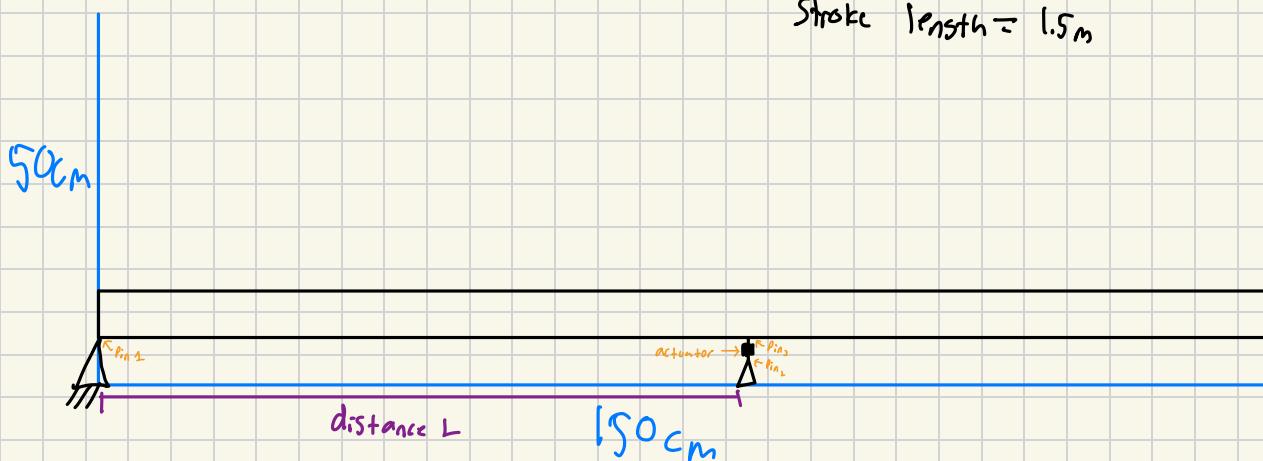


Portfolio

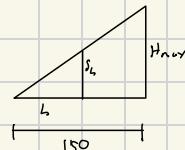
Part 1

use Actuator RSX Max force = 294 kN
Stroke length = 1.5m



decrease L to increase height lifted

increase L to increase weight lifted



$$\sum M_{P_{in}} \Rightarrow L \cdot F_{actuator} - 150 \cdot F_w = 0$$

$$L \cdot F_{actuator} = 150 \cdot F_w$$

$$M_{max} = \frac{L}{150} \cdot F_{actuator}$$

$$\frac{SL}{L} = \frac{H_{max}}{150\text{cm}}$$

$$H_{max} = \frac{150\text{cm} \cdot 150\text{cm}}{L}$$

$$L = 50\text{cm} \text{ because}$$

any extra height above

1.5m seems excessive

and we still have an

allowance M_{max} of 0.981.806 kg

11:46PM Fri Oct 10

< Portfolio spread sheet

	A	B	C
1	L	m(max) kg	H(max) m
2	1	199.7961264	225
3	10	1997.961264	22.5
4	20	3995.922528	11.25
5	30	5993.883792	7.5
6	40	7991.845056	5.625
7	50	9989.80632	4.5
8	75	14984.70948	3
9	100	19979.61264	2.25
10	125	24974.5158	1.8

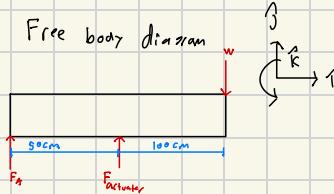
Portfolio Part 2



Assume weight is maximum possible weight

$$M = 9984 \text{ kN}$$

Free body diagram



$$\sum F_y: F_A + F_{act\ max} = W$$

$$W = M_g$$

$$W = 9984 \cdot 1.81$$

$$\sum M_A: 50\text{cm} F_{act} = 180\text{cm} W$$

$$W = 0.744 \text{ kN}$$

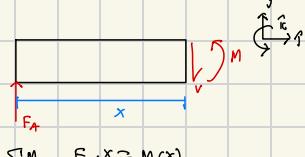
$$F_{act} = 3W$$

$$F_{act} = 203.18 \text{ kN}$$

$$F_A = W - F_{act} = -105.18 \text{ kN}$$

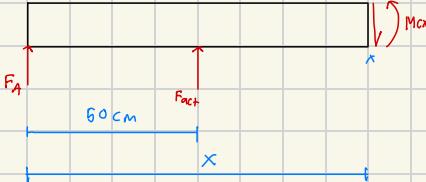
Break into spans

Span 1 Pin to actuator



$$\sum M_x: F_A \cdot x = M(x)$$

Span 2 actuator to weight



Span 1

$$\sum M_x: M(x) = F_A \cdot x + F_{act}(x - 0.5m)$$

$$EI y'' = M(x)$$

$$EI y'' = M(x)$$

$$EI y'_1 = \frac{F_A x^2}{2} + F_{act} \left(\frac{x^2}{2} - 0.5m x \right) + C_1$$

$$EI y_1(x) = \frac{F_A x^3}{6} + C_1 x + C_2$$

$$EI y_1(x) = \frac{F_A x^3}{6} + F_{act} \left(\frac{x^3}{6} - \frac{0.5m x^2}{2} \right) + C_3 x + C_4$$

$$\text{use BC } y(0) = 0 \rightarrow C_2 = 0$$

$$y_1'(0.5) = y_2'(0.5)$$

$$\frac{F_A \cdot (0.5)^2}{2} + C_1 = \frac{F_A (0.5)^2}{2} + F_{act} \left(\frac{(0.5)^2}{2} - 0.5 \cdot 0.5 \right) + C_3$$

$$y_1(0.5) = y_2(0.5)$$

$$\frac{F_A \cdot (0.5)^3}{6} + C_1 \cdot (0.5) = \frac{F_A (0.5)^3}{6} + F_{act} \left(\frac{(0.5)^3}{6} - \frac{(0.5)^3}{2} \right) + C_3 (0.5) + C_4$$

$$y_2(1.5) = 0$$

$$0 = \frac{F_A (1.5)^3}{6} + F_{act} \left(\frac{(1.5)^3}{6} - \frac{(1.5)^3}{2} \right) + C_3 (1.5) + C_4$$

Use software to solve system of equations

$$C_1 = 2241575$$

$$C_3 = 5144605$$

$$C_4 = \frac{73495}{6}$$

Use software for max deflection location

$$x = 1.5$$

$$Y_{max} = Y(1.5) = \frac{330727.5}{EI}$$

Limiting deflection for under 2%:

$$Y_{max} = Y(1.5) = \frac{330727.5}{EI}$$

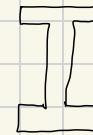
$$Y_{max} < 0.02 \cdot L \rightarrow Y_{max} < 0.03 \text{ m}$$

$$E_{steel} = 200 \text{ GPa} \quad I > \frac{330727.5}{200 \times 10^9 \cdot 0.03}$$

$I > 0.000055 \text{ m}^4$ note: At first this number seemed very low but typical allowable bending is in the range of mm's so a lower number makes some sense

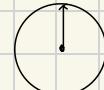
I initially wanted to use I-beam cross section since they are strong in bending and there is no torsion which is what they are weakest at.

The proposed cross section



But I think Shear will be the limiting case so

I went with a rod instead



$$I = \frac{\pi}{4} r^4$$

$$\left(\frac{\pi r^4}{4}\right)^{1/4} = r$$

$$r = 0.0015 \text{ m}$$