

CSC446 Report

Peiqi Wang, Eris Zhang

April 10, 2019

Contents

1	Introduction	2
2	Linear Elasticity	2
2.1	Background	2
2.2	Problem Assumptions	3
2.3	Differential Equations	4
2.4	Boundary Value Problem	5
3	Finite Elements	5
3.1	Weak Form	5
3.2	Variational Form	5
3.3	Discretization	6
3.4	Element Stiffness Matrix	7
3.5	Global Stiffness Matrix	8
3.6	Dirichlet Boundary Condition	9
4	Iterative Methods	9
4.1	Stationary Iteration	10

1 Introduction

In computational fabrication, it is crucial to quantify how a 3D printed objects might break or fail using linear elasticity simulations. [1] [2] [2] To enable interactive design and optimization of shapes such that failure cases are minimized, there has been work to increase the speed of simulation with novel interpolation algorithms [3] or with the help of modern parallel hardware. [4]. A particularly interesting and relevant work uses matrix-free iterative methods to perform real time simulation on GPU. [5] [6]

The aim of this report is to explore potential methods to visualize failure cases, i.e. fractures, breakage, etc. for solid shapes of elastic material in real time. For our use case, precision of simulation is only important up to the visualization not deviating from the ground truth by human perception. To exploit this assumption, we explored different iterative methods to solve the linear system resulted from finite element discretization. We explored linear basis functions on both tetrahedron element and regular hexahedron element. In the latter case, we can avoid the costly operation of numerically computing integrals at each element, which opens up opportunity to use matrix-free iterative method with substantial reduction in memory and time cost.

2 Linear Elasticity

2.1 Background

Continuum mechanics deals with behavior of material modeled as a continuous mass. The study of elastic material is a subset of continuum mechanics that models the ability of a material to resist influence and return to its original configuration when the influence is removed. The following definitions are important to elasticity theory and provide important background for rest of the report. [7]

1. **(stress)** describes internal force over a differential area at a point, which can be represented as a symmetric second order tensor σ ,

$$\sigma = \begin{bmatrix} \sigma_{11} & \sigma_{12} & \sigma_{13} \\ \sigma_{21} & \sigma_{22} & \sigma_{23} \\ \sigma_{31} & \sigma_{32} & \sigma_{33} \end{bmatrix}$$

where

$$\sigma_{ij} = \lim_{dA_i \rightarrow 0} \frac{F_j}{dA_i}$$

represents force applied in j -th coordinate axis over a differential plane orthogonal to i -th coordinate axis.

2. **(strain)** describes deformation by relative displacements, which can be represented

as a symmetric second order tensor $\boldsymbol{\varepsilon}$,

$$\boldsymbol{\varepsilon} = \begin{bmatrix} \varepsilon_{11} & \varepsilon_{12} & \varepsilon_{13} \\ \varepsilon_{21} & \varepsilon_{22} & \varepsilon_{23} \\ \varepsilon_{31} & \varepsilon_{32} & \varepsilon_{33} \end{bmatrix}$$

where ε_{ii} are axial strains and ε_{ij} where $i \neq j$ are shearing strains.

3. **(displacement)** describes how much a point moves from referenced to deformed configuration, which can be represented as component-wise displacement, \mathbf{u}

$$\mathbf{u} = (u_1, u_2, u_3)$$

4. **(yield)** a point on a stress-strain curve of a material that indicates the limit of elastic behavior. Material experiencing stress over the yield point will suffer from permanent deformations.
5. **(von Mises yielding criterion)** Ultimately, we are interested in computing the stress field over all points within a solid geometry, from which we can deduce material yielding by comparing the effective stress σ_e with material-specific yield stress σ_y . Simply, we use Von Mises' criterion to determine yielding,

$$\sigma_e = \frac{1}{\sqrt{2}} \sqrt{(\sigma_{11} - \sigma_{22})^2 + (\sigma_{22} - \sigma_{33})^2 + (\sigma_{33} - \sigma_{11})^2 + 6(\sigma_{23}^2 + \sigma_{31}^2 + \sigma_{12}^2)} > \sigma_y$$

2.2 Problem Assumptions

Here, we lay down simplifying assumptions for our problem.

1. **(Analytic stress/strain)** If we assume that stress and strain field is analytic in small neighborhood of a point, we can guarantee convergent Taylor series expansion, which leads to simpler formulation of differential equation governing materials' elastic behavior. This assumption, for example, helped with derivation of Green-Lagrangian strain tensor

$$\varepsilon_{ij} = (u_{i,j} + u_{j,i} - u_{k,i}u_{k,j})$$

2. **(Small deformations)** Typically, elastic material exhibit a nonlinear relationship between stress and strain. However, if we assume for small deformation only, stress-strain is linear, hence the name *linear* elasticity. We can employ Hooke's law to model the relationship between stress and strain.

$$\boldsymbol{\sigma} = \mathbf{C} : \boldsymbol{\varepsilon}$$

where \mathbf{C} is a fourth-order stiffness tensor.

3. **(Isotropic homogeneous material)** Typically, material exhibit direction or location specific behavior. If materials are characterized by properties independent of orientation and coordinate system, we can reduce the number of material-dependent constants to 2 (i.e. λ, μ),

$$\boldsymbol{\sigma} = \mathbf{C} : \boldsymbol{\varepsilon} \quad \rightarrow \quad \boldsymbol{\sigma} = 2\mu\boldsymbol{\varepsilon} + \lambda \text{tr} \boldsymbol{\varepsilon} \mathbf{I}$$

Also note that the stiffness tensor is symmetric, i.e. $\mathbf{C}_{ijkl} = \mathbf{C}_{klij}$. This is true even for anisotropic materials.

4. **(Static equilibrium)** We are only interested in modeling static objects. With this assumption, we obtain a simpler formula from Newton's second law on conservation of linear and angular momentum.

$$\nabla \cdot \boldsymbol{\sigma} + \mathbf{f} = \rho \ddot{\mathbf{u}} \quad \rightarrow \quad \nabla \cdot \boldsymbol{\sigma} + \mathbf{f} = 0$$

where \mathbf{f} are body forces, and ρ is density of the material.

2.3 Differential Equations

From physical laws and simplifying assumptions, we can derive a set of equations governing linear elasticity with respect to the stress field $\boldsymbol{\sigma}$, the strain field $\boldsymbol{\varepsilon}$, and the displacement field \mathbf{u} . We express the equations in tensor notations.

1. **(Equilibrium Equations)** express equilibrium condition imposed to the stress field by Newton's second law

$$\nabla \cdot \boldsymbol{\sigma} + \mathbf{f} = 0 \quad \sigma_{ji,j} + f_i = 0$$

2. **(Kinematics Equations)** express deformation with respect to displacement without reference to forces that created them.

$$\boldsymbol{\varepsilon} = \frac{1}{2} (\nabla \mathbf{u} + \nabla \mathbf{u}^T) \quad \varepsilon_{ij} = \frac{1}{2} (u_{i,j} + u_{j,i})$$

3. **(Constitutive Law)** describes behavior of material under stress. We approximate behavior of linear elastic material with Hooke's Law

$$\boldsymbol{\sigma} = \mathbf{C} : \boldsymbol{\varepsilon} \quad \sigma_{ij} = C_{ijkl} \varepsilon_{kl}$$

where \mathbf{C} is a 4-th order stiffness tensor encoding material properties. Specifically, under the assumption of isotropic material, the relationship is simplified to

$$\boldsymbol{\sigma} = 2\mu\boldsymbol{\varepsilon} + \lambda \text{tr} \boldsymbol{\varepsilon} \mathbf{I} \quad \sigma_{ij} = 2\mu\varepsilon_{ij} + \lambda\varepsilon_{kk}\delta_{ij}$$

where λ, μ are Lamé's first and second parameter can be written as material-specific constants - Poisson's ratio ν and Young's modulus E ,

$$\mu = \frac{E}{2(1+\nu)} \quad \lambda = \frac{E\nu}{(1+\nu)(1-2\nu)}$$

2.4 Boundary Value Problem

For our purposes, it is easier to eliminate stress and strain to seek a set of equations where the displacement field is the primary unknown. We can achieve this by writing the strain $\boldsymbol{\sigma}$ in the equilibrium equation as a function of displacement \mathbf{u} using the constitutive law and kinematics equations. In other words, we seek displacement field \mathbf{u} in \mathbb{R}^3 satisfying

$$\begin{aligned} -\nabla \cdot \mathbf{C}(\boldsymbol{\varepsilon}(\mathbf{u})) &= \mathbf{f} & \mathbf{x} \in \Omega \\ \mathbf{u} &= 0 & \mathbf{x} \in \Gamma \end{aligned}$$

where $\Omega \in \mathbb{R}^3$ is a closed and bounded region occupied by a linear elastic solid, with boundary $\partial\Omega$. $\Gamma \subset \partial\Omega$ is part of boundary that the object cannot be displaced. $\mathcal{V} := \{\mathbf{v} \in H(\Omega, \mathbb{R}^3) \mid \mathbf{v}|_{\Gamma} = \mathbf{0}\}$ is a first Sobolev space satisfying the zero boundary condition on Γ . $\boldsymbol{\varepsilon}$ is a linear differential operator over the displacement field \mathbf{u} , while \mathbf{C} is a linear operator over the strain field $\boldsymbol{\varepsilon}(\mathbf{u})$, which is positive definite for linear elastic isotropic material. It is easy to see that $\mathcal{L} := -\nabla \cdot (\mathbf{C}\boldsymbol{\varepsilon}(\cdot))$ is a linear differential operator.

3 Finite Elements

It is conventional to use finite element method to solve linear elasticity equations for 3-dimensional objects with complex geometry. We will formulate the boundary value problem in both the weak and variational form. The two formulations are equivalent in our setup. We will then discuss computer implementation strategies on tetrahedron elements, and briefly on regular hexahedron elements.

3.1 Weak Form

The goal is to estimate \mathbf{u} in a finite dimensional function space $\mathcal{V}_m \subset \mathcal{V}$ spanned by a set of basis functions $\{\mathbf{u}_1, \dots, \mathbf{u}_m\} \subset \mathcal{V}$. We can formulate linear elasticity weakly as

$$\int_{\Omega} \mathbf{C}\boldsymbol{\varepsilon}(\mathbf{u}) : \boldsymbol{\varepsilon}(\mathbf{v}) d\mathbf{x} = \int_{\Omega} \mathbf{f} \cdot \mathbf{u} d\mathbf{x}$$

for all $\mathbf{v} \in \mathcal{V}$. The left hand side is obtained via Green's first identity assuming a pure displacement formulation with no traction force on the boundary. [8] We can define a bilinear form $\tilde{a}(\cdot, \cdot)$

$$\tilde{a}(\mathbf{u}, \mathbf{v}) = \langle \mathcal{L}\mathbf{u}, \mathbf{v} \rangle = \int_{\Omega} \mathbf{C}\boldsymbol{\varepsilon}(\mathbf{u}) : \boldsymbol{\varepsilon}(\mathbf{v}) d\mathbf{x}$$

where $\langle \cdot, \cdot \rangle$ is L^2 norm over Ω .

3.2 Variational Form

By theorem 9.1 in [9] and the following proposition, $\mathcal{L}\mathbf{u} = \mathbf{f}$ is the Euler-Lagrange equation of following variational problem

$$\mathcal{J}(\mathbf{u}) := \tilde{a}(\mathbf{u}, \mathbf{u}) - 2 \langle \mathbf{f}, \mathbf{u} \rangle$$

where $\mathcal{J} : \mathcal{V} \rightarrow \mathbb{R}$ is a functional and $\mathbf{u} \in \mathcal{V}$. The theorem also guarantees the uniqueness and existence of solution for the boundary value problem. Therefore, we can solve for an equivalent optimization problem

$$\min_{\mathbf{u} \in \mathcal{V}} \mathcal{J}(\mathbf{u}) \quad \text{where} \quad \mathcal{J}(\mathbf{u}) = \underbrace{\frac{1}{2} \int_{\Omega} \mathbf{C} \boldsymbol{\varepsilon}(\mathbf{u}) : \boldsymbol{\varepsilon}(\mathbf{u}) d\mathbf{x}}_{\text{potential energy}} - \underbrace{\int_{\Omega} \mathbf{f} \cdot \mathbf{u} d\mathbf{x}}_{\text{work}}$$

This amounts to applying the principle of least work to mechanical systems. Simply, we are minimizing the internal potential energy and work done by external force (e.g. gravity) on the object.

3.3 Discretization

We construct a mesh \mathcal{T} that partitions the domain Ω where each $\Omega^e \in \mathcal{T}$ is a tetrahedron and n is number of vertices of the mesh. The mesh is generated with `tetgen`, which is a delaunay-based tetrahedralization algorithm that generates mesh with good geometric properties. We define a linear nodal basis function that is 0 at all vertices of the mesh, except has value of 1 at one vertex. Note the boundary value equations are vector valued, so we compose the same scalar valued nodal basis function for each component of the vector field. Therefore, we arrive at $3n$ linear basis that form a finite dimensional subspace of \mathcal{V} ,

$$\mathcal{V}_{3n} = \text{span} \{ \varphi_1, \dots, \varphi_{3n} \}$$

The variational formulation over the discretized domain is then as follows,

$$\min_{\mathbf{d} \in \mathbb{R}^{3n}} \mathcal{J}_{3n}(\mathbf{d}) \quad \text{where} \quad \mathcal{J}_{3n}(\mathbf{d}) := \mathcal{J}(\mathbf{u}) = \sum_{\Omega^e \in \mathcal{T}} \left[\frac{1}{2} \int_{\Omega^e} \mathbf{C} \boldsymbol{\varepsilon}(\mathbf{u}) : \boldsymbol{\varepsilon}(\mathbf{u}) d\mathbf{x} - \int_{\Omega^e} \mathbf{f} \cdot \mathbf{u} d\mathbf{x} \right]$$

where $\mathbf{d} \in \mathbb{R}^{3n}$ are coefficients to linear nodal basis functions and could be interpreted as displacements in 3 coordinate directions at each vertex of the mesh. The displacement field \mathbf{u} and coefficient \mathbf{d} are related as follows in the above equation,

$$\mathbf{u} = \begin{pmatrix} \sum_{i=1}^n \xi_i \varphi_i \\ \sum_{i=1}^n \eta_i \varphi_i \\ \sum_{i=1}^n \zeta_i \varphi_i \end{pmatrix} = \begin{bmatrix} \varphi_1 & 0 & 0 & \varphi_2 & 0 & 0 & \cdots & \varphi_n & 0 & 0 \\ 0 & \varphi_1 & 0 & 0 & \varphi_2 & 0 & \cdots & 0 & \varphi_n & 0 \\ 0 & 0 & \varphi_1 & 0 & 0 & \varphi_2 & \cdots & 0 & 0 & \varphi_n \end{bmatrix} \begin{bmatrix} \xi_1 \\ \eta_1 \\ \zeta_1 \\ \xi_2 \\ \eta_2 \\ \zeta_2 \\ \vdots \\ \xi_n \\ \eta_n \\ \zeta_n \end{bmatrix} = \boldsymbol{\varphi} \mathbf{d}$$

where φ_{ij} is nodal basis for i -component of vector field at vertex j . Here, we impose a node-based ordering, in particular, we can write

$$\mathbf{d}^e = \left(\underbrace{\xi_1 \ \eta_1 \ \zeta_1}_{\mathbf{d}_1^e} \ \underbrace{\xi_2 \ \eta_2 \ \zeta_2}_{\mathbf{d}_2^e} \ \underbrace{\xi_3 \ \eta_3 \ \zeta_3}_{\mathbf{d}_3^e} \ \underbrace{\xi_4 \ \eta_4 \ \zeta_4}_{\mathbf{d}_4^e} \right)$$

as the nodal displacements at each vertex of a single tetrahedron $\Omega^e \in \mathcal{T}$. It is important to note that when integrating over the volume of an element, the nodal basis is a function of the variable of integration \mathbf{x} , while the coefficients \mathbf{d} is not a function of \mathbf{x} , i.e.

$$\mathbf{u}^e(\mathbf{x}) = \boldsymbol{\varphi}^e(\mathbf{x})\mathbf{d}^e$$

where

$$\boldsymbol{\varphi}^e(\mathbf{x}) = \begin{bmatrix} \varphi_1(\mathbf{x}) & 0 & 0 & \cdots & \varphi_4(\mathbf{x}) & 0 & 0 \\ 0 & \varphi_1(\mathbf{x}) & 0 & \cdots & 0 & \varphi_4(\mathbf{x}) & 0 \\ 0 & 0 & \varphi_1(\mathbf{x}) & \cdots & 0 & 0 & \varphi_4(\mathbf{x}) \end{bmatrix} \quad \mathbf{d}^e = \begin{bmatrix} \xi_1 \\ \eta_1 \\ \zeta_1 \\ \xi_2 \\ \eta_2 \\ \zeta_2 \\ \xi_3 \\ \eta_3 \\ \zeta_3 \\ \xi_4 \\ \eta_4 \\ \zeta_4 \end{bmatrix}$$

3.4 Element Stiffness Matrix

We will proceed with derivation of solution for the variational formulation of the problem on the discretized domain. In practice, it is more convenient to represent integrands in matrix notation. For a single tetrahedron,

$$\frac{1}{2} \int_{\Omega^e} \mathbf{C} \boldsymbol{\varepsilon}(\mathbf{u}^e) : \boldsymbol{\varepsilon}(\mathbf{u}^e) d\mathbf{x} - \int_{\Omega^e} \mathbf{f} \cdot \mathbf{u}^e d\mathbf{x} = \frac{1}{2} \int_{\Omega^e} \boldsymbol{\varepsilon}(\mathbf{u}^e)^T \mathbf{C} \boldsymbol{\varepsilon}(\mathbf{u}^e) d\mathbf{x} - \int_{\Omega^e} \mathbf{f}^T \mathbf{u}^e d\mathbf{x}$$

Evaluating linear basis function for any $\mathbf{x} = (x, y, z)$ over a tetrahedron with 4 vertices \mathbf{x}_i for $i = 1, \dots, 4$ is equivalent to determining the barycentric coordinate of \mathbf{x} . This is straight-forward to compute by using the inverse of the following linear relationship,

$$\begin{bmatrix} 1 \\ x \\ y \\ z \end{bmatrix} = \begin{bmatrix} 1 & 1 & 1 & 1 \\ x_1 & x_2 & x_3 & x_4 \\ y_1 & y_2 & y_3 & y_4 \\ z_1 & z_2 & z_3 & z_4 \end{bmatrix} \begin{bmatrix} \varphi_1 \\ \varphi_2 \\ \varphi_3 \\ \varphi_4 \end{bmatrix}$$

Linear operators \mathbf{C} and ε can be represented using matrices as follows

$$\varepsilon = \begin{bmatrix} \partial/\partial x & 0 & 0 \\ 0 & \partial/\partial x & 0 \\ 0 & 0 & \partial/\partial x \\ \partial/\partial y & \partial/\partial x & 0 \\ 0 & \partial/\partial z & \partial/\partial y \\ \partial/\partial z & 0 & \partial/\partial x \end{bmatrix} \quad \mathbf{C} = \begin{bmatrix} \lambda + 2\mu & \lambda & \lambda & 0 & 0 & 0 \\ \lambda & \lambda + 2\mu & \lambda & 0 & 0 & 0 \\ \lambda & \lambda & \lambda + 2\mu & 0 & 0 & 0 \\ 0 & 0 & 0 & \mu & 0 & 0 \\ 0 & 0 & 0 & 0 & \mu & 0 \\ 0 & 0 & 0 & 0 & 0 & \mu \end{bmatrix}$$

To simplify the expression, we want to write $\varepsilon(\mathbf{u}^e)$ with respect to \mathbf{d}^e . We can define

$$\mathbf{B}^e = \varepsilon \boldsymbol{\varphi}^e = [\mathbf{B}_1 \quad \mathbf{B}_2 \quad \mathbf{B}_3 \quad \mathbf{B}_4] \quad \mathbf{B}_i = \begin{bmatrix} \partial\varphi_i/\partial x & 0 & 0 \\ 0 & \partial\varphi_i/\partial x & 0 \\ 0 & 0 & \partial\varphi_i/\partial x \\ \partial\varphi_i/\partial y & \partial\varphi_i/\partial x & 0 \\ 0 & \partial\varphi_i/\partial z & \varphi_i\partial/\partial y \\ \partial\varphi_i/\partial z & 0 & \partial\varphi_i/\partial x \end{bmatrix}$$

such that

$$\varepsilon(\mathbf{u}^e) = \varepsilon \boldsymbol{\varphi}^e \mathbf{d}^e = \mathbf{B}^e \mathbf{d}^e$$

Therefore the functional reduces to

$$\begin{aligned} \frac{1}{2} \int_{\Omega^e} \varepsilon(\mathbf{u}^e)^T \mathbf{C} \varepsilon(\mathbf{u}^e) d\mathbf{x} - \int_{\Omega^e} \mathbf{f}^T \mathbf{u}^e d\mathbf{x} &= \frac{1}{2} \int_{\Omega^e} (\mathbf{B}^e \mathbf{d}^e)^T \mathbf{C} (\mathbf{B}^e \mathbf{d}^e) d\mathbf{x} - \int_{\Omega^e} \mathbf{f}^T \boldsymbol{\varphi}^e \mathbf{d}^e d\mathbf{x} \\ &= \frac{1}{2} (\mathbf{d}^e)^T \left(\int_{\Omega^e} (\mathbf{B}^e)^T \mathbf{C} \mathbf{B}^e d\mathbf{x} \right) \mathbf{d}^e - \left(\int_{\Omega^e} \mathbf{f}^T \boldsymbol{\varphi}^e d\mathbf{x} \right) \mathbf{d}^e \\ &= \frac{1}{2} (\mathbf{d}^e)^T \mathbf{K}^e \mathbf{d}^e - \mathbf{F}^e \mathbf{d}^e \end{aligned}$$

where $\mathbf{K}^e \in \mathbb{R}^{12 \times 12}$ is termed *element stiffness matrix*,

$$\mathbf{K}^e = \int_{\Omega^e} (\mathbf{B}^e)^T \mathbf{C} \mathbf{B}^e d\mathbf{x} \quad \mathbf{F}^e = \int_{\Omega^e} \mathbf{f}^T \boldsymbol{\varphi}^e d\mathbf{x}$$

Tetrahedron elements has the nice property that \mathbf{B}^e is a constant function of \mathbf{x} . Therefore, $\mathbf{K}^e = (\mathbf{B}^e)^T \mathbf{C} \mathbf{B}^e |\Omega^e|$ is constant inside the element, where $|\cdot|$ denotes *volume of*. In general, this is not true. We need to resort to 3-dimensional Gaussian quadrature to numerically integrate the integral for elements like regular hexahedron.

3.5 Global Stiffness Matrix

Now return to the original functional,

$$\mathcal{J}_{3n}(\mathbf{d}) = \sum_{\Omega^e \in \mathcal{T}} \left[\frac{1}{2} (\mathbf{d}^e)^T \mathbf{K}^e \mathbf{d}^e - \mathbf{F}^e \mathbf{d}^e \right] = \frac{1}{2} \mathbf{d}^T \mathbf{K} \mathbf{d} - \mathbf{d}^T \mathbf{F}$$

for some $\mathbf{K} \in \mathbb{R}^{3nx3n}$, $\mathbf{F} \in \mathbb{R}^{3nx1}$. The last step is always possible, as \mathcal{J}_{3n} is quadratic with respect to \mathbf{d} . Here \mathbf{K} is termed *global stiffness matrix*. In practice, we need to keep a consistent indexing (node-based ording) to map the contribution of element stiffness matrix \mathbf{K}^e to corresponding location of global stiffness matrix \mathbf{K} . In spirit of Ritz Method, we set the gradient of \mathcal{J}_{3n} to $\mathbf{0}$,

$$\nabla_{\mathbf{d}} \left(\frac{1}{2} \mathbf{d}^T \mathbf{K} \mathbf{d} - \mathbf{d}^T \mathbf{F} \right) = \mathbf{0} \quad \Rightarrow \quad \mathbf{K} \mathbf{d} = \mathbf{F}$$

Solving the boundary value problem is then reduced to solving a linear system $\mathbf{K} \mathbf{d} = \mathbf{F}$. In general, \mathbf{K} is a symmetric positive definite matrix.

3.6 Dirichlet Boundary Condition

We can enforce the dirichlet boundary condition, by considering a subset of linear equations. Specifically, let $\mathbf{I} = \mathbf{D} + \tilde{\mathbf{D}}$, where $\mathbf{D}_{ii} = 1$ for all i corresponding to vertices at Dirichlet boundary. We can rewrite the system of linear equations as follows,

$$\begin{aligned} \mathbf{K}(\mathbf{D} + \tilde{\mathbf{D}})\mathbf{d} &= \mathbf{F} \\ \mathbf{K}\tilde{\mathbf{D}}\mathbf{d} &= \mathbf{F} - \mathbf{K}\mathbf{D}\mathbf{x} \\ \tilde{\mathbf{D}}\mathbf{K}\tilde{\mathbf{D}}\mathbf{d} &= \tilde{\mathbf{D}}\mathbf{F} - \tilde{\mathbf{D}}\mathbf{K}\mathbf{D}\mathbf{x} \end{aligned}$$

where $\tilde{\mathbf{D}}\mathbf{K}\mathbf{D}\mathbf{x} = \mathbf{0}$, so we have

$$\tilde{\mathbf{D}}\mathbf{K}\tilde{\mathbf{D}}\mathbf{d} = \tilde{\mathbf{D}}\mathbf{F}$$

Effectively, we zero-ed out entries on both left and right hand side of the linear system at indices corresponding to vertices on Dirichlet boundary. We can collapse the matrix \mathbf{K} by removing zero rows/columns and obtain a matrix that remains to be symmetric positive definite.

4 Iterative Methods

We are left with the task of solving the linear system

$$\mathbf{A}\mathbf{x} = \mathbf{b}$$

where \mathbf{A} is a sparse symmetric positive definite matrix. Cholesky decomposition is the standard method to solve a symmetric positive definite linear system. However, exact solution is unnecessary for the purpose of visualization. In this section, we will explore a few iterative method for computing approximate solutions that are *good enough*.

4.1 Stationary Iteration

Consider the standard splitting of \mathbf{A} ,

$$\mathbf{A} = \mathbf{D} - \mathbf{L} - \mathbf{U}$$

where \mathbf{D} , \mathbf{L} , \mathbf{U} are the diagonal, negative of strictly lower-triangular, and negative of strictly upper-triangular parts of \mathbf{A} . We consider a linear one-step stationary scheme

$$\mathbf{x}^{(k+1)} = \mathbf{H}\mathbf{x}^{(k)} + \mathbf{v}$$

for some \mathbf{H} , \mathbf{v} . Specifically, we are interested in stationary methods that are convergent for symmetric positive definite \mathbf{A} . Initially, we started with Jacobi's method,

$$\mathbf{H} = \mathbf{P}^{-1}(\mathbf{L} + \mathbf{U}) = \mathbf{I} - \mathbf{D}^{-1}\mathbf{A} \quad \mathbf{v} = \mathbf{P}^{-1}\mathbf{b}$$

and element-wise iteration

$$\mathbf{x}_i^{(k+1)} = \frac{1}{a_{ii}} \left(b_i - \sum_{j \neq i} a_{ij} \mathbf{x}_j^{(k)} \right)$$

Jacobi's iteration is embarrassingly parrallel, as \mathbf{x}^{k+1} is only dependent upon previous \mathbf{x}^k and so every iteration can be fully parallelised. However, positive definiteness of \mathbf{A} is not sufficient for convergence. In general, linear elasticity problem does not satisfy $\rho(\mathbf{H}) < 1$, which is an equivalent condition for convergence given nonsingular \mathbf{A}, \mathbf{P} . [9] Instead, we used weighted Jacobi,

$$\mathbf{H} = \mathbf{I} - \omega \mathbf{D}^{-1} \mathbf{A} \mathbf{x}_i^{(k+1)} = \frac{1}{a_{ii}} \left(b_i - \sum_{j \neq i} a_{ij} \mathbf{x}_j^{(k)} \right)$$

which is convergent given symmetric positive definite \mathbf{A}

References

- [1] Qingnan Zhou, Julian Panetta, and Denis Zorin. “Worst-case structural analysis”. In: *ACM Transactions on Graphics* 32.4 (2013), p. 1. DOI: [10.1145/2461912.2461967](https://doi.org/10.1145/2461912.2461967).
- [2] Timothy Langlois et al. “Stochastic structural analysis for context-aware design and fabrication”. In: *ACM Transactions on Graphics* 35.6 (2016), pp. 1–13. DOI: [10.1145/2980179.2982436](https://doi.org/10.1145/2980179.2982436).
- [3] Adriana Schulz et al. “Interactive design space exploration and optimization for CAD models”. In: *ACM Transactions on Graphics* 36.4 (2017), pp. 1–14. DOI: [10.1145/3072959.3073688](https://doi.org/10.1145/3072959.3073688).
- [4] Christian Dick, Joachim Georgii, and Rüdiger Westermann. “A real-time multigrid finite hexahedra method for elasticity simulation using CUDA”. In: *Simulation Modelling Practice and Theory* 19.2 (2011), pp. 801–816. DOI: [10.1016/j.simpat.2010.11.005](https://doi.org/10.1016/j.simpat.2010.11.005).
- [5] Praveen Yadav and Krishnan Suresh. “Large Scale Finite Element Analysis Via Assembly-Free Deflated Conjugate Gradient”. In: *Journal of Computing and Information Science in Engineering* 14.4 (2014), p. 041008. DOI: [10.1115/1.4028591](https://doi.org/10.1115/1.4028591).
- [6] Carsten Konke. “Matrix-free voxel-based finite element method for materials with heterogeneous microstructures”. In: (2018).
- [7] Olivier A Bauchau and James I Craig. “Structural analysis”. In: (2009).
- [8] Magnus Jorstad. “Multigrid preconditioning of the linear elasticity equation”. In: (2016).
- [9] A Iserles. “A first course in the numerical analysis of differential equations”. In: (2009).