



EARTHQUAKE AND NUCLEAR PLANT -
IMPROVING AND SUSTAINING SAFETY

3D Non-linear Wave Propagation by Spectral Element Method

Filippo Gatti^{1,2}

¹Laboratoire MSSMat - CentraleSupélec

²DICA - Politecnico di Milano

14 décembre 2016





Set-up and Nomenclature

PRELIMINARIES

$$\underline{\underline{I}} = \sum_{n=1}^3 \mathbf{e}_n \otimes \mathbf{e}_n$$

2^{nd} order Identity Tensor

$$\underline{\underline{A}} = \underline{\underline{A}}_D + \frac{1}{3} \mathcal{I}_1^A \underline{\underline{I}}$$

Tensor decomposition : deviatoric and spherical part

$$\underline{\underline{A}} : \underline{\underline{A}} = Tr \left(\underline{\underline{A}} \underline{\underline{A}}^T \right)$$

2^{nd} order Tensor Double Contraction

$$\mathcal{I}_1^A = Tr \left(\underline{\underline{A}} \right) = \underline{\underline{I}} : \underline{\underline{A}}$$

Tensor 1^{st} Invariant

$$\mathcal{J}_1^A = Tr \left(\underline{\underline{A}}_D \right) = 0$$

Deviatoric Tensor 1^{st} Invariant

$$\mathcal{J}_2^A \left(\underline{\underline{A}} \right) = \frac{1}{2} \underline{\underline{A}}_D : \underline{\underline{A}}_D$$

Deviatoric Tensor 2^{nd} Invariant



Set-up and Nomenclature

PRELIMINARIES

$$f(\underline{\underline{\mathbf{A}}})$$

Tensor Valued Scalar Function

$$\underline{\underline{\nabla}}_{\mathbf{A}} f = \sum_{m,n=1}^3 \frac{\partial f}{\partial A_{mn}} \mathbf{e}_m \otimes \mathbf{e}_n$$

Gradient of Tensor Valued Scalar Function

$$f(\underline{\underline{\mathbf{A}}}; \underline{\underline{\mathbf{B}}})$$

Tensor Bi-Valued Scalar Function

$$\mathcal{K}_1^{A-B} = \underline{\underline{\mathbf{A}}} : \underline{\underline{\mathbf{B}}}$$

Tensor 1st Joint Invariant

$$\mathcal{K}_2^{A-B} = \underline{\underline{\mathbf{A}}} : \underline{\underline{\mathbf{B}}}^2$$

Tensor 2nd Joint Invariant

$$\mathcal{K}_3^{A-B} = \underline{\underline{\mathbf{A}}}^2 : \underline{\underline{\mathbf{B}}}$$

Tensor 3rd Joint Invariant

$$\mathcal{K}_4^{A-B} = \underline{\underline{\mathbf{A}}}^2 : \underline{\underline{\mathbf{B}}}^2$$

Tensor 4th Joint Invariant

$$\text{Objectivity} : f(\underline{\underline{\mathbf{Q}}}^T \underline{\underline{\mathbf{A}}} \underline{\underline{\mathbf{Q}}}; \underline{\underline{\mathbf{Q}}}^T \underline{\underline{\mathbf{B}}} \underline{\underline{\mathbf{Q}}}) = f(\underline{\underline{\mathbf{A}}}; \underline{\underline{\mathbf{B}}}) \quad \forall \underline{\underline{\mathbf{Q}}} \in SO(3)$$

$$\rightarrow f(\underline{\underline{\mathbf{A}}}; \underline{\underline{\mathbf{B}}}) = f(\mathcal{I}_m^A, \mathcal{I}_m^B, \mathcal{K}_m^{A-B})$$



Set-up and Nomenclature

PRELIMINARIES

$$f(\underline{\underline{\mathbf{A}}}; \underline{\underline{\mathbf{B}}}) = f(\underline{\underline{\mathbf{A}}} - \underline{\underline{\mathbf{B}}})$$

Tensor Bi-Valued Scalar Function

$$\mathcal{L}_1^{A-B} = \underline{\underline{\mathbf{I}}} : (\underline{\underline{\mathbf{A}}} - \underline{\underline{\mathbf{B}}})$$

Tensor 1st Invariant

$$\mathcal{L}_2^{A-B} = \underline{\underline{\mathbf{I}}} : (\underline{\underline{\mathbf{A}}} - \underline{\underline{\mathbf{B}}})^2$$

Tensor 2nd Invariant

$$\mathcal{L}_3^{A-B} = \underline{\underline{\mathbf{I}}} : (\underline{\underline{\mathbf{A}}} - \underline{\underline{\mathbf{B}}})^3$$

Tensor 3rd Invariant

$$\text{Objectivity} : f(\underline{\underline{\mathbf{Q}}}^T \underline{\underline{\mathbf{A}}} \underline{\underline{\mathbf{Q}}}; \underline{\underline{\mathbf{Q}}}^T \underline{\underline{\mathbf{B}}} \underline{\underline{\mathbf{Q}}}) = f(\underline{\underline{\mathbf{A}}}; \underline{\underline{\mathbf{B}}}) \quad \forall \underline{\underline{\mathbf{Q}}} \in SO(3)$$

$$\rightarrow f(\underline{\underline{\mathbf{A}}}; \underline{\underline{\mathbf{B}}}) = f(\mathcal{L}_m^{A-B})$$



Set-up and Nomenclature

PRELIMINARIES

$$f\left(\underline{\underline{\mathbf{A}}}; \underline{\underline{\xi}}\right)$$

$\underline{\underline{\xi}}$ Structural Hidden Variables

$$\underline{\underline{\xi}} = \left[\underline{\underline{\mathbf{A}}}; \underline{\underline{\mathbf{a}}}; a \right]$$

Hidden Variables : Tensors, Vectors and Scalars

$$\text{Objectivity} : f\left(\underline{\underline{\mathbf{Q}}}^T \underline{\underline{\mathbf{A}}} \underline{\underline{\mathbf{Q}}}; \underline{\underline{\mathbf{Q}}} \underline{\underline{\xi}}\right) = f\left(\underline{\underline{\mathbf{A}}}; \underline{\underline{\xi}}\right) \forall \underline{\underline{\mathbf{Q}}} \in SO(3)$$

$$\rightarrow \underline{\underline{\mathbf{Q}}} \underline{\underline{\xi}} = \left[\underline{\underline{\mathbf{Q}}}^T \underline{\underline{\mathbf{A}}} \underline{\underline{\mathbf{Q}}}; \underline{\underline{\mathbf{Q}}} \underline{\underline{\mathbf{a}}}; a \right]$$



Set-up and Nomenclature

STRAIN

$$\underline{\underline{\epsilon}} = \underline{\underline{e}} + \frac{1}{3} \mathcal{I}_1^\epsilon \underline{\underline{I}} \quad \text{Deviatoric and Spherical decomposition}$$

$$\mathcal{I}_1^\epsilon = \text{Tr}(\underline{\underline{\epsilon}}) = \epsilon_{vol} \quad 1^{st} \text{ Strain Tensor Invariant} = \text{Volumetric Strain}$$

$$\mathcal{J}_2^\epsilon(\underline{\underline{\epsilon}}) = \frac{1}{2} \underline{\underline{e}} : \underline{\underline{e}} \quad 2^{nd} \text{ Strain Tensor Invariant}$$

$$\underline{\underline{\dot{\epsilon}}} = \underline{\underline{\dot{\epsilon}}}^{el} + \underline{\underline{\dot{\epsilon}}}^{pl} \quad \text{Small Strain Additivity}$$

Assumption : $\mathcal{I}_1^{\epsilon^{pl}} = 0 \rightarrow \underline{\underline{\dot{\epsilon}}}^{pl} = \underline{\underline{\dot{e}}}^{pl}$



Set-up and Nomenclature

HARDENING VARIABLES

$$\underline{\underline{\eta}} = \left[\underline{\underline{\alpha}}; r \right]$$

Kinematic Internal Variables

$$\underline{\underline{\chi}} \left(\underline{\underline{\eta}} \right) = \left[\underline{\underline{X}} \left(\underline{\underline{\alpha}} \right); R(r) \right]$$

Static Internal Variables

$$\underline{\underline{X}} \left(\underline{\underline{\alpha}} \right)$$

Back Stress (Kinematic Hardening)

$$R(r)$$

Yield Limit (Isotropic Hardening)

Assumption : $\underline{\underline{\chi}} = -\dot{\lambda} \nabla_{\underline{\underline{\eta}}} g$

NL model constitutive equations

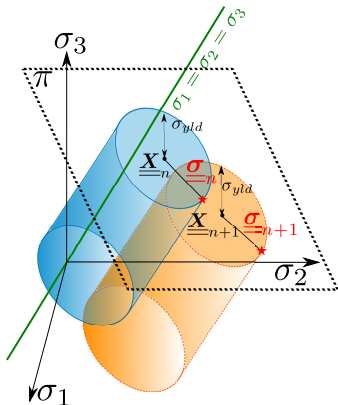


FIGURE 1 – Von Mises yield locus in principal stress 3D space

- ▶ Von Mises yield locus :

$$f(\underline{\underline{\sigma}}; \underline{\underline{\chi}}) \leq 0$$

$$f = \sqrt{3\mathcal{J}_2(\underline{\underline{\sigma}}^D - \underline{\underline{\chi}})} - R(r)$$

$$\underline{\underline{D}}_{\sigma} f = \frac{3}{2} \frac{\underline{\underline{\sigma}}^D - \underline{\underline{\chi}}}{\sqrt{3\mathcal{J}_2(\underline{\underline{\sigma}}^D - \underline{\underline{\chi}})}}$$

$$\dot{\lambda} = \sqrt{\frac{2}{3}} \|\dot{\underline{\underline{\epsilon}}}^{pl}\| = \sqrt{\frac{2}{3}} \|\dot{\underline{\underline{\epsilon}}}^{pl}\|$$

- ▶ Non-associative flow-rule

$$g(\underline{\underline{\sigma}}; \underline{\underline{\chi}}) = f(\underline{\underline{\sigma}}; \underline{\underline{\chi}}) + \underbrace{\frac{3}{4} \frac{\kappa}{C} \text{Tr}(\underline{\underline{\chi}} \cdot \underline{\underline{\chi}}^T)}_{\text{fading memory}}$$

$$\dot{\underline{\underline{\epsilon}}}^{pl} = \dot{\underline{\underline{\epsilon}}}^{pl} = \dot{\lambda} \underline{\underline{D}}_{\sigma} g$$



Nonlinear Hardening

- Prager kinematic hardening :

$$\underline{\underline{\mathbf{X}}} = \frac{2}{3} C \underline{\underline{\alpha}} \rightarrow \dot{\underline{\underline{\mathbf{X}}}} = \frac{2}{3} C \dot{\underline{\underline{\alpha}}}$$

$$\dot{\underline{\underline{\alpha}}} = -\dot{\lambda} \underline{\underline{D_{\mathbf{X}} \mathbf{g}}} = \dot{\lambda} \left(\underline{\underline{D_{\sigma} \mathbf{g}}} - \underbrace{\frac{3}{2} \frac{\kappa}{C} \underline{\underline{\mathbf{X}}}}_{\text{recall term}} \right)$$

- Isotropic hardening :

$$R = R(r) = R \left(\int_0^t \|\dot{\underline{\underline{\epsilon}}}^{pl}\| dt \right) = \sigma_{yld} + R_{\infty} (1 - e^{-br})$$

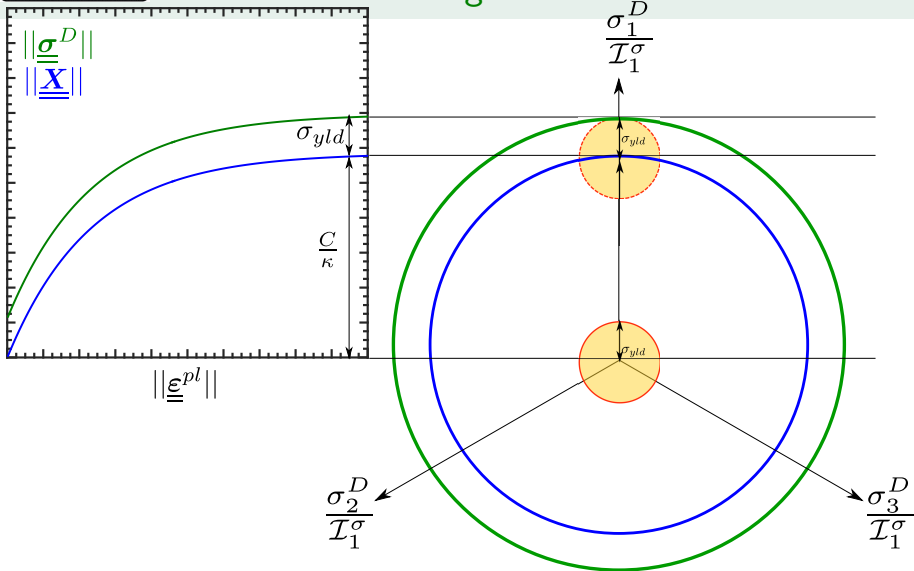
$$\dot{R} = b (R_{\infty} + \sigma_{yld} - R) \dot{r}$$

$$\dot{r} = -\dot{\lambda} \frac{\partial g}{\partial R} = \dot{\lambda} = \sqrt{\frac{2}{3}} \|\dot{\underline{\underline{\epsilon}}}^{pl}\|$$

Deviatoric stress plane :

- $\underline{\underline{\mathbf{X}}}$: back-stress **centre** of moving yield locus
- R : evolving **radius** of yield locus
- σ_{yld} : first yielding limit

Nonlinear Hardening





Nonlinear Hardening

Hardening saturation

- ▶ Isotropic hardening

$$\dot{R} = 0 \rightarrow R = R_{\infty} + \sigma_{yld}$$

- ▶ Kinematic hardening

$$\|\underline{\dot{\mathbf{X}}}\| = 0 = \|\underline{\dot{\boldsymbol{\alpha}}}\|$$

$$\rightarrow \|\underline{\mathbf{X}}\| = \frac{C}{\kappa}$$



Elastic-Plastic solution

► Hardening modulus h

$$h = C - \frac{3}{2} \frac{(\underline{\underline{\sigma}}^D - \underline{\underline{X}}) : \underline{\underline{X}}}{\sqrt{3\mathcal{J}_2(\underline{\underline{\sigma}}^D - \underline{\underline{X}})}} + b(R_\infty + \sigma_{yld} - R)$$

► Plastic multiplier $\dot{\lambda}$

$$\dot{\lambda} = \frac{\langle \underline{\underline{\nabla}}_\sigma f : \mathbb{D}^{el} : \underline{\underline{\dot{\epsilon}}} \rangle}{h + \underline{\underline{\nabla}}_\sigma f : \mathbb{D}^{el} : \underline{\underline{\nabla}}_\sigma g}$$

► Elastic-Plastic stiffness matrix \mathbb{D}^{ep}

$$\underline{\underline{\dot{\sigma}}} = \mathbb{D}^{ep} : \underline{\underline{\dot{\epsilon}}} \Rightarrow \mathbb{D}^{ep} = \mathbb{D}^{el} - \frac{\mathbb{D}^{el} : \underline{\underline{\nabla}}_\sigma g \otimes \mathbb{D}^{el} : \underline{\underline{\nabla}}_\sigma f}{h + \underline{\underline{\nabla}}_\sigma f : \mathbb{D}^{el} : \underline{\underline{\nabla}}_\sigma g}$$



Numerical implementation : key points

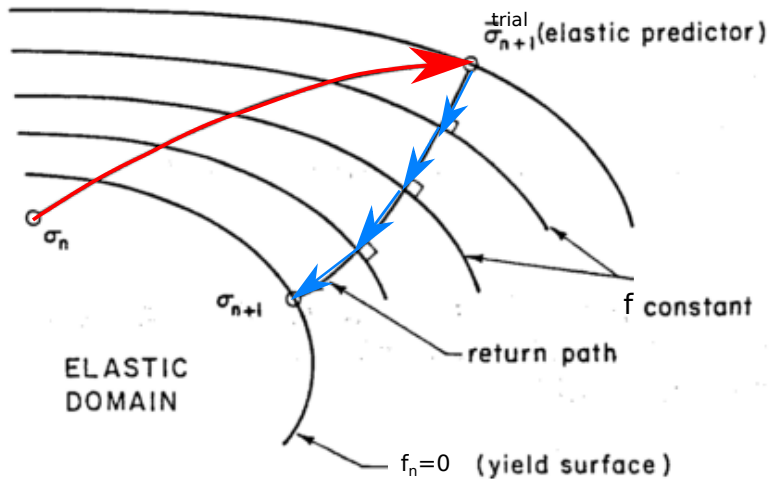
Sub-stepping explicit algorithm ($\Delta t_n \ll$)

1. Updating current state : t_n
2. Strain increment : $\underline{\underline{\Delta \varepsilon}}_n = \int_{t_n}^{t_{n+1}} \underline{\underline{\dot{\varepsilon}}}(s) ds = \frac{1}{\Delta t_n} \int_0^1 \left[\underline{\underline{\dot{\varepsilon}}}^{el}(s) + \underline{\underline{\dot{\varepsilon}}}^{pl}(s) \right] ds$
3. Elastic trial stress state prediction : $\underline{\underline{\Delta \sigma}}_n^{trial} = \mathbb{D}^{el} : \underline{\underline{\Delta \varepsilon}}_n$
4. Plasticity-Check : $f \left(\underline{\underline{\sigma}}_n + \underline{\underline{\Delta \sigma}}_n^{trial}; \underline{\underline{\chi}}_n \right)$
 - ▶ $f < 0$: elastic step \rightarrow next strain increment
 - ▶ $f \geq 0$: elastic-plastic step \rightarrow **PLASTIC CORRECTION !**

Plastic Correction :

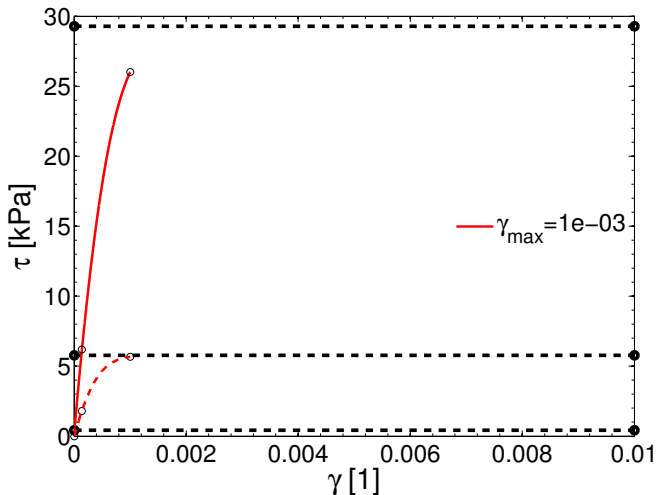
- ▶ sub-stepping method (radial return) **Sloan et al. 2001** :
 $f \left(\underline{\underline{\sigma}}_n + \underline{\underline{\Delta \sigma}}_n^{trial-k}; \underline{\underline{\chi}}_n \right) = 0$
- ▶ hardening update : $\underline{\underline{\chi}}_n \rightarrow \underline{\underline{\chi}}_{n+1}$
- ▶ drift correction : $f \left(\underline{\underline{\sigma}}_n + \underline{\underline{\Delta \sigma}}_n^{trial-k} + \underline{\underline{\Delta \sigma}}_n^{drift}; \underline{\underline{\chi}}_{n+1} \right) = 0$

Numerical implementation : key points



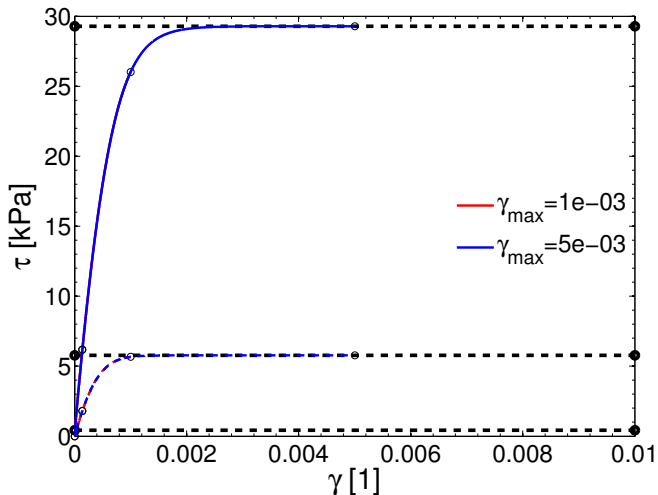
Numerical example

LINEAR MONOTONIC strain path



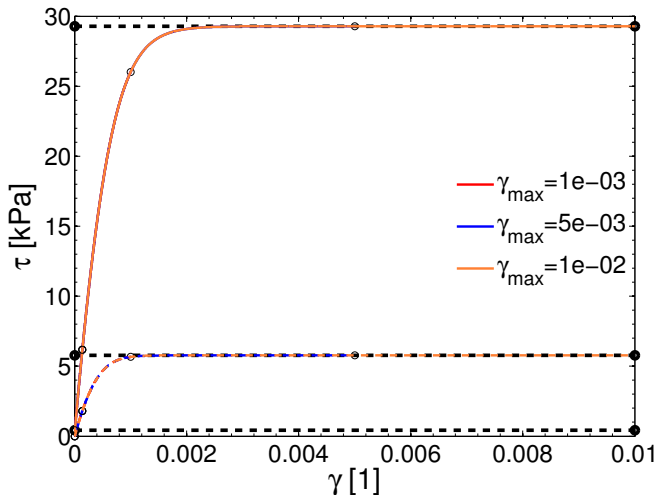
Numerical example

LINEAR MONOTONIC strain path



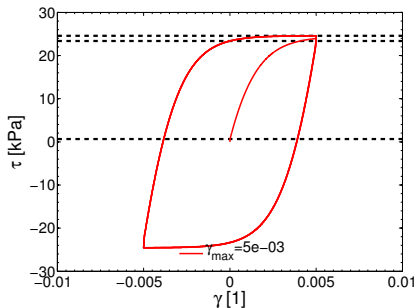
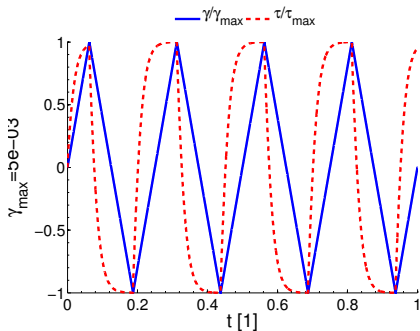
Numerical example

LINEAR MONOTONIC strain path



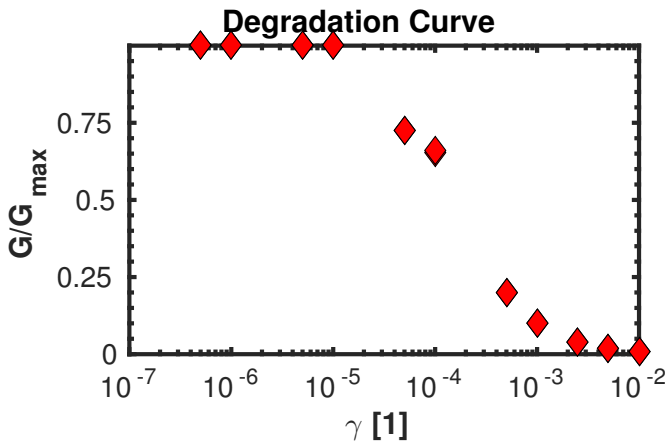
Numerical example

SAW-TOOTH strain path



Numerical example

SAW-TOOTH strain path





SEM implementation

FOLLOW UP

- ▶ **3D implementation in SEM3D**
- ▶ **2D implementation in SPEED** (to be coupled with DRM?)
- ▶ **Model calibration** for clays and sands
- ▶ **Validation & Verification :**
 - ▶ Soil column test
 - ▶ Non-linear random soil material

GRAZIE DELL'ATTENZIONE
MERCI DE VOTRE ATTENTION
THANKS FOR THE ATTENTION

Filippo Gatti[3, 7]

filippo.gatti@centralesupelec.fr

filippo.gatti@polimi.it



`https://github.com/FilLTP89`

REFERENCES & ACKNOWLEDGEMENTS

JP Bardet, K Ichii, and CH Lin.

EERA - A computer program for Equivalent-linear Earthquake site Response Analyses of layered soil deposits.
University of Southern California, August 2000.

I.A. Beresnev and Kuo-Rang Wen.

Nonlinear Soil Response-A Reality?

Bulletin of the Seismological Society of America, 86(6) : 1984-1970, 1996.

Filippo Gatti, Fernando Lopez-Caballero, and Didier Clouteau.

One-Dimensional Seismic Response at the Nuclear Power Plant of Kashiwazaki-Kariwa during the 2007 Niigata-Chuetsu-Oki Earthquake.

In J. Kruijs, Y. Tsompanagos, and B.M.V. Topping, editors, *Proceedings of the Fifteenth International Conference on Civil, Structural and Environmental Engineering Computing*, number 157, pages 4-15. Stirlingshire, UK., 2015. Civil-Comp Press.

I. M. Niiss.

Procedures for Selecting Earthquake Ground Motions at Rock Sites.
Technical report, 1991.

G Masing.

On Heyn's hardening theory of metals due to inner elastic stresses.

Wiss. Veröff. Siemens-Konzern, 3 : 231-239, 1929.

Alain Pecker.

Dynamique des Sols.

Presses de l'Ecole Nationale des Ponts et Chaussées, 1984.

C. Smeraldi, F. Gatti, and R. Paolucci.

An artificial neural network approach to support physics-based generation of broadband earthquake ground motions, Septembre 2016.

The work carried out under the SINAPS@ project has benefited from French funding managed by the National Research Agency under the program "Future Investments" - SINAPS@ reference No. ANR-11-RSNR-0022.