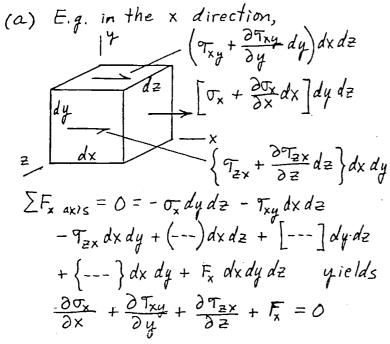
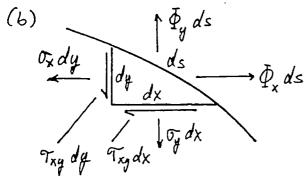
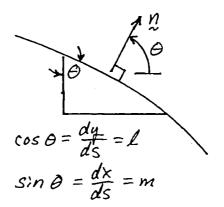
3.1-1







Thickness immaterial; can take as unity.

$$\sum F_{x} = 0 = -\sigma_{x} dy - \tau_{xy} dx + \Phi_{x} ds$$

$$\geq \tau_{y} - \nu = -\sigma_{y} dx - \tau_{xy} dy + \Phi_{y} ds$$

$$\Phi_{x} = \sigma_{x} \frac{dy}{ds} + \tau_{xy} \frac{dx}{ds}$$

$$\Phi_{y} = \sigma_{xy} \frac{dy}{ds} + \sigma_{y} \frac{dx}{ds}$$

$$\Phi_{x} = l\sigma_{x} + m\tau_{xy}$$

$$\Phi_{y} = l\sigma_{xy} + m\sigma_{y}$$

[3.1-2]
(a)
$$\nabla_{x} = -6a_{1}x^{2}, \quad \sigma_{y} = 12a_{1}x^{2}, \quad T_{xy} = 12a_{1}y^{2}$$

$$|Y|b$$

$$|Q|b$$

$$|Q|b$$

$$|Q|c$$

x-dir.
$$\nabla_{x,x} + \nabla_{xy,y} = -12a_{i}x + 24a_{i}y \neq 0$$

y-dir. $\nabla_{xy,x} + \nabla_{y,y} = 0 + 0 = 0$
Equil. not satisfied; field not possible.

3.1-3

$$\nabla_{x} = 3a_{1}x^{2}y, \quad \nabla_{y} = a_{1}y^{3}, \quad \nabla_{xy} = -3a_{1}xy^{2}$$
Equilibrium:
$$\nabla_{x,x} + \nabla_{xy,y} = 6a_{1}xy - 6a_{1}xy = 0$$

$$\nabla_{xy,x} + \nabla_{y,y} = -3a_{1}y^{2} + 3a_{1}y^{2} = 0$$
Check compatibility:
$$\varepsilon_{x} = \frac{1}{E} (3a_{1}x^{2}y - \nu a_{1}y^{3})$$

$$\varepsilon_{y} = \frac{1}{E} (a_{1}y^{3} - 3\nu a_{1}x^{2}y)$$

$$\nabla_{xy} = \frac{2(1+\nu)}{E} (-3a_{1}xy^{2})$$

$$\varepsilon_{x,y} + \varepsilon_{y,xx} \Rightarrow \nabla_{xy,xy}$$

$$\frac{1}{E} (-6\nu a_{1}y - 6\nu a_{1}y) \Rightarrow \frac{2(1+\nu)}{E} (-6a_{1}y)$$

$$\nu \Rightarrow 1 + \nu \quad \text{Not possible.}$$

$$E_{x} = u_{1x} = a_{2} + 2a_{4}x + a_{5}y$$

$$E_{y} = V_{1y} = a_{4} + a_{11}x + 2a_{12}y$$

$$V_{xy} = u_{1} + V_{1x} = a_{3} + a_{5}x + 2a_{6}y + a_{8} + 2a_{10}x + a_{11}y$$

$$Let A = \frac{E}{1 - \nu^{2}} \cdot Then$$

$$\sigma_{x} = A(E_{x} + \nu E_{y}), \quad \sigma_{y} = A(E_{y} + \nu E_{x}), T_{x} = GY_{xy}$$

$$Equilibrium \ eqs. \ become$$

$$\sigma_{x,x} + T_{xy,y} = A(2a_{4} + \nu a_{11}) + G(2a_{6} + a_{11})$$

$$T_{xy,x} + \sigma_{y,y} = G(a_{5} + 2a_{10}) + A(2a_{12} + \nu a_{5})$$

$$When \ are \ these \ 2 \ eqs, \ satisfied?$$

$$With G = (1 - \nu)A/2,$$

$$2a_{4} + \nu a_{11} + \frac{1 - \nu}{2}(2a_{6} + a_{11}) = 0$$

$$\frac{1 - \nu}{2}(a_{5} + 2a_{10}) + 2a_{12} + \nu a_{5} = 0$$
or
$$2a_{4} + (1 - \nu)a_{6} + \frac{1 + \nu}{2}a_{11} = 0$$

$$2a_{12} + (1 - \nu)a_{10} + \frac{1 + \nu}{2}a_{5} = 0$$

(a)
$$\Delta V = V_{final} - V_{original}$$

$$\Delta V = \left[(1 + \epsilon_x) dx (1 + \epsilon_y) dy (1 + \epsilon_z) dz \right] - \left[dx dy dz \right]$$

$$\Delta V = (\epsilon_x + \epsilon_y + \epsilon_z) dx dy dz + higher order tems$$

$$\Delta V = (\epsilon_x + \epsilon_y + \epsilon_z) dx dy dz \quad \text{for small strains}$$

$$\frac{\Delta V}{V} = \frac{\Delta V}{dx dy dz} = \epsilon_x + \epsilon_y + \epsilon_z$$

(b) Use 3D form of Eqs. 3.1-4, with
$$\nabla_x = 0_p = 0_2 = -p$$
:
$$\epsilon_x = \frac{-p}{E} (1-2\nu) \qquad \epsilon_y = \epsilon_x \qquad \epsilon_z = \epsilon_x$$
Hence $\frac{\Delta V}{V} = -\frac{3p}{E} (1-2\nu)$ and $\frac{\Delta V/V}{P} = -\frac{3(1-2\nu)}{E}$
(c) As $\nu \to 0.5$, $\frac{\Delta V}{V} \to 0$

(a)
$$\Delta V = V_{final} - V_{original}$$

$$\Delta V = \left[(1 + \epsilon_x) d_x (1 + \epsilon_y) d_y (1 + \epsilon_z) d_z \right] - \left[d_x d_y d_z \right]$$

$$\Delta V = (\epsilon_x + \epsilon_y + \epsilon_z) d_x d_y d_z + higher order tems$$

$$\Delta V = (\epsilon_x + \epsilon_z + \epsilon_z) d_x d_y d_z \quad \text{for small strains}$$

$$\frac{\Delta V}{V} = \frac{\Delta V}{d_x d_y d_z} = \epsilon_x + \epsilon_y + \epsilon_z$$

(b) Use 3D form of Eqs. 3.1-4, with
$$\nabla_x = 0_7 = 0_2 = -p$$
:
$$E_x = \frac{-P}{E} (1-2\nu) \qquad E_g = E_x \qquad E_z = E_x$$
Hence $\frac{\Delta V}{V} = -\frac{3p}{E} (1-2\nu)$ and $\frac{\Delta V/V}{P} = -\frac{3(1-2\nu)}{E}$
(c) As $\nu \to 0.5$, $\frac{\Delta V}{V} \to 0$

$$N_{1} = \frac{(2-x)(3-x)}{(2)(3)} = \frac{1}{6}(6-5x+x^{2})$$

$$N_{2} = \frac{-x(3-x)}{(-2)(1)} = \frac{1}{2}(3x-x^{2})$$

$$N_{3} = \frac{-x(2-x)}{(-3)(-1)} = \frac{1}{3}(-2x+x^{2})$$

(a)
$$\sum_{i=1}^{3} N_{i} = \frac{1}{6} (6-5x+x^{2}+9x-3x^{2}-4x+2x^{2}) = 1$$

(b)
$$N_{1,x} = \frac{1}{6}(-5+2x)$$
 $\sum_{i=1}^{3} N_{i,x} = \frac{1}{6}(-5+2x)$
 $N_{2,x} = \frac{1}{2}(3-2x)$ $+9-6x-4+4x$
 $N_{3,x} = \frac{1}{3}(-2+2x)$ = 0

3.2-2

If all 4 $\phi_i = 1$, the shape is \downarrow^{ϕ} 1 \downarrow_{1} \downarrow_{0}

Clearly $\phi \neq 1$ throughout $(\Sigma N_i = 1)$ if $\Sigma N_i \phi_i = \phi$ gives 1 = 1 for $\phi = \phi_i = 1$. Also, not all N_i have the same units.

$$\begin{aligned} & \epsilon_{\rm x} = u_{,\rm x} = a_{\rm z} + 2a_{\rm 4} \times + a_{\rm 5} \, q \\ & \epsilon_{\rm y} = v_{,\rm y} = a_{\rm 9} + a_{\rm 11} \times + 2a_{\rm 12} \, q \\ & v_{\rm xy} = u_{,\rm 9} + v_{,\rm x} = a_{\rm 3} + a_{\rm 5} \times + 2a_{\rm 6} \, q + a_{\rm 8} + 2a_{\rm 10} \times + a_{\rm 11} \, q \\ & Let \ A = \frac{E}{1 - \nu z} \cdot Then \\ & \sigma_{\rm x} = A \left(\epsilon_{\rm x} + \nu \epsilon_{\rm y} \right), \ \sigma_{\rm y} = A \left(\epsilon_{\rm y} + \nu \epsilon_{\rm x} \right), T_{\rm xy} = G v_{\rm xy} \\ & Equilibrium \ eqs. \ become \\ & \sigma_{\rm x,x} + \sigma_{\rm xy,y} = A \left(2a_{\rm 4} + \nu a_{\rm 11} \right) + G \left(2a_{\rm 6} + a_{\rm 11} \right) \\ & \sigma_{\rm xy,x} + \sigma_{\rm y,y} = G \left(a_{\rm 5} + 2a_{\rm 10} \right) + A \left(2a_{\rm 12} + \nu a_{\rm 5} \right) \\ & When \ are \ these \ 2 \ eqs. \ satisfied? \\ & With \ G = (1 - \nu)A/2, \\ & 2a_{\rm 4} + \nu a_{\rm 11} + \frac{1 - \nu}{2} \left(2a_{\rm 6} + a_{\rm 11} \right) = 0 \\ & \frac{1 - \nu}{2} \left(a_{\rm 5} + 2a_{\rm 10} \right) + 2a_{\rm 12} + \nu a_{\rm 5} = 0 \\ & \text{or} \\ & 2a_{\rm 4} + \left(1 - \nu \right) a_{\rm 6} + \frac{1 + \nu}{2} a_{\rm 11} = 0 \\ & 2a_{\rm 12} + \left(1 - \nu \right) a_{\rm 10} + \frac{1 + \nu}{2} a_{\rm 5} = 0 \end{aligned}$$

3,2-3

(a)
$$\phi = \sum N_i \phi_i$$
; use Eqs. 3.2-7

$$\phi = \frac{(3-x)(5-x)(8-x)}{(2)(4)(7)} 2 + \frac{(1-x)(5-x)(8-x)}{(-2)(2)(5)} 2$$

$$+ \frac{(1-x)(3-x)(8-x)}{(-4)(-2)(3)} 2 + \frac{(1-x)(3-x)(5-x)}{(-7)(-5)(-3)} 5$$
(b) $\phi_0 = \frac{3 \cdot 5 \cdot 8}{56} 2 + \frac{1 \cdot 5 \cdot 8}{-20} 2 + \frac{1 \cdot 3 \cdot 8}{24} 2 + \frac{1 \cdot 3 \cdot 5}{-105} 5$

$$= \frac{120}{28} - \frac{40}{10} + \frac{24}{12} - \frac{15}{21} = 1.57$$
Similarly
$$\phi_2 = \frac{1 \cdot 3 \cdot 6}{28} - \frac{(-1) \cdot 3 \cdot 6}{10} + \frac{(-1) \cdot 1 \cdot 6}{12} - \frac{(-1) \cdot 1 \cdot 3}{21} = 2.086$$

$$\phi_4 = \frac{(-1) \cdot 1 \cdot 4}{28} - \frac{(-3) \cdot 1 \cdot 4}{10} + \frac{(-3) \cdot (-1) \cdot 4}{12} - \frac{(-3) \cdot (-1) \cdot 1}{21} = 1.914$$

$$\phi_7 = \frac{(-4) \cdot (-2) \cdot 1}{28} - \frac{(-6) \cdot (-2) \cdot 1}{10} + \frac{(-6) \cdot (-4) \cdot 1}{12} - \frac{(-6) \cdot (-4) \cdot (-2)}{21} = 3.371$$

First two of Eqs. 3.2-8 yield $a_1 = \phi_1$, $a_2 = \phi_{,\times,1}$ The second two equations become

$$\begin{bmatrix} L^{2} & L^{3} \\ 2L & 3L^{2} \end{bmatrix} \begin{Bmatrix} \alpha_{3} \\ \alpha_{4} \end{Bmatrix} = \begin{cases} \phi_{2} - \phi_{1} - L\phi_{1}x_{1} \\ \phi_{1}x_{2} - \phi_{1}x_{1} \end{cases},$$

$$\begin{cases} a_{3} \\ \alpha_{4} \end{Bmatrix} = \frac{1}{L^{4}} \begin{bmatrix} 3L^{2} & -L^{2} \\ -2L & L^{2} \end{bmatrix} \begin{Bmatrix} \phi_{2} - \phi_{1} - L\phi_{2}x_{1} \\ \phi_{1}x_{2} - \phi_{1}x_{1} \end{Bmatrix} = \begin{cases} (-3\phi_{1} - 2L\phi_{1}x_{1} + 3d_{2} - L\phi_{1}x_{2})/L^{2} \\ (2\phi_{1} + L\phi_{1}x_{1} - 2\phi_{2} + L\phi_{1}x_{2})/L^{3} \end{Bmatrix}$$

$$\begin{bmatrix} A \end{bmatrix} = \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ -3/L^{2} & -2/L & 3/L^{2} & -1/L \\ 2/L^{3} & 1/L^{2} & -2/L^{3} & 1/L^{2} \end{bmatrix}$$

with $[X] = [1 \times x^2 \times^3]$,

$$[X][A]^{-1} = \left[1 - \frac{3x^{2}}{L^{2}} + \frac{2x^{3}}{L^{3}}, x - \frac{2x^{2}}{L} + \frac{x^{3}}{L^{2}}, \frac{3x^{2}}{L^{2}} - \frac{2x^{3}}{L^{3}}, -\frac{x^{2}}{L} + \frac{x^{3}}{L^{2}}\right]$$

$$N_{1} \qquad N_{2} \qquad N_{3} \qquad N_{4}$$

$$\phi = [X]\{a\} = [1 \times x^{2}]\{a_{1}^{2}\}$$

$$\phi_{,x} = [0 \ 1 \ 2x]\{a_{1}^{2}\}$$
Evaluate given conditions (let $\Theta_{1} = \phi_{,x1}$)
$$\begin{cases} \phi_{1} \\ \Theta_{1} \\ \phi_{2} \end{cases} = \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 1 & L & L^{2} \end{bmatrix} \begin{cases} a_{1} \\ a_{2} \\ a_{3} \end{cases} \quad a_{1} = \phi_{1}, a_{2} = \Theta_{1}$$

$$\begin{cases} a_{1} \\ a_{2} \\ a_{3} \end{cases} = \begin{bmatrix} 1 & 0 & 0 \\ 0 & L & L^{2} \end{bmatrix} \begin{cases} a_{1} \\ a_{2} \\ a_{3} \end{cases} \quad \phi_{2} - \phi_{1} - L \phi_{1} = L^{2} a_{3}$$

$$\begin{cases} a_{1} \\ a_{2} \\ a_{3} \end{cases} = \begin{bmatrix} 1 & 0 & 0 \\ 0 & L & L^{2} \end{bmatrix} \begin{cases} \phi_{1} \\ \theta_{1} \\ \theta_{2} \end{cases} = \begin{bmatrix} A \\ A \end{bmatrix}^{-1} \{d\}$$

$$\phi = [X][A]^{-1}\{d\} = [N]\{d\} \quad \text{where}$$

$$[N] = \begin{bmatrix} 1 - \frac{x^{2}}{L^{2}} & x(1 - \frac{x}{L}) & \frac{x^{2}}{L^{2}} \end{bmatrix} \cdot Also$$

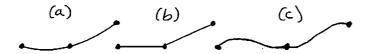
$$[N] = \begin{bmatrix} 1 - \frac{x^{2}}{L^{2}} & x(1 - \frac{x}{L}) & \frac{x^{2}}{L^{2}} \end{bmatrix} \cdot Also$$

$$[N_{,x}] = \begin{bmatrix} -\frac{2x}{L^{2}} & 1 - \frac{2x}{L} & \frac{2x}{L^{2}} \end{bmatrix}$$

$$\vdots \quad A + x = 0 \quad A + x = L \quad 1 \quad N_{1} \quad N_{2} \quad N_{3} \quad N_{2} \quad N_{2} \quad N_{3} \quad N_{3} \quad N_{3} \quad N_{3} \quad N_{3} \quad N_{4} \quad N_$$

3,2-6

Lagrange: 3 pts. define parabola. Piecewise Co: two straight lines. Piecewise C': two cubics.



(a)
$$u = \begin{bmatrix} 1 & x \end{bmatrix} \begin{Bmatrix} a_1 \\ a_2 \end{Bmatrix}$$
, $\begin{bmatrix} B_{\alpha} \end{bmatrix} = \frac{\partial}{\partial x} \begin{bmatrix} 1 & x \end{bmatrix} = \begin{bmatrix} 0 & 1 \end{bmatrix}$

$$\begin{bmatrix} k_{\alpha} \end{bmatrix} = \int_{0}^{L} \begin{bmatrix} B_{\alpha} \end{bmatrix}^{T} \begin{bmatrix} B_{\alpha} \end{bmatrix} AE dx = AEL \begin{bmatrix} 0 & 0 \\ 0 & 1 \end{bmatrix}$$

Let
$$x_1 = 0$$
, $x_2 = L$ in Eq. 3.2-4. Then $[A] = \begin{bmatrix} 1 & 0 \\ 1 & L \end{bmatrix}$, $[A]^{-1} = \begin{bmatrix} 1 & 0 \\ -1/L & 1/L \end{bmatrix}$
 $[k] = [A]^{-T}[k_a][A]^{-1} = \begin{bmatrix} 1 & -1/L \\ 0 & 1/L \end{bmatrix} (AEL \begin{bmatrix} 0 & 0 \\ -1/L & 1/L \end{bmatrix}) = \frac{AE}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$

(b)
$$V = \begin{bmatrix} 1 \times x^2 \times^3 \end{bmatrix} \begin{cases} a_1 \\ a_2 \\ a_3 \\ a_4 \end{cases}, \frac{d^2_{V}}{dx^2} = \begin{bmatrix} 0 & 0 & 2 & 6x \end{bmatrix} \begin{cases} a_1 \\ a_2 \\ a_3 \\ a_4 \end{cases} = \begin{bmatrix} B_a \end{bmatrix} \begin{cases} a_1 \\ a_2 \\ a_3 \\ a_4 \end{cases}$$

[A] is calculated in Problem 3.2-4

$$\begin{bmatrix} k \\ = \begin{bmatrix} A \end{bmatrix}^{-1} \begin{bmatrix} k_a \\ A \end{bmatrix}^{-1} = \begin{bmatrix} 1 & 0 & -3/L^2 & 2/L^3 \\ 0 & 1 & -2/L & 1/L^2 \\ 0 & 0 & 3/L^2 & -2/L^3 \\ 0 & 0 & -1/L & 1/L^2 \end{bmatrix} \begin{bmatrix} 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & -2 & 0 & 2 \\ 6 & 0 & -6 & 6L \end{bmatrix}$$

$$\Gamma L = F \begin{bmatrix}
12/L^{3} & 6/L^{2} & -12/L^{3} & 6/L^{2} \\
6/L^{2} & 4/L & -6/L^{2} & 2/L \\
-12/L^{3} & -6/L^{2} & 12/L^{3} & -6/L^{2} \\
6/L^{2} & 2/L & -6/L^{2} & 4/L
\end{bmatrix}$$

[A) Follow the method of Prob. 3.3-1. That is, use the "a-basis", $u = [1 \times x^2 x^3] \{g\}$, $[k_a] = \int_0^1 [B_a]^T [B_a] A E dx$, where $[B_a] = [0 \ 1 \ 2x \ 3x^2]$, $[k] = [A]^T [k_a] [A]^T$, and $[A]^T$ is computed in Prob. 3.2-4. $AE \begin{bmatrix} 0 & 0 & 0 & 0 \\ 0 & L & L^2 & L^3 \\ 0 & L^3 \frac{3}{2} L^4 \frac{9}{5} L^5 \end{bmatrix}$ $AE \begin{bmatrix} 0 & 0 & 0 & 0 \\ -1 & 0 & 1 & 0 \\ -1 & -2/L & 1/L^2 \\ 0 & 0 & 3/L^2 & -2/L^3 \\ 0 & 0 & -1/L & 1/L^2 \end{bmatrix}$ $AE \begin{bmatrix} 1 & 0 & -3/L^2 & 2/L^3 \\ 0 & 1 & -2/L & 1/L^2 \\ 0 & 0 & 3/L^2 & -2/L^3 \\ 0 & 0 & -1/L & 1/L^2 \end{bmatrix}$ $AE \begin{bmatrix} 36 & 3L & -36 & 3L \\ 3L & 4L^2 & -3L & -L^2 \\ -36 & -3L & 36 & -3L \\ 3L & -L^2 & -3L & 4L^2 \end{bmatrix}$ $AE \begin{bmatrix} 1 & 0 & -3/L^2 & 2/L^3 \\ 0 & 0 & -1/L & 1/L^2 \end{bmatrix}$ $AE \begin{bmatrix} 36 & 3L & -36 & 3L \\ 3L & 4L^2 & -3L & -L^2 \\ -3L & 4L^2 \end{bmatrix}$ $AE \begin{bmatrix} 1 & 0 & -3/L^2 & 2/L^3 \\ 0 & 0 & -1/L & 1/L^2 \end{bmatrix}$ $AE \begin{bmatrix} 36 & 3L & -36 & 3L \\ 3L & 4L^2 & -3L & -L^2 \\ -3L & 4L^2 \end{bmatrix}$

(b) Must not enforce interelement continuity of E_x at these locations. Where two elements meet, can condense the E_x d.o.f. in one of them before assembly.

$$\lceil \nu \rceil = \frac{AE}{-8} \begin{vmatrix} 7 & -8 & 1 \\ -8 & 16 & -8 \end{vmatrix}$$

3.4-1 Beam theory:
$$I = \frac{t}{12}(2c)^3 = \frac{2tc^3}{3}$$

$$V_D = V_F = -\frac{ML^2}{2EI} = -\frac{3ML^2}{4Etc^3}$$

$$U_F = -U_D = C\frac{ML}{EI} = \frac{3ML}{2Etc^2}$$

$$\epsilon_x = \frac{u_D}{L} = -\frac{3M}{2Etc^2}$$
, $\epsilon_y = \frac{v_F - v_D}{2c} = 0$,

$$\delta_{xy} = -\frac{u_D}{2c} + \frac{v_D}{L} + \frac{u_F}{2c} = \frac{u_F}{c} + \frac{v_D}{L} = \frac{3ML}{4E+c^3}$$

with
$$v=0$$
, $\sigma_x = E \epsilon_x = \frac{3M}{2tc^2}$, $\sigma_y = E \epsilon_y = 0$, $\tau_{xy} = \frac{E}{2} \gamma_{xy} = \frac{3ML}{4tc^3}$

with
$$\sigma_x$$
 & σ_{xy} from FEA, $\frac{\tau_{xy}}{\sigma_x} = \frac{L}{2c} \rightarrow \infty$ as $\frac{L}{c} \rightarrow \infty$

Obtain [B] from Eq. 3.4-10

$$\{\mathcal{E}_{1}^{2} = [\mathcal{B}_{1}^{2}]\{\mathcal{L}_{1}^{2} = \begin{bmatrix} -1/L & 0 & 1/L & 0 & 0 & 0 \\ 0 & -1/2c & 0 & 0 & 0 & 1/2c \\ -1/2c & -1/L & 0 & 1/L & 1/2c & 0 \end{bmatrix} \begin{bmatrix} 0 \\ u_{0} \\ v_{0} \\ 0 \\ 0 \end{bmatrix}$$

$$\epsilon_{x} = \frac{v_{10}}{L} = -\frac{3M}{2Etc^{2}}$$
, $\epsilon_{y} = 0$, $\delta_{xy} = \frac{V_{D}}{L} = -\frac{3ML}{4Etc^{3}}$

with
$$v = 0$$
, $\sigma_x = E \epsilon_x = -\frac{3M}{2tc^2}$, $\sigma_y = E \epsilon_y = 0$

$$\tau_{xy} = \frac{E}{2} v_{xy} = -\frac{3ML}{8tc^3}$$

Beam theory : as in part (a)

with
$$\sigma_{x} & \tau_{xy} \text{ from } FEA$$
, $\frac{\tau_{xy}}{\sigma_{x}} = \frac{L}{4c} \longrightarrow \infty$ as $\frac{L}{c} \rightarrow \infty$

(a) The only nonzero dio.f. are us and vs. From Eq. 3.4-10,

$$\begin{cases} \mathcal{E}_{x} \\ \mathcal{E}_{y} \\ \mathcal{V}_{xy} \end{cases} = \begin{bmatrix} \mathcal{B} \\ \mathcal{E}_{x} \\ \mathcal{E}_{x} \\ \mathcal{E}_{x} \\ \mathcal{E}_{x} \end{cases} = \begin{bmatrix} \mathcal{O} & \mathcal{O} \\ \mathcal{O} & 1/a \\ 1/a & \mathcal{O} \end{bmatrix} \begin{cases} u_{3} \\ v_{3} \end{cases} \quad \text{For } z = 0,$$

$$\begin{bmatrix} k \end{bmatrix} = Et \frac{a^2}{2} \begin{bmatrix} 0 & 0 & 1/a \\ 0 & 1/a & 0 \end{bmatrix} \begin{bmatrix} 1 & 1/2 \end{bmatrix} \begin{bmatrix} 0 & 0 \\ 0 & 1 \\ 1 & 0 \end{bmatrix}$$

$$\begin{bmatrix} k \end{bmatrix} = \frac{Et}{2} \begin{bmatrix} 0 & 0 & 1 \\ 0 & 1 & 0 \end{bmatrix} \begin{bmatrix} 0 & 0 \\ 0 & 1 \\ 1/2 & 0 \end{bmatrix} = \frac{Et}{2} \begin{bmatrix} 1/2 & 0 \\ 0 & 1 \end{bmatrix}$$

(b) 4 triangles each resist P with ϵ_y stiffness Et/4 4 triangles each resist P with κ_y stiffness Et/2 Net stiffness = $4\left(\frac{Et}{4} + \frac{Et}{2}\right) = 3Et$, so $V = \frac{P}{3Et}$ at center node (independent of a).

3.4-4 With u_1 and v_1 the only nonzero dof., $\epsilon_x = 0$, $\epsilon_y = \frac{v_1}{b}$, $\delta_{xy} = \frac{u_1}{b}$ $\{\xi_i^2\} = \left[\frac{B}{B}\right] \{\xi_i^2\} = \frac{1}{b} \begin{bmatrix} 0 & 0 \\ 0 & 1 \end{bmatrix} \begin{bmatrix} u_1 \\ v_i \end{bmatrix} \qquad \text{For } v = 0,$ $[k] = E(abt) \frac{1}{b^2} \begin{bmatrix} 0 & 0 & 1 \\ 0 & 1 & 0 \end{bmatrix} \begin{bmatrix} 1 & 1 & 1 \\ 0 & 1 & 0 \end{bmatrix}$ $[k] = \frac{Eat}{b} \begin{bmatrix} 0 & 0 & 1 \\ 0 & 1 & 0 \end{bmatrix} \begin{bmatrix} 0 & 0 \\ 0 & 1 \\ 0 & 1 \end{bmatrix} = \frac{Eat}{b} \begin{bmatrix} 1/2 & 0 \\ 0 & 1 \end{bmatrix}$

(a) Let
$$\phi = u$$
 or v . ϕ depends only on y . Assume $\phi = c_1 + c_2 y + c_3 y^2$ (a)

Substitute:

$$\phi = 0$$
 at $y = 0$, $\therefore c_1 = 0$
 $\phi = 0$ at $y = b$, $0 = c_2b + c_3b^2$
 $\phi = 1$ at $y = 2b$, $1 = c_2(2b) + c_3(4b^2)$
Solve: $c_2 = -\frac{1}{2b}$, $c_3 = \frac{1}{2b^2}$
Eq. (a) becomes $\phi = -\frac{y}{2b} + \frac{y^2}{2b^2}$

(b) On
$$y=0$$
, $N_4=1-\left(\frac{x}{a}\right)^2$
Zero at $x=\pm a$, unity at $x=0$
On $y=b$, $N_4=-\left(\frac{x}{a}\right)^2+\frac{1}{4}$
Zero at $x=\pm (a/2)$
At $y=2b$, $N_4=1-2+1=0$

At
$$y = 2b$$
, $N_4 = 1 - 2 + 1 = 0$

(c) Here
$$u = N_3 u_3 + N_4 u_4$$
, $V = N_3 V_3 + N_4 V_4$
 $E_x = \frac{\partial N_3}{\partial x} u_3 + \frac{\partial N_4}{\partial x} u_4 = -\frac{2x}{a^2} u_4$

$$\epsilon_y = \frac{\partial N_3}{\partial y} V_3 + \frac{\partial N_4}{\partial y} V_4
= \left(-\frac{1}{2b} + \frac{y}{b^2} \right) V_3 + \left[-\frac{1}{b} + \frac{y}{2b^2} \right] V_4$$

$$\delta_{xy} = \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} = (---)u_3 + [---]u_4 - \frac{2x}{a^2}v_4$$

 $= a_3 + a_5$

Eqs. 3.6-4 and 3.6-5 show that
$$[N]\{d\} = \begin{cases} a_1 + a_2 x + a_3 y \\ a_4 + a_5 x + a_6 y \end{cases}$$

(b)
$$\epsilon_{x} = \frac{1}{4ab} \left[-(b-y) \ 0 \ b-y \ 0 \ b+y \ 0 \ -(b+y) \ 0 \right] \{ \frac{1}{ab} \}$$

$$= a_{2}$$

$$\epsilon_{0} = \frac{1}{4ab} \left[0 \ -(a-x) \ 0 \ -(a+x) \ 0 \ a+x \ 0 \ a-x \right] \{ \frac{1}{ab} \}$$

$$= a_{6}$$

$$\delta_{xy} = \frac{1}{4ab} \left[-(a-x) \ -(b-y) \ -(a+x) \ b-y \ a+x \ b+y \ a-x \ -(b+y) \right] \{ \frac{1}{ab} \}$$

3.6-3 Beam theory will give up = - UD, VF = VD (a) From Eq. 3.6-6, From beam theory, Prob. 3.4-1, $u_b = -\frac{3ML}{2Etc^2}$, $v_b = -\frac{3ML'}{4Etc^3}$ Hence $\epsilon_{x} = \frac{3MLy}{4Ftahc^{2}}, \quad \epsilon_{y} = 0, \quad \gamma_{xy} = \frac{3ML(a+x)}{4ahEtc^{2}} - \frac{3ML^{2}}{8aEtc^{3}}$ For $\nu = 0$, $\sigma_x = E \epsilon_x = \frac{3MLy}{4+ahc^2}$, $\sigma_y = E \epsilon_y = 0$, $T_{xy} = \frac{E}{2} T_{xy} = \frac{3ML(a+x)}{8abtc^2} - \frac{3ML^2}{16atc^3}$ But L= 2a and c=b, so $\sigma_{x} = \frac{3My}{2+13}$, $\sigma_{y} = 0$, $T_{xy} = \frac{3Mx}{413+}$ (in the coordinate system of Fig. 3.6-1). In this system, $\sigma_{\rm X} = \frac{My}{I} = \frac{3My}{2tb^3}$, $\sigma_{\rm F} = \sigma_{\rm Xg} = 0$ according to beam theory. (b) From beam theory, Prob. 3,4-2, $u_0 = \frac{3PL^2}{4F+r^2}$, $v_0 = \frac{PL^3}{2Etc^3}$ Hence $\epsilon_{x} = -\frac{3PL^{2}y}{8abE+c^{2}}, \epsilon_{y} = 0, \quad \chi_{xy} = \frac{PL^{3}}{10E+c^{3}} = \frac{3PL^{2}(a+x)}{8abE+c^{2}}$ For $\nu=0$, $\sigma_{x}=E\epsilon_{x}=-\frac{3PL^{2}u}{8ah+c^{2}}$, $\sigma_{y}=E\epsilon_{y}=0$, $T_{xy} = \frac{E}{2} Y_{xy} = \frac{PL^3}{40 + c^3} - \frac{3PL^2(a+x)}{160h + c^2}$ But L=2a and c=6, so $\sigma_{x} = \frac{3Pay}{2+13}$, $\sigma_{y} = 0$, $\tau_{xy} = \frac{Pa}{1+L^{3}}(a-3x)$ (in the coordinate system of Fig. 3.6-1). In this system,

(in the coordinate system of Fig. 3.6-1). In this system, $\sigma_{x} = \frac{My}{I} = \frac{P(a-x)y}{t(2b)^{3}/12} = \frac{3P(a-x)y}{2tb^{3}}, \quad \sigma_{y} = 0, \text{ and}$ $\sigma_{xy} = \frac{3}{2} \frac{P}{2tb} = \frac{3P}{4tb} \quad \text{according to beam theory.}$

(a) $K_{48,39}$ is associated with V_{24} and U_{20} . Since node 24 is not in element j, el. j does not contribute to $K_{48,39}$ (b) $K_{37,37} = k_{1,1}$ of el. j. [E] is diagonal (since v = 0), [E] = $I0^7 [1 \ 1 \ \frac{1}{2} \ 1$. We need only col. 1 of [B], Eq. 3.6-6 $k_{1,1} = \frac{1}{16} \begin{bmatrix} 1 \ -(1-y) \end{bmatrix} \begin{bmatrix} -(1-y) \ 0 \end{bmatrix} \begin{bmatrix} -(1-y) \ -\frac{1}{2}(1-x) \end{bmatrix}$ After integration, $k_{1,1} = 10^7/2$. (c) $K_{59,61} = k_{3,5}$ of el. j. Need cols. 3 8 5 of [B], Eq. 3.6-6. As in (b), $k_{3,5} = \frac{1}{16} \begin{bmatrix} 1-y \ 0 \end{bmatrix} \begin{bmatrix} 1+y \ 0 \end{bmatrix} \begin{bmatrix} 1-y \ 0 \end{bmatrix} \begin{bmatrix} 1+y \ 0 \end{bmatrix} \begin{bmatrix} 1-y \ 0 \end{bmatrix} \begin{bmatrix} 1+y \ 0 \end{bmatrix} \begin{bmatrix} 1-y \ 0 \end{bmatrix} \begin{bmatrix} 1+y \ 0 \end{bmatrix} \begin{bmatrix} 1-y \ 0 \end{bmatrix} \begin{bmatrix} 1+y \ 0 \end{bmatrix} \begin{bmatrix} 10^7(1) \ 0 \ 0 \end{bmatrix} dx dy$ After integration, $k_{3,5} = 0$

Method 1: Evaluate u=a1+a2x + azy + axy at nodes, solve for a's, gather coefficients of u, uz, uz, u4. $u_1 = a_1$ $u_2 = u_1 + a_2(2a)$; $a_2 = \frac{u_2 - u_1}{2a}$ Node 2 $u_4 = u_1 + a_3(2b)$; $a_3 = \frac{u_4 - u_1}{2L}$ Node 4 $u_3 = u_1 + \frac{u_2 - u_1}{2a} 2a + \frac{u_4 - u_1}{2b} 2b + a_4(2a)(2b)$ from which $a_4 = \frac{u_1 - u_2 + u_3 - u_4}{4ab}$ $u = u_1 + \frac{u_2 - u_1}{2b} x + \frac{u_4 - u_1}{2b} y + \frac{u_1 - u_2 + u_3 - u_4}{2b} xy$ $u = \left(1 - \frac{x}{2a} - \frac{y}{2h} + \frac{xy}{4ah}\right)u_1 + \left(\frac{x}{2a} - \frac{xy}{4ah}\right)u_2$ $+\left(\frac{xy}{44}\right)u_3 + \left(\frac{y}{2h} - \frac{xy}{44h}\right)u_4$ Coefficients of the ui are the Ni. Method 2: Take the product of onedimensional Ni (as for a bar element). $N_1 = \frac{2a - x}{2a} \frac{2b - y}{2h}$ $N_2 = \frac{x}{2a} \frac{2b - y}{2h}$ $N_4 = \frac{2a - x}{2a} \frac{4}{2b}$ $N_3 = \frac{\times}{2a} \frac{y}{2h}$ Method 3: In Egs. 3.6-4, replace x by x-a and y by y-b.

3.6-6

Consider the lower-left element of the four-element structure.

In the assembled structure, v3 is the only nonzero d.o.f., so we need only this one stiffness coefficient.

$$k = \frac{1}{16a^4} \begin{cases} d \cdot a \\ 0 \cdot a + x \cdot a + y \end{cases} \begin{bmatrix} E \\ E/2 \end{bmatrix} \begin{cases} 0 \\ a + x \\ a + y \end{cases} t dx dy$$

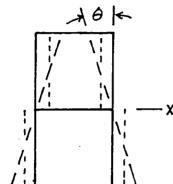
$$k = \frac{Et}{16a^4} \int_{-a}^{a} \left[(a+x)^2 + \frac{(a+y)^2}{2} \right] dx dy = \frac{Et}{16a^4} \left[\frac{16a^4}{3} + \frac{8a^4}{3} \right] = \frac{Et}{2}$$

For the four-element structure, $V_{center} = \frac{P/4}{F+1/2} = \frac{P}{2F+1/2}$

Assume that beam theory would provide almost the exact tip deflection, and recall Eq. 3.6-11:

 $\frac{\theta_{el}}{\theta_b} = \frac{1 - \nu^2}{1 + \frac{1 - \nu}{2} \left(\frac{a}{b}\right)^2}$

- (a) Expect the 2nd mesh to be better, since a/b is half as large in the 2nd mesh
 - (6) Two elements of the first mesh, with bending deformation:



Smaller dashed lines represent deformation due to ϵ_x in elements. Assume this is exact. Remainder comes from rotation of element faces and suffers from parasitic shear: with $\nu=0$,

$$\frac{\theta_{el}}{\theta_b} = \frac{1}{1 + \frac{1}{2}(1)} = \frac{2}{3}$$

Assume that V_{xy} and E_{x} contribute equally, therefore estimate error as the average:

$$e \approx \frac{1}{2} \left[0 + \left(1 - \frac{2}{3} \right) \right] \approx 17\%$$

For the second mesh, $\frac{a}{b} = \frac{1}{2}$, so with v = 0

$$\frac{\partial el}{\partial b} = \frac{1}{1 + \frac{1}{2}(\frac{1}{2})^2} = 0.89$$
 i.e. $e \approx 11\%$

3.7-1

$$\phi_{152} = \frac{-x(a-x)}{2a^{2}}\phi_{1} + \frac{(a-x)(a+x)}{a^{2}}\phi_{5} + \frac{x(a+x)}{2a^{2}}\phi_{2}$$

$$\phi_{43} = \frac{a-x}{2a}\phi_{4} + \frac{a+x}{2a}\phi_{3}$$

$$\phi = \frac{b-y}{2b}\phi_{152} + \frac{b+y}{2b}\phi_{43} = \sum_{i=1}^{5} N_{i}\phi_{i}$$

$$N_{1} = \frac{-x(a-x)(b-y)}{4a^{2}b}, N_{2} = \frac{x(a+x)(b-y)}{4a^{2}b}$$

$$N_{3} = \frac{(a+x)(b+y)}{4ab}, N_{4} = \frac{(a-x)(b+y)}{4ab}$$

$$N_{5} = \frac{(a^{2}-x^{2})(b-y)}{2a^{2}b}$$

3.7-2

(a)
$$\phi_{184} = \sum_{i=1}^{3} \overline{N}_{i} * (\phi_{i} \text{ on } x=-a)$$

$$\phi_{597} = \sum_{i=1}^{3} \overline{N}_{i} * (\phi_{i} \text{ on } x=0)$$

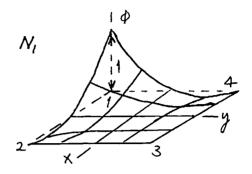
$$\phi_{263} = \sum_{i=1}^{3} \overline{N}_{i} * (\phi_{i} \text{ on } x=+a)$$

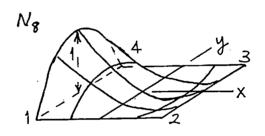
for which \overline{N}_1 , \overline{N}_2 , \overline{N}_3 come from Eq. 3.2-7 with $x \to y$, $x_1 \to -b$, $x_2 = 0$, $x_3 \to b$ $\overline{N}_1 = \frac{-y(b-y)}{2b^2}$, $\overline{N}_2 = \frac{(-b-y)(b-y)}{-b(b)} = \frac{b^2-y^2}{b^2}$ $\overline{N}_3 = \frac{(-b-y)(-y)}{-2b(-b)} = \frac{y(b+y)}{2b^2}$. In 9-node el., $\phi = \frac{-x(a-x)}{2a^2}\phi_{84} + \frac{a^2-x^2}{a^2}\phi_{597} + \frac{x(a+x)}{2a^2}\phi_{263}$

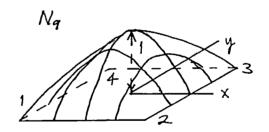
 $\phi = N_{a}\phi_{184} + N_{b}\phi_{5q7} + N_{c}\phi_{263}$ $\phi = N_{a}(\vec{N}_{1}\phi_{1} + \vec{N}_{2}\phi_{8} + \vec{N}_{3}\phi_{4}) + N_{b}(\vec{N}_{1}\phi_{5} + \vec{N}_{2}\phi_{q} + \vec{N}_{3}\phi_{7}) + N_{c}(\vec{N}_{1}\phi_{2} + \vec{N}_{2}\phi_{6} + \vec{N}_{3}\phi_{3})$ $\phi = \sum_{i=1}^{q} N_{i}\phi_{i} \quad \text{where}$ $\frac{q}{2} N_{i}\phi_{i} \quad \text{where}$

 $N_{1} = N_{a} \overline{N}_{1} = xy(a-x)(b-y)/4a^{2}b^{2}$ $N_{2} = N_{c} \overline{N}_{1} = -xy(a+x)(b-y)/4a^{2}b^{2}$ $N_{3} = N_{c} \overline{N}_{3} = xy(a+x)(b+y)/4a^{2}b^{2}$ $N_{4} = N_{a} \overline{N}_{3} = -xy(a-x)(b+y)/4a^{2}b^{2}$ $N_{-} = N. \overline{N}_{1} = -(a^{2}-x^{2})y(b-y)/2a^{2}b^{2}$ $N_{6} = N_{c} N_{z} = x(a+x)(b^{2}-y^{2})/2a^{2}b^{2}$ $N_{7} = N_{b} \overline{N}_{3} = (a^{2}-x^{2})y(b+y)/2a^{2}b^{2}$ $N_{8} = N_{a} \overline{N}_{z} = -x(a-x)(b^{2}-y^{2})/2a^{2}b^{2}$ $N_{9} = N_{0} \overline{N}_{1} = -(a^{2}-x^{2})(b^{2}-y^{2})/2a^{2}b^{2}$ $N_{1} = -(x^{2}-x^{2})(b^{2}-y^{2})/a^{2}b^{2}$

(b)

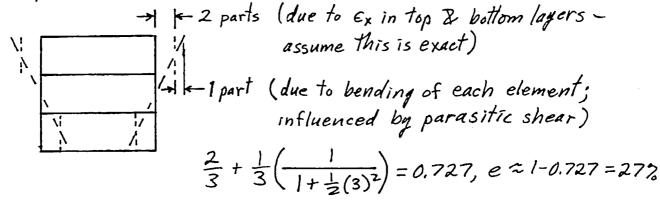






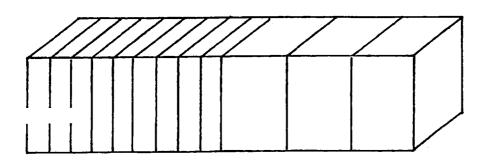
(c) Aspect ratio
$$\frac{1}{3}$$
, $\frac{1}{1+\frac{1}{2}(\frac{1}{3})^2} = 0.947$, $e \approx 1-0.947 = 5.3\%$

(b) Aspect ratio 3



(c) Aspect ratio 1,
$$\frac{1}{1+\frac{1}{2}(1)^2} = 0.667$$
, $e \approx 1-0.667 = 33\%$

A better 12-element mesh (for the given loading):



Rigid-body lateral translation \bar{u} : $\{d\} = [\bar{u} \ O \ \bar{u} \ O]^T$ Rigid-body rotation through small angle Θ about node 1: $\{d\} = [O \ \Theta \ L\Theta \ \Theta]^T$

In both cases, straightforward multiplication shows that [B]{d} and [k]{d} are both zero.

(a)
$$\frac{1}{a} \xrightarrow{\frac{1}{\beta}} W = a + \beta \times \text{ gives}$$

$$w_1 = a, \theta_1 = \beta$$

$$w_2 = a + \beta L, \theta_2 = \beta$$

Subs. into given field; get $w = \alpha + \beta x$ 01

(b) From given field,
$$w_{,xx} = (\theta_2 - \theta_1)/L$$

By beam theory: let $M_0 = const.$ moment

$$EIw_{,xx} = M_o (a)$$

$$\int_{\theta_1}^{\theta_2} EIdw_{,x} = \int_{0}^{M_o} dx$$

$$Eliminate M_o$$
between (a) 2
$$(b)', get$$

$$EI(\theta_2 - \theta_1) = M_oL (b)$$

$$w_{,xx} = (\theta_2 - \theta_1)/L$$

Eliminate Mo
between (a) I
(b); get
$$w_{ixx} = (\theta_2 - \theta_1)/L$$

(c)
$$w_{,xx} = \lfloor B \rfloor \{d\} = \left[0, -\frac{1}{L}, 0, \frac{1}{L}\right] \left[w_i \theta_i w_j \theta_j\right]$$

and develops no nodal forces in response to 0, & B. [k] has rank 1.

(a)

Evaluate u= [1, x2]{a} at nodes.

$$\begin{cases} u_1 \\ u_2 \end{cases} = \begin{bmatrix} 1 & 0 \\ 1 & L^2 \end{bmatrix} \begin{cases} a_1 \\ a_2 \end{cases}, \begin{cases} a_1 \\ a_2 \end{cases} = \frac{1}{L^2} \begin{bmatrix} L^2 & 0 \\ -1 & 1 \end{bmatrix} \begin{cases} u_1 \\ u_2 \end{cases} = \begin{bmatrix} A \end{bmatrix} \{ u_1 \}$$

$$u = \begin{bmatrix} 1 \\ X^2 \end{bmatrix} \begin{bmatrix} A \end{bmatrix}^{-1} \{ u_1 \} = \begin{bmatrix} L^2 - X^2 & X^2 \\ L^2 & L^2 \end{bmatrix} \{ u_1 \}$$

$$\epsilon_{x} = \begin{bmatrix} -\frac{2x}{L^2} & \frac{2x}{L^2} \end{bmatrix} \{ u_1 \} = \begin{bmatrix} B \\ X \end{bmatrix} \{ u_1 \}$$

$$\begin{bmatrix} K \end{bmatrix} = \begin{bmatrix} L \\ B \end{bmatrix}^{T} \begin{bmatrix} B \\ X \end{bmatrix} AE d_{X} = \frac{4AE}{3L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

Too large by factor $\frac{4}{3}$. Source is defective u: no x^1 term present, so state of constant ϵ_x is not possible.

(b) Evaluate
$$u = [x, x^{2}]\{a\}$$
 at nodes.
$$\begin{cases} u_{1} \\ u_{2} \end{cases} = \begin{bmatrix} -\frac{L}{2} & \frac{L^{2}}{4} \\ \frac{L}{2} & \frac{L^{2}}{4} \end{bmatrix} \begin{cases} a_{1} \\ a_{2} \end{cases}, \begin{cases} a_{1} \\ a_{2} \end{cases} = \frac{1}{L^{2}} \begin{bmatrix} -L & L \\ 2 & 2 \end{bmatrix} \begin{bmatrix} u_{1} \\ u_{2} \end{cases} = \begin{bmatrix} A \end{bmatrix}^{-1} \{u\}$$

$$u = [x, x^{2}][A]^{-1} \{u\} = \begin{bmatrix} \frac{2x^{2}-Lx}{L^{2}} & \frac{2x^{2}+Lx}{L^{2}} \end{bmatrix} \{u\}$$

$$\epsilon_{x} = \begin{bmatrix} \frac{4x-L}{L^{2}} & \frac{4x+L}{L^{2}} \end{bmatrix} \{u\} = \begin{bmatrix} B \end{bmatrix} \{u\}$$

$$[K] = \begin{bmatrix} L/2 & L \\ L^{2} & L \end{bmatrix}^{T} [B] AE dx = \frac{AE}{3L} \begin{bmatrix} 7 & 1 \\ 1 & 7 \end{bmatrix}$$

body translation). Source of trouble is defective u: no constant term present.

Exact ans:
$$u_z = \frac{PL}{AE}$$
, $u_3 = 2\frac{PL}{AE}$

(a) $\frac{4AE}{3L} \begin{bmatrix} 2 & -1 \\ -1 & 1 \end{bmatrix} \begin{bmatrix} u_2 \\ u_3 \end{bmatrix} = \begin{cases} 0 \\ P \end{cases}$, $\begin{cases} u_2 \\ u_3 \end{cases} = \frac{PL}{3/4} \begin{cases} 3/4 \\ AE \end{cases}$

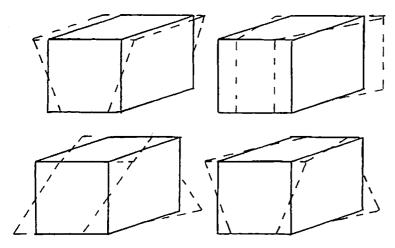
$$\sigma_{x=0} = E[O, O]\{d\} = O \quad (\text{i=xact } \sigma_x \text{ is } P/A \text{ for all } x)$$

(b) $AE \begin{bmatrix} 14 & 1 \\ 1 & 7 \end{bmatrix} \begin{bmatrix} u_2 \\ u_3 \end{bmatrix} = \begin{cases} 0 \\ P \end{cases}$, $\begin{cases} u_1 \\ u_3 \end{cases} = \frac{PL}{AE} \begin{cases} -3/97 \\ 42/97 \end{cases}$

$$\sigma_{x=0} = E \begin{bmatrix} -3L \\ L^2 \end{cases}$$
, $\frac{-L}{L^2} \begin{bmatrix} u_1 \\ u_2 \end{bmatrix} = -\frac{1}{L} \frac{-3PL}{97AE} = \frac{3P}{97A}$

at $x=0$ in structure; at $x=-\frac{L}{2}$

In element.



With the node numbering of Fig. 3.8 la, x-direction nodal dio.f. of the foregoing states are, with c a constant,

3.10-1

3.10-2

Consider e.q. a quadrilateral element, and apply Eq. 3.10-1 to each side.

$$\frac{\delta_{m_1}}{L_1} = \frac{1}{8}(\omega_2 - \omega_1)$$

$$\frac{\delta_{m_2}}{L_2} = \frac{1}{8}(\omega_3 - \omega_2)$$

$$\frac{\delta_{m_3}}{L_3} = \frac{1}{8}(\omega_4 - \omega_3)$$

$$\frac{\delta_{m_4}}{L_4} = \frac{1}{8}(\omega_1 - \omega_4)$$

Add; thus
$$\sum_{i=1}^{4} \frac{\delta_{mi}}{L_{i}} = 0$$

Eq. 3.11-6:
$$\begin{cases} F_{4} \\ F_{7} \\ F_{3} \end{cases} = \frac{a}{15} \begin{bmatrix} 4 & 2 - 1 \\ 2 & 16 & 2 \\ -1 & 2 & 4 \end{bmatrix} \begin{bmatrix} 9_{4} \\ 9_{7} \\ 9_{3} \end{bmatrix}$$
[H]

Unit thickness

(a)
$$\begin{cases} F_4 \\ F_7 \\ F_3 \end{cases} = \begin{bmatrix} H \end{bmatrix} \begin{cases} \sigma \\ 0 \\ -\sigma \end{cases} = \frac{\sigma a}{3} \begin{cases} 1 \\ 0 \\ -1 \end{cases}$$

(a) $\begin{cases} F_4 \\ F_7 \\ F_3 \end{cases} = \begin{bmatrix} H \end{bmatrix} \begin{cases} \sigma \\ 0 \\ -\sigma \end{cases} = \frac{\sigma a}{3} \begin{cases} 1 \\ 0 \\ -1 \end{cases}$ Couple-minent: $M = F_4(2a) = \frac{2\sigma a^2}{3}$ Flexure formula: $M = \frac{\sigma r}{c} = \frac{\sigma (2a)^3/12}{a} = \frac{2\sigma a^2}{3}$

(b)
$$\begin{cases} F_4 \\ F_7 \\ F_3 \end{cases} = \begin{bmatrix} H \end{bmatrix} \begin{cases} 0 \\ \sigma/2 \\ \sigma \end{cases} = \frac{\sigma a}{3} \begin{cases} 0 \\ 2 \\ 1 \end{cases}$$

 $M = \frac{\sigma I}{c} = \frac{\sigma (4a)^3/12}{c} = \frac{8\sigma a^2}{a^2}$

Contribution of half the section is $\frac{M}{2} = \frac{40a^2}{3}$

(c)
$$\{F_4\}_{F_7} = [H] \{0\}_{0} = \frac{\pi a}{15} \{2\}_{16}$$
 Shear force: $F_4 + F_7 + F_3 = \frac{4\pi a}{3}$ Beam theory (parabolic distribution): $\pi = \frac{3}{5} \text{ V}$ $V = 4\pi a$

$$\gamma = \frac{3}{2} \frac{V}{2a}$$
, $V = \frac{4\gamma a}{3}$

With [N] from Eq. 3.11-5, and constant q,

$$\begin{cases}
F_1 \\
F_2 \\
F_3
\end{cases} = \int_{-a}^{a} [N] q dx = \int_{-a}^{a} \frac{1}{2a^2} \begin{cases} x(x-a) \\ 2(a^2-x^2) \\ x(x+a) \end{cases} q dx$$

$$= \frac{q}{2a^{2}} \left\{ \frac{x^{3} - ax^{2}}{3} - \frac{ax^{2}}{2} \right\}^{2} = \frac{q}{2a^{2}} \left\{ \frac{2a^{3}/3}{8a^{3}/3} \right\} = q \left\{ \frac{a/3}{4a/3} \right\} = F \left\{ \frac{1/6}{2/3} \right\}$$

$$= \frac{q}{2a^{2}} \left\{ \frac{x^{3} - ax^{2}}{3} + \frac{ax^{2}}{2} \right\}^{2} = \frac{q}{2a^{2}} \left\{ \frac{2a^{3}/3}{3} \right\} = q \left\{ \frac{a/3}{4a/3} \right\} = F \left\{ \frac{1/6}{2/3} \right\}$$

$$= \frac{q}{2a^{2}} \left\{ \frac{x^{3} - ax^{2}}{3} + \frac{ax^{2}}{2} \right\}^{2} = \frac{q}{2a^{2}} \left\{ \frac{2a^{3}/3}{3} \right\} = q \left\{ \frac{a/3}{4a/3} \right\} = F \left\{ \frac{1/6}{2} \right\}$$

where F=2ga

3.11-3

$$M_1 \delta \theta_{21} = \int_0^L v \, q \, dx$$
 where $v = N_2 \delta \theta_{21}$ and q is constant
Hence $M_1 = q \int_0^L (x - \frac{2x^2}{L} + \frac{x^3}{L^2}) dx = q L^2 (\frac{1}{2} - \frac{2}{3} + \frac{1}{4}) = \frac{q L^2}{12}$

$$\{x_{e}\} = \begin{cases} x - \frac{x^{3}}{L^{2}} + \frac{x^{4}}{2L^{3}} \\ |x|^{2} + \frac{x^{4}}{2L^{3}} + \frac{x^{4}}{4L^{2}} \\ \frac{x^{2}}{L^{2}} - \frac{2x^{3}}{3L} + \frac{x^{4}}{4L^{2}} \\ -\frac{x^{3}}{3L} + \frac{x^{4}}{4L^{2}} \\ 0 \end{cases}$$

$$\begin{cases} x - \frac{x^{3}}{L^{2}} + \frac{x^{4}}{2L^{3}} \\ \frac{x^{2}}{L^{2}} - \frac{2x^{3}}{2L^{3}} \\ -\frac{x^{3}}{3L} + \frac{x^{4}}{4L^{2}} \\ 0 \end{cases}$$

$$\{r_{e}\} = q \begin{cases} 0.40625L \\ 0.05729L^{2} \\ 0.09375L \\ -0.02604L^{2} \end{cases} = \frac{qL}{8}$$

$$= \frac{qL}{8}$$

(6)
$$\left\{ \int_{-\infty}^{\infty} e^{-t} dt \right\}_{x}^{T} M_{c}$$
 where $\left[\frac{dN}{dx} \right]_{x}^{T}$ is evaluated at $x = \frac{L}{2}$

From Ni in Fig. 3.2-4 we obtain

$$\begin{cases} -\frac{6x}{L^{2}} + \frac{6x^{2}}{L^{3}} \\ 1 - \frac{4x}{L} + \frac{3x^{2}}{L^{2}} \\ \frac{6x}{L^{2}} - \frac{6x^{2}}{L^{3}} \\ -\frac{2x}{L} + \frac{3x^{2}}{L^{2}} \\ x = \frac{L}{2} \end{cases} = \begin{cases} -\frac{c}{2L} + \frac{c}{4L} \\ 1 - 2 + \frac{3}{4L} \\ -\frac{1}{4L} \end{cases} = \begin{cases} -\frac{3}{2L} \\ -\frac{1}{4L} \\ -\frac{1}{4L} \end{cases}$$

$$\frac{L}{2} + \frac{L}{2} \rightarrow \frac{3M_c}{2L}$$

$$\frac{3M_c}{2L}$$

$$\frac{3M_c}{2L}$$

$$3.11-5$$

$$V = \begin{bmatrix} N_4 & N_7 & N_3 \end{bmatrix}^T \begin{Bmatrix} V_4 \\ V_7 \\ V_3 \end{Bmatrix}$$

Apply Eq. 3.2-7:
$$N_4 = \frac{-x(a-x)}{a(2a)} = -\frac{x(a-x)}{2a^2}$$

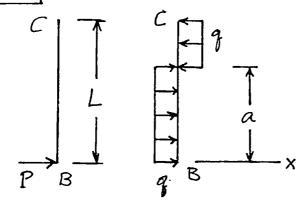
$$N_7 = \frac{(-a-x)(a-x)}{-a(a)} = \frac{a^2-x^2}{a^2}$$

$$N_3 = \frac{(-a-x)(-x)}{-2a(-a)} = \frac{x(a+x)}{2a^2}$$

$$\begin{cases} F_4 \\ F_7 \\ F_3 \end{cases} = \begin{cases} N_4 \\ N_7 \\ N_3 \end{cases}_{x=\frac{a}{a}} = \begin{cases} -\frac{(a/2)^2}{2a^2} \\ \frac{a^2-(a/2)^2}{a^2} \\ \frac{a/2(3a/2)}{2a^2} \end{cases} F = \begin{cases} -1/8 \\ 3/4 \end{cases}_{x=\frac{a}{a}} = \begin{cases} \frac{3F}{4} & \frac{3F}{8} \\ \frac{3}{4} & \frac{3F}{4} \end{cases}$$

$$F_4 + F_7 + F_3 = F$$
 $\sum M_4 = \frac{3F}{4}a + \frac{3F}{8}(2a) = \frac{3Fa}{4}$





where
$$q = \frac{(1+V2)P}{L}$$

$$a = \frac{L}{V2}$$

x-direction force:
$$qa-q(L-a)=q(2a-L)$$

$$=\frac{(1+\sqrt{2})P}{L}(\frac{2L}{\sqrt{2}}-L)$$

$$=\frac{(1+\sqrt{2})(2-\sqrt{2})P}{\sqrt{2}}$$

$$=\frac{2+\sqrt{2}-2}{\sqrt{2}}P=P$$

Moment about B:
$$qa\frac{a}{2} - q(L-a)\frac{L+a}{2} = \frac{q}{2}(2a^2-L^2)$$

= $\frac{q}{2}(L^2-L^2) = 0$

3.11-7

Evaluate Ni of Eqs. 3.6-4 at Q $(x=\frac{a}{2}, y=0)$;

$$\{r_e\} = \begin{bmatrix} N \end{bmatrix}_a^T \begin{Bmatrix} P_x \\ P_y \end{Bmatrix} = \frac{1}{4ab} \begin{bmatrix} ab/2 & 0 & 3ab/2 & 0 & 3ab/2 & 0 & ab/2 & 0 \\ 0 & ab/2 & 0 & 3ab/2 & 0 & 3ab/2 & 0 & ab/2 & P_y \end{Bmatrix}$$

$$\begin{cases} r_e \\ = \frac{1}{8} \begin{bmatrix} 1 & 0 & 3 & 0 & 3 & 0 & 1 & 0 \end{bmatrix}^T \begin{cases} 8 \\ 6 \\ 6 \\ \end{cases}$$

$$\begin{cases} r_e \\ = \begin{bmatrix} 1 & \frac{3}{4} & 3 & \frac{9}{4} & 3 & \frac{9}{4} & 1 & \frac{3}{4} \end{bmatrix}^T$$

3.11-8 Get Ni from Eq. 3.2-7. Let
$$a = \frac{L}{2}$$
. $B_i = \frac{d}{dx} N_i$
 $N_i = -\frac{x(a-x)}{2a^2}$ $N_2 = \frac{a^2-x^2}{a^2}$ $N_3 = \frac{x(a+x)}{2a^2}$
 $B_1 = -\frac{a-2x}{2a^2}$ $B_2 = -\frac{2x}{a^2}$ $B_3 = \frac{a+2x}{2a^2}$

$$\{r_{e}\} = \int \left[\mathbb{E}\right]^{T} \left[\mathbb{E}\right] \{\xi_{o}\} dV = \begin{bmatrix} a \\ B_{2} \\ B_{3} \end{bmatrix} = \left(\alpha T_{3} \frac{a+x}{2a}\right) A dx$$

$$\{r_{e}\} = \frac{EA \alpha T_{2}}{4a^{3}} \begin{cases} a \\ -(a-2x) \\ -4x \\ a+2x \end{cases} \{a+x\} dx = \frac{EA \alpha T_{3}}{4a^{3}} \begin{cases} a \\ -a^{2}+x+2x^{2} \\ -4ax-4x^{2} \\ a^{2}+3x+2x^{2} \end{cases} dx$$

$$\{r_{e}\} = \frac{EA \alpha T_{3}}{4a^{3}} \begin{cases} -2a^{3}+4a^{3}/3 \\ -8a^{3}/3 \\ 2a^{3}+4a^{3}/3 \end{cases} = \frac{EA \alpha T_{3}}{4a^{3}} \begin{cases} -2a^{3}/3 \\ -8a^{3}/3 \\ 10a^{3}/3 \end{cases} = \frac{EA \alpha T_{3}}{6} \begin{cases} -1 \\ -4 \\ 5 \end{cases}$$

 $\{r_e\} = -\int [\mathcal{B}]^T \{g_o\} dV$

To show Σ rei = 0, we ask if $LIJ\{re\}$ = 0, where LIJ is a row vector filled with 1's. We get Σ rei = 0 if $LIJ\{B_i\}$ = 0, where $\{B_i\}$ is a column of $[B_i]$ for all j in $[B_i]$. Now $[B_i] = [\partial_i][N]$, so each row of $[B_i]$ (and each column of $[B_i]$) is a derivative of $[N_i]$, But $\Sigma N_i = 1$ for C° elements, so $\Sigma N_{i,x} = 0$.

$$\begin{split} & [E] \{ \mathcal{E}_{o} \} = \frac{E}{1 - \nu^{2}} \begin{bmatrix} 1 & \nu & O \\ \nu & 1 & O \\ O & O & \frac{1 - \nu}{2} \end{bmatrix} \begin{pmatrix} x T_{o} x \\ x T_{o} x \\ O \end{pmatrix} = \frac{E \times T_{o} x}{1 + \nu} \begin{cases} 1 \\ 1 \\ O \end{cases} \\ & \{ \mathcal{L}_{e} \} = \left\{ \left[\mathcal{E}_{o} \right]^{T} [E] \{ \mathcal{E}_{o} \} t dx dy, \ N_{i} = \frac{(a \pm x)(b \pm y)}{4ab} \right\} \end{split}$$

Ni,x indep. of x but [6] linear in x, so integration w.r.t. x with limits -a to ta yields zero x-direction nodal loads.

{xe} =
$$\frac{E \times T_0}{(1+\nu)4ab}$$

{re} = $\frac{E \times T_0}{(1+\nu)4ab}$

{re} = $\frac{E^2 \times T_0}{(1+\nu)4ab}$

3.11-11

(a) First the nodal loads all 3 cases.

$$[N] = \left\lfloor \frac{L-s}{L} \right\rfloor, F = \frac{c}{A}(L_T-x)$$
 $\{r_e\} = \int_{0}^{L} N_1^T F dV$ where $dV = Ads$

One elements: in the first, $s = x$, $L_T = 2L$

in the second, $x = L + s$, $L_T = 2L$
 $\{r_e\}_1 = \frac{cL^2}{6} \left\{\frac{5}{4}\right\}$, $\{r_e\}_2 = \frac{cL^2}{6} \left\{\frac{2}{1}\right\}$

Three elements: $L_T = 3L$; in the respective elements, $x = s$, $x = L + s$, $x = 2L + s$.

 $\{r_e\}_1 = \frac{cL^2}{6} \left\{\frac{8}{4}\right\}$, $\{r_e\}_2 = \frac{cL^2}{6} \left\{\frac{2}{1}\right\}$

Three elements: $L_T = 3L$; in the respective elements, $x = s$, $x = L + s$, $x = 2L + s$.

 $\{r_e\}_1 = \frac{cL^2}{6} \left\{\frac{8}{4}\right\}$, $\{r_e\}_2 = \frac{cL^2}{6} \left\{\frac{2}{4}\right\}$. $\{r_e\}_2 = \frac{cL^2}{6} \left\{\frac{2}{1}\right\}$

(b) One e(. $\frac{AE}{L} u_2 = \frac{cL^2}{6} \left\{\frac{5}{4}\right\}$, $\{r_e\}_3 = \frac{cL^2}{6} \left\{\frac{4}{1}\right\}$
 $\begin{cases} u_2 \\ u_3 \end{cases} = \frac{cL^3}{6AE} \left\{\frac{7}{8}\right\} = \frac{cL^3}{6EA} \left\{\frac{7}{8}\right\}$ (exact)

 $\begin{cases} u_2 \\ u_3 \end{cases} = \frac{cL^3}{6AE} \left\{\frac{7}{8}\right\} = \frac{cL^3}{6EA} \left\{\frac{7}{1}\right\}$ (exact)

 $\begin{cases} u_2 \\ u_3 \end{cases} = \frac{cL^3}{6AE} \left\{\frac{7}{8}\right\} = \frac{cL^3}{6EA} \left\{\frac{1}{1}\right\}$ (exact)

 $\begin{cases} u_2 \\ u_3 \end{cases} = \frac{cL^3}{6AE} \left\{\frac{7}{8}\right\} = \frac{cL^3}{6EA} \left\{\frac{1}{1}\right\}$ (exact)

 $\begin{cases} u_2 \\ u_3 \end{cases} = \frac{cL^3}{6AE} \left\{\frac{7}{8}\right\} = \frac{cL^3}{6EA} \left\{\frac{7}{1}\right\}$ (exact)

 $\begin{cases} u_2 \\ u_3 \end{cases} = \frac{cL^3}{6AE} \left\{\frac{7}{8}\right\} = \frac{cL^3}{6EA} \left\{\frac{7}{1}\right\}$ (exact)

 $\begin{cases} u_2 \\ u_3 \end{cases} = \frac{cL^3}{6AE} \left\{\frac{7}{8}\right\} = \frac{cL^3}{6EA} \left\{\frac{7}{1}\right\}$ (exact)

 $\begin{cases} u_2 \\ u_3 \end{cases} = \frac{cL^3}{6AE} \left\{\frac{7}{8}\right\} = \frac{cL^3}{6EA} \left\{\frac{7}{1}\right\}$ (exact)

 $\begin{cases} u_2 \\ u_3 \end{cases} = \frac{cL^3}{6AE} \left\{\frac{7}{8}\right\} = \frac{cL^3}{6EA} \left\{\frac{7}{1}\right\}$ (exact)

 $\begin{cases} u_2 \\ u_3 \end{cases} = \frac{cL^3}{6AE} \left\{\frac{7}{8}\right\} = \frac{cL^3}{6EA} \left\{\frac{7}{1}\right\}$ (exact)

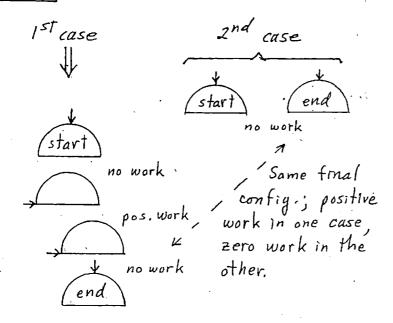
 $\begin{cases} u_2 \\ u_3 \end{cases} = \frac{cL^3}{6AE} \left\{\frac{7}{8}\right\} = \frac{cL^3}{6EA} \left\{\frac{7}{1}\right\}$ (exact)

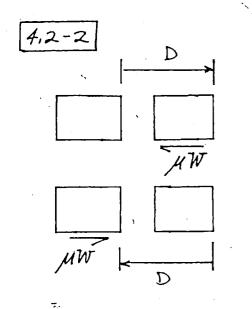
 $\begin{cases} u_2 \\ u_3 \end{cases} = \frac{cL^3}{6AE} \left\{\frac{7}{8}\right\} = \frac{cL^3}{6EA} \left\{\frac{7}{1}\right\}$ (exact)

 $\begin{cases} u_4 \\ u_5 \end{cases} = \frac{cL^3}{6AE} \left\{\frac{7}{8}\right\} = \frac{cL^3}{6EA} \left\{\frac{7}{1}\right\} = \frac{cL^3}{6AE} \left\{\frac{7}{1}\right\}$
 $\begin{cases} u_4 \\ u_5 \end{cases} = \frac{cL^3}{6AE} \left\{\frac{7}{8}\right\} = \frac{cL^3}{6EA} \left\{\frac{7}{1}\right\} = \frac{cL^3}{6AE} \left\{\frac$

end







Work of friction force:

(- UW) D

µW (-D)

Net work of friction force in -2 µWD Not zero, so not conservative.