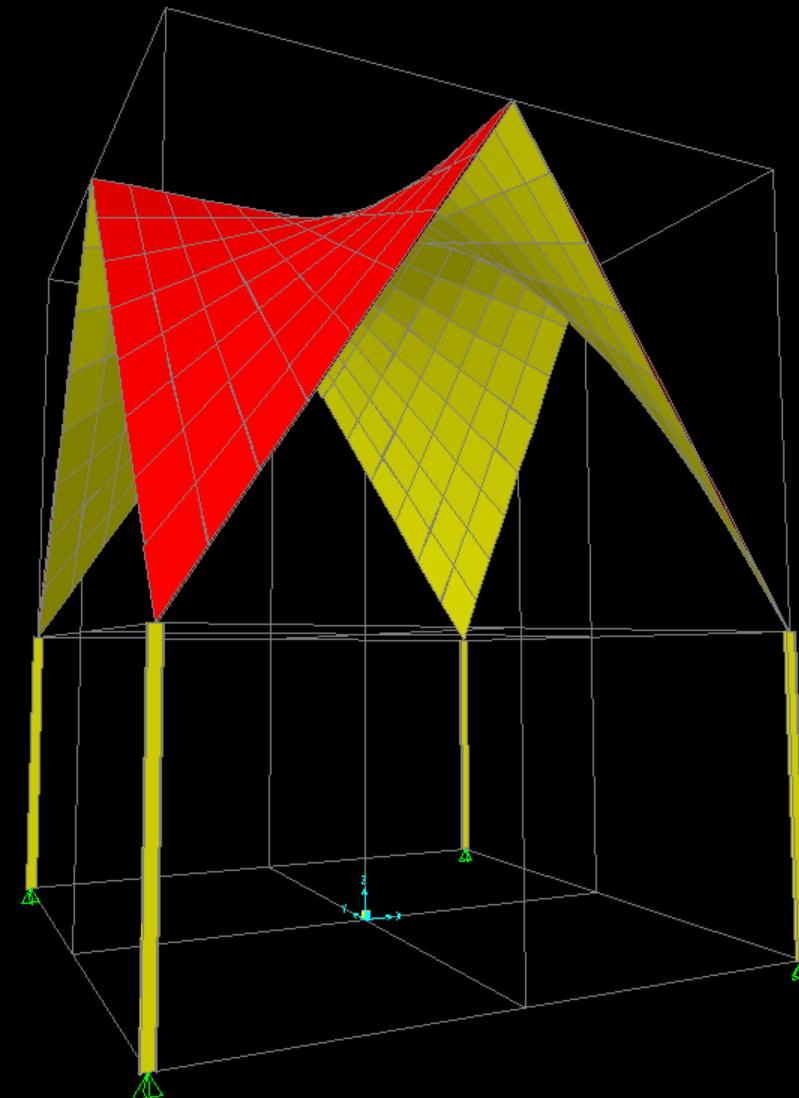
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3-D View



3-D View

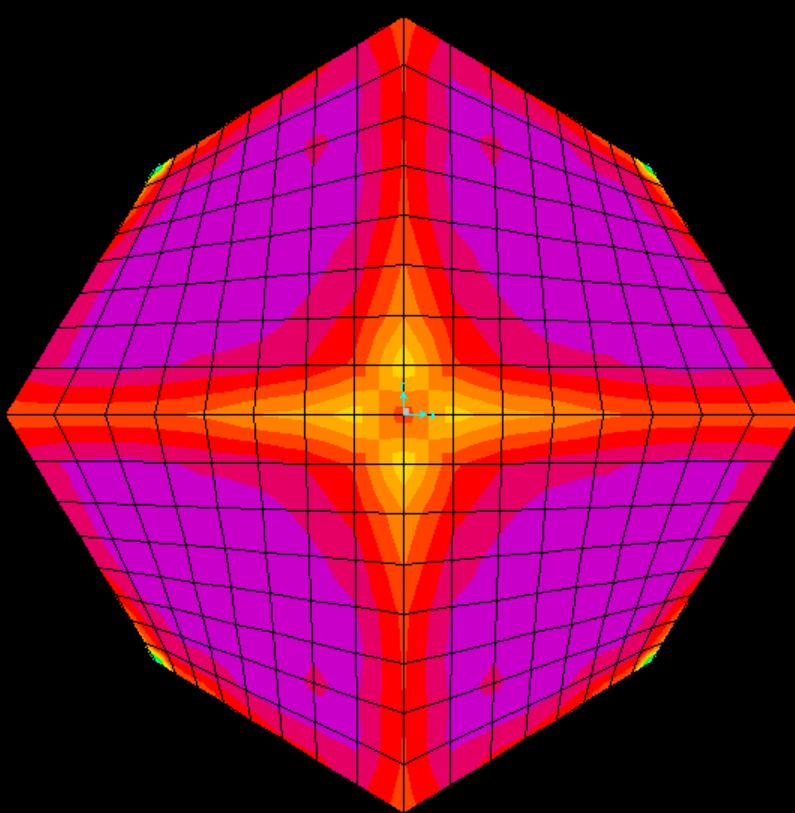


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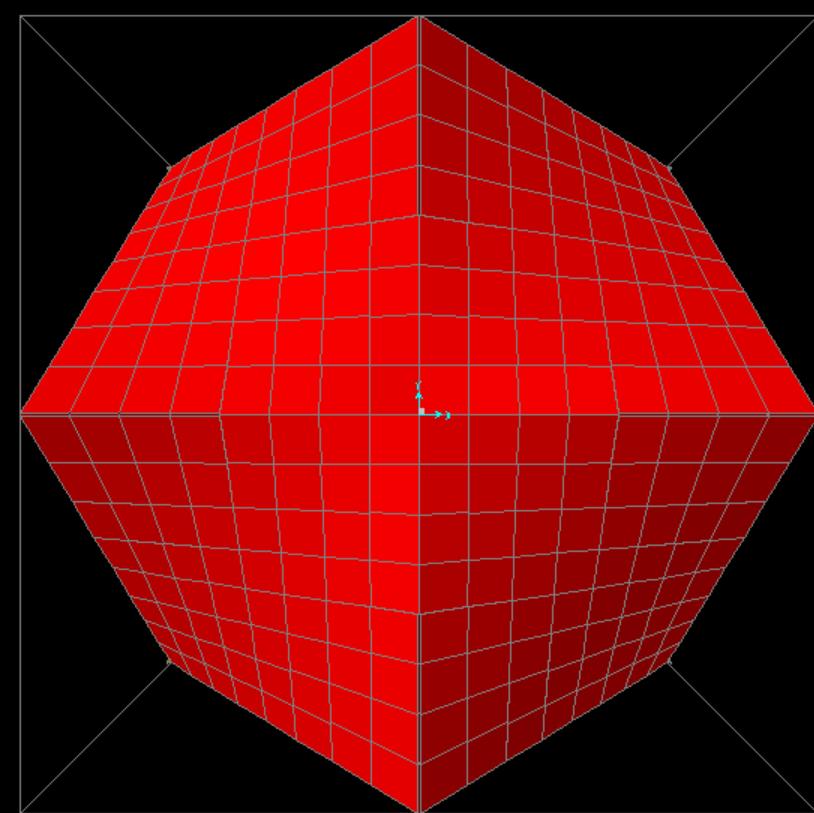
File Edit View Define Bridge Draw Select Assign Analyze Display Design Options Help



Stress SVM Diagram (DEAD)



3-D View



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**DG Bank, Berlin,  
Germany, 2001, Frank  
Gehry; Schlaich,  
Bergemann und Partners**

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25.9.2001

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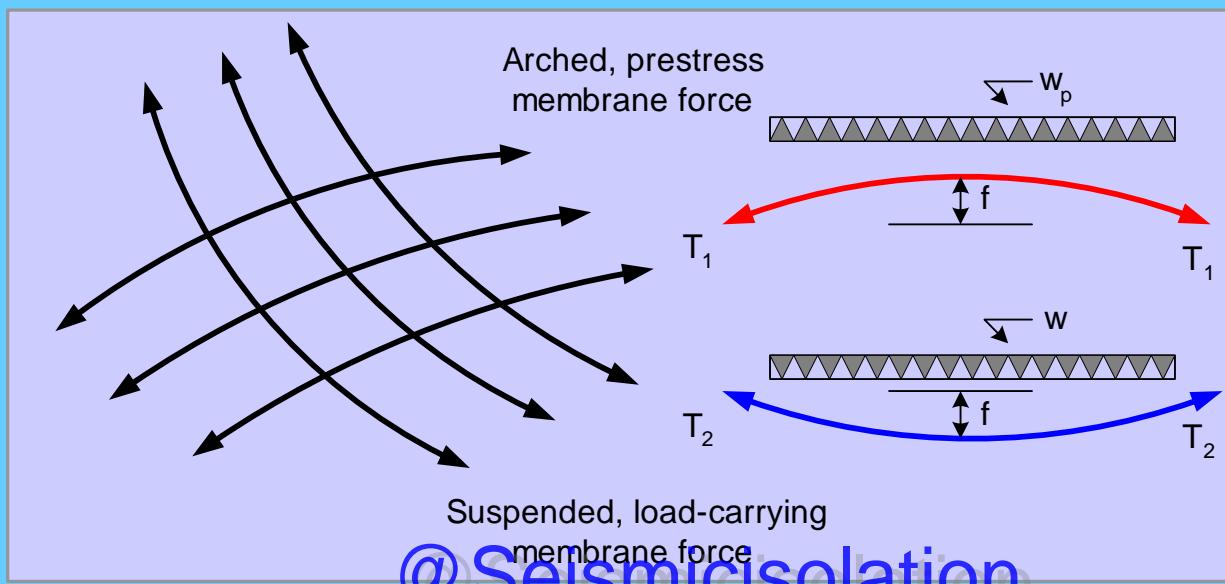
## TENSILE SURFACES



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# STRUCTURAL BEHAVIOR

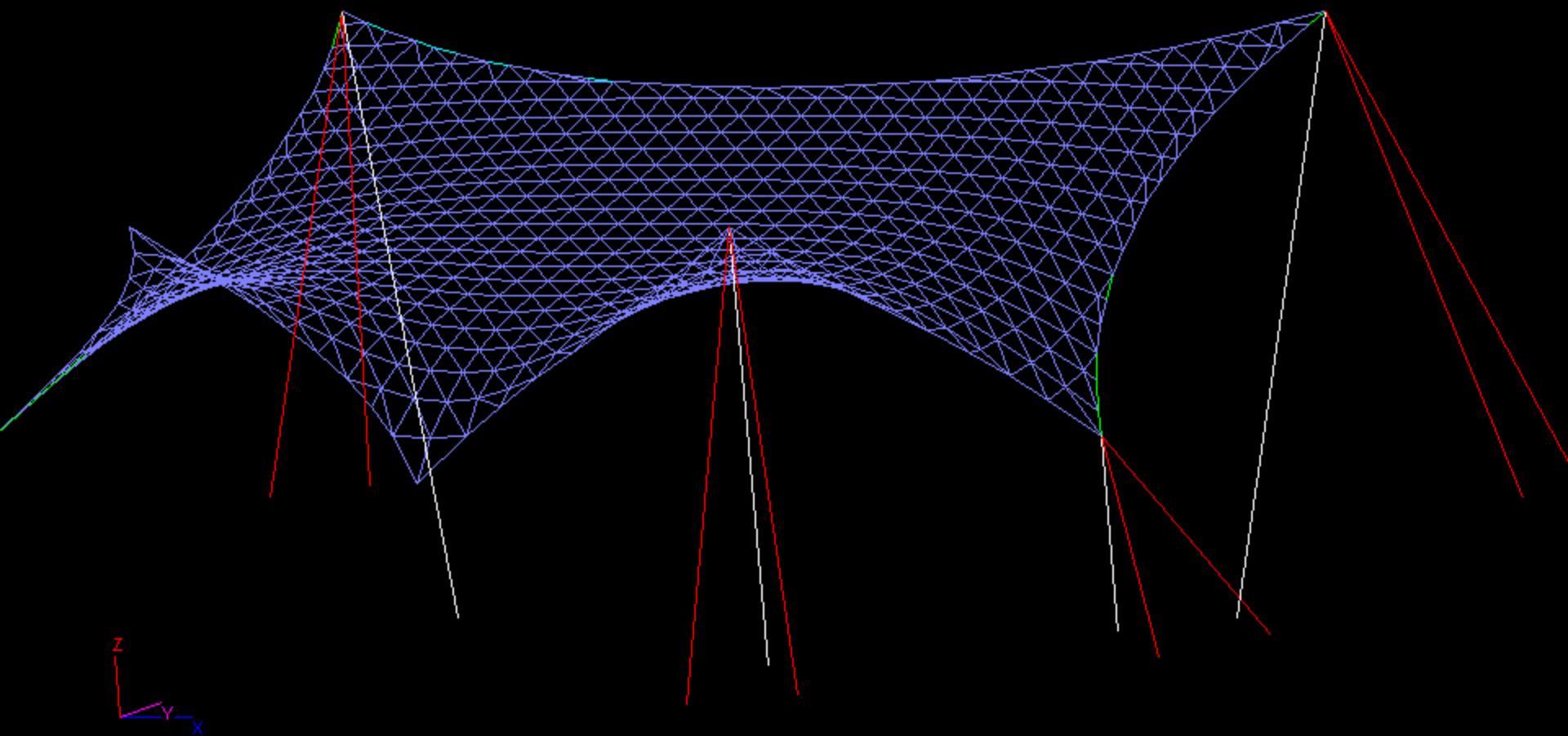
Soft membranes must adjust their shape (because they are flexible) to the loading so that they can respond in tension. The membrane surface must have double curvature of anticlastic geometry to be stable. The basic shape is defined mathematically as a **hyperbolic paraboloid**. In cable-nets under gravity loads, the main (convex, suspended, lower load bearing) cable is prevented from moving by the secondary (concave, arched, upper, bracing, etc.) cable, which is prestressed and pulls the suspended layer down, thus stabilizing it. Visualize the initial surface tension analogous to the one caused by internal air pressure in pneumatic structures.



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standard View

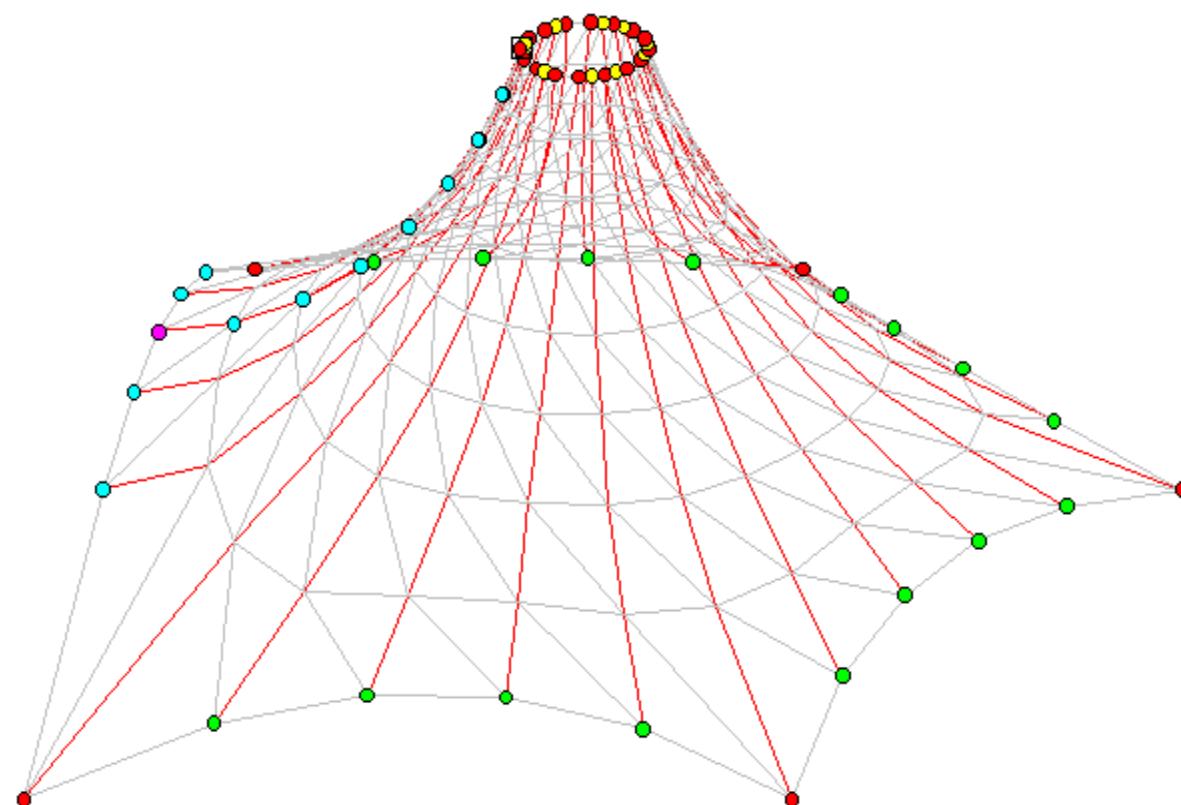


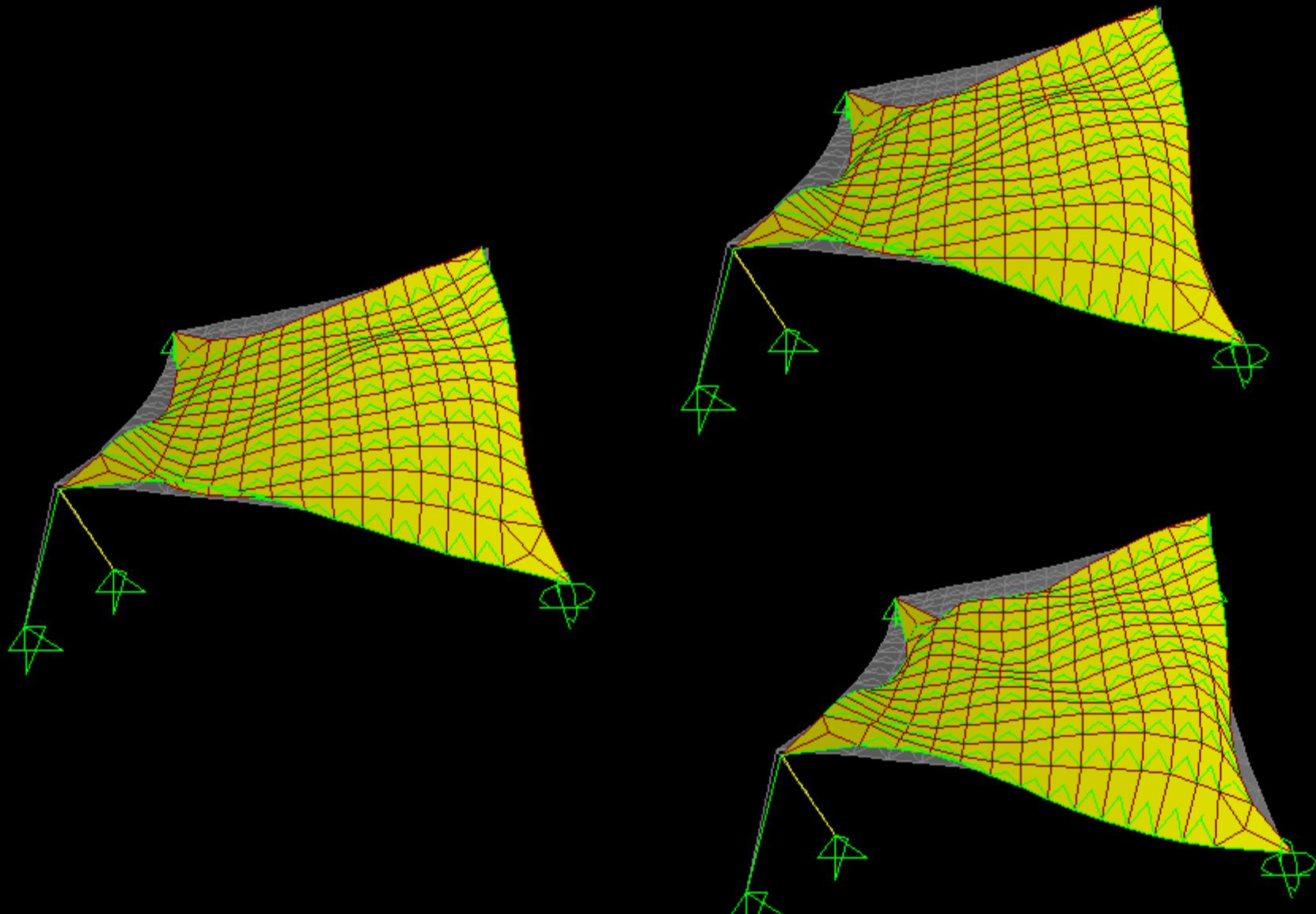
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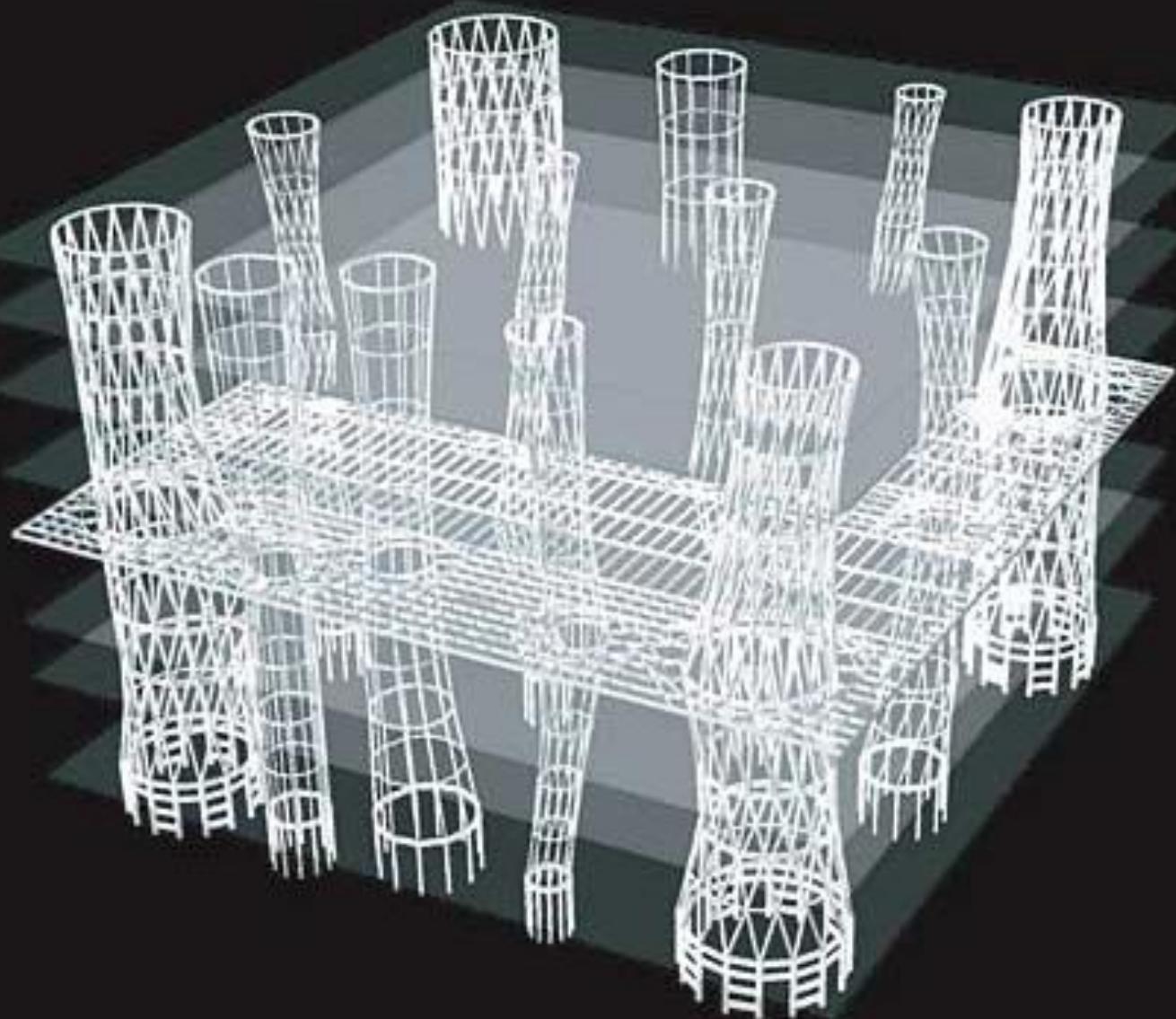


# THE NEW GEOMETRY

Daniel Libeskind BERLIN CITY EDGE  
© Seismicisolation



Sendai Mediatheque, Sendai City, Japan, 2001, Tadao Ito + Mutsuro Sasaki



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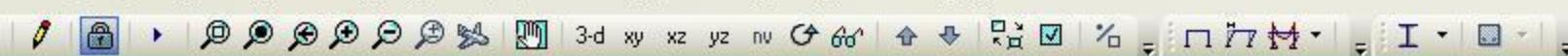
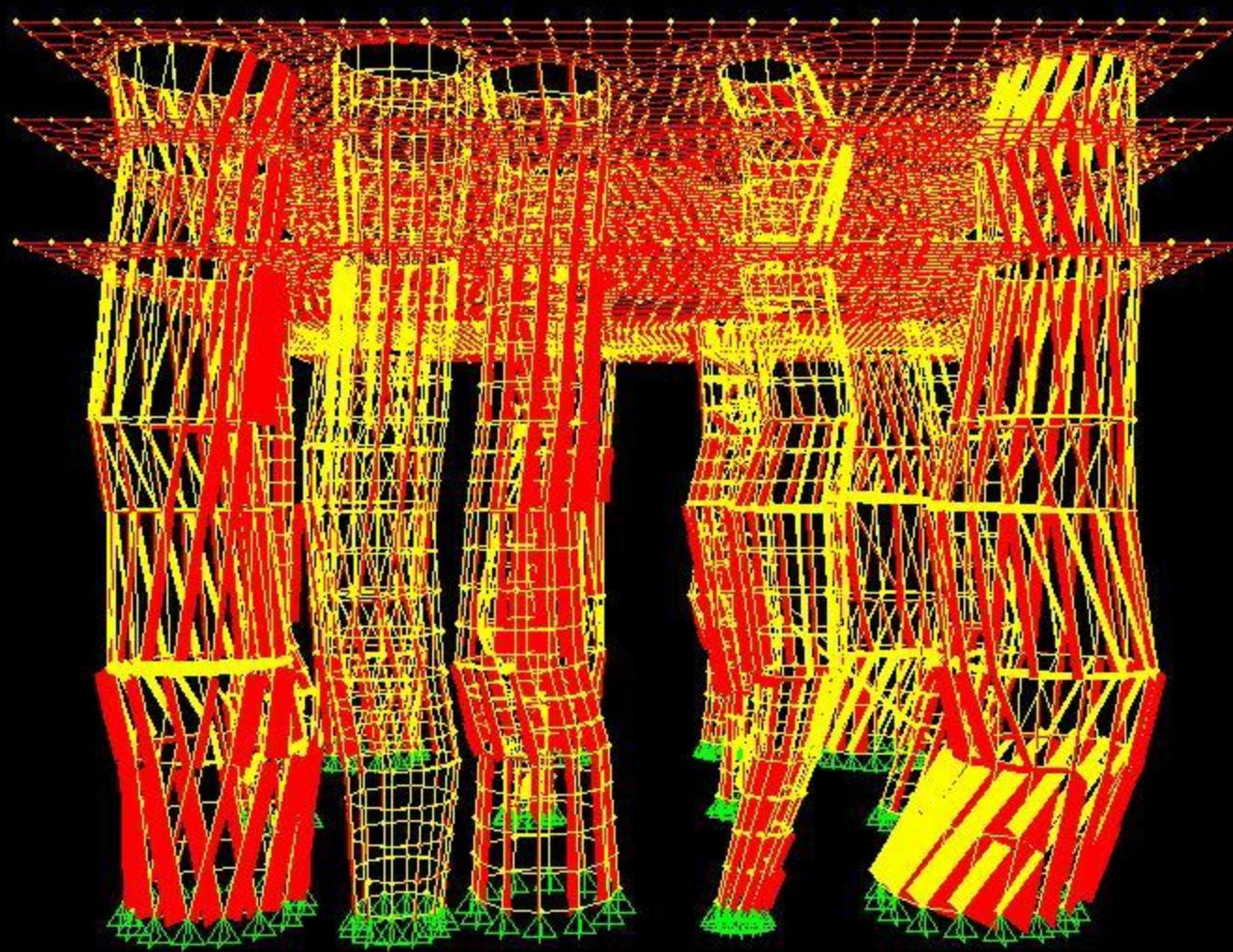
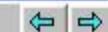


Diagram (G)



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ent for detailed diagram



ine Bridge Draw Select Assign Analyze Display Design Options Help

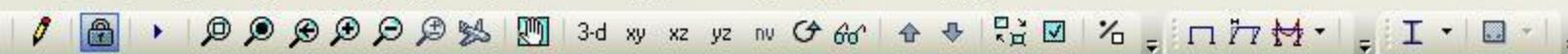


Diagram (G)

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Line Bridge Draw Select Assign Analyze Display Design Options Help

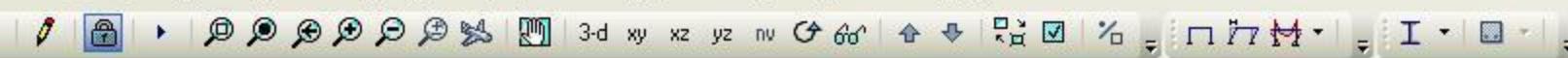
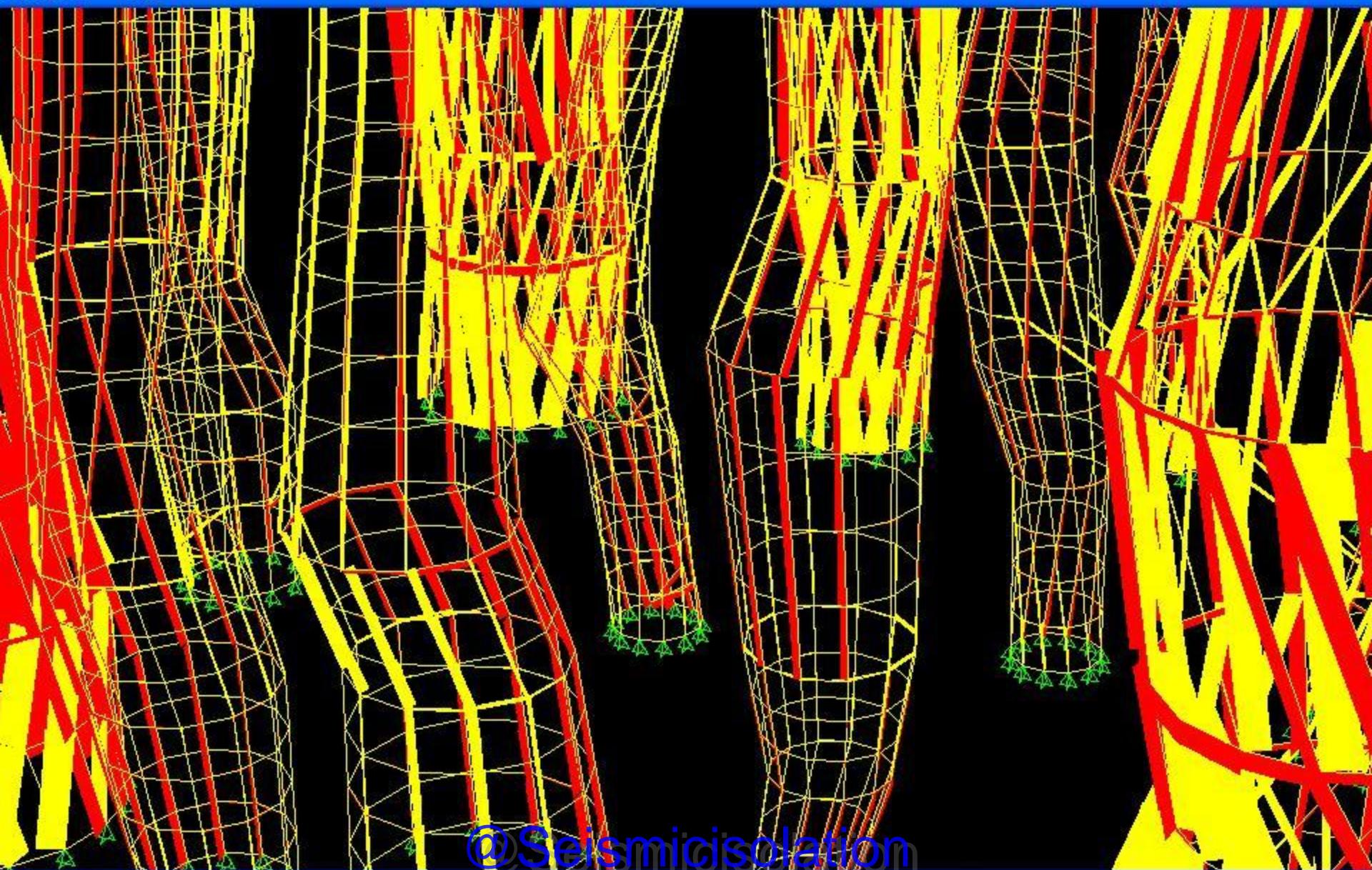
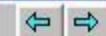


Diagram (W)

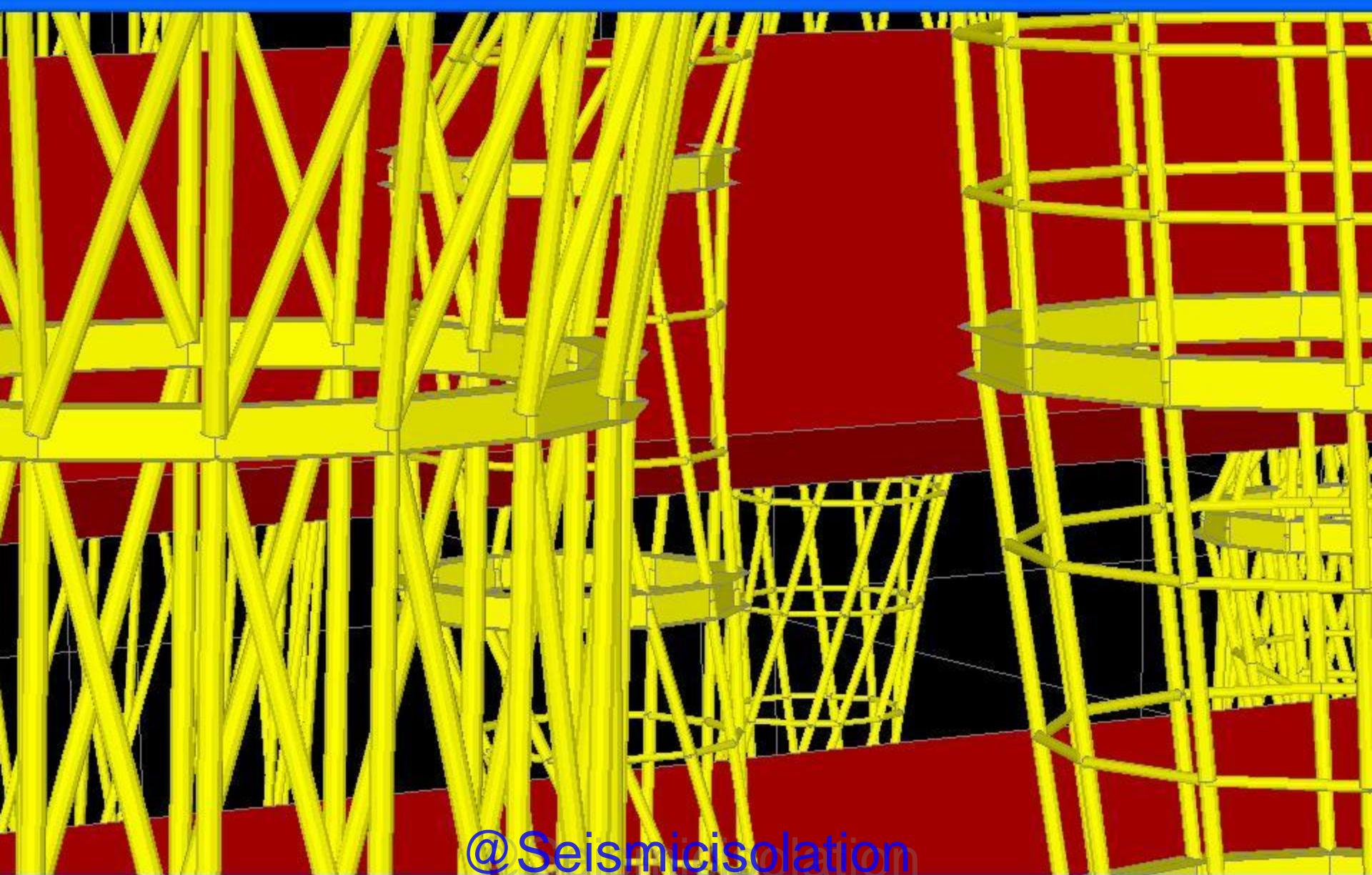
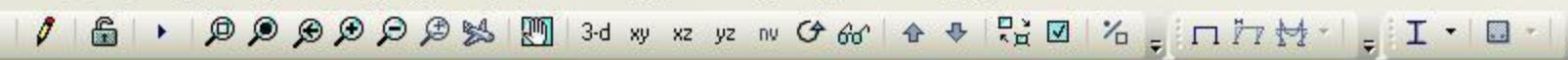


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Get detailed diagram



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X0.00 Y0.00 Z0.00

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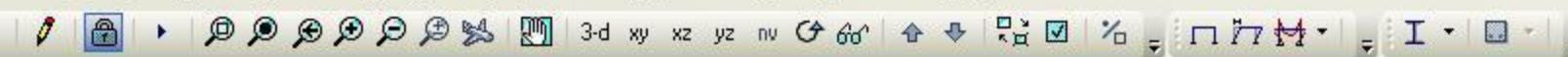
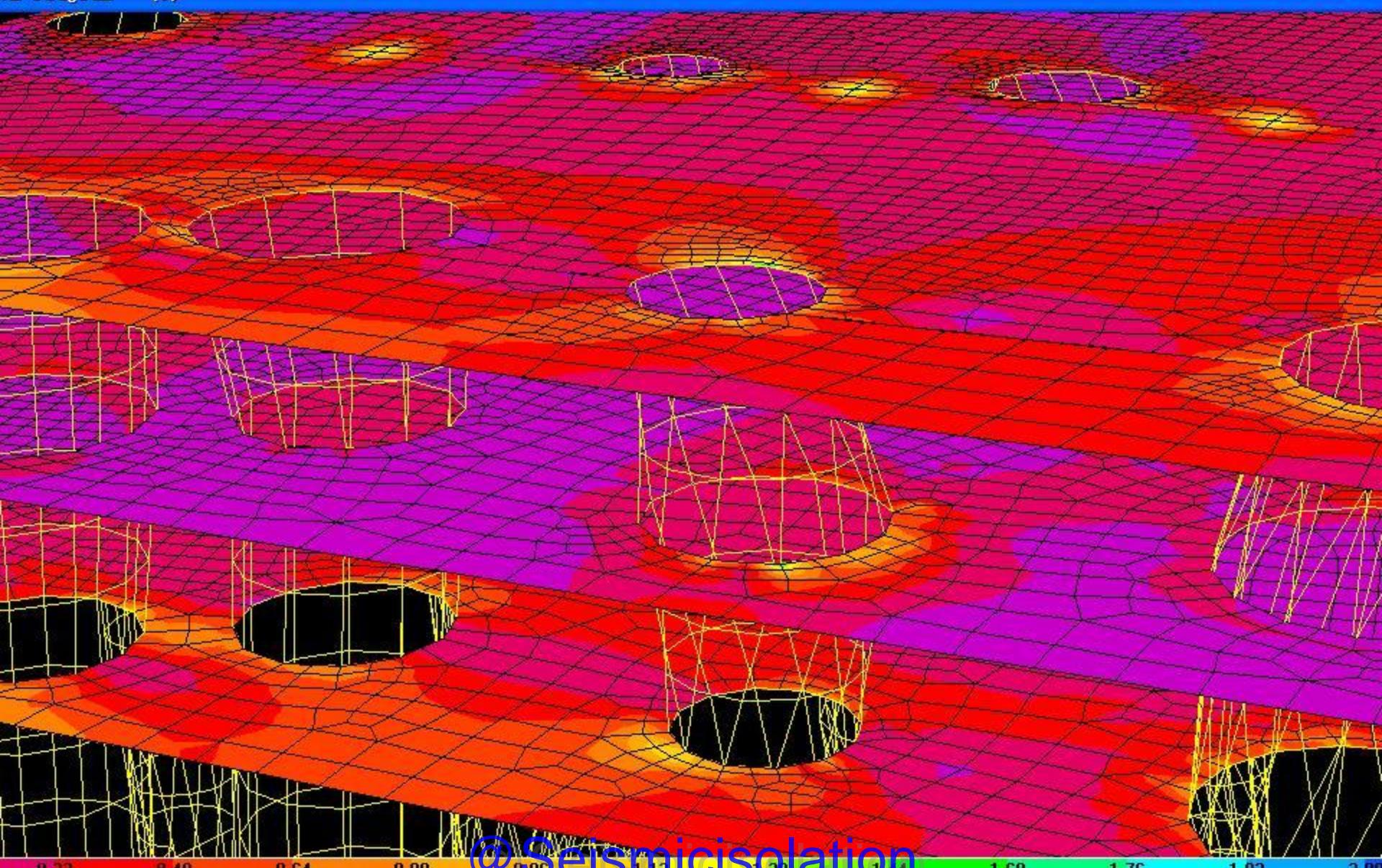
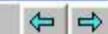


Diagram (G)



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and drag to mark zoom region



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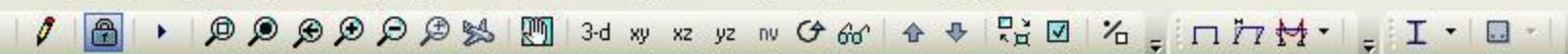
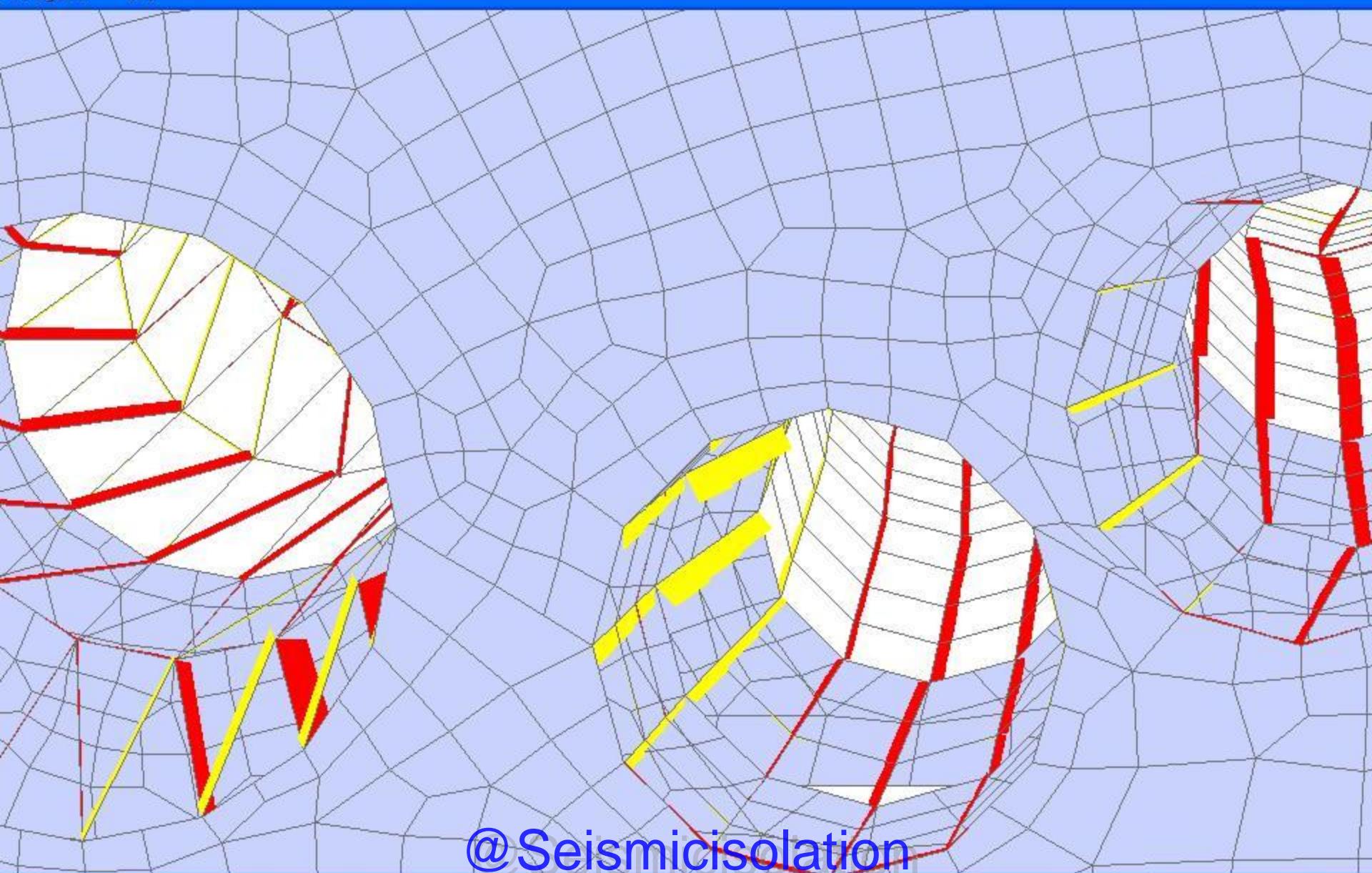
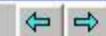
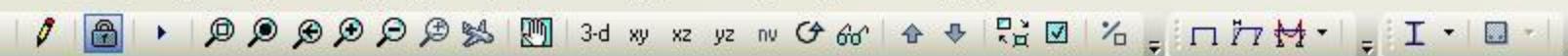


Diagram (G)

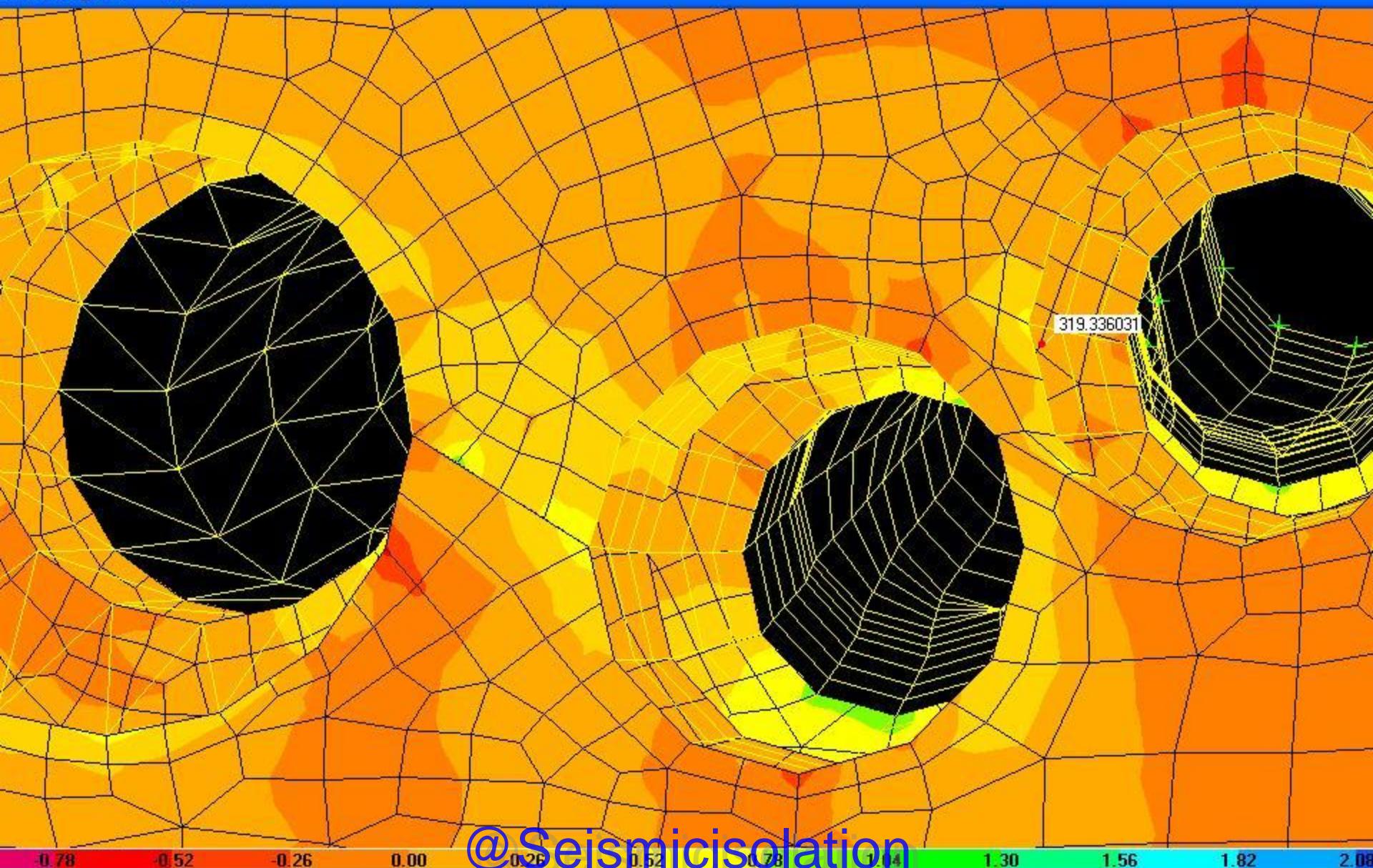


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MAX Diagram (G)

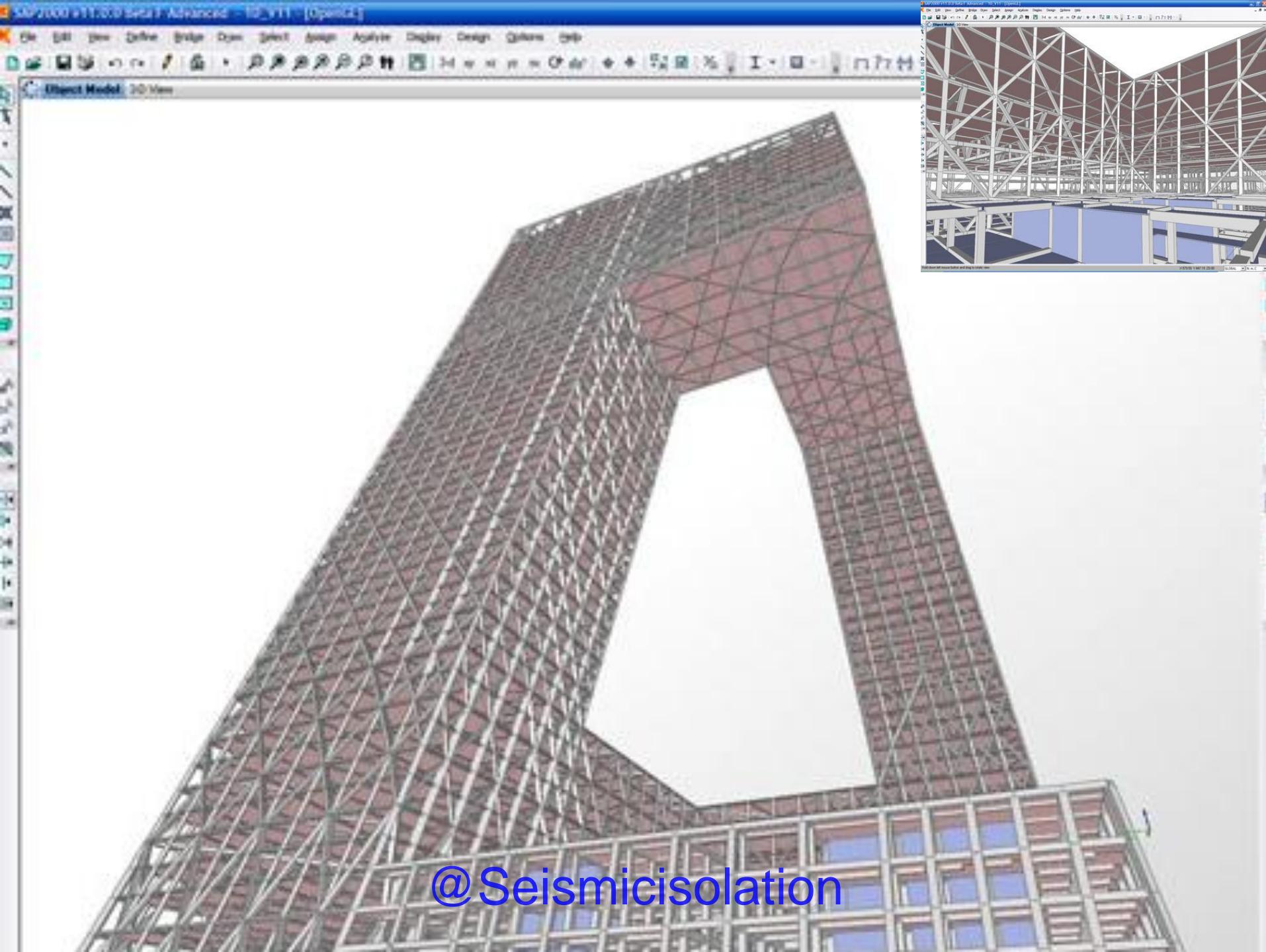


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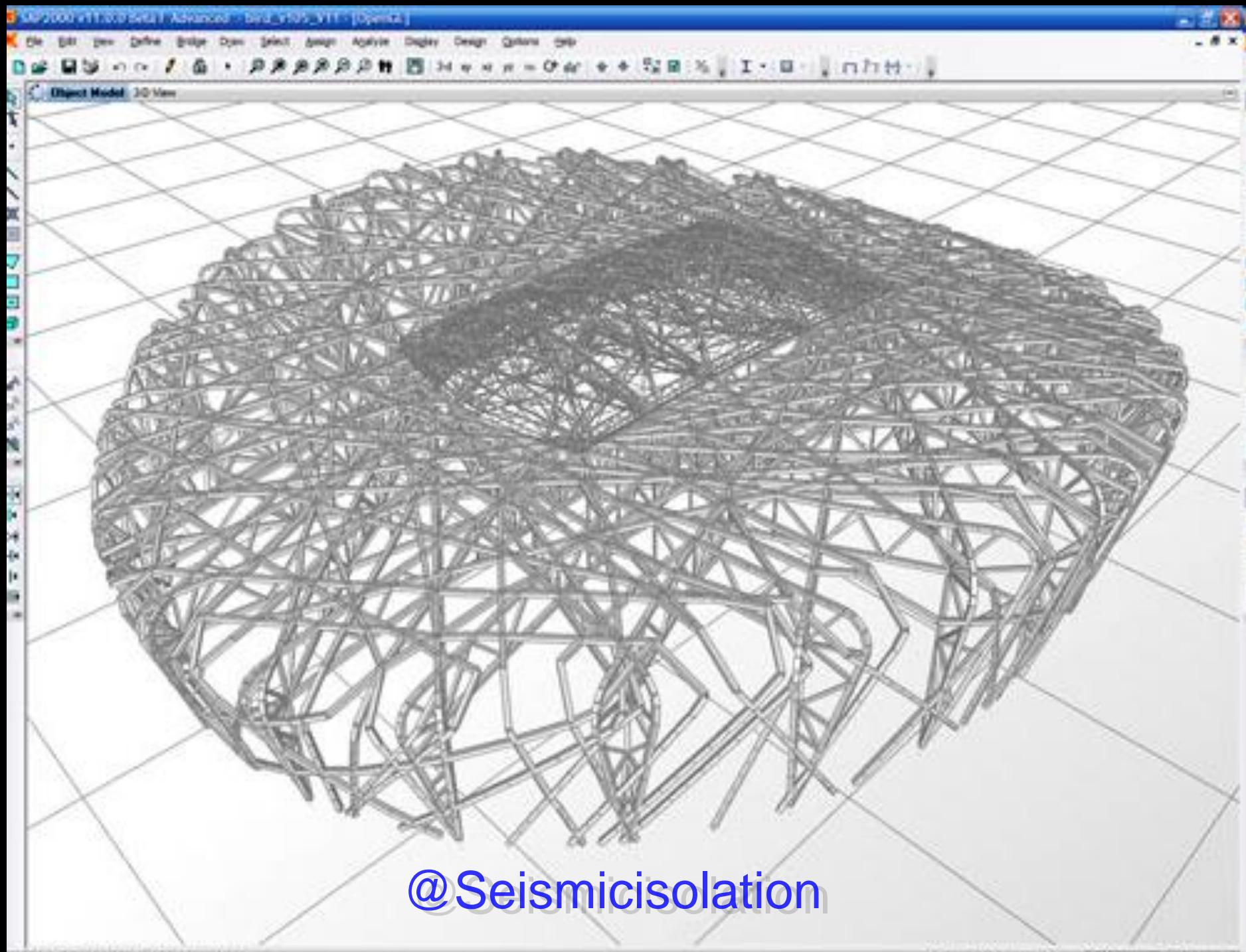
CCTV Headquarters and TVCC Building, Beijing, 2008, Rem Koolhaas and Ole Scheeren, Arup Eng.



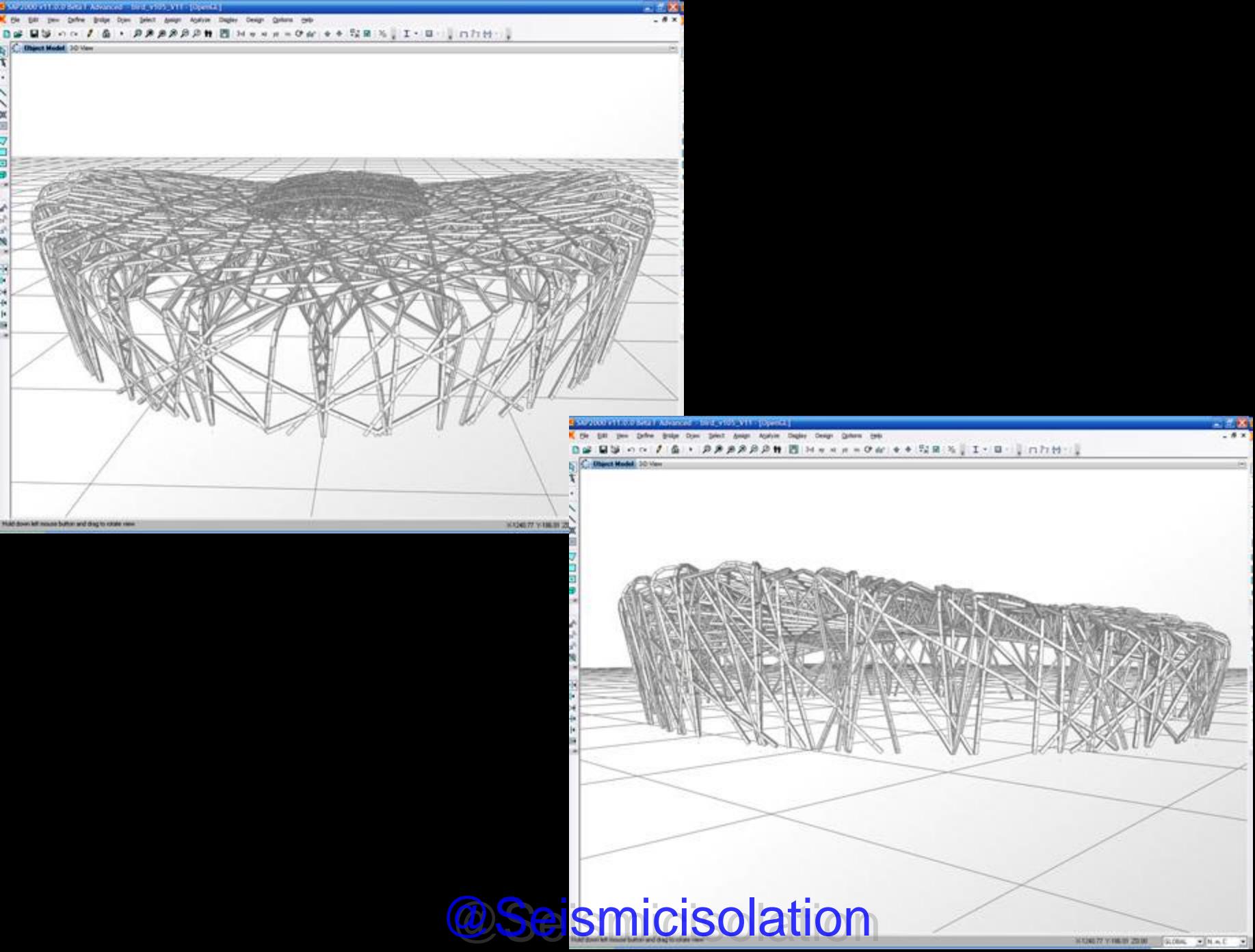
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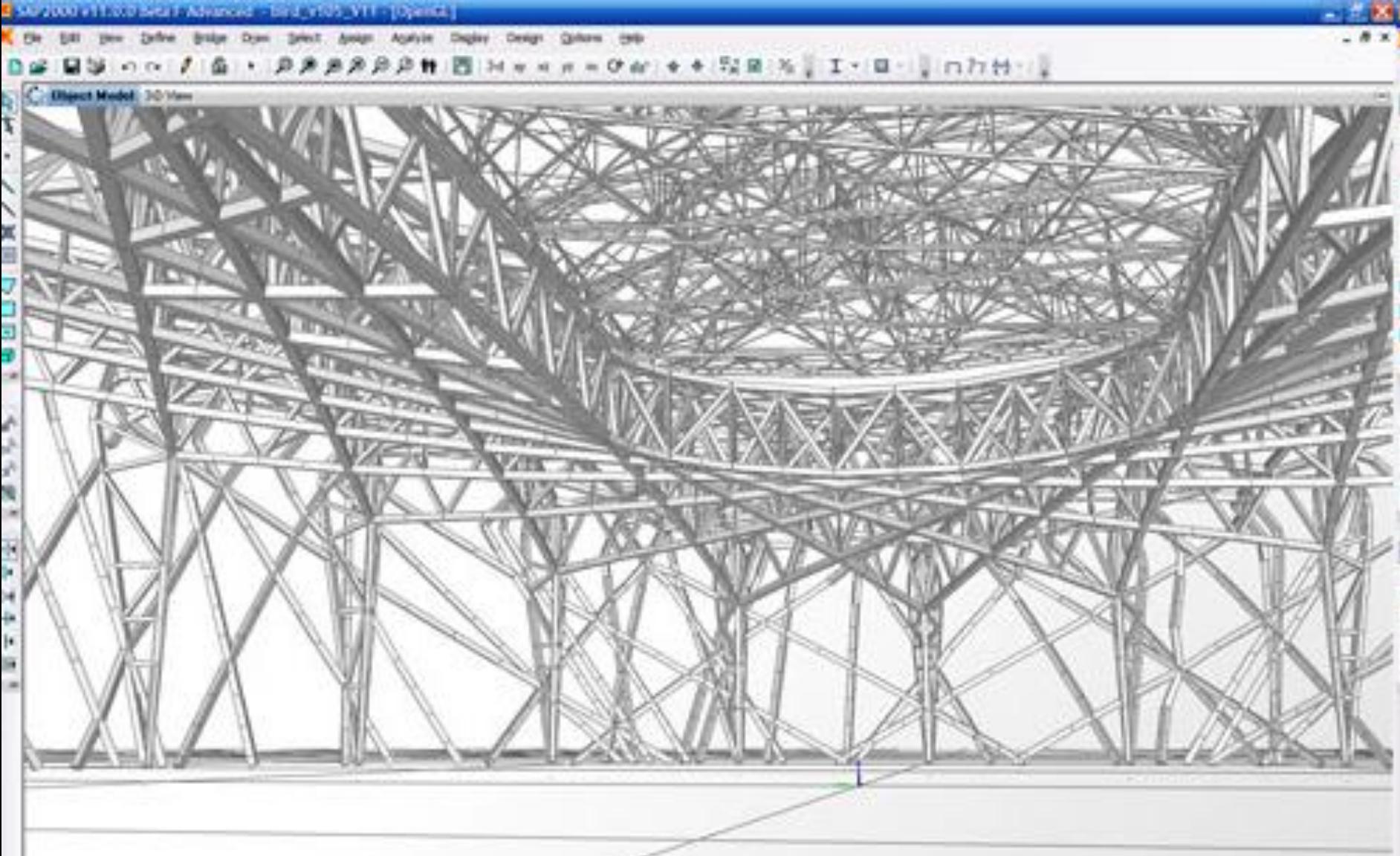
Beijing National Stadium, 2008, Herzog and De Meuron Arch, Arup Eng.  
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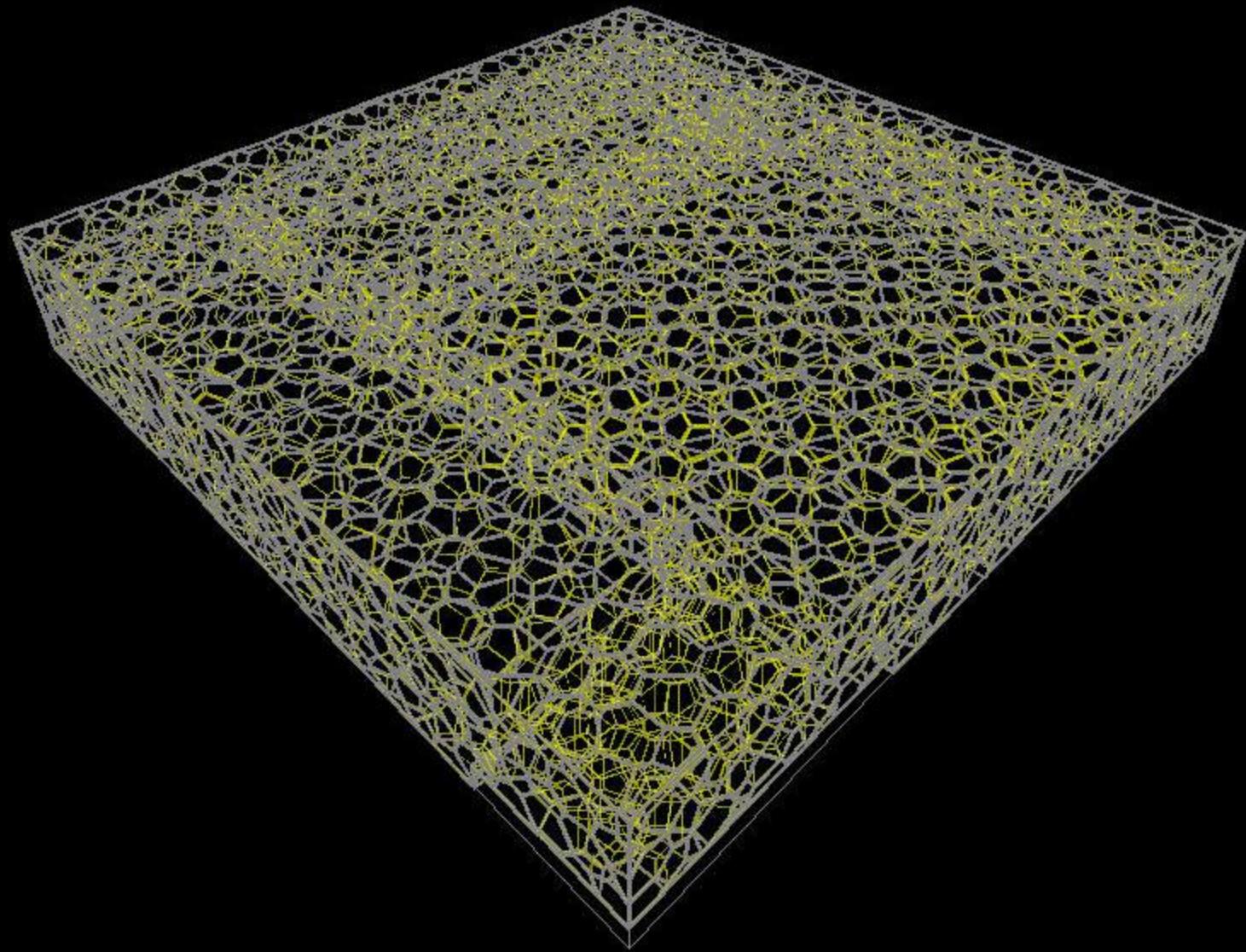
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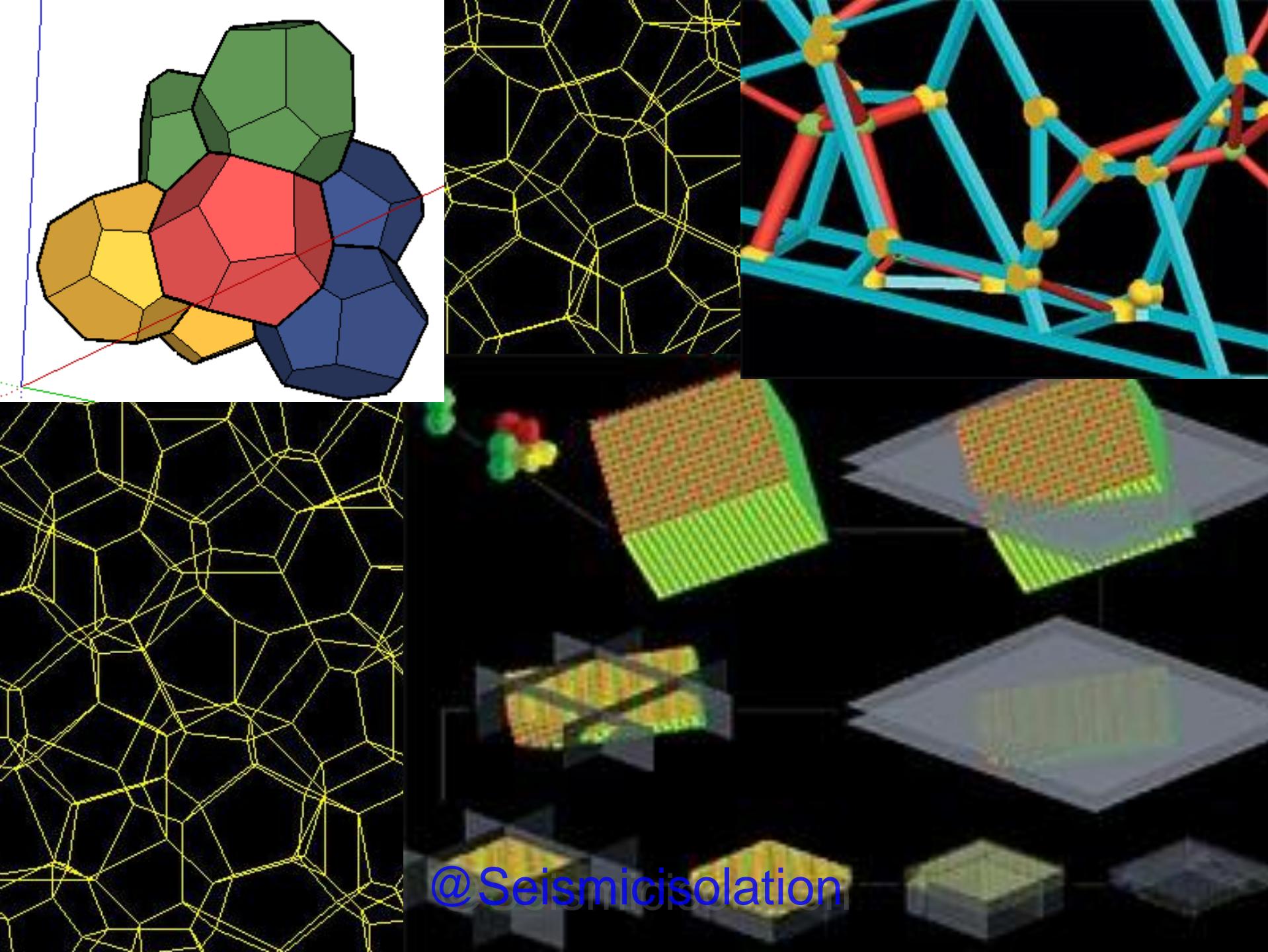
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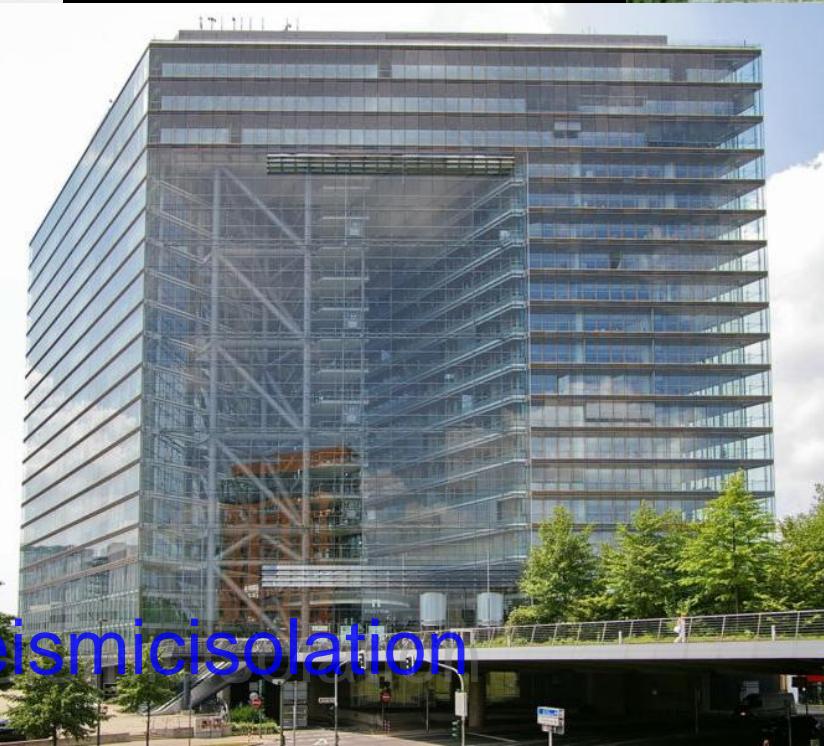
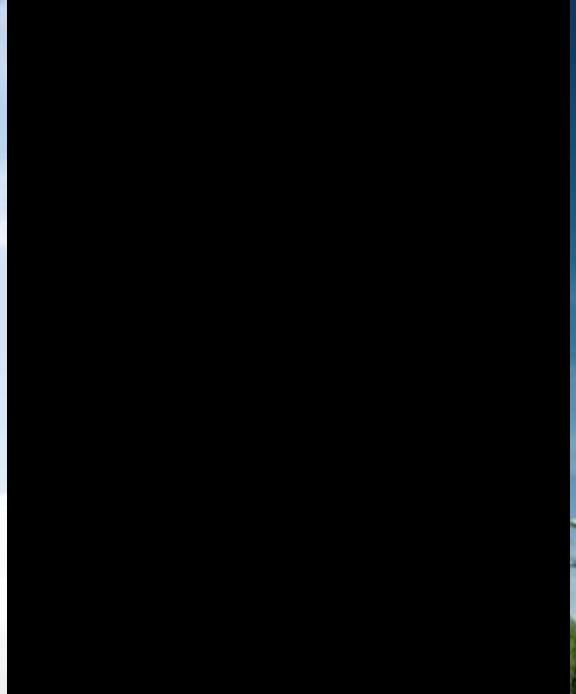
National Swimming Center, Beijing, 2008, Arup Arch and Eng  
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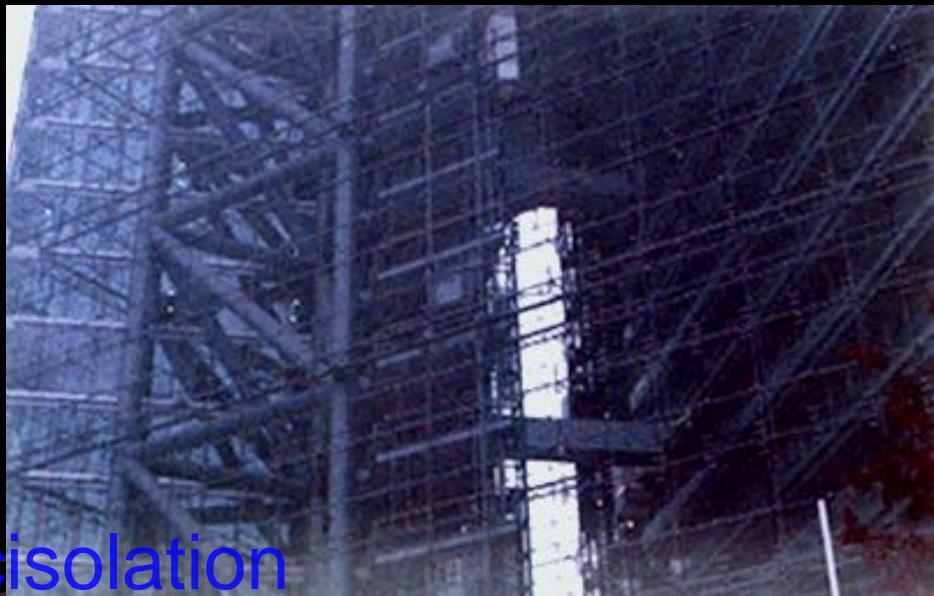
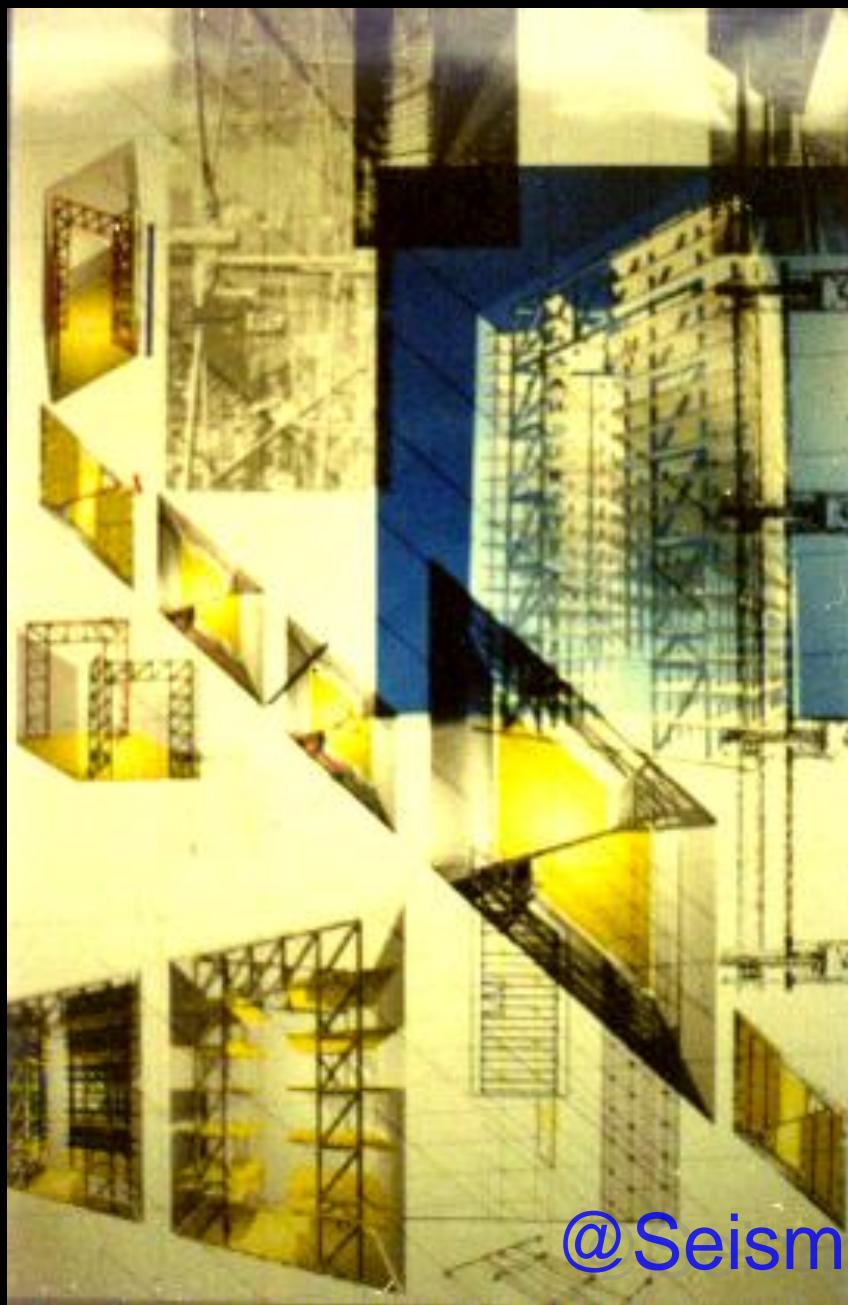
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Duesseldorf City Gate,  
Duesseldorf, Germany, 1997, [@Seismicisolation](#)  
H. Petzinka + Fink Arch

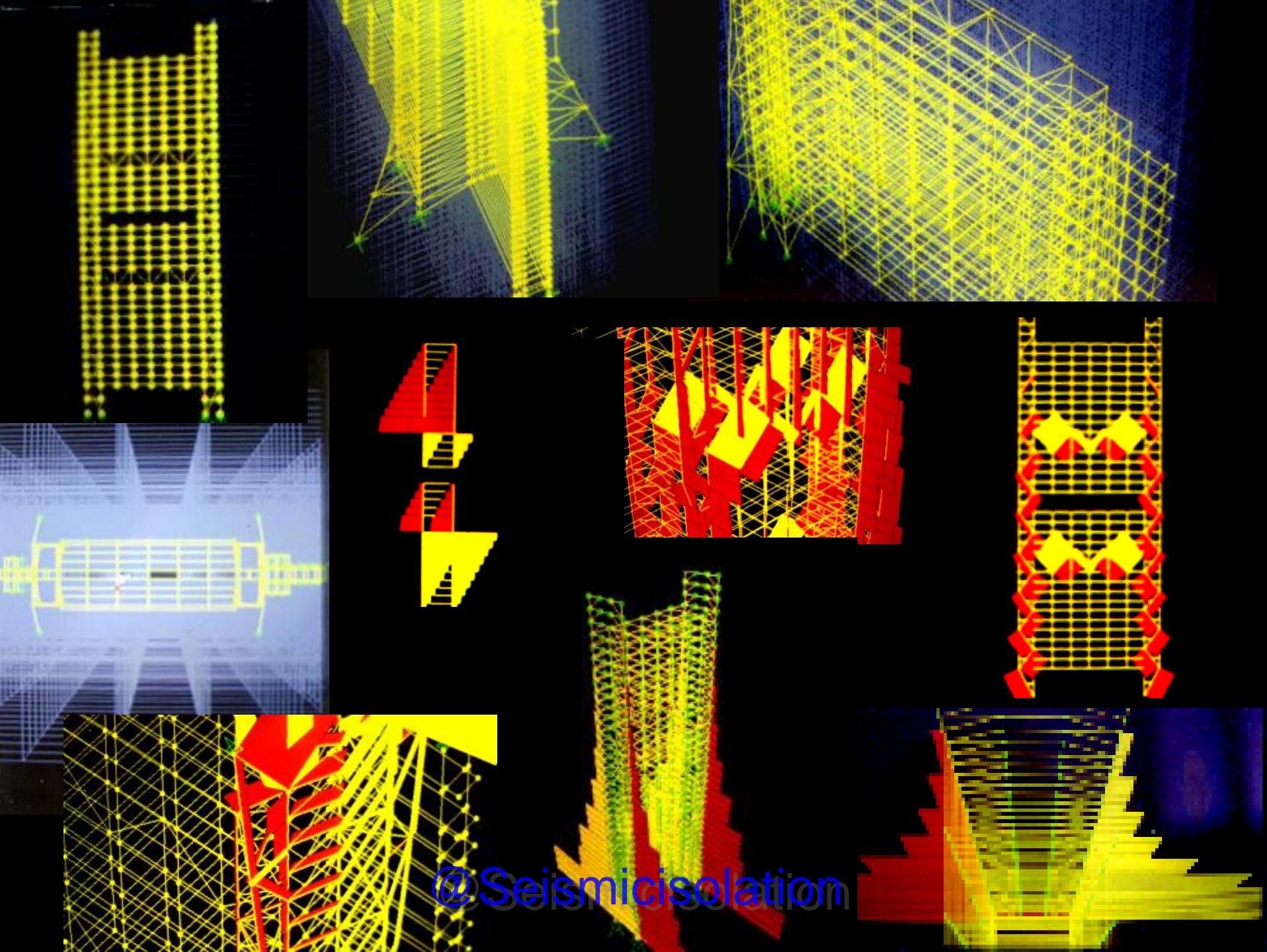


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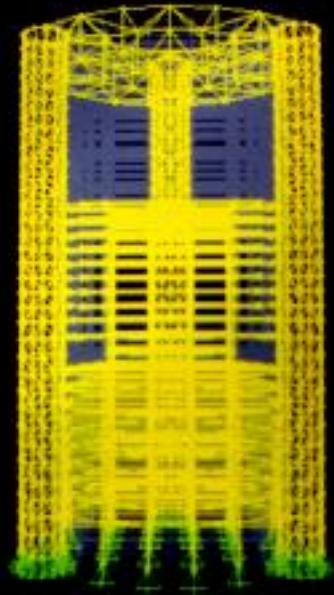
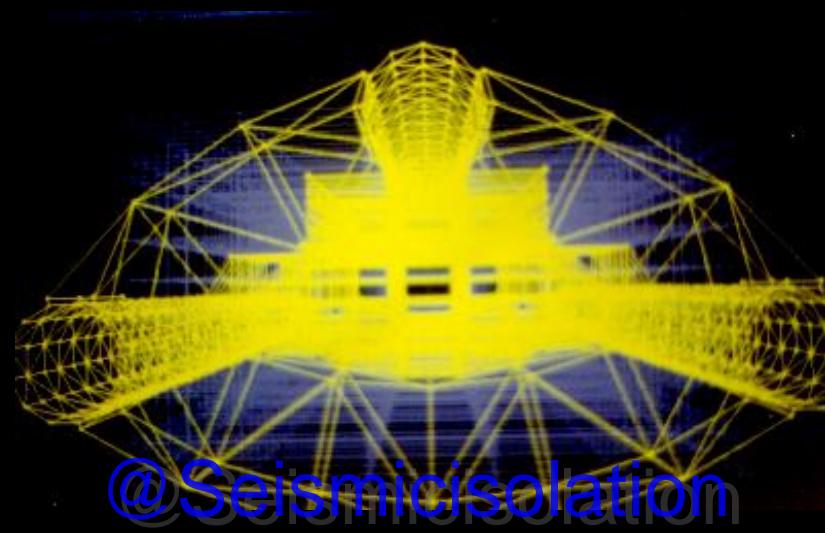
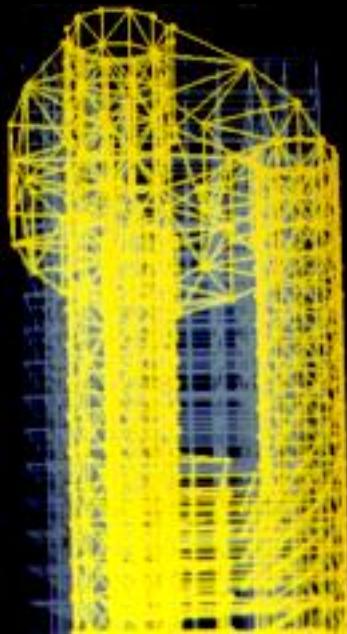
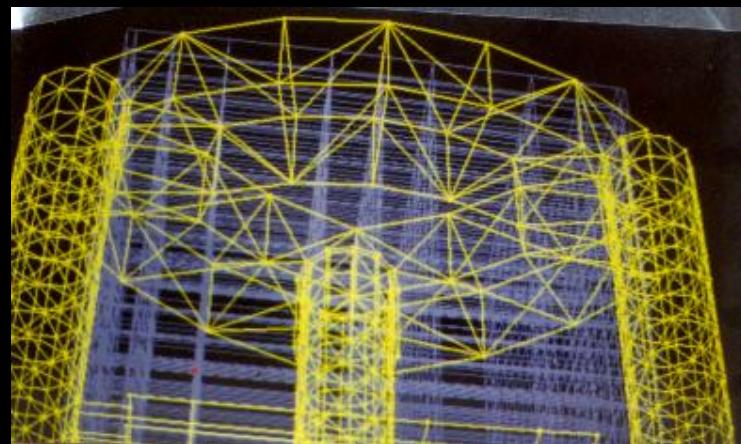
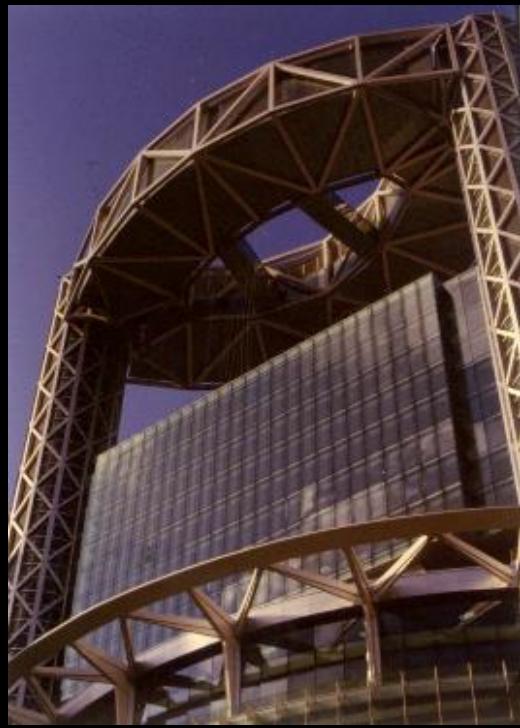
**Seoul Broadcasting Center**, Seoul, 2003, Richard Rogers Arch. And Buro Happold Struct. Eng



**Samsung** Samsung Jongro Tower, Seoul, 1999, Rafael Vinoly



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