



II grafički

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1. Prema podacima iz projekta, stub preseka 50/55cm prenosi na temelj sledeća opterećenja:

$$N = 2200 + 20a + 50b \text{ kN}$$

$$M_x = -(350 + (b + 2a)) \text{ kNm}$$

$$H_x = -(55 + a + b) \text{ kN}$$

$$M_y = -(270 + a/2 + b/3) \text{ kNm}$$

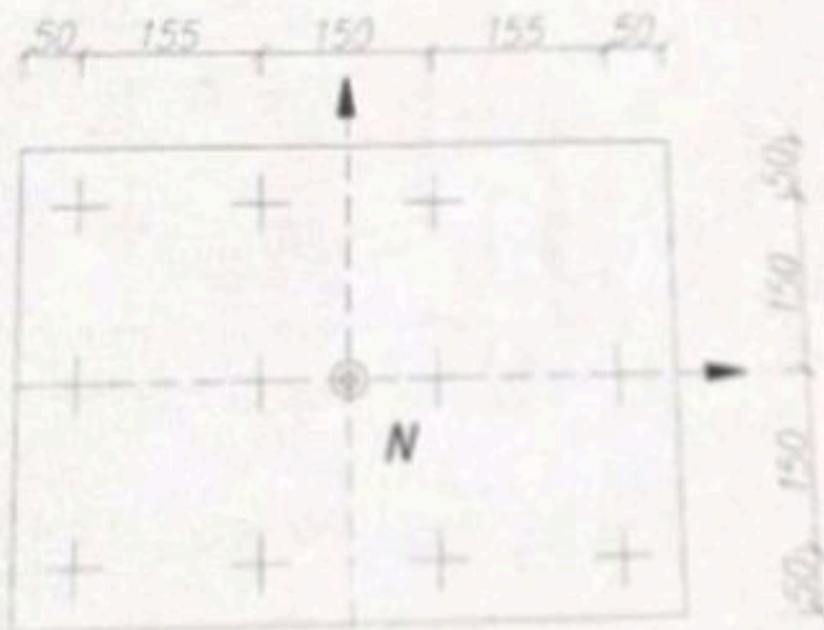
$$H_y = -(80 + b + a/2) \text{ kN}$$

Stub je fundiran, na gotovim AB šipovima, prečnika 230, u tlu sledećih karakteristika:

- I sloj, dubine 4.0m: $\gamma = 18.0 \text{ kN/m}^3$, $c = 75 \text{ kN/m}^2$, $\varphi = 27^\circ$

- II sloj: $\gamma = 22.5 \text{ kN/m}^3$, $c = 12 \text{ kN/m}^2$, $\varphi = 29^\circ$

Raspored i broj šipova (4x3) je prethodno određen, međutim, prilikom pobijanja šipova došlo je do otkazivanja pojedinih šipova. Sračunati dužinu šipova za dispoziciju prikazanu na slici:



Debljina naglavne grede iznosi 80cm, a njena gornja površina je na 30cm iznad površine terena.

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PK

008-023/20 $\alpha=2^\circ$; $\beta=3^\circ$

$$N = 2200 + 20 \cdot \alpha + 50 \cdot \beta \text{ kN} = 2200 + 20 \cdot 2 + 50 \cdot 3 = 2330 \text{ kN}$$

$$M_y = -(350 - (\alpha + 2\beta)) \text{ kNm} = -(350 - (2 + 2 \cdot 3)) \text{ kNm} = -343 \text{ kNm}$$

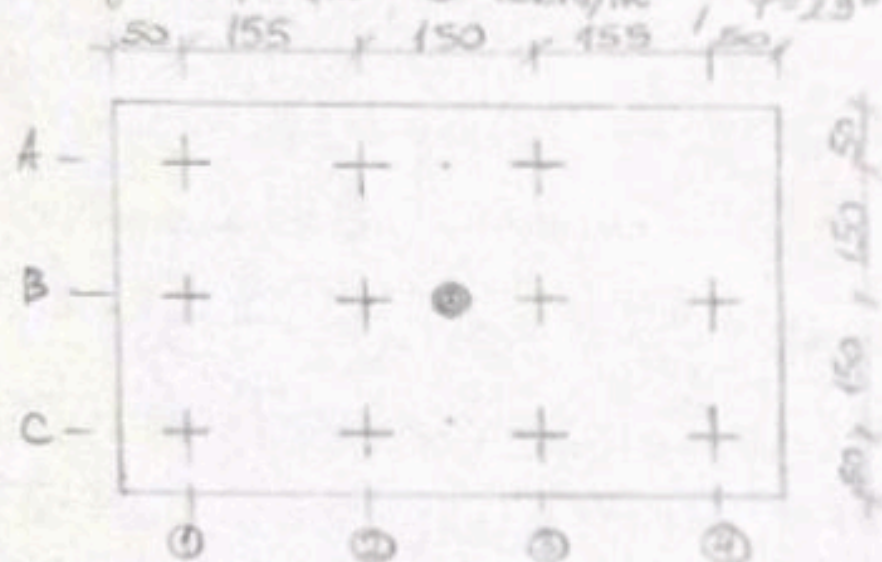
$$H_y = -(55 - \alpha + \beta) \text{ kN} = -(55 - 2 + 3) \text{ kN} = -56 \text{ kN}$$

$$M_x = -(270 + \alpha/2 + \beta/3) \text{ kNm} = -(270 + 2/2 + 3/3) \text{ kNm} = -272 \text{ kNm}$$

$$H_x = -(80 + \beta + \alpha/2) \text{ kN} = -(80 + 3 + 2/2) \text{ kN} = -84 \text{ kN}$$

I sloj: dubina 4.0m; $\gamma = 13.0 \text{ kN/m}^3$; $c = 4 \text{ kN/m}^2$; $\varphi = 27^\circ$

II sloj: $\gamma = 22.5 \text{ kN/m}^3$; $c = 92 \text{ kN/m}^2$; $\varphi = 29^\circ$



$$X_T = \frac{3 \cdot 1.55 + 3 \cdot 3.05 + 2 \cdot 4.6}{11} = 2.031 \text{ m}$$

$$Y_T = \frac{4 \cdot 1.5 + 3 \cdot 3.0}{11} = 1.3636 \text{ m}$$

0 odnos na XOy :

$$e_x = 2.3 - 2.031 = 0.269$$

$$e_y = 1.5 - 1.3636 = 0.1364 \text{ m}$$

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$$M_x^H = N \cdot e_y = 2350 \cdot 0,1364 = 323,956 \text{ kNm}$$

$$M_y^H = N \cdot e_x = 2350 \cdot 0,103 = 243,51 \text{ kNm}$$

2. Koordinate položaja šipova u odnosu na bazu
šipova

$$X_{1-1} = -2,031 \text{ m}$$

$$Y_{1-1} = 1,6364 \text{ m}$$

$$X_{2-2} = -0,541 \text{ m}$$

$$Y_{2-2} = 0,1364 \text{ m}$$

$$X_{3-3} = 0,959 \text{ m}$$

$$Y_{3-3} = -1,3636 \text{ m}$$

$$X_{4-4} = 2,509 \text{ m}$$

$$\sum X_i^2 = 3 \cdot 2,031^2 + 3 \cdot 0,541^2 + 3 \cdot 0,541^2 + 3 \cdot 0,959^2 + 2 \cdot 2,509^2 = 29,344 \text{ m}^2$$

$$\sum Y_i^2 = 3 \cdot 1,6364^2 + 4 \cdot 0,1364^2 + 4 \cdot 1,3636^2 = 15,55 \text{ m}^2$$

* Proračun sile u šipovima

$$S_i = \frac{\sum V}{n} \pm \frac{\sum M_y}{\sum X_i^2} \cdot X_{max} \pm \frac{\sum M_x}{\sum Y_i^2} \cdot Y_{max}$$

$$\sum V = N + G_t = 2350 + 20 \cdot 4 \cdot 5,6 \cdot (0,8 + 0,3) = 2822,8 \text{ kN}$$

$$\sum M_x = -M_x + H_y \cdot h_t - N \cdot e_y = -272 + 56 \cdot 0,8 - 2350 \cdot 0,1364 = -323,956 \text{ kNm}$$

$$\sum M_y = -M_y - H_x \cdot h_t + N \cdot e_x = -343 - 24 \cdot 0,8 + 2350 \cdot 0,103 = 243,51 \text{ kNm}$$

Šip A3

$$S_{A-3} = \frac{2822,8}{11} + \frac{243,51}{29,344} \cdot 0,959 + \frac{323,956}{15,55} \cdot 1,6364 = 323,207$$

Šip B4

$$S_{B-4} = \frac{2822,8}{11} + \frac{243,51}{29,344} \cdot 2,509 + \frac{323,956}{15,55} \cdot 0,1364 = 274,561$$

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- Horizontalna sila

$$S_H = \frac{\sqrt{84^2 + 56^2}}{11} = 9.178 \text{ kN} < 20 \text{ kN}$$

$$S_5, \text{ daer} = Q \cdot A_g + P \cdot A_p$$

$$Q = \gamma \cdot r \cdot N_{\gamma R} + G_D \cdot K_s \cdot N_{2R} + C_m \cdot N_{cR}$$

$$\gamma = \gamma_{11} = 22.5 \text{ kN/m}^3 ; r = \frac{d_2}{2} = \frac{30}{2} = 15 \text{ cm}$$

$$G_D = \gamma_1 \cdot h_1 + \gamma_2 \cdot h_2 = 19 \cdot 4 + 22.5 \cdot h_2$$

$$\tan \varphi_m = \frac{\tan \varphi_2}{1.5} = \frac{\tan 29^\circ}{1.5} \Rightarrow \varphi_m = 20^\circ = \varphi$$

$$\begin{cases} N_{cR} = 75 \\ N_{2R} = 12 \\ N_{\gamma R} = 4 \end{cases}$$

$$K_{s2} = 1 - \sin \varphi_m = 0.66$$

$$Q = 22.5 \cdot 0.15 \cdot 4 + (76 + 22.5 h_2) \cdot 0.66 \cdot 12 + \frac{12}{2.5} \cdot 75$$

$$Q = 975.42 + 178.2 h_2$$

$$A_2 = \frac{0.3^2 \pi}{4} = 0.071 \text{ m}^2$$

$$Q A_2 = (975.42 + 178.2 h_2) \cdot 0.071 \text{ m}^2 = 69.255 + 12.652 h_2$$

$$P A_r = p_1 A_{p1} + p_2 A_{p2}$$

$$A_{p1} = d_5 \cdot \pi \cdot h_5 = 0.3 \pi \cdot (0.8 + 0.3 + \frac{4}{2})$$

$$A_{p2} = d_5 \cdot \pi \cdot h_2 = 0.3 \pi \cdot h_2 = 0.94 \cdot h_2 \text{ m}^2$$

- Specifično trenje po ovotaku σ_{1R}

$$P = C_m + G_D \cdot K_s \cdot \tan \varphi_m ; C_m = \frac{C_1}{F_s} = \frac{7}{2.5} = 2.8 \text{ kN/m}^2$$

$$P_1 = C_{m1} + G_{D1} \cdot K_{s1} \cdot \tan \varphi_{m1} ; G_{D1} = (1.1 + 4 \cdot \frac{1.1}{2}) \cdot 19 = 48.45 \text{ kN/m}^2$$

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$$\tan \varphi_{m1} = \frac{\tan \varphi_1}{1.5} = \frac{\tan 27^\circ}{1.5} = 0.34 \Rightarrow \varphi_{m1} = 18^\circ$$

$$K_{s1} = 1 - \sin \varphi_{m1} = 0.691$$

$$P_1 = 2.8 + 48.45 \cdot 0.691 \cdot 0.34 = 14.83 \text{ kN/m}^2$$

$$P_1 A_{p1} = 14.83 \cdot 2.92 = 41.41 \text{ kN}$$

$$P_2 = C_{m2} + \sigma_{g2} \cdot K_{s2} \cdot \tan \varphi_{m2} ; C_{m2} = \frac{C_2}{F_2} = \frac{12}{2.5} = 4.8$$

$$\sigma_{g2} = 4 \cdot \gamma_1 + \frac{h_2}{2} \cdot \gamma_2 = 4 \cdot 19 + \frac{22.5}{2} \cdot h_2 = 76 + 11.25 h_2$$

$$K_{s2} = 1 - \sin \varphi_{m2} = 0.66$$

$$\tan \varphi_{m2} = \frac{\tan \varphi_2}{1.5} = \frac{\tan 29^\circ}{1.5} = 0.37 \Rightarrow \varphi_{m2} = 20^\circ$$

$$P_2 = 4.8 + (76 + 11.25 h_2) \cdot 0.66 \cdot 0.37 = 23.36 + 2.75 h_2$$

$$P_2 A_{p2} = (23.36 + 2.75 h_2) \cdot 0.34 \cdot h_2 = 21.96 h_2 + 2.58 h_2^2$$

$$P A_p = P_1 A_{p1} + P_2 A_{p2} = 41.41 + 21.96 h_2 + 2.58 h_2^2$$

$$\underbrace{69.255 + 12.652 h_2}_{2A_g} + \underbrace{41.41 + 21.96 h_2 + 2.58 h_2^2}_{P A_p} = 323.207$$

$$2.58 h_2^2 + 34.612 h_2 - 212.542 = 0$$

$$\Rightarrow h_2 = 4.578 \text{ m}$$

$$L_{\bar{s}} = 0.5 + h_1 + h_2 = 0.5 + 2.9 + 4.578 = 7.978 \text{ m}$$

$$\Rightarrow \boxed{L_{\bar{s}} = 8 \text{ m}}$$

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