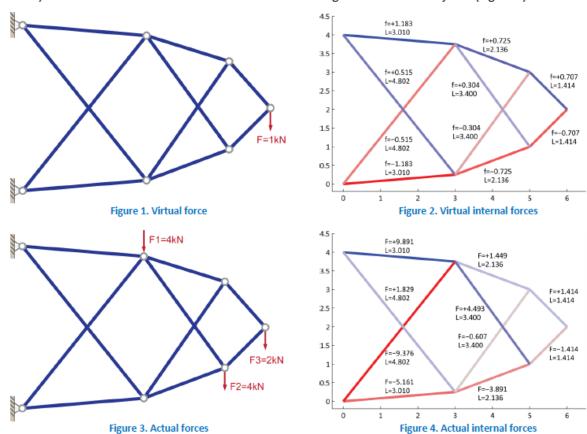
CIVE 546 – Structural Design Optimization (SDO)

HW2-Solution

Problem 1

a) The forces in all members of truss are obtained using the method of the joints (Figure 1).



b) Virtual forces, actual forces, material and element properties are tabulated as:

Member	f(kN)	F (kN)	L (m)	f FL (kN ² m)	A (m ²)	E (kN/m²)	EA(kN)
1	-1.183	-5.161	3.010	18.380	0.0900	20000	1800
2	-0.725	-3.891	2.136	6.026	0.0400	20000	800
3	-0.707	-1.414	1.414	1.414	0.0225	20000	450
4	1.183	9.891	3.010	35.225	0.0900	20000	1800
5	0.725	1.449	2.136	2.244	0.0400	20000	800
6	0.707	1.414	1.414	1.414	0.0225	20000	450
7	-0.514	-9.376	4.802	23.144	0.0100	20000	200
8	0.514	1.829	4.802	4.515	0.0100	20000	200
9	-0.304	-0.607	3.400	0.627	0.0100	20000	200
10	0.304	4.493	3.400	4.644	0.0100	20000	200

From the table above

$$\Delta = \sum_{i=1}^{n} \frac{f_i F_i L_i}{A_i E_i} = 0.2111m$$

c) We can rewrite an auxiliary objective function by introducing the Lagrange multiplier as

$$\mathcal{L}(\boldsymbol{A}, \lambda) = \sum_{i=1}^{n} A_i L_i + \lambda \left(\sum_{i=1}^{n} \frac{f_i F_i}{A_i E_i} L_i - \Delta_{max} \right)$$

Based on the necessary condition for extremum, we can compute

$$\begin{split} \frac{\partial \mathcal{L}(\mathbf{A}, \lambda)}{\partial A_i} &= L_i - \lambda \frac{f_i F_i}{{A_i}^2 E_i} L_i = 0 \\ \frac{\partial \mathcal{L}(\mathbf{A}, \lambda)}{\partial \lambda} &= \sum_{i=1}^n \frac{f_i F_i}{A_i E_i} L_i - \Delta_{max} = 0 \end{split}$$

Solving two equations in terms of the Lagrange multiplier, we can obtain

$$\lambda = \left[\frac{1}{\Delta_{max}} \sum_{i=1}^{n} \left[\frac{f_i F_i}{E_i} \right]^{0.5} L_i \right]^2 = 11.033$$

d) Optimal cross-sectional areas are

$$A_i^* = \sqrt{\lambda \frac{f_i F_i}{E_i}} = 3.322 \cdot \sqrt{\frac{f_i F_i}{E_i}}$$

 $A^* = [0.0580 \ 0.0394 \ 0.0235 \ 0.0803 \ 0.0241 \ 0.0235 \ 0.0516 \ 0.0228 \ 0.0101 \ 0.0274]^{\mathrm{T}} (m^2).$

Problem 2

The second moment of inertia and the first moment of area are:

$$\begin{split} I(b,t_w) &= 2 \cdot b \cdot t_f \cdot \left(\frac{d-t_f}{2}\right)^2 + \frac{t_w}{12} \cdot \left(d-2 \cdot t_f\right)^3 \\ Q(b,t_w) &= b \cdot t_f \cdot \left(\frac{d-t_f}{2}\right) + \frac{t_w}{2} \cdot \left(d-2 \cdot t_f\right)^2 \end{split}$$

a) The objective function is volume:

$$F(b, t_w) = 2 \cdot b \cdot t_f + (d - 2 \cdot t_f) \cdot t_w$$

The flange must be larger than the web thickness:

$$g_1(b, t_w) = -b + t_w \le 0$$

The minimum web thickness is 0.25 in:

$$g_2(b, t_w) = 0.25 - t_w \le 0$$

The flange width cannot be negative:

$$g_3(b, t_w) = -b \le 0$$

Stress due to bending moment has to be lower than σ_y :

$$g_4(b, t_w) = \frac{M}{I(b, t_w)} \left(\frac{d}{2}\right) - \sigma_y \le 0$$

Stress due to shear has to be lower than τ_v :

$$g_5(b, t_w) = \frac{V \cdot Q(b, t_w)}{t_w \cdot I(b, t_w)} - \tau_y \le 0$$

Classification:

- Constraints g₂ and g₃ are box constraints (lower bound)
- Constraint g₁ is a linear inequality
- 3. Constraints g_4 and g_5 are nonlinear inequalities
- b) Taking the unknown vector to be

$$\mathbf{x} = \begin{pmatrix} b \\ t_w \end{pmatrix}$$

The input data for fmincon is

$$\underbrace{\begin{bmatrix} -1 & 1 \end{bmatrix}}_{\mathbf{A}} \mathbf{x} \leq \underbrace{0}_{\mathbf{b}}$$

$$\mathbf{A}_{eq} = \emptyset \quad \mathbf{b}_{eq} = \emptyset$$

$$\mathbf{lb} = \begin{bmatrix} 0 & 0.25 \end{bmatrix}$$

$$\mathbf{ub} = \emptyset$$

With g_4 and g_5 defined in a separate nonlinear constraint function. Note that there are no nonlinear equalities, only inequalities.

SectionNLCON.m

```
function [c,ceq]=SectionNLCON(d,b,tw,tf,M,V)

sigma_y = 36;
tau_y = sigma_y / sqrt(3);

[I,Q]=SectionIQ(d,b,tw,tf);
sigma=M/I*(d/2);
tau=V*Q/(I*tw);

c = [ sigma-sigma_y tau-tau_y ]';
ceq = [];

function [I,Q]=SectionIQ(d,b,tw,tf)
I = 2*b*tf*(d/2-tf/2)^2 + tw/12*(d-2*tf)^3;
Q = b*tf*(d/2-tf/2) + tw/2*(d/2-tf)^2;
return
```

The main function to solve the problem is

The output for the problem is

```
The optimal values are
b = 12.741877 in
tw = 0.499635 in
>>
```