

Úa
Compendium

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Introduction

$\dot{U}a$ is a finite-element ice flow model. This document provides some theoretical background information to $\dot{U}a$. It is not a manual. This is a ‘life’ document, i.e. it is constantly being modified and changed and the current form of the document is in no means final. It contains some general material on glacier mechanics, a bit on the FE method, and lots of some very specific $\dot{U}a$ related stuff. Most of the material related strictly to glacier mechanics is from a course given at Caltech in 2014.

Notation and definitions

Typical problem geometry is depicted in Fig. 10.1 and the main geometrical variables listed in Table 1. Upper and lower glacier surfaces are denoted by s and b , respectively, while the ocean surface and the bedrock are denoted by S and B , respectively. The ice thickness is $h = s - b$ and is always positive. The distance from bedrock (B) to the ocean surface (S) is $H = S - B$, and this quantity can be either positive or negative, depending on location.

The maximal ice thickness possible without grounding is denoted by h_f and is

$$h_f := (S - B)\rho_o/\rho,$$

where ρ and ρ_o are the ice and the ocean densities, respectively. The ice grounds if the ice thickness exceeds h_f , that is whenever $h > h_f$. Note that h_f becomes negative for $B > S$, i.e. whenever the bedrock (B) is located above sea level (S). In that case we always have $h > h_f$ for any positive ice thickness h and the ice is always grounded. We also define the positive flotation thickness, h_f^+ , as

$$h_f^+ = h_f \mathcal{H}(h_f),$$

where \mathcal{H} is the Heaviside function, defined as

$$\mathcal{H}(x) = \begin{cases} 0 & \text{for } x < 0 \\ 1/2 & \text{for } x = 0 \\ 1 & \text{for } x > 0 \end{cases}$$

The function $\mathcal{H}(h - h_f)$ (equal to one if grounded, zero if afloat), is needed frequently and we define \mathcal{G} as

$$\mathcal{G} = \mathcal{H}(h - h_f)$$

Hence

$$\mathcal{G} = \mathcal{H}(h - h_f) = \begin{cases} 0 & \text{over floating areas} \\ 1/2 & \text{at the grounding line} \\ 1 & \text{over grounded areas} \end{cases} \quad (1)$$

The variable \mathcal{G} can be thought of as a ‘grounding’ parameter.

The freeboard is

$$f := s - S,$$

and the draft d is defined as

$$d := \begin{cases} S - b, & \text{if } S > b \\ 0, & \text{otherwise} \end{cases}$$

which can also be written as

$$d = \mathcal{H}(H)(S - b),$$

The ‘positive’ ocean depth H^+ is defined as

$$H^+ := \mathcal{H}(H)H, \quad (2)$$

i.e. $H^+ = H$ if $H > 0$ and zero otherwise.

To distinguish between continuous quantities and discrete quantities we use bold face for the latter. In a finite-element context we might write

$$f(x) = f_q \phi_q(x)$$



Figure 1: Geometrical variables: Glacier surface (s), glacier bed (b), ocean surface (S), ocean floor (B), glacier thickness ($h = s - b$), ocean depth ($H = S - B$), glacier draft ($d = S - b$), glacier freeboard ($f = s - S$).

Table 1: List of variables

s	upper glacier surface
b	lower glacier surface
S	ocean surface
B	bedrock / ocean floor
$h := s - b$	glacier thickness
$H := S - B$	ocean depth (pos. or neg. depending on location)
$H^+ = \mathcal{H}(H)H$	(positive) ocean depth
$d := \mathcal{H}(H)(S - b)$	glacier draft (positive by definition)
$f := s - S$	freeboard (always positive)
$\mathcal{G} := \mathcal{H}(h - h_f)$	grounding mask, 1 if ice grounded, 0 otherwise.
$h_f := \rho_o H / \rho$	flotation thickness (maximal ice thickness without grounding)
$h_f^+ := \rho_o H^+ / \rho$	positive flotation thickness
α	tilt of coordinate system
ρ	ice density
ρ_o	ocean density
$\varrho = \rho(1 - \rho/\rho_o)$	
(u_b, v_b, w_b)	xyz components of basal velocity
(u_s, v_s, w_s)	xyz components of surface velocity
$\tau_{xx}, \tau_{yy}, \tau_{xy}$ etc.	deviatoric stress components
$\sigma_{xx}, \sigma_{yy}, \sigma_{xy}$ etc.	Cauchy stress components
$\dot{\epsilon}_{xx}, \dot{\epsilon}_{yy}, \dot{\epsilon}_{xy}$ etc.	strain rates
g	gravitational acceleration

Here f is a continuous function, f_q the nodal variables, and ϕ_q the shape/form functions. We sometimes group the nodal variables together into a vector writing

$$\mathbf{f} = [f_1, f_1, \dots, f_N]^T$$

and then

$$f(x) = \mathbf{f}^T \boldsymbol{\phi}$$

Note that f and f_q very are different quantities. If, for a vector \mathbf{f} , we need to refer to the q element of the vector, we write $[\mathbf{f}]_q$, i.e.

$$f_q = [\mathbf{f}]_q$$

The matrix representation of a continuous operator $L : H_1 \rightarrow H_2$ is written in bold as \mathbf{L} . The elements of the matrix are $L_{pq} = [\mathbf{L}]_{pq}$.

The L^2 norm is

$$(f, g)_{L^2} = \int f(x) g(x) dx$$

where f and g are square intergrable functions and the l^2 norm is

$$(\mathbf{f}, \mathbf{g})_{l^2} = \mathbf{f}^T \cdot \mathbf{g}$$

where \mathbf{f} and \mathbf{g} are vectors. The inner products define corresponding L^2 and l^2 norms.

We often need to linearise various quantities, which we do by calculating directional derivatives. The directional derivative $D\mathbf{f}$ of a function \mathbf{f} of the variable \mathbf{x} in the direction \mathbf{v} is defined as

$$D\mathbf{f}[\mathbf{x}; \mathbf{v}] = \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} \mathbf{f}(\mathbf{x} + \epsilon \mathbf{v})$$

Often we think of \mathbf{v} being a small perturbation to \mathbf{x} in which case we write

$$D\mathbf{f}[\mathbf{x}; \Delta \mathbf{x}] = \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} \mathbf{f}(\mathbf{x} + \epsilon \Delta \mathbf{x})$$

and may then simply write $D\mathbf{f}[\mathbf{x}]$. Another common notation for the directional derivative of the function $\mathbf{f}(\mathbf{x})$ in the direction \mathbf{v} is $\nabla_{\mathbf{v}} \mathbf{f}(\mathbf{x})$.



Figure 2: Distribution of integration points with the unit reference triangle and the degree of precision.

Part I

Úa Stuff

Chapter 1

Equations of ice flow

1.1 Shallow Ice Stream Approximation (SSTREAM/SSA)

The shallow-ice stream (SSTREAM/SSA/Shelfy) equations are

$$\partial_x(h(2\tau_{xx} + \tau_{yy})) + \partial_y(h\tau_{xy}) - t_{bx} = (\rho gh \partial_x s + \frac{1}{2}h^2 g \partial_x \rho) \cos \alpha - \rho gh \sin \alpha \quad (1.1)$$

$$\partial_y(h(2\tau_{yy} + \tau_{xx})) + \partial_x(h\tau_{xy}) - t_{by} = (\rho gh \partial_y s + \frac{1}{2}h^2 g \partial_y \rho) \cos \alpha \quad (1.2)$$

where α is the tilt of the coordinate system with respect to the gravity vector. Defining the *resistive stress tensor* as

$$\mathbf{R} = \begin{pmatrix} 2\tau_{xx} + \tau_{yy} & \tau_{xy} \\ \tau_{xy} & 2\tau_{yy} + \tau_{xx} \end{pmatrix} \quad (1.3)$$

and

$$\nabla_{xy} = (\partial_x, \partial_y)$$

the field equations can be written in a compact form as

$$\nabla_{xy} \cdot (h \mathbf{R}) - \mathbf{t}_{bh} = \rho gh \nabla_{xy} s + \frac{1}{2}gh^2 \nabla_{xy} \rho, \quad (1.4)$$

for $\alpha = 0$, where

$$\mathbf{t}_{bh} = \begin{pmatrix} t_{bx} \\ t_{by} \end{pmatrix}$$

is the horizontal part of the basal stress vector (basal traction)

$$\mathbf{t}_b = \boldsymbol{\sigma} \hat{\mathbf{n}} - (\hat{\mathbf{n}} \cdot \boldsymbol{\sigma} \hat{\mathbf{n}}) \hat{\mathbf{n}},$$

with $\hat{\mathbf{n}}$ being a unit normal vector to the bed pointing into the ice.

Note that it is the horizontal component of the basal traction that enters the field equations. We will sometimes just write \mathbf{t}_b instead of \mathbf{t}_{bh} which is a slight abuse of notation, and strictly speaking incorrect.

1.2 Shallow Ice Shelf (SSHELF/SSA)

The shallow ice shelf approximation is simply the shallow ice stream approximation with the drag term dropped.

Since

$$s = S + (1 - h\rho/\rho_o)$$

we have over a floating ice shelf

$$\rho gh \nabla_{xy} s = \frac{1}{2} \rho g \nabla_{xy} h^2$$

and the momentum equations have the form

$$\nabla_{xy} \cdot (h \mathbf{R}) = \frac{1}{2} \rho g \nabla_{xy} h^2 + \frac{1}{2} gh^2 \nabla_{xy} \rho, \quad (1.5)$$

or if we skip the spatial density gradient

$$\nabla_{xy} \cdot \left(\mathbf{R} - \frac{1}{2} \rho g \begin{pmatrix} h & 0 \\ 0 & h \end{pmatrix} \right) = 0 \quad (1.6)$$

1.3 Shallow Ice Sheet (SSHEET/SIA)

The shallow ice sheet equations for the deformational velocities are

$$(u, v) = -E |\nabla_{xy}s|^{n-1} (h^{n+1} - (s-z)^{n+1}) (\partial_x s, \partial_y s)$$

where

$$E = \frac{2A}{n+1} (\rho g)^n$$

The vertically integrated flux is

$$q_x = \int_b^s \rho u \, dz,$$

giving

$$(q_x, q_y) = -\rho D |\nabla_{xy}s|^{(n-1)} h^{n+2} (\partial_x s, \partial_y s)$$

where

$$D = \frac{2A}{n+2} (\rho g)^n$$

or

$$(q_x, q_y) = F \rho h (u, v)$$

with

$$F = \frac{n+1}{n+2}$$

where

$$|\nabla_{xy}s| = \sqrt{(\partial_x s)^2 + (\partial_y s)^2}$$

1.4 Equation of mass conservation

The prognostic equation is a vertically integrated expression of mass conservation. The local form of the mass-conservation equation is

$$\nabla \cdot (\rho \mathbf{v}) + \partial_t \rho = 0 \quad (1.7)$$

The kinematic boundary conditions at the upper and lower surface are written as

$$\partial_t s + u_s \partial_x s + v_s \partial_y s - w_s = a_s \quad \text{at} \quad z = s \quad (1.8)$$

$$\partial_t b + u_b \partial_x b + v_b \partial_y b - w_b = -a_b \quad \text{at} \quad z = b \quad (1.9)$$

The mass balance distributions, a_b and a_s , are in the units of meters of water equivalent per time. Note the sign convention for a_s and a_b used in (1.8) and (1.9). Mass flux into the ice is defined positive irrespective over which surface it takes place. Surface accumulation is positive, melting always negative.

The horizontal ice flux is defined as

$$\mathbf{q}_{xy} = \int_b^s \rho \mathbf{v}_{xy} \, dz$$

Focusing on the flow-line case for the moment we find that

$$\begin{aligned} \partial_x q_x &= \int_b^s \partial_x (\rho u) \, dz \\ &= \int_b^s \partial_x (\rho u) \, dz + \rho u_s \partial_x s - \rho u_b \partial_x b \\ &= - \int_b^s (\partial_z (\rho w) + \partial_t \rho) \, dz + \rho u_s \partial_x s - \rho u_b \partial_x b \\ &= -\rho w_s + \rho w_b - h \partial_t \rho + \rho u_s \partial_x s - \rho u_b \partial_x b \\ &= -h \partial_t \rho - \rho w_s + \rho u_s \partial_x s + \rho w_b - \rho u_b \partial_x b \\ &= -h \partial_t \rho + \rho a_s - \rho \partial_t s + \rho a_b + \rho \partial_t b \\ &= -h \partial_t \rho + \rho a - \rho \partial_t h \end{aligned}$$

where

$$a = a_s + a_b$$

and therefore, once the y component has been added

$$\rho \partial_t h + \nabla_{xy} \cdot \mathbf{q}_{xy} + h \partial_t \rho = \rho a \quad (1.10)$$

In most modelling work of large ice masses it is assumed that the density ρ is uniform in space and does not vary with time. In $\dot{U}a$ we relax this condition somewhat and only assume that the density at a given location does not change with time, i.e.

$$\partial_t \rho = 0.$$

but allow the density to vary in the horizontal ($\partial_x \rho$ and $\partial_y \rho$ are not assumed to be equal to zero). Hence the mass conservation equations (1.7) becomes

$$\nabla \cdot (\rho \mathbf{v}) = 0.$$

and (1.10)

$$\rho \partial_t h + \nabla_{xy} \cdot \mathbf{q}_{xy} = \rho a \quad (1.11)$$

or

$$\rho \partial_t h + \partial_x(\rho h u) + \partial_y(\rho h v) = \rho a \quad (1.12)$$

for a velocity field that does not change with depth.

Eq. (1.11) is the form of the mass continuity equation used in $\dot{U}a$. The effect of horizontal gradients in (vertically integrated) density are also included in the momentum equations (10.60).

Vertical velocities

Note that

$$w_s = u_s \partial_x s + w_b - u_b \partial_x b - \frac{1}{\rho} \partial_x q_x - \frac{h}{\rho} \partial_t \rho$$

or

$$w_s = u_s \partial_x s + \partial_t b + a_b - \frac{1}{\rho} \partial_x q_x - \frac{h}{\rho} \partial_t \rho$$

which can be used to calculate vertical velocities.

If the bed elevation does not change with time and if furthermore $\partial_t \rho = 0$, we have the special case

$$w_s = u_s \partial_x s + a_b - \frac{1}{\rho} \partial_x q_x$$

1.5 Sliding law

The power-law type Weertman sliding law is

$$\mathbf{T} \boldsymbol{\sigma} \hat{\mathbf{n}} + C^{-1/m} |\mathbf{T} \mathbf{v}|^{1/m-1} \mathbf{T} \mathbf{v} = 0$$

where

$$\mathbf{T} = \mathbf{1} - \hat{\mathbf{n}} \otimes \hat{\mathbf{n}}$$

is the tangential operator. One can also define the normal operator \mathbf{N} as

$$\mathbf{N} = \hat{\mathbf{n}} \otimes \hat{\mathbf{n}}$$

and we have

$$\boldsymbol{\sigma} = \mathbf{N} \boldsymbol{\sigma} + \mathbf{T} \boldsymbol{\sigma}$$

The basal drag caused by ice flow only acts over the grounded parts, a fact which we can express

$$\mathbf{t}_b = \mathcal{H}(h - h_f) \mathbf{f}(\mathbf{v}_b)$$

or as

$$\mathbf{t}_b = \mathcal{G} \mathbf{f}(\mathbf{v}_b)$$

where \mathcal{G} is the grounding parameter defined by Eq. (1)

Different formulations are

$$\mathbf{t}_b = \mathcal{G} C^{-1/m} |\mathbf{v}_b|^{1/m-1} \mathbf{v}_b \quad (1.13)$$

$$\mathbf{v}_b = \mathcal{G}^{-m} C |\mathbf{t}_b|^{m-1} \mathbf{t}_b \quad (1.14)$$

$$|\mathbf{v}_b| = \mathcal{G}^{-m} C |\mathbf{t}_b|^m \quad (1.15)$$

where \mathbf{v}_b is the tangential velocity, i.e. the basal sliding velocity

$$\begin{aligned} \mathbf{v}_b &= \mathbf{v} - (\hat{\mathbf{n}}^T \cdot \mathbf{v}) \hat{\mathbf{n}} \\ \mathbf{v}_b &= \mathbf{T} \mathbf{v} \end{aligned}$$

and \mathbf{t}_b is the tangential component of the basal traction

$$\mathbf{t}_b = -\mathbf{T}(\boldsymbol{\sigma} \hat{\mathbf{n}})$$

We refer to \mathbf{t}_b as *basal shear traction*. Note that the shear traction is, in general, not equal to basal shear stress.

Since

$$\mathcal{G} = \mathcal{G}^m$$

for any power m we do not strictly need to include the stress exponent m in any equations involving \mathcal{H} . However if we use an approximation to Heaviside function \mathcal{H} in the definition of \mathcal{G} given by Eq. (1), then this stress exponent should be included.

The basal slipperiness C has the physical dimensions of velocity divide by stress to the power m . It can be useful to think of C as the ratio

$$C = \frac{C_v}{C_\tau^m}$$

where C_v has the dimensions of velocity and C_τ the dimensions of stress. We can then write (1.13) and (1.14) as

$$\mathbf{t}_b = \mathcal{G} C_\tau \left(\frac{|\mathbf{v}_b|}{C_v} \right)^{1/m} \frac{\mathbf{v}_b}{|\mathbf{v}_b|} \quad (1.16)$$

$$\mathbf{v}_b = C_v \left(\frac{|\mathbf{t}_b|}{\mathcal{G} C_\tau} \right)^m \frac{\mathbf{t}_b}{|\mathbf{t}_b|} \quad (1.17)$$

If we write

$$\mathbf{t}_b = \beta^2 \mathbf{v}_b$$

then

$$\beta^2 = C^{-1/m} |\mathbf{v}_b|^{1/m-1}$$

In \mathcal{U} the power-law sliding law is given as

$$\begin{pmatrix} t_{bx} \\ t_{by} \end{pmatrix} = \mathcal{G} \beta^2 \begin{pmatrix} u_b \\ v_b \end{pmatrix}$$

where

$$\beta^2 = (C + C_0)^{-1/m} (u^2 + v^2 + u_o^2)^{(1-m)/2m}$$

where C_0 and u_0 are some (small) regularisation parameters.

1.5.1 Weertman sliding law limits

If we now consider the limit $m \rightarrow +\infty$ we see from Eq. (1.14) that must have $|\mathbf{t}_b| \rightarrow C_\tau$ over the grounded areas (where $\mathcal{G} = 1$) for the velocity to remain finite. With increasing m , the basal velocity \mathbf{v}_b becomes increasingly sensitive to basal shear traction, and in the limit $m \rightarrow +\infty$ the velocity can be considered to become infinitely sensitive to basal shear traction. For $m \rightarrow +\infty$, $1/m \rightarrow 0$ and inserting $1/m = 0$ into Eq. (1.16) gives

$$\mathbf{t}_b = C_\tau \frac{\mathbf{v}_b}{|\mathbf{v}_b|} \quad \text{for } m \rightarrow +\infty.$$

Hence, the basal shear traction is determined by C_τ , and C_τ can be considered to be a yield stress. In this particular limit it therefore arguably better to recast the 'sliding' law as

$$\mathbf{t}_b = \tau^* \frac{\mathbf{v}_b}{|\mathbf{v}_b|} \quad \text{for } m \rightarrow +\infty,$$

where $\tau^* = C_\tau$ is a yield stress, and the 'sliding law' is now simply a stress condition for the basal shear traction and τ^* a property of the bed (e.g. till) that is determined by some other physical principle. Using this viewpoint, in the limit $m \rightarrow +\infty$ the parameter C_v has no effect on the solution, but can be calculated afterwards from the velocity. The basal sliding law does not impose any direct constraints on the basal sliding velocity, i.e. for given basal shear traction, the basal velocity can have any value. The basal velocity can still be calculated by solving the SSTREAM/SSA equations provided the velocity is set to a value somewhere along the boundary by the boundary conditions (In this limit the SSTREAM field equations only provide constraints on the gradients of velocities). In the SIA equations the velocity can not be determined using the momentum equation (as by definition there is no direct functional relationship between velocity and basal shear traction), and must be determined from other consideration (such as mass conservation).

Considering the opposite limit where $m \rightarrow 0$, we see from Eq. (1.16) that now it is the basal shear traction that becomes infinitely sensitive to basal velocity, or conversely, the basal velocity becomes in this limit independent of basal shear traction. Inserting $m = 0$ into Eq. (1.14) gives

$$\mathbf{v}_b = C_v \frac{\mathbf{t}_b}{|\mathbf{t}_b|} \quad \text{for } m \rightarrow 0.$$

This is a limit which is (I find) difficult to understand in physical terms. The basal velocity is now no longer a function of the stress state and must be determined by some other physical principle. What physical conditions at the bed would force the basal sliding velocity to obtain some particular value?

Note that the limits $m \rightarrow +\infty$ and $m \rightarrow 0$ are fundamentally different. In the former the basal shear traction is fixed, in the latter the basal velocity.

1.6 Ocean drag term

To simulate drag exerted on the ice by the ocean we add an ocean drag term over the floating section of the form

$$\mathbf{t}_b^o = \mathcal{H}(h_f - h) C_o^{-1/m_o} |\mathbf{v}_b - \mathbf{v}_o|^{1/m_o - 1} (\mathbf{v}_b - \mathbf{v}_o)$$

where \mathbf{v}_o is the velocity of the ocean current.

The sea-ice literature suggest $m_o = 1/2$, i.e.

$$\mathbf{t}_b^o = \mathcal{H}(h_f - h) C_o^{-2} |\mathbf{v}_b - \mathbf{v}_o| (\mathbf{v}_b - \mathbf{v}_o)$$

and defines

$$D_o = C_o^{-2}$$

where

$$D_o = \rho_o c_o$$

and typically $c_o = 0.0055$. Hence

$$C_o = \frac{1}{\sqrt{D_o}} = \frac{1}{\sqrt{\rho_o c_o}} \approx 0.4 \quad [\sqrt{(m/s)/Pa}]$$

The total drag is a sum of that due to basal sliding and ocean currents.

$$\mathbf{t}_b = \mathcal{G} \beta^2 \mathbf{v} + \mathcal{H}(h_f - h) \beta_o^2 (\mathbf{v} - \mathbf{v}_o)$$

1.7 Flow law

Glen's flow law is

$$\dot{\epsilon}_{ij} = A\tau^{n-1}\tau_{ij},$$

where

$$\tau = \sqrt{\tau_{ij}\tau_{ij}/2}$$

The flow law can also be written as

$$\tau_{ij} = A^{-1/n} \dot{\epsilon}^{(1-n)/n} \dot{\epsilon}_{ij}, \quad (1.18)$$

where

$$\dot{\epsilon} = \sqrt{\dot{\epsilon}_{ij}\dot{\epsilon}_{ij}/2}$$

which in the Shallow Ice Stream Approximation takes the form

$$\dot{\epsilon} = \sqrt{(\dot{\epsilon}_{xx})^2 + (\dot{\epsilon}_{yy})^2 + \dot{\epsilon}_{xx}\dot{\epsilon}_{yy} + (\dot{\epsilon}_{xy})^2} \quad (1.19)$$

$$= ((\partial_x u)^2 + (\partial_y v)^2 + \partial_x u \partial_y v + (\partial_x v + \partial_y u)^2/4)^{1/2} \quad (1.20)$$

If we write

$$\tau_{ij} = 2\eta\dot{\epsilon}_{ij}$$

then η is the effective viscosity given by

$$\eta = \frac{1}{2} A^{-1/n} \dot{\epsilon}^{(1-n)/n}$$

or

$$\eta = \frac{1}{2} A^{-1/n} ((\partial_x u)^2 + (\partial_y v)^2 + \partial_x u \partial_y v + (\partial_x v + \partial_y u)^2/4)^{(1-n)/2n}$$

1.8 Floating relationships

Where the ice is afloat, we have

$$\rho g h = \rho_o g d.$$

If the ice thickness is greater than $\rho_o H / \rho$ the ice is grounded. For a given bedrock geometry B and ocean surface S the ice is floating provided $h < h_f$ where

$$h_f := \rho_o H / \rho, \quad (1.21)$$

and for $h \geq h_f$ the glacier is grounded.

Where the glacier is afloat, i.e. $h \leq h_f$, the following relations hold:

$$h = \rho_o d / \rho = \frac{s - S}{1 - \rho / \rho_o} = \frac{\rho_o}{\rho} (S - b), \quad (1.22)$$

$$b = \frac{\rho s - \rho_o S}{\rho - \rho_o} = S - \frac{\rho}{\rho_o} h, \quad (1.23)$$

$$s = S + (1 - \rho / \rho_o) h = (1 - \rho_o / \rho) b + \frac{\rho_o}{\rho} S, \quad (1.24)$$

$$f = (1 - \rho / \rho_o) h. \quad (1.25)$$

Furthermore, if $\partial_x S = 0$ the slopes of the upper and the lower boundary are related through

$$b \partial_x s - s \partial_x b = S \partial_x h, \quad (1.26)$$

and also

$$\partial_x s = (1 - \rho / \rho_o) \partial_x h.$$

At the grounding line we have:

$$\begin{aligned} h &= h_f \\ d &= H. \end{aligned}$$

where h_f is defined by (1.21)

1.9 Expressing geometrical variables in terms of ice thickness

It is advantageous to be able to express geometrical variables such as s , b , and d in terms of ice thickness h .

It is easy to see that

$$s = \mathcal{H}(h - h_f) (h + B) + \mathcal{H}(h_f - h) (S + (1 - \rho/\rho_o) h), \quad (1.27)$$

$$b = \mathcal{H}(h - h_f) B + \mathcal{H}(h_f - h) (S - \rho h/\rho_o), \quad (1.28)$$

and that

$$d = \mathcal{H}(H) [\mathcal{H}(h_f - h) \rho h/\rho_o + \mathcal{H}(h - h_f) H], \quad (1.29)$$

i.e.

$$d = \begin{cases} H, & \text{if } h > h_f \text{ and } H > 0 \\ \rho h/\rho_o, & \text{if } h < h_f \text{ and } H > 0 \\ 0, & \text{if } H < 0 \end{cases}$$

The draft is always $0 \leq d \leq \rho h/\rho_o$.

Eq. (1.29) can be simplified a bit further by noticing that if $H > 0$ then $\mathcal{H}(H)\mathcal{H}(h_f - h) = \mathcal{H}(h_f - h)$. On the other hand if $H < 0$ then $\mathcal{H}(H) = 0$ but so is $\mathcal{H}(h_f - h)$ because if $H = S - B < 0$ then $h_f = \frac{\rho_o}{\rho}(S - B) < 0$ and since h is always positive we have $\mathcal{H}(h_f - h) = 0$, i.e.

$$\mathcal{H}(H)\mathcal{H}(h_f - h) = \mathcal{H}(h_f - h)$$

and d can be therefore be written as

$$d = \mathcal{H}(h_f - h) \rho h/\rho_o + \mathcal{H}(H)\mathcal{H}(h - h_f) H. \quad (1.30)$$

or as

$$d = \mathcal{H}(h_f - h) \rho h/\rho_o + \mathcal{H}(h - h_f) H^+. \quad (1.31)$$

using

$$H^+ := \mathcal{H}(H) H$$

1.10 Stress boundary conditions at an ice front

We consider the case of an ice front in contact with water of a given depth. The treatment is general and includes the cases of zero water depth, i.e. glacier terminating on land, and a floating ice front, i.e. a glacier terminating in an ocean.

At the calving front the jump condition is

$$\boldsymbol{\sigma} \cdot \hat{\mathbf{n}}_{xy} = -p_o \hat{\mathbf{n}}_n.$$

where p_o is the hydrostatic ocean pressure, and

$$\hat{\mathbf{n}}_{xy} = (n_x, n_y, 0)^T,$$

is a unit normal pointing horizontally outward from the ice front. The vertically integrated form of this stress condition is

$$\int_b^s (\sigma_{xx} n_x + \sigma_{xy} n_y) dz = - \int_b^S p_o n_x dz \quad \text{on } \Gamma_2 \quad (1.32)$$

$$\int_b^s (\sigma_{xy} n_x + \sigma_{yy} n_y) dz = - \int_b^S p_o n_y dz \quad \text{on } \Gamma_2 \quad (1.33)$$

If the draft d at the ice front is zero, i.e. if the ice front is fully grounded, then $S < b$, the right hand sides of (10.91) and (10.92) are to be set to zero.

Using

$$\sigma_{xx} = 2\tau_{xx} + \tau_{yy} + \sigma_{zz},$$

and with

$$\sigma_{zz} = -\rho g(s - z),$$

(where we have set $\alpha = 0$), it follows that

$$\begin{aligned} \int_b^s \sigma_{xx} dz &= \int_b^s (2\tau_{xx} + \tau_{yy}) dz - \int_b^s \rho g(s - z) dz \\ &= h(2\tau_{xx} + \tau_{yy}) - \frac{\rho g}{2} h^2. \end{aligned}$$

The x component of the vertically integrated ocean pressure acting on the calving front is

$$\begin{aligned} - \int_b^S p_o n_x dz &= - \int_b^S \rho_o g(S - z) n_x dz \\ &= - \frac{1}{2} \rho_o g(S - b)^2 \\ &= - \frac{1}{2} \rho_o g d^2. \end{aligned}$$

Boundary conditions (10.91) and (10.92) can therefore be written as

$$h(2\tau_{xx} + \tau_{yy})n_x + h\tau_{xy}n_y = \frac{g}{2}(\rho h^2 - \rho_o d^2)n_x, \quad (1.34)$$

$$h(2\tau_{yy} + \tau_{xx})n_y + h\tau_{xy}n_x = \frac{g}{2}(\rho h^2 - \rho_o d^2)n_y, \quad (1.35)$$

or more compactly as

$$\mathbf{R} \cdot \hat{\mathbf{n}}_{xy} = \frac{g}{2h}(\rho h^2 - \rho_o d^2)\hat{\mathbf{n}}_{xy}. \quad (1.36)$$

The boundary condition (10.95) is valid for both grounded and floating ice edges.

1.10.1 Floating

In the particular case where the calving front is afloat, $\rho h = \rho_o d$ boundary conditions (10.93) and (10.94) simplify to

$$h(2\tau_{xx} + \tau_{yy})n_x + h\tau_{xy}n_y = \frac{1}{2}\rho gh^2 n_x \quad (1.37)$$

$$h(2\tau_{yy} + \tau_{xx})n_y + h\tau_{xy}n_x = \frac{1}{2}\rho gh^2 n_y \quad (1.38)$$

where

$$\varrho := \rho(1 - \rho/\rho_o),$$

Written in terms of the velocity components the boundary conditions along a floating ice front are:

$$\eta h(4\partial_x u + 2\partial_y v)n_x + \eta h(\partial_x v + \partial_y u)n_y = \frac{\varrho gh^2}{2}n_x, \quad (1.39)$$

$$\eta h(\partial_x v + \partial_y u)n_x + \eta h(4\partial_y v + 2\partial_x u)n_y = \frac{\varrho gh^2}{2}n_y. \quad (1.40)$$

1.10.2 Grounded

On the other hand if the ice terminates on land then $d = 0$ and

$$h(2\tau_{xx} + \tau_{yy})n_x + h\tau_{xy}n_y = \frac{g}{2}\rho h^2 n_x, \quad (1.41)$$

$$h(2\tau_{yy} + \tau_{xx})n_y + h\tau_{xy}n_x = \frac{g}{2}\rho h^2 n_y. \quad (1.42)$$

1.11 Boundary condition at a glacier terminus as a natural boundary condition

For solving (1.1) and (1.2) it is advantageous to modify the equations in such a way that the boundary conditions (10.93) and (10.94) become the ‘natural’ boundary conditions. Furthermore, for an implicit

time integration with respect to both velocities, grounding-line position, and ice thickness, it is of advantage to write all evolving geometrical variables (s , b) in terms of the ice thickness h .

The key idea is to rewrite (assuming $\alpha = 0$) the Eqs. (1.1) and (1.2) as

$$\partial_x(h(2\tau_{xx} + \tau_{yy})) + \partial_y(h\tau_{xy}) - t_{bx} = \frac{1}{2}g\partial_x(\rho h^2 - \rho_o d^2) + g(\rho h - \rho_o d)\partial_x b, \quad (1.43)$$

$$\partial_y(h(2\tau_{yy} + \tau_{xx})) + \partial_x(h\tau_{xy}) - t_{by} = \frac{1}{2}g\partial_y(\rho h^2 - \rho_o d^2) + g(\rho h - \rho_o d)\partial_y b, \quad (1.44)$$

(note the d term is not missing a $\mathcal{H}(H)$ because d is automatically zero whenever $\mathcal{H}(H) = 0$.) where it has been used that $\partial_x \rho_o = \partial_y \rho_o = 0$ and $\partial_x S = \partial_y S = 0$. Note that in Eqs. (1.43) and (1.44) the second terms on the right hand sides are automatically zero where the ice is afloat and that this formulation can also be used if the ice density varies in the horizontal.

The equality of the right-hand terms in (1.1) and (1.43) (for $\alpha = 0$) follows from

$$\begin{aligned} \frac{1}{2}g\partial_x(\rho h^2 - \rho_o d^2) + g(\rho h - \rho_o d)\partial_x b &= \frac{1}{2}gh^2\partial_x \rho + g(\rho h\partial_x h - \rho_o d\partial_x d) + g(\rho h - \rho_o d)\partial_x b \\ &= \frac{1}{2}gh^2\partial_x \rho + g(\rho h\partial_x(s - b) - \rho_o d\partial_x d) + g(\rho h - \rho_o d)\partial_x b \\ &= \frac{1}{2}gh^2\partial_x \rho + g(\rho h\partial_x s - \rho_o d\partial_x d) - g\rho_o d\partial_x b \\ &= \frac{1}{2}gh^2\partial_x \rho + g(\rho h\partial_x s - \rho_o d\partial_x(\mathcal{H}(H)(S - b))) - g\rho_o d\partial_x b \\ &= \frac{1}{2}gh^2\partial_x \rho + g\rho h\partial_x s - g\rho_o d(S - b)\partial_x \mathcal{H}(H) \end{aligned}$$

and the last term is (in an integrated sense) zero

$$\begin{aligned} \int g\rho_o d(S - b)\partial_x \mathcal{H}(H) dx &= \int g\rho_o d(S - b)\partial_H \mathcal{H}(H) \partial_x H dx \\ &= \int g\rho_o d(S - b)\delta(H) \partial_x H dx \\ &= g\rho_o d(S - b) \quad (\text{for } x \text{ where } H = 0) \\ &= 0 \end{aligned}$$

where the last step follows from the fact that where $H = 0$, we have $S = b$, because if $H = 0$, then $h_f = \rho H / \rho_o = 0$, and hence $h \geq h_f$ because h is never negative. Where $H = 0$ the ice is therefore grounded, and $B = b$ and therefore $S - b = S - B = H = 0$, so $S = b$.

Hence

$$g\rho h\partial_x s + \frac{1}{2}gh^2\partial_x \rho = \frac{1}{2}g\partial_x(\rho h^2 - \rho_o d^2) + g(\rho h - \rho_o d)\partial_x b. \quad (1.45)$$

Because $\rho_o d \leq \rho h$, with the equality sign fulfilled where the ice is afloat, the second terms on the right-hand sides of Eqs. (1.43) and (1.44) are positive where the ice is both partly and fully grounded, and zero where it is afloat. Therefore

$$\begin{aligned} g(\rho h - \rho_o d)\partial_x b &= \mathcal{H}(h - h_f)g(\rho h - \rho_o d)\partial_x b \\ &= \mathcal{H}(h - h_f)g(\rho h - \rho_o d)\partial_x B \\ &= \mathcal{H}(h - h_f)g(\rho h - \rho_o H^+)\partial_x B \end{aligned}$$

where we used (1.31) and

$$d = \mathcal{H}(h_f - h)\rho h / \rho_o + \mathcal{H}(h - h_f)H^+,$$

and hence

$$\mathcal{H}(h - h_f)d = \mathcal{H}(h - h_f)H^+,$$

in an integrated sense.

The basal drag terms are also zero where the ice is afloat and the system can therefore be written as

$$\partial_x(h(2\tau_{xx} + \tau_{yy})) + \partial_y(h\tau_{xy}) - t_{bx} = \frac{1}{2}g\partial_x(\rho h^2 - \rho_o d^2) + g\mathcal{H}(h - h_f)(\rho h - \rho_o H^+)\partial_x B \quad (1.46)$$

$$\partial_y(h(2\tau_{yy} + \tau_{xx})) + \partial_x(h\tau_{xy}) - t_{by} = \frac{1}{2}g\partial_y(\rho h^2 - \rho_o d^2) + g\mathcal{H}(h - h_f)(\rho h - \rho_o H^+)\partial_y B \quad (1.47)$$

This form suggest how we can get boundary condition (10.95) to be the natural boundary condition of our FE formulation. We simply need to take into the boundary integral the first terms on the left and right-hand sides of (1.46) and (1.47) and the details are given in Section 2.1.

Written in terms of the velocity components:

$$\begin{aligned} \partial_x(h\eta(4\partial_x u + 2\partial_y v)) + \partial_y(h\eta(\partial_y u + \partial_x v)) - t_{bx} = \\ \frac{1}{2}g\partial_x(\rho h^2 - \rho_o d^2) + g\mathcal{H}(h - h_f)(\rho h - \rho_o H^+)\partial_x B \end{aligned} \quad (1.48)$$

$$\begin{aligned} \partial_y(h\eta(4\partial_y v + 2\partial_x u)) + \partial_x(h\eta(\partial_x v + \partial_y u)) - t_{by} = \\ \frac{1}{2}g\partial_y(\rho h^2 - \rho_o d^2) + g\mathcal{H}(h - h_f)(\rho h - \rho_o H^+)\partial_y B \end{aligned} \quad (1.49)$$

1.12 SSTREAM in 1HD

In one horizontal dimension (1HD), i.e. in the flow-line case, the SSTREAM equation becomes

$$4\partial_x(h\eta\partial_x u) - t_{bx} = \frac{1}{2}g\partial_x(\rho h^2 - \rho_o d^2) + g\mathcal{H}(h - h_f)(\rho h - \rho_o H^+)\partial_x B$$

with

$$\eta = \frac{1}{2}A^{-1/n}\dot{\epsilon}^{(1-n)/n} = \frac{1}{2}A^{-1/n}|\partial_x u|^{(1-n)/n}$$

if we use Glen's flow law, and with

$$t_{bx} = \mathcal{H}(h - h_f) C^{-1/m} |u|^{1/m-1} u$$

if we use Weertman sliding law.

Inserting Glen's flow law we get

$$2\partial_x(A^{-1/n} h |\partial_x u|^{(1-n)/n} \partial_x u) - t_{bx} = \frac{1}{2}g\partial_x(\rho h^2 - \rho_o d^2) + g\mathcal{H}(h - h_f)(\rho h - \rho_o H^+)\partial_x B \quad (1.50)$$

and if we can assume that $u > 0$ and $\partial_x u > 0$ then the SSTREAM equation is

$$2\partial_x \left(A^{-1/n} h (\partial_x u)^{1/n} \right) - \mathcal{H}(h - h_f) C^{-1/m} u^{1/m} = \rho g h \partial_x s + \frac{1}{2} g h \partial_x \rho \quad (1.51)$$

Eq. (1.51) is a fairly common way of writing down the SSTREAM/SSA equation in 1HD.

Chapter 2

Finite-element implementation

2.1 FE formulation of the diagnostic equations

. In the FE method the inner product of the field equations with a test function is formed. The inner product is

$$\langle \phi, \theta \rangle = \iint_{\Omega} \phi \theta \, dx \, dy$$

where ϕ and θ are some functions. One form of Green's theorem states that

$$\iint_{\Omega} \phi \partial_x \theta \, dx \, dy = - \iint_{\Omega} \partial_x \phi \theta \, dx \, dy + \oint_{\Gamma} \phi \theta \, n_x \, d\Gamma$$

Applying the Green's theorem on the stress terms and the first term on the right-hand side of that of Eq. (1.46), i.e. x direction, leads to

$$\begin{aligned} 0 = & \iint_{\Omega} \left(h(2\tau_{xx} + \tau_{yy}) \partial_x \phi + h\tau_{xy} \partial_y \phi - \frac{1}{2}g(\rho h^2 - \rho_o d^2) \partial_x \phi + t_{bx} N \right. \\ & \left. + \phi g \mathcal{H}(h - h_f)(\rho h - \rho_o H^+) \partial_x B \right) dx \, dy \\ & - \oint_{\Gamma} (h(2\tau_{xx} + \tau_{yy}) \phi n_x + h\tau_{xy} \phi n_y - \frac{1}{2}g(\rho h^2 - \rho_o d^2) \phi n_x) \, d\Gamma \end{aligned}$$

Performing the same calculation of the y direction results in boundary terms that are identically equal to zero for the boundary condition (10.95). The natural boundary condition is therefore exactly (10.95) and covers not only the case of a fully floating ice front, but that of a grounded and partially grounded ice fronts as well. Using Eq. (1.29) the draft (d) appearing the equations above can be written in terms of the ice thickness (h). This formulation is therefore well suited as a starting point for a linearisation around h required for a fully implicit solution of transient flow.

Expressing this equation in terms of the velocity components u and v

$$\begin{aligned} 0 = & \iint_{\Omega} (h\eta(4\partial_x u + 2\partial_y v) \partial_x \phi + h\eta(\partial_y u + \partial_x v) \partial_y \phi + \mathcal{H}(h - h_f) \beta^2 u \phi \\ & - \frac{1}{2}g(\rho h^2 - \rho_o d^2) \partial_x \phi + \phi g \mathcal{H}(h - h_f)(\rho h - \rho_o H^+) \partial_x B) dx \, dy \\ & - \oint_{\Gamma} (h\eta(4\partial_x u + 2\partial_y v) \phi n_x + h\eta(\partial_y u + \partial_x v) \phi n_y - \frac{1}{2}g(\rho h^2 - \rho_o d^2) N) n_x \, d\Gamma \end{aligned} \quad (2.1)$$

$$\begin{aligned} 0 = & \iint_{\Omega} (h\eta(4\partial_y v + 2\partial_x u) \partial_y \phi + h\eta(\partial_x v + \partial_y u) \partial_x \phi + \mathcal{H}(h - h_f) \beta^2 v \phi \\ & - \frac{1}{2}g(\rho h^2 - \rho_o d^2) \partial_y \phi + \phi g \mathcal{H}(h - h_f)(\rho h - \rho_o H^+) \partial_x B) dx \, dy \\ & - \oint_{\Gamma} (h\eta(4\partial_y v + 2\partial_x u) \phi n_y + h\eta(\partial_x v + \partial_y u) \phi n_x - \frac{1}{2}g(\rho h^2 - \rho_o d^2) N) n_y \, d\Gamma \end{aligned} \quad (2.2)$$

where the corresponding expression in y direction has been added.

2.2 FE formulation of the prognostic equations

$\hat{U}a$ allows for fully implicit time integration with respect to both geometry, grounding-line migration, and velocity. This approach is not limited by the CFL condition and is unconditionally stable allowing for arbitrarily large time steps irrespective of spatial discretisation. The time step is only limited by the convergence radius of the Newton-Raphson method.

The recommended option in a transient run is to use a fully implicit Θ method combined with the consistent streamline-upwind Petrov-Galerkin method (SUPG). This is the default option.

In $\hat{U}a$ a semi-implicit approach (implicit with respect to geometry, explicit with respect to velocity) is also implemented. Unless memory is a limiting factor, the fully implicit approach is always preferable to the semi-implicit (staggered) approach.

Experience shows the Θ method to give good results when used in a combination with a fully implicit forward time integration. For a semi-implicit approach a third-order Taylor Galerkin (TG3) is a better approach.

In 2HD both Θ and TG3 have been implemented for both staggered and implicit approach. (The 1HD fully implicit was only done using the Θ method and not using TG3.) Using TG3 in 1HD staggered approach resulted in a great improvement over the Θ method. It appears that in the implicit approach there is no great advantage of using TG3.

There is no separate diffusion term added to the prognostic equations in $\hat{U}a$, and no shock-stabilisation term either. Even just using the fully implicit approach without SUPG generally gives good results. But using SUPG is nevertheless recommended, especially for problems involving grounding line migration.

2.2.1 Mass flux equation

The vertically integrated form of the mass conservation equation used in $\hat{U}a$ is Eq. (1.11).

2.2.2 Θ method or the ‘generalized trapezoidal rule’

In the Θ method the left-hand side is approximated by the discrete first-order derivative $\Delta h / \Delta t = (h_1 - h_0) / (t_1 - t_0)$ and the right-hand side is replaced by a weighted average of the values at time step $t = t_1$ and $t = t_0$, i.e.

$$\frac{\Delta h}{\Delta t} = \Theta \partial_t h_1 + (1 - \Theta) \partial_t h_0$$

where

$$\begin{aligned} \rho \partial_t h_0 &= \rho a_0 - \partial_x q_{x0} - \partial_y q_{y0} \\ \rho \partial_t h_1 &= \rho a_1 - \partial_x q_{x1} - \partial_y q_{y1} \end{aligned}$$

and $0 \leq \Theta \leq 1$. For $\Theta > 0$ the resulting system is implicit with respect to both thickness (h) and velocity (u and v).

2.2.3 Third order implicit Taylor Galerkin (TG3)

This method is also referred to as fourth-order Crank-Nicolson time-stepping.

First expand h at time step 1 and 0 using third order Taylor expansion as

$$\begin{aligned} h_1 &= h_0 + \Delta t \partial_t h_0 + \frac{(\Delta t)^2}{2} \partial_{tt}^2 h_0 + \frac{(\Delta t)^3}{6} \partial_{ttt}^3 h_0, \\ h_0 &= h_1 - \Delta t \partial_t h_1 + \frac{(\Delta t)^2}{2} \partial_{tt}^2 h_1 - \frac{(\Delta t)^3}{6} \partial_{ttt}^3 h_1, \end{aligned}$$

adding and simplifying gives

$$\frac{1}{\Delta t} (h_1 - h_0) = \frac{1}{2} (\partial_t h_0 + \partial_t h_1) + \frac{\Delta t}{4} (\partial_{tt}^2 h_0 - \partial_{tt}^2 h_1) + \frac{(\Delta t)^2}{12} (\partial_{ttt}^3 h_0 + \partial_{ttt}^3 h_1) \quad (2.3)$$

Note that including only the first term of the Taylor expansion is equal to using the Θ method with $\Theta = 1/2$.)

Then replace the third-order derivative is expressed through finite differences giving

$$\begin{aligned}\frac{1}{\Delta t}(h_1 - h_0) &= \frac{1}{2}(\partial_t h_0 + \partial_t h_1) + \frac{\Delta t}{4}(\partial_{tt}^2 h_0 - \partial_{tt}^2 h_1) + \frac{(\Delta t)^2}{12\Delta t}(\partial_{tt}^2(h_1 - h_0) + \partial_{tt}^2(h_1 - h_0)) \\ &= \frac{1}{2}(\partial_t h_0 + \partial_t h_1) + \frac{\Delta t}{4}(\partial_{tt}^2 h_0 - \partial_{tt}^2 h_1) + \frac{\Delta t}{6}\partial_{tt}^2(h_1 - h_0)\end{aligned}$$

i.e.

$$h_1 - h_0 = \frac{\Delta t}{2}(\partial_t h_0 + \partial_t h_1) + \frac{(\Delta t)^2}{12}(\partial_{tt}^2 h_0 - \partial_{tt}^2 h_1) \quad (2.4)$$

The final step is to replace time derivatives with spatial derivatives through repeated use of the prognostic equation.

The flux \mathbf{q} is a time dependent function of both h and \mathbf{v} , using the prognostic equation the second time derivative of h can be written as

$$\begin{aligned}\rho\partial_{tt}^2 h &= \rho\partial_t a - \nabla_{xy} \cdot \partial_t \mathbf{q} \\ &= \rho\partial_t a - \nabla_{xy} \cdot (\partial_h \mathbf{q} \partial_t h + (\nabla_{uv} \mathbf{q}) \cdot \partial_t \mathbf{v}) \\ &= \rho\partial_t a - \nabla_{xy} \cdot (\partial_h \mathbf{q}(\rho a - \nabla_{xy} \cdot \mathbf{q})/\rho + (\nabla_{uv} \mathbf{q}) \cdot \partial_t \mathbf{v})\end{aligned}$$

or

$$\rho\partial_{tt}^2 h = \rho\partial_t a - \nabla_{xy} \cdot (\partial_h \mathbf{q}(\rho a - \nabla_{xy} \cdot \mathbf{q})/\rho + (\nabla_{uv} \mathbf{q}) \cdot \partial_t \mathbf{v}) \quad (2.5)$$

where $\nabla_{uv} := (\partial_u, \partial_v)$ is the *horizontal velocity gradient operator*.

Third-order Taylor-Galerkin (TG3) for SSHEET/SSA

Using (2.5) in the SSHEET/SIA approximation where

$$\mathbf{q} = \mathbf{v}(h),$$

we find that

$$\rho\partial_{tt}^2 h = \rho\partial_t a - \nabla_{xy} \cdot (\partial_h \mathbf{q}(\rho a - \nabla_{xy} \cdot \mathbf{q})).$$

TG3 for SSTREAM/SSA

Using (2.5) in the SSTREAM/SSA approximation where $\mathbf{q} = \rho h \mathbf{v}$ we find¹

$$\rho\partial_{tt}^2 h = \rho\partial_t a - \nabla_{xy} \cdot (\mathbf{v}(\rho a - \nabla_{xy} \cdot \mathbf{q}) + \rho h \partial_t \mathbf{v}) \quad (2.7)$$

The Third-Order-Taylor-Galerkin (TG3) method is obtained by inserting Eqs. (1.12) and (2.7) into Eq. (2.4) leading to

$$\begin{aligned}\langle \rho(h_1 - h_0), N \rangle &= \frac{\Delta t}{2} (\langle \rho a_0 - \nabla_{xy} \cdot \mathbf{q}_0, N \rangle + \langle \rho a_1 - \nabla_{xy} \cdot \mathbf{q}_1, N \rangle) \\ &\quad + \frac{1}{2} \frac{\Delta t^2}{6} (\langle \rho a_0 - \nabla_{xy} \cdot \mathbf{q}_0, \mathbf{v}_0 \cdot \nabla_{xy} N \rangle - \langle \rho a_1 - \nabla_{xy} \cdot \mathbf{q}_1, \mathbf{v}_1 \cdot \nabla_{xy} N \rangle)\end{aligned} \quad (2.8)$$

(where a few terms involving $\partial_t u$ and $\partial_t a$ have been omitted as well as the boundary terms, see below). This is from suitable for a fully implicit approach, i.e. where both thickness and velocity is solved for implicitly. Note that the higher-order terms (i.e. those of second and third order) in the implicit TG3 method for $t = t_0$ and $t_1 = t_0 + \Delta t$ have opposite signs. In steady-state they will therefore cancel each other out.

¹This expression can also be derived operating on each component as follows

$$\begin{aligned}\partial_{tt}^2 h &= \partial_t a - \partial_{tx}^2(hu) - \partial_{ty}^2(hv) \\ &= \partial_t a - \partial_x(h\partial_t u + u\partial_t h) - \partial_y(h\partial_t v + v\partial_t h)\end{aligned}$$

leading to

$$\partial_{tt}^2 h = \partial_t a - \partial_x(h\partial_t u + u(a - \partial_x(hu) - \partial_y(hv))) - \partial_y(h\partial_t v + v(a - \partial_x(hu) - \partial_y(hv))) \quad (2.6)$$

which is identical to Eq. (2.7).

In more detail the TG3 system is as follows (missing ρ in a number of places):

$$\begin{aligned}
0 = & \frac{1}{\Delta t}(h_1 - h_0) \\
& - \frac{1}{2}(a_0 - \partial_x(q_{x0}) - \partial_y(q_{0y}) + a_1 - \partial_x(q_{x1}) - \partial_y(q_{y1})) \\
& - \frac{\Delta t}{12}(\partial_t a_0 - \partial_x(h_0 \partial_t u_0 + u_0(a_0 - \partial_x(q_{x0}) - \partial_y(q_{0y}))) - \partial_y(h_0 \partial_t v_0 + v_0(a_0 - \partial_x(q_{x0}) - \partial_y(q_{0y})))) \\
& + \frac{\Delta t}{12}(\partial_t a_1 - \partial_x(h_1 \partial_t u_1 + u_1(a_1 - \partial_x(q_{x1}) - \partial_y(q_{y1}))) - \partial_y(h_1 \partial_t v_1 + v_1(a_1 - \partial_x(q_{x1}) - \partial_y(q_{y1}))))
\end{aligned} \tag{2.9}$$

For (2.9) corresponding Galerkin system is

$$\begin{aligned}
0 = & \int (h_1 - h_0 - \frac{\Delta t}{2}(a_0 - \partial_x(q_{x0}) - \partial_y(q_{0y}) + a_1 - \partial_x(q_{x1}) - \partial_y(q_{y1}))) N dA \\
& - \frac{\Delta t^2}{12} \int (\partial_t a_0 N + (h_0 \partial_t u_0 + u_0(a_0 - \partial_x(q_{x0}) - \partial_y(q_{0y}))) \partial_x N \\
& \quad + (h_0 \partial_t v_0 + v_0(a_0 - \partial_x(q_{x0}) - \partial_y(q_{0y}))) \partial_y N) dA \\
& + \frac{\Delta t^2}{12} \int (\partial_t a_1 N + (h_1 \partial_t u_1 + u_1(a_1 - \partial_x(q_{x1}) - \partial_y(q_{y1}))) \partial_x N \\
& \quad + (h_1 \partial_t v_1 + v_1(a_1 - \partial_x(q_{x1}) - \partial_y(q_{y1}))) \partial_y N) dA \\
& + \frac{\Delta t^2}{12} \oint ((h_0 \partial_t u_0 + u_0(a_0 - \partial_x(q_{x0}) - \partial_y(q_{0y}))) n_x \\
& \quad + (h_0 \partial_t v_0 + v_0(a_0 - \partial_x(q_{x0}) - \partial_y(q_{0y}))) n_y) N d\gamma \\
& - \frac{\Delta t^2}{12} \oint ((h_1 \partial_t u_1 + u_1(a_1 - \partial_x(q_{x1}) - \partial_y(q_{y1}))) n_x \\
& \quad + (h_1 \partial_t v_1 + v_1(a_1 - \partial_x(q_{x1}) - \partial_y(q_{y1}))) n_y) N d\gamma
\end{aligned} \tag{2.10}$$

where the second order spatial derivatives have been eliminated through partial integration.

The boundary term can be written as

$$\begin{aligned}
& \frac{\Delta t^2}{12} \oint ((h_0 \partial_t u_0 + u_0(a_0 - \partial_x(q_{x0}) - \partial_y(q_{0y}))) n_x \\
& \quad + (h_0 \partial_t v_0 + v_0(a_0 - \partial_x(q_{x0}) - \partial_y(q_{0y}))) n_y) N d\gamma \\
& - \frac{\Delta t^2}{12} \oint ((h_1 \partial_t u_1 + u_1(a_1 - \partial_x(q_{x1}) - \partial_y(q_{y1}))) n_x \\
& \quad + (h_1 \partial_t v_1 + v_1(a_1 - \partial_x(q_{x1}) - \partial_y(q_{y1}))) n_y) N d\gamma \\
& = \frac{\Delta t^2}{12} \oint ((h_0 \partial_t u_0 + u_0 \partial_t h_0) n_x + (h_0 \partial_t v_0 + v_0 \partial_t h_0) n_y) N d\gamma \\
& - \frac{\Delta t^2}{12} \oint ((h_1 \partial_t u_1 + u_1 \partial_t h_1) n_x + (h_1 \partial_t v_1 + v_1 \partial_t h_1) n_y) N d\gamma \\
& = \frac{\Delta t^2}{12} \oint (\partial_t(q_{x0}) n_x + \partial_t(q_{0y}) n_y) N d\gamma - \frac{\Delta t^2}{12} \oint (\partial_t(q_{x1}) n_x + \partial_t(h_1 v_1) n_y) N d\gamma \\
& = \frac{\Delta t^2}{12} \oint \partial_t(h_0 \mathbf{v}_0 - h_1 \mathbf{v}_1) \cdot \mathbf{n} N d\gamma
\end{aligned}$$

showing that it disappears if $\partial_t q = 0$ over the boundary. Experience suggests that this boundary term can be ignored.

2.3 Consistent Streamline-Upwind Petrov-Galerkin (SUPG)

The standard SUPG is on the form

$$< \rho \partial_t h + \nabla \mathbf{q} - \rho \mathbf{a}, N + M > = 0 \tag{2.11}$$

where M is a perturbation to the test-function space. In the literature various forms for M have been suggested. One such form is

$$M = \tau \mathbf{v} \cdot \nabla N$$

where τ is a parameter with the dimension of time. Note that in (2.11) the added term is applied to all terms, including time derivative. This is sometimes referred to as a 'consistent' weighting. The extra terms are interpreted element-wise, as

$$\langle \rho \partial_t h + \nabla \mathbf{q} - \rho a, N \rangle + \beta \sum_e \langle \rho \partial_t h + \nabla \mathbf{q} - \rho a, \tau \mathbf{v} \cdot \nabla N \rangle = 0$$

The extra term, which is considered as a correction term, is zero for an exact solution in the classical sense.

There is no one single accepted/optimal way of selecting τ , and in the literature various definition has been proposed.

The SUPG was initially introduced for equations on the form $\partial_t h + \mathbf{v} \cdot \nabla h - \nabla \cdot k \nabla h + g = 0$ and in this case, and for linear elements and regular grids, the optimal value for τ is

$$\tau = \frac{l}{2|\mathbf{v}|} \left(\coth \text{Pe} - \frac{1}{\text{Pe}} \right). \quad (2.12)$$

where the Péclet number is

$$\text{Pe} = |\mathbf{v}|l/(2k)$$

with k the diffusivity and l is a measure of the (local) element size. In the limiting case where $k \rightarrow 0$, the equation becomes hyperbolic, $\text{Pe} \rightarrow +\infty$, and τ simply becomes

$$\tau = \frac{l}{2|\mathbf{v}|}$$

and perturbation term to the test function has the form

$$M = \frac{l}{2} \frac{\mathbf{v}}{|\mathbf{v}|} \cdot \nabla N$$

If on the other hand $\text{Pe} \rightarrow 0$ then (2.12) leads to

$$\tau = \frac{l}{2|\mathbf{v}|} \frac{\text{Pe}}{3} = \frac{l}{2|\mathbf{v}|} \frac{|\mathbf{v}|l}{6k} = \frac{l^2}{12k}$$

in which case

$$M = \frac{l^2}{12k} \mathbf{v} \cdot \nabla N$$

The value of τ given by (2.12) does not depend on Δt . For transient problems with high Péclet number it has been suggested using

$$\tau = \tau_t$$

where

$$\tau_t := \Delta t/2 \quad (2.13)$$

as well as

$$\tau = \tau_s$$

where

$$\tau_s := \frac{l}{2|\mathbf{v}|} \quad (2.14)$$

The first definition is a temporal criterion while the second is a spatial criterion. The first form is often used in transient situations where diffusion is small and the problem either hyperbolic or close to being hyperbolic. The second form is often used for convection diffusion problems involving temperature such as $\mathbf{v} \cdot \nabla T - \nabla \cdot (k \nabla T) + f = 0$ (no time dependency).

A guidance as to how to select τ is looking at some specific limits. The SUPG correction term should vanish if $\Delta t \rightarrow 0$, if $l \rightarrow 0$, and also if $|\mathbf{v}| \rightarrow 0$. A smart choice could then be

$$\tau = \tau_1$$



Figure 2.1: SUPG

where

$$\tau_1 := \frac{l}{2|\mathbf{v}|} \kappa \quad (2.15)$$

with

$$\kappa = \coth \xi - \frac{1}{\xi} \quad (2.16)$$

where

$$\xi = |\mathbf{v}| \Delta t / l$$

is the element Courant number and l is a characteristic local element size. For $\xi \ll 1$ we have $\kappa \sim \xi/3$, and

$$\tau = \frac{l}{2|\mathbf{v}|} \frac{|\mathbf{v}| \Delta t}{3l} = \Delta t / 6$$

hence

$$M = \frac{\Delta t}{6} \mathbf{v} \cdot \nabla N$$

showing that the perturbation term goes to zero as either $\Delta t \rightarrow 0$ and $|\mathbf{v}| \rightarrow 0$. If $l \rightarrow 0$ then $\kappa \rightarrow 1$ and $\tau \rightarrow 0$, so all the above listed limits are obtained with τ given by (2.15). In summary, for $\tau = \tau_1$ given by (2.15) we have

$$M = \frac{l\kappa}{2|\mathbf{v}|} \mathbf{v} \cdot \nabla N$$

and the following limits

$$M \rightarrow 0 \quad \text{when} \quad \Delta t \rightarrow 0$$

$$M \rightarrow 0 \quad \text{when} \quad |\mathbf{v}| \rightarrow 0$$

$$M \rightarrow 0 \quad \text{when} \quad l \rightarrow 0$$

$$M \rightarrow \frac{l}{2} \frac{\mathbf{v}}{|\mathbf{v}|} \cdot \nabla N \quad \text{when} \quad \Delta t \rightarrow \infty \quad (2.17)$$

$$M \rightarrow \frac{l}{2} \frac{\mathbf{v}}{|\mathbf{v}|} \cdot \nabla N \quad \text{when} \quad |\mathbf{v}| \rightarrow \infty \quad (2.18)$$

$$M \rightarrow \frac{\Delta t}{6} \mathbf{v} \cdot \nabla N \quad \text{when} \quad l \rightarrow \infty \quad (2.19)$$

In the literature it is shown that (2.17) and (2.18) is the optimal choice in the convective limit, i.e. for large element Courant numbers. Limit (2.19) can be justified using Taylor-Galerkin approach (see below).

The local element size, l , can be defined in a number of similar ways leading to different numerical pre-factors to τ and ξ . The exact functional relationship between κ and ξ is also not uniquely defined.

Another option of creating a smooth transition between the temporal and spatial criteria 2.13 and 2.14 is to select τ as

$$\tau = \tau_2$$

where

$$\tau_2 := \left(\frac{1}{\tau_t} + \frac{1}{\tau_s} \right)^{-1} \quad (2.20)$$

which can also be written as

$$\tau_2 := \frac{1}{2} \frac{\Delta t}{1 + \xi} \quad (2.21)$$

Expression (2.21) gives the limits

$$\begin{aligned} M &\rightarrow 0 && \text{when } \Delta t \rightarrow 0 \\ M &\rightarrow 0 && \text{when } |\mathbf{v}| \rightarrow 0 \\ M &\rightarrow 0 && \text{when } l \rightarrow 0 \\ M &\rightarrow \frac{l}{2} \frac{\mathbf{v}}{|\mathbf{v}|} \cdot \nabla N && \text{when } \Delta t \rightarrow \infty \\ M &\rightarrow \frac{l}{2} \frac{\mathbf{v}}{|\mathbf{v}|} \cdot \nabla N && \text{when } |\mathbf{v}| \rightarrow \infty \\ M &\rightarrow \frac{\Delta t}{2} \mathbf{v} \cdot \nabla N && \text{when } l \rightarrow \infty \end{aligned} \quad (2.22)$$

Apart from a different numerical factor in the last limit, all limits are the same as obtained using definition (2.15).

Summarizing

$$\begin{aligned} \tau_1 &:= \frac{l}{2|\mathbf{v}|} \kappa \\ &= \frac{l}{2|\mathbf{v}|} \left(\coth \xi - \frac{1}{\xi} \right) \\ &= \frac{l}{2|\mathbf{v}|} \left(\coth \left(\frac{|\mathbf{v}| \Delta t}{l} \right) - \frac{l}{|\mathbf{v}| \Delta t} \right) \end{aligned}$$

Since

$$\begin{aligned} \frac{l}{2|\mathbf{v}|} &= \frac{\Delta t}{2} \frac{l}{|\mathbf{v}| \Delta t} \\ &= \frac{\Delta t}{2} \frac{1}{\xi} \end{aligned}$$

These two above listed options for τ can also be written as

$$\tau_1 = \frac{\Delta t}{2} \frac{1}{\xi} (\coth \xi - 1/\xi) \quad (2.23)$$

$$\tau_2 = \frac{\Delta t}{2} \frac{1}{1 + \xi} \quad (2.24)$$

and they are shown in Fig. 2.1b as functions of ξ for $\Delta t/2 = 1$. Only for $|\mathbf{v}| \Delta t < 2l$ is there any significant difference, and the difference is never larger than a factor 3 obtained in the limit $\xi \rightarrow 0$. It appears unlikely that there will be any significant resulting differences between selecting $\tau = \tau_1$ or $\tau = \tau_2$ (see (2.23) and (2.24)). Currently the SUPG implementation in $\hat{U}a$ uses $\tau = \tau_1$.

Implementing SUPG implicitly using the Θ method leads to

$$\begin{aligned} 0 &= \langle \rho(h_1 - h_0)/\Delta t + (1 - \Theta)(\nabla_{xy} \cdot \mathbf{q}_0 - a_0) + \Theta(\nabla_{xy} \cdot \mathbf{q}_1 - a_1), N \rangle \\ &+ \beta \langle \rho(h_1 - h_0)/\Delta t + (1 - \Theta)(\nabla_{xy} \cdot \mathbf{q}_0 - a_0) + \Theta(\nabla_{xy} \cdot \mathbf{q}_1 - a_1), \tau((1 - \Theta)\mathbf{v}_0 + \Theta\mathbf{v}_1) \cdot \nabla_{xy} N \rangle \end{aligned}$$

Here the perturbation to the test-function space is a weighted average over the values at the beginning and the end of the time step. This adds another source of non-linearity to the problem. Experience showed this to reduce the radius of convergence considerably and to increase grumpiness on a personal level. Former can be avoided by using the value of perturbation term at the beginning of the time step, i.e.

$$\begin{aligned} 0 &= \langle \rho(h_1 - h_0)/\Delta t + (1 - \Theta)(\nabla_{xy} \cdot \mathbf{q}_0 - a_0) + \Theta(\nabla_{xy} \cdot \mathbf{q}_1 - a_1), N \rangle \\ &+ \beta \langle \rho(h_1 - h_0)/\Delta t + \nabla_{xy} \cdot \mathbf{q}_0 - a_0, \tau \mathbf{v}_0 \cdot \nabla_{xy} N \rangle \end{aligned}$$

2.4 SIA-motivated diffusion

$$\mathbf{q} = \mathbf{q}^b + \mathbf{q}^d$$

where

$$\mathbf{q}^b = \mathbf{v}h$$

and

$$\mathbf{q}^d = \rho D h^{n+2} |\nabla_{xy} s|^{n-1} \nabla s$$

where

$$D = \frac{2A}{n+2} (\rho g)^n$$

$$s = (h + B)\mathcal{H}(h - h_f) + (1 - \mathcal{H}(h - h_f))(S + (1 - \rho/\rho_o) h)$$

and therefore (almost)

$$\partial_x s = (\partial_x h + \partial_x B)\mathcal{H}(h - h_f) + (1 - \mathcal{H}(h - h_f))(S + (1 - \rho/\rho_o) \partial_x h)$$

This motivates adding a SIA based diffusion term

$$-D < |\nabla_{xy} s|^{n-1} h^{n+2} \nabla s | \nabla_{xy} N >$$

However, this is (currently) not done in $\hat{U}a$.

In 1HD the SIA form of the continuity equation can be written as

$$\rho \partial_t s + \partial_x (k \partial_x s) = \rho a$$

with

$$k := \frac{2\rho A (\rho g)^n}{n+2} |\partial_x s|^{n-1} h^{n+2}$$

suggesting a Peclet number

$$\text{Pe} = \frac{uL}{k}$$

2.5 Connection between third order Taylor-Galerkin (TG3) and streamline-upwind Petrov-Galerkin (SUPG)

In the context of SUPG, the TG3 system given by (2.8) can be thought of as also introducing extra weighting term beside the standard N term. But those additional weighting terms are only applied to the source term (a) and the spatial term, and not to the time-derivative term. Furthermore, as mentioned above in a steady-state these extra weighting terms cancel each other out.

It is instructive to consider as well the case where a second-order forward Taylor expansion

$$h_1 = h_0 + \Delta t \partial_t h_0 + \frac{(\Delta t)^2}{2} \partial_{tt}^2 h_0 \quad (2.25)$$

is used instead of the centred expansion given by Eq. (2.4). Inserting (1.12) and (2.7) into (2.25) gives

$$0 = h_1 - h_0 - \Delta t (a - \nabla_{xy} \cdot \mathbf{q}) - \frac{(\Delta t)^2}{2} (\partial_t a - \nabla_{xy} \cdot (\mathbf{v}(a - \nabla_{xy} \cdot \mathbf{q}) + h \partial_t \mathbf{v}))$$

The Galerkin system is

$$\begin{aligned} < \rho(h_1 - h_0), N > = \Delta t < \rho a - \nabla_{xy} \cdot \mathbf{q}, N > \\ &+ \frac{(\Delta t)^2}{2} < \rho \partial_t a, N > - \frac{(\Delta t)^2}{2} < \rho h \partial_t \mathbf{v}, N > \\ &+ \frac{(\Delta t)^2}{2} < \rho a - \nabla_{xy} \cdot \mathbf{q}, \mathbf{v} \cdot \nabla_{xy} N > \end{aligned}$$

where a partial integration has been used to get rid of second order derivatives (not writing the boundary terms). The last term is similar to what in some other ad-hoc methods is introduced as a stabilisation

term. This term only acts in the direction of flow and is zero transverse to the flow direction. In the above expression all terms are to be evaluated at the beginning of the interval. This approach is usually referred to as the second-order explicit Taylor-Galerkin (TG2e) method. If we evaluate all terms by taking the mean value over the time interval, we get an implicit method, but now the second-order terms do not cancel out in steady-state, and the resulting method is quite similar to the streamline-upwind Petrov-Galerkin.

Dropping the time derivatives of a and \mathbf{v} , we can rewrite the above system as

$$\langle \rho(h_1 - h_0)/\Delta t, N \rangle = \langle \rho a - \nabla_{xy} \cdot \mathbf{q}, N + \frac{1}{2} \Delta t \mathbf{v} \cdot \nabla_{xy} N \rangle$$

showing that the TG2e results in an ‘inconsistent’ weighting with $\tau = \Delta t$ and $\beta = 1/2$.

Comparing 2.8 with (2.11), TG3 can be interpreted as a some sort of Petrov-Galerkin method where only the spatial terms and the source terms are multiplied by a modified test function. In TG3 the modified test function is

$$N + \frac{\Delta t}{6} \mathbf{v} \cdot \nabla_{xy} N, \quad (2.26)$$

wheres in SUPG it has the form

$$N + \beta \tau \mathbf{v} \cdot \nabla_{xy} N. \quad (2.27)$$

The weighting is done inconsistently in TG3, i.e. not over the time-derivative. Apart for the inconsistent weighting used in TG3, the SUPG is equal to TG3 provided the two adjustable parameters β and τ are selected as $\beta = 1/6$ and $\tau = \Delta t$.

The TG3 methods follows automatically from a third-order Taylor expansion and involves no adjustable parameters. The SUPG is in essence a heuristic method.

2.6 Implementing fully-implicit

In a fully implicit approach using the Newton-Raphson iteration the unknowns at time-step 1 are written in incremental form as

$$\begin{aligned} u_1^{i+1} &= \Delta u + u_1^i, \\ v_1^{i+1} &= \Delta v + v_1^i, \\ h_1^{i+1} &= \Delta h + h_1^i, \end{aligned}$$

where u_1^{i+1} is the estimate for u_1 at Newton-Raphson iteration step i . (Note that $\Delta h \neq h_1 - h_0$ and that $\Delta h \rightarrow 0$ with increasing i .)

2.6.1 First-order fully implicit

Taking only the first-order Taylor terms from (2.10), and only considering the x components for the time being, gives

$$\begin{aligned} 0 &= \frac{\rho}{\Delta t} (\Delta h + h_1^i - h_0) \\ &\quad - \frac{1}{2} (\rho(a_0 + a_1) - \partial_x(\rho u_0 h_0) - \partial_x(\rho(\Delta h + h_1^i)(\Delta u + u_1^i))) \end{aligned}$$

If the specific mass balance is a function of thickness, i.e.

$$a = a(h)$$

then an additional term must be added to the matrix on the left-hand side, and the right-hand side terms must be evaluated within the NR loop every time that the thickness is updated. The first-order equation is then

$$\begin{aligned} 0 &= \frac{\rho}{\Delta t} (\Delta h + h_1^i - h_0) \\ &\quad - \frac{1}{2} (\rho(a_0(h_0) + a_1(h_1) + \partial_h a_1|_{h_1} \Delta h) - \partial_x(q_{x0}) - \partial_x((\Delta h + h_1^i)(\Delta u + u_1^i))) \end{aligned}$$

Ignoring second-order terms and taking the terms involving the unknown Δh to the left-hand side leads to

$$\begin{aligned} \rho \frac{\Delta h}{\Delta t} + \frac{1}{2} (\partial_x(\rho u_1^i \Delta h + \rho h_1^i \Delta u) + \partial_y(\rho v_1^i \Delta h + \rho h_1^i \Delta v) - \rho \partial_h a|_{h_1} \Delta h) \\ = \frac{\rho}{2} (a_0 + a_1) - \frac{\rho}{\Delta t} (h_1^i - h_0) - \frac{1}{2} (\partial_x(q_{x0}) + \partial_x(\rho h_1^i u_1^i) + \partial_y(q_{y0}) + \partial_y(\rho h_1^i v_1^i)) \end{aligned} \quad (2.28)$$

where the corresponding y terms have been added.

2.6.2 Fully implicit SSTREAM time integration with the Θ method

$$K^{uu} \Delta \mathbf{u} := D\mathbf{r}_x(\mathbf{u}_1^i, \mathbf{v}_1^i, \mathbf{h}_1^i)[\Delta \mathbf{u}]$$

$$\begin{bmatrix} K^{uu} & K^{uv} & K^{uh} \\ K^{vu} & K^{vv} & K^{vh} \\ K^{hu} & K^{hv} & K^{hh} \end{bmatrix} \begin{bmatrix} \Delta \mathbf{u} \\ \Delta \mathbf{v} \\ \Delta \mathbf{h} \end{bmatrix} = \begin{bmatrix} \mathbf{r}_u \\ \mathbf{r}_v \\ \mathbf{r}_h \end{bmatrix} \quad (2.29)$$

We go from time step $t = t_0$ to $t = t_1$ and solve for the unknown values for \mathbf{u} , \mathbf{v} , and \mathbf{h} , at $t = t_1$ (\mathbf{u}_1 , \mathbf{v}_1 , and \mathbf{h}_1) given their respective values at $t = t_0$ (\mathbf{u}_0 , \mathbf{v}_0 , and \mathbf{h}_0).

$$\begin{aligned} \mathbf{u}_1^{i+1} &= \mathbf{u}_1^i + \Delta \mathbf{u} \\ \mathbf{v}_1^{i+1} &= \mathbf{v}_1^i + \Delta \mathbf{v} \\ \mathbf{h}_1^{i+1} &= \mathbf{h}_1^i + \Delta \mathbf{h} \end{aligned}$$

For notational simplicity we omit the i superscript and it is to be understood that the values of η , β^2 , h , u , and v are the estimated values at iteration i .

At element level the matrices are

$$\begin{aligned} [K^{uu}]_{pq} &= \int_{\Omega} \{ 4\eta h \partial_x N_p \partial_x N_q + h\eta \partial_y N_p \partial_y N_q + \mathcal{H}(h - h_f) \beta^2 N_p N_q \\ &\quad + h D_{eu} (4\partial_x u + 2\partial_y v) \partial_x N_p + h D_{eu} (\partial_x v + \partial_y u) \partial_y N_p \\ &\quad + D_b u u N_p N_q \} dx dy \end{aligned}$$

$$\begin{aligned} [K^{vv}]_{pq} &= \int_{\Omega} \{ 4\eta h \partial_y N_p \partial_y N_q + h\eta \partial_x N_p \partial_x N_q + \mathcal{H}(h - h_f) \beta^2 N_p N_q \\ &\quad + h D_{ev} (4\partial_y v + 2\partial_x u) \partial_y N_p + h D_{ev} (\partial_x v + \partial_y u) \partial_x N_p \\ &\quad + D_b v v N_p N_q \} dx dy \end{aligned}$$

$$\begin{aligned} [K^{uv}]_{pq} &= \int_{\Omega} \{ h\eta (2\partial_x N_p \partial_y N_q + \partial_y N_p \partial_x N_q) \\ &\quad + h D_{ev} (4\partial_x u + 2\partial_y v) \partial_x N_p + h D_{ev} (\partial_x v + \partial_y u) \partial_y N_p \\ &\quad + D_b u v N_p N_q \} dx dy \end{aligned}$$

$$\begin{aligned} [K^{vu}]_{pq} &= \int_{\Omega} \{ h\eta (2\partial_y N_p \partial_x N_q + \partial_x N_p \partial_y N_q) \\ &\quad + h D_{eu} (4\partial_y v + 2\partial_x u) \partial_y N_p + h D_{eu} (\partial_x v + \partial_y u) \partial_x N_p \\ &\quad + D_b u v N_p N_q \} dx dy \end{aligned}$$

(floating only (original version))

$$\begin{aligned} [K^{xh}]_{pq} &= \int_{\Omega} \{ \eta (4\partial_x u + 2\partial_y v) \partial_x N_p N_q + \eta (\partial_y u + \partial_x v) \partial_y N_p N_q \\ &\quad + \delta(h - h_f) \beta^2 u N_p N_q \\ &\quad + \rho g (\mathcal{H}(h - h_f) + h \delta(h - h_f)) ((\rho/\rho_o \partial_x h - \partial_x H) \cos \alpha - \sin \alpha) N_p N_q \\ &\quad + \frac{\rho^2 g}{\rho_o} h \mathcal{H}(h - h_f) \cos \alpha N_p \partial_x N_q \\ &\quad - \rho g h \cos \alpha \partial_x N_p N_q \} dx dy \end{aligned}$$

)

(floating only (corrected July 2011))

$$\begin{aligned}
[K^{xh}]_{pq} = & \int_{\Omega} \left\{ \eta(4\partial_x u + 2\partial_y v) \partial_x N_p N_q + \eta(\partial_y u + \partial_x v) \partial_y N_p N_q \right. \\
& + \delta(h - h_f) \beta^2 u N_p N_q \\
& + \rho g \mathcal{H}(h - h_f) ((\rho/\rho_o \partial_x h - \partial_x H) \cos \alpha - \sin \alpha) N_p N_q \\
& + \frac{\rho^2 g}{\rho_o} h \mathcal{H}(h - h_f) \cos \alpha N_p \partial_x N_q \\
& + \frac{\rho^2}{\rho_o} \delta(h - h_f) \cos \alpha h \partial_x h N_p N_q \\
& \left. - \rho g h \cos \alpha \partial_x N_p N_q \right\} dx dy
\end{aligned}$$

)

(general case

$$\begin{aligned}
[K^{uh}]_{pq} = & \int_{\Omega} \left\{ \eta(4\partial_x u + 2\partial_y v) \partial_x N_p N_q + \eta(\partial_y u + \partial_x v) \partial_y N_p N_q \right. \\
& + \delta(h - h_f) \beta^2 u N_p N_q \\
& + \rho g \mathcal{H}(h - h_f) \partial_x B \cos \alpha N_p N_q - \rho g \sin \alpha N_p N_q \\
& \left. - \rho g h \left(1 - \frac{\rho}{\rho_o} \mathcal{H}(h_f - h) \right) \cos \alpha \partial_x N_p N_q \right\}
\end{aligned}$$

)

(floating only version:

$$\begin{aligned}
[K^{vh}]_{pq} = & \int_{\Omega} \left\{ \eta(4\partial_y v + 2\partial_x u) \partial_y N_p N_q + \eta(\partial_x v + \partial_y u) \partial_x N_p N_q \right. \\
& + \delta(h - h_f) \beta^2 v N_p N_q \\
& + \rho g (\mathcal{H}(h - h_f) + h \delta(h - h_f)) (\rho/\rho_o \partial_y h - \partial_y H) \cos \alpha N_p N_q \\
& + \frac{\rho^2 g}{\rho_o} h \mathcal{H}(h - h_f) \cos \alpha N_p \partial_y N_q \\
& \left. - \rho g h \cos \alpha \partial_y N_p N_q \right\} dx dy
\end{aligned}$$

)

(general case

$$\begin{aligned}
[K^{vh}]_{pq} = & \int_{\Omega} \left\{ \eta(4\partial_y v + 2\partial_x u) \partial_y N_p N_q + \eta(\partial_x v + \partial_y u) \partial_x N_p N_q \right. \\
& + \delta(h - h_f) \beta^2 v N_p N_q \\
& + \rho g (\mathcal{H}(h - h_f) + h \delta(h - h_f)) (\rho/\rho_o \partial_y h - \partial_y H) \cos \alpha N_p N_q \\
& + \frac{\rho^2 g}{\rho_o} h \mathcal{H}(h - h_f) \cos \alpha N_p \partial_y N_q \\
& \left. - \rho g h \cos \alpha \partial_y N_p N_q \right\} dx dy
\end{aligned}$$

)

$$[K^{hu}]_{pq} = \theta(\partial_x h N_q + h \partial_x N_q) N_p$$

$$[K^{hv}]_{pq} = \theta(\partial_y h N_q + h \partial_y N_q) N_p$$

$$[K^{hh}]_{pq} = (N_q/\Delta t + \theta(\partial_x u N_q + u \partial_x N_q + \partial_y v N_q + v \partial_y N_q)) N_p$$

In the equations the quantities D_{eu} , D_{ev} , E , and D_b , which arise because of the linearisation of η and β^2 , are given by

$$D_{eu} = E((2\partial_x u + \partial_y v)\partial_x N_q + \frac{1}{2}(\partial_x v + \partial_y u)\partial_y N_q) \quad (2.30)$$

$$D_{ev} = E((2\partial_y v + \partial_x u)\partial_y N_q + \frac{1}{2}(\partial_x v + \partial_y u)\partial_x N_q) \quad (2.31)$$

$$(2.32)$$

$$E = \frac{1-n}{4n} A^{-1/n} \epsilon^{(1-3n)/n}$$

$$D_b := (1/m - 1) C^{-1/m} |\mathbf{v}|^{(1-3m)/m}$$

Third-order Taylor-Galerkin fully implicit

The terms in

$$\begin{aligned} \frac{\Delta t^2}{12} \int & (\partial_t a_1 N + (h_1 \partial_t u_1 + u_1(a_1 - \partial_x(q_{x1}) - \partial_y(q_{y1})))\partial_x N \\ & + (h_1 \partial_t v_1 + v_1(a_1 - \partial_x(q_{x1}) - \partial_y(q_{y1})))\partial_y N) dA \end{aligned} \quad (2.33)$$

from (2.10), need to be linearised. Starting with

$$h_1 \partial_t u_1 + u_1(a_1 - \partial_x(q_{x1}) - \partial_y(q_{y1}))$$

and inserting $h^{i+1} = \Delta h + h_1^i$ etc. gives

$$(\Delta h + h_1^i) \partial_t(\Delta u + u_1^i) + (\Delta u + u_1^i)(a_1 - \partial_x((\Delta h + h_1^i)(\Delta u + u_1^i)) - \partial_y((\Delta h + h_1^i)(\Delta v + v_1^i)))$$

and first ignoring only some second-order terms

$$\partial_t u_1^i \Delta h + h_1^i \partial_t \Delta u + h_1^i \partial_t u_1^i + (a_1 \Delta u + u_1^i a_1) - (\Delta u + u_1^i)(\partial_x(u_1^i \Delta h + h_1^i \Delta u + h_1^i u_1^i) + \partial_y(v_1 \Delta h + h_1^i \Delta v + h_1^i v_1^i))$$

and then ignoring the remaining second-order terms

$$\begin{aligned} & ((u_1^i - u_0)/\Delta t)(\Delta h + h_1^i) + a_1 \Delta u + u_1^i a_1 \\ & - u_1^i(\partial_x(u_1^i \Delta h + h_1^i \Delta u + h_1^i u_1^i) + \partial_y(v_1 \Delta h + h_1^i \Delta v + h_1^i v_1^i)) \\ & - (\partial_x(h_1^i u_1^i) + \partial_y(h_1^i v_1^i)) \Delta u \end{aligned}$$

where $\partial_t u_1^i = (u_1^i - u_0)/\Delta t$ and $\partial_t \Delta u$ has been set to zero. Now shifting the unknowns over to the left-hand side

$$\begin{aligned} & \partial_t u_1^i \Delta h + a_1 \Delta u \\ & - u_1^i(\partial_x(u_1^i \Delta h + h_1^i \Delta u) + \partial_y(v_1 \Delta h + h_1^i \Delta v)) \\ & - (\partial_x(h_1^i u_1^i) + \partial_y(h_1^i v_1^i)) \Delta u \\ & = u_1^i(\partial_x(h_1^i u_1^i) + \partial_y(h_1^i v_1^i)) - u_1^i a_1 - \partial_t u_1^i h_1^i \end{aligned}$$

and adding the y terms

$$\begin{aligned} & (\partial_t u_1^i \partial_x N + \partial_t v_1^i \partial_y N) \Delta h + a_1 (\partial_x N \Delta u + \partial_y N \Delta v) \\ & - (u_1^i \partial_x N + v_1^i \partial_y N)(\partial_x(u_1^i \Delta h + h_1^i \Delta u) + \partial_y(v_1 \Delta h + h_1^i \Delta v)) \\ & - (\partial_x(h_1^i u_1^i) + \partial_y(h_1^i v_1^i)) (\partial_x N \Delta u + \partial_y N \Delta v) \\ & = (\partial_x N u_1^i + \partial_y N v_1^i) (\partial_x(h_1^i u_1^i) + \partial_y(h_1^i v_1^i)) - a_1 (\partial_x N u_1^i + \partial_y N v_1^i) - (\partial_t u_1^i \partial_x N + \partial_t v_1^i \partial_y N) h_1^i \end{aligned}$$

Then adding the remaining higher-order Taylor terms in (2.10), involving fields from time-step zero, to the right-hand side gives

$$\begin{aligned}
& (\partial_t u_1^i \partial_x N + \partial_t v_1^i \partial_y N) \Delta h \\
& + (\partial_x N \Delta u + \partial_y N \Delta v) (a_1 - \partial_x(h_1^i u_1^i) + \partial_y(h_1^i v_1^i)) \\
& - (\partial_x N u_1^i + \partial_y N v_1^i) (\partial_x(u_1^i \Delta h + h_1^i \Delta u) + \partial_y(v_1^i \Delta h + h_1^i \Delta v)) \\
& = \partial_t(a_0 - a_1)N \\
& - (u_1^i \partial_x N + v_1^i \partial_y N)(a_1 - \partial_x(h_1^i u_1^i) + \partial_y(h_1^i v_1^i)) - h_1^i (\partial_t u_1^i \partial_x N + \partial_t v_1^i \partial_y N) \\
& + (u_0 \partial_x N + v_0 \partial_y N)(a_0 - \partial_x(q_{x0}) + \partial_y(q_{0y})) + h_0 (\partial_t u_0 \partial_x N + \partial_t v_0 \partial_y N)
\end{aligned}$$

Now adding first-order Taylor terms from (2.28) to the expression above

$$\begin{aligned}
& \Delta h N + \frac{\Delta t}{2} (\partial_x(u_1^i \Delta h + h_1^i \Delta u) + \partial_y(v_1^i \Delta h + h_1^i \Delta v)) N \\
& + \gamma(\partial_t u_1^i \partial_x N + \partial_t v_1^i \partial_y N) \Delta h \\
& + \gamma(\partial_x N \Delta u + \partial_y N \Delta v) (a_1 - \partial_x(h_1^i u_1^i) + \partial_y(h_1^i v_1^i)) \\
& - \gamma(\partial_x N u_1^i + \partial_y N v_1^i) (\partial_x(u_1^i \Delta h + h_1^i \Delta u) + \partial_y(v_1^i \Delta h + h_1^i \Delta v)) \\
& = \left(h_0 - h_1^i + \frac{\Delta t}{2}(a_0 + a_1) - \frac{\Delta t}{2}(\partial_x(q_{x0}) + \partial_x(h_1^i u_1^i) + \partial_y(q_{0y}) + \partial_y(h_1^i v_1^i)) \right) N \\
& + \gamma \partial_t(a_0 - a_1)N \\
& - \gamma(u_1^i \partial_x N + v_1^i \partial_y N)(a_1 - \partial_x(h_1^i u_1^i) + \partial_y(h_1^i v_1^i)) - \gamma h_1^i (\partial_t u_1^i \partial_x N + \partial_t v_1^i \partial_y N) \\
& + \gamma(u_0 \partial_x N + v_0 \partial_y N)(a_0 - \partial_x(q_{x0}) + \partial_y(q_{0y})) + \gamma h_0 (\partial_t u_0 \partial_x N + \partial_t v_0 \partial_y N)
\end{aligned}$$

which can also be written as

$$\begin{aligned}
& \Delta h N + (\kappa N - \gamma(\partial_x N u_1^i + \partial_y N v_1^i)) (\partial_x(u_1^i \Delta h + h_1^i \Delta u) + \partial_y(v_1^i \Delta h + h_1^i \Delta v)) \\
& + \gamma(\partial_t u_1^i \partial_x N + \partial_t v_1^i \partial_y N) \Delta h \\
& + \gamma(\partial_x N \Delta u + \partial_y N \Delta v) (a_1 - \partial_x(h_1^i u_1^i) - \partial_y(h_1^i v_1^i)) \\
& = (h_0 - h_1^i) N \\
& + (\kappa N + \gamma(u_0 \partial_x N + v_0 \partial_y N)) (a_0 - \partial_x(q_{x0}) - \partial_y(q_{0y})) \\
& + (\kappa N - \gamma(u_1^i \partial_x N + v_1^i \partial_y N)) (a_1 - \partial_x(h_1^i u_1^i) - \partial_y(h_1^i v_1^i)) \\
& + \gamma \partial_t(a_0 - a_1)N \\
& - \gamma h_1^i (\partial_t u_1^i \partial_x N + \partial_t v_1^i \partial_y N) \\
& + \gamma h_0 (\partial_t u_0 \partial_x N + \partial_t v_0 \partial_y N)
\end{aligned}$$

where

$$\gamma = \frac{(\Delta t)^2}{12}$$

and

$$\kappa = \frac{\Delta}{2}$$

2.6.3 Semi-implicit: uv explicit, and h implicit

In the semi-implicit approach h_1 is treated as the unknown while h_0, u_1, v_1, h_0, v_0 are assumed to be known.

Taking the unknown h_1 in (2.10) to the left-hand side gives

$$\begin{aligned}
& \int (h_1 + \frac{\Delta t}{2} (\partial_x(q_{x1}) + \partial_y(q_{y1}))) N dA \\
& + \frac{\Delta t^2}{12} \int ((h_1 \partial_t u_1 - u_1 (\partial_x(q_{x1}) + \partial_y(q_{y1}))) \partial_x N + (h_1 \partial_t v_1 - v_1 (\partial_x(q_{x1}) + \partial_y(q_{y1}))) \partial_y N) dA \\
& - \frac{\Delta t^2}{12} \oint ((h_1 \partial_t u_1 - u_1 (\partial_x(q_{x1}) + \partial_y(q_{y1}))) n_x + (h_1 \partial_t v_1 - v_1 (\partial_x(q_{x1}) + \partial_y(q_{y1}))) n_y) N d\gamma \\
& = \int (h_0 + \frac{\Delta t}{2} (a_0 + a_1 - \partial_x(q_{x0}) - \partial_y(q_{y0}))) N dA \\
& + \frac{\Delta t^2}{12} \int (\partial_t(a_0 - a_1) N + (h_0 \partial_t u_0 - u_1 a_1 + u_0 (a_0 - \partial_x(q_{x0}) - \partial_y(q_{y0}))) \partial_x N \\
& \quad + (h_0 \partial_t v_0 - v_1 a_1 + v_0 (a_0 - \partial_x(q_{x0}) - \partial_y(q_{y0}))) \partial_y N) dA \\
& - \frac{\Delta t^2}{12} \oint ((h_0 \partial_t u_0 - u_1 a_1 + u_0 (a_0 - \partial_x(q_{x0}) - \partial_y(q_{y0}))) n_x \\
& \quad + (h_0 \partial_t v_0 - v_1 a_1 + v_0 (a_0 - \partial_x(q_{x0}) - \partial_y(q_{y0}))) n_y) N d\gamma
\end{aligned}$$

2.7 Transient implicit SSHEET/SIA with the Θ method

An implicit method is used where h_0 and h_1 are the ice thicknesses at the beginning and the end of the time step, respectively. The system is solved using NR, and we write

$$\begin{aligned}
h^{i+1} &= h^i + \Delta h \\
s^{i+1} &= s^i + \Delta h
\end{aligned}$$

where i is the number of the non-linear iteration step. Provided the method converges, Δh goes to zero with increasing i . In the following we simply write h instead of h^i .

$$(h + \Delta h - h_0)/\Delta t = -(1 - \Theta) \nabla_{xy} \cdot \mathbf{q}_0 - \Theta \nabla_{xy} \cdot \mathbf{q}_1 (h + \Delta h)$$

$$\int_{\Omega} (h + \Delta h - h_0) N \Omega = -(1 - \Theta) \Delta t \int_{\Omega} N \nabla_{xy} \cdot \mathbf{q}_0 - \Theta \Delta t \int_{\Omega} N \nabla_{xy} \cdot \mathbf{q}_1 (h + \Delta h) \quad (2.34)$$

To use the above equation we need to know the perturbation in flux due to perturbation in thickness. The simplest way of perturbing $q(h)$ with respect to h is to find

$$\delta q = \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} q(h + \epsilon \Delta h).$$

As using SSHEET for floating ice shelves is somewhat questionable we here only consider the case of grounded ice². Where grounded

$$q_x(h) = -\rho D ((\partial_x h + \partial_x B)^2 + (\partial_y h + \partial_y B)^2)^{(n-1)/2} h^{n+2} \partial_x s,$$

and

$$\Delta s = \Delta h,$$

with

$$D = \frac{2A}{n+2} (\rho g)^n$$

²In general the surface is related to the ice thickness and bed through

$$s = (h + B) \mathcal{H}(h - h_f) + (S + (1 - \rho/\rho_o)h) \mathcal{H}(h_f - h).$$

Therefore

$$\begin{aligned}
\delta q_x &= \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} q_x(h + \epsilon \Delta h) \\
&= -D \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} D \left(\partial_x(s + \epsilon \Delta h)^2 + \partial_y(s + \epsilon \Delta h)^2 \right)^{(n-1)/2} (h + \epsilon \Delta h)^{n+2} \partial_x(s + \epsilon \Delta h) \\
&= -D |\nabla_{xy}s|^{n-1} h^{n+2} \partial_x \Delta h \\
&\quad - D(n+2) |\nabla_{xy}s|^{n-1} h^{n+1} \partial_x s \Delta h \\
&\quad - D(n-1) |\nabla_{xy}s|^{n-3} (\partial_x s \partial_x \Delta h + \partial_y s \partial_y \Delta h) h^{n+2} \partial_x s
\end{aligned}$$

The first term of the final expression shown above is the perturbation in flux due to increase in slope, the second one the perturbation due to increase in thickness. The third term is non-linear terms that vanishes for $n = 1$. This third term represents changes flux caused by a change in effective viscosity due to perturbations in slope. This third terms shows that a change in slope in y direction gives rise to an increase in flux in x direction.

As expected, for a negative unperturbed surface slope ($\partial_x s < 0$), both a positive perturbation in thickness ($\Delta h > 0$), and an increase in (negative) surface slope ($\partial_x \Delta h < 0$), result in a positive perturbation in flux ($\delta q_x > 0$).

2.7.1 SSHEET with no-flux natural boundary condition

Using a variant of Gauss theorem given by Eq. (B.3) we write (2.34) on the form

$$\int_{\Omega} N \nabla_{xy} \cdot \mathbf{q} \, d\Omega = \oint_{\partial\Omega} N \mathbf{q} \cdot \hat{\mathbf{n}} \, d\Gamma - \int_{\Omega} \nabla_{xy} N \cdot \mathbf{q} \, d\Omega \quad (2.35)$$

Applying (2.35) on (linearised) (2.34) and assuming that $\mathbf{q} \cdot \hat{\mathbf{n}} = 0$ on $\partial\Omega$, i.e. ice flux across the boundary is zero (homogeneous Neumann boundary condition), gives

$$\begin{aligned}
\int_{\Omega} (h + \Delta h - h_0) N \, d\Omega &= (1 - \Theta) \Delta t \int_{\Omega} \mathbf{q}_0 \cdot \nabla_{xy} N \, d\Omega \\
&\quad + \Theta \Delta t \int_{\Omega} \mathbf{q}_1^i \cdot \nabla_{xy} N \, d\Omega \\
&\quad + \Theta \Delta t \int_{\Omega} \Delta \mathbf{q} \cdot \nabla_{xy} N \, d\Omega
\end{aligned}$$

or

$$\begin{aligned}
&\int_{\Omega} \Delta h N \, d\Omega - \Theta \Delta t \int_{\Omega} \Delta \mathbf{q} \cdot \nabla_{xy} N \, d\Omega \\
&= - \int_{\Omega} (h - h_0) N \, d\Omega \\
&\quad + (1 - \Theta) \Delta t \int_{\Omega} \mathbf{q}_0 \cdot \nabla_{xy} N \, d\Omega \\
&\quad + \Theta \Delta t \int_{\Omega} \mathbf{q}_1^i \cdot \nabla_{xy} N \, d\Omega
\end{aligned}$$

where $\mathbf{q}^{i+1} = \mathbf{q}^i + \Delta \mathbf{q}$ is the NR iteration, with

$$\begin{aligned}
\Delta q_x &= -D(n-1) h^{n+2} |\nabla_{xy}s|^{n-3} \partial_x s (\partial_x s \partial_x \Delta h + \partial_y s \partial_y \Delta h) \\
&\quad - D(n+2) |\nabla_{xy}s|^{n-1} h^{n+1} \partial_x s \Delta h \\
&\quad - D |\nabla_{xy}s|^{n-1} h^{n+2} \partial_x \Delta h
\end{aligned}$$

Collecting terms, and writing s and h instead of s^{i+1} and h^{i+1} , respectively, and s_0 and h_0 instead of s^i and h^i , gives

$$\begin{aligned}
& \int N_p N_q \Delta h_q d\Omega \\
& + \Theta \Delta t \int D(n+2) |\nabla_{xy} s|^{n-1} h^{n+1} (\partial_x N_p \partial_x s + \partial_y N_p \partial_y s) N_q \Delta h_q d\Omega \\
& + \Theta \Delta t \int D |\nabla_{xy} s|^{n-1} h^{n+2} (\partial_x N_p \partial_x N_q + \partial_y N_p \partial_y N_q) \Delta h_q d\Omega \\
& + \Theta \Delta t \int D(n-1) h^{n+2} |\nabla_{xy} s|^{n-3} (\partial_x N_p \partial_x s + \partial_y N_p \partial_y s) (\partial_x s \partial_x N_q + \partial_y s \partial_y N_q) \Delta h_q d\Omega \\
& = \int (h_0 - h) N_p d\Omega \\
& + (1 - \Theta) \Delta t \int (q_{x0} \partial_x N_p + q_{y0} \partial_y N_p) d\Omega \\
& + \Theta \Delta t \int (q_{x1} \partial_x N_p + q_{y1} \partial_y N_p) d\Omega
\end{aligned}$$

where

$$\begin{aligned}
q_{x0} &= D |\nabla_{xy} s_0|^{(n-1)} h_0^{n+2} \partial_x s_0 \\
q_{y0} &= D |\nabla_{xy} s_0|^{(n-1)} h_0^{n+2} \partial_y s_0 \\
q_{x1} &= D |\nabla_{xy} s_1|^{(n-1)} h_1^{n+2} \partial_x s_1 \\
q_{y1} &= D |\nabla_{xy} s_1|^{(n-1)} h_1^{n+2} \partial_y s_1
\end{aligned}$$

and

$$D = \frac{2A(\rho g)^n}{n+2}$$

2.7.2 Transient SSHEET/SIA with a free-flux natural boundary condition

To arrive at a formulation where free flux is the natural boundary condition we express the flux in terms of the deformational velocity as‘

$$\mathbf{q} = F h \mathbf{v}_d$$

For a ‘free-flux’ boundary condition this is the flux at the in and outflow boundaries.

We solve

$$\int_{\Omega} u_d N \Omega = \int_{\Omega} E |\nabla_{xy} s|^{n-1} h^{n+1} \partial_x s d\Omega \quad (2.36)$$

$$\int_{\Omega} v_d N \Omega = \int_{\Omega} E |\nabla_{xy} s|^{n-1} h^{n+1} \partial_y s d\Omega \quad (2.37)$$

$$\int_{\Omega} (h_1 - h_0) N \Omega = -(1 - \Theta) \Delta t \int_{\Omega} F N \nabla_{xy} \cdot h_0 \mathbf{v}_{d0} \Omega - \Theta \Delta t \int_{\Omega} F N \nabla_{xy} \cdot h \mathbf{v}_{d1} d\Omega \quad (2.38)$$

for u_d , v_d , and h as unknowns. Writing this as a coupled system with u_d , v_d , and h all as unknowns gives a system of first order differential equations, rather than one second order equation. This eliminates the need to get rid of a second spatial derivative and the natural boundary conditions now corresponds to a ‘free-flux’ condition.

NR with

$$\begin{aligned}
u^{i+1} &= u^i + \Delta u \\
v^{i+1} &= v^i + \Delta v \\
h^{i+1} &= h^i + \Delta h \\
s^{i+1} &= s^i + \Delta h
\end{aligned}$$

and linearising gives

$$\begin{aligned}
u + \Delta u &= E |\nabla_{xy} s|^{n-1} h^{n+1} \partial_x s \\
&\quad + E(n-1) |\nabla_{xy} s|^{n-3} h^{n+1} \partial_x s (\partial_x s \partial_x \Delta h + \partial_y s \partial_y \Delta h) \\
&\quad + E(n+1) |\nabla_{xy} s|^{n-1} h^n \partial_x s \Delta h \\
&\quad + E |\nabla_{xy} s|^{n-1} h^{n+1} \partial_x \Delta h
\end{aligned}$$

Taking the Δ terms to one side

$$\begin{aligned}
& - \Delta u_q \\
& + E(n-1) |\nabla_{xy} s|^{n-3} h^{n+1} \partial_x s (\partial_x s \partial_x \Delta h + \partial_y s \partial_y \Delta h) \\
& + E(n+1) |\nabla_{xy} s|^{n-1} h^n \partial_x s \Delta h \\
& + E |\nabla_{xy} s|^{n-1} h^{n+1} \partial_x \Delta h \\
& = u - E |\nabla_{xy} s|^{n-1} h^{n+1} \partial_x s
\end{aligned}$$

Galerkin, u term:

$$\begin{aligned}
& - \langle N_p, N_q \rangle \Delta u_q \\
& + (n-1) \langle N_p, E |\nabla_{xy} s|^{n-3} h^{n+1} \partial_x s (\partial_x s \partial_x N_q + \partial_y s \partial_y N_q) \rangle \Delta h_q \\
& + (n+1) \langle N_p, E |\nabla_{xy} s|^{n-1} h^n \partial_x s N_q \rangle \Delta h_q \\
& + \langle N_p, E |\nabla_{xy} s|^{n-1} h^{n+1} \partial_x N_q \rangle \Delta h_q \\
& = \langle N_p, u - E |\nabla_{xy} s|^{n-1} h^{n+1} \partial_x s \rangle
\end{aligned}$$

The corresponding v term is obtained by replacing u with v and derivatives with respect to x by derivatives with respect to y ,

Galerkin q term:

$$\begin{aligned}
& \langle N_p, N_q \rangle \Delta h_q + \Delta t \Theta F \langle N_p, \partial_x u N_q + u \partial_x N_q + \partial_y v N_q + v \partial_y N_q \rangle \Delta h_q \\
& \quad + \Delta t \Theta F \langle N_p, \partial_x h N_q + h \partial_x N_q \rangle \Delta u_q \\
& \quad + \Delta t \Theta F \langle N_p, \partial_y h N_q + h \partial_y N_q \rangle \Delta v_q \\
& = \Delta t \langle N_p, (1 - \Theta) a_0 + \Theta a_1 \rangle \\
& \quad - \langle N_p, (h - h_0) \rangle \\
& \quad - \Delta t F \langle N_p, ((1 - \Theta)(\partial_x(h_0 u_0) + \partial_y(h_0 v_0)) + \Theta(\partial_x(hu) + \partial_y(hv))) \rangle
\end{aligned}$$

2.8 Method of characteristics

Think of

$$\partial_t h + \partial_x(uh) = a$$

as

$$D_t h + h \partial_x u = a$$

with

$$D_t h = \partial_t h + u \partial_x h$$

Along the characteristics I have

$$D_t h = \lim_{\Delta t \rightarrow 0} \frac{1}{\Delta t} (h(x, t) - h(x - u \Delta t, t - \Delta t))$$

and if I just write

$$h(x - u \Delta t, t - \Delta t) = h(x, t - \Delta t) - d_x h(x, t - \Delta t) u \Delta t$$

and take the limit I (of course) just get

$$\partial_t h + \partial_x(uh) = a$$

again.

The question seem to me to be about how to approximate the variation along the characteristic. If I write

$$h(x - u \Delta t, t - \Delta t) = h_0 - d_x h_0 u \Delta t + \frac{1}{2} d_{xx}^2 h_0 (u \Delta t)^2$$

where h_0 is evaluated at x and at $t - \Delta t$, I get a ‘correction’ term and the discretized version is

$$\frac{1}{\Delta t} (h_1 - h_0 + d_x h_0 u \Delta t + \frac{1}{2} d_{xx}^2 h_0 (u \Delta t)^2) + h d_x u = a$$

It is a bit unclear in the above expression at what time to evaluate u . I could go for the average value over the time step, but I will only know it at the previous time step anyhow.

One way of interpreting the above equation is

$$\frac{1}{\Delta t} (h_1 - h_0) + d_x h u + \frac{1}{2} u^2 \Delta t d_{xx}^2 h + h d_x u = a$$

and then evaluate all terms at some time within the time step, i.e. the Θ method. The above equation can be written as

$$\frac{1}{\Delta t} (h_1 - h_0) + \partial_x (hu) + \frac{\Delta t}{2} u^2 \partial_{xx}^2 h = a$$

In combination with the Θ method I get

$$\frac{1}{\Delta t} (h_1 - h_0) = \Theta \left(a_1 - \partial_x (q_{x1}) - \frac{1}{2} u_1 \Delta t \partial_{xx}^2 (u_1 h_1) \right) + (1 - \Theta) \left(a_0 - \partial_x (q_{x0}) - \frac{1}{2} u_0 \Delta t \partial_{xx}^2 (u_0 h_0) \right)$$

(did not write the a correction term)

2.9 Taylor-Galerkin

We have

$$\partial_t h + \partial_x (uh) = a$$

we write

$$h_1 = h_0 + \Delta t \partial_t h_0 + \frac{(\Delta t)^2}{2} \partial_{tt}^2 h_0$$

which is a second-order accurate Euler method.

Inserting we get

$$\begin{aligned} h_1 &= h_0 + \Delta t (a - \partial_x (hu)) + \frac{(\Delta t)^2}{2} \partial_t (a - \partial_x (uh)) \\ &= h_0 + \Delta t (a - \partial_x (hu)) + \frac{(\Delta t)^2}{2} (\partial_t a - \partial_{xt}^2 (uh)) \\ &= h_0 + \Delta t (a - \partial_x (hu)) + \frac{(\Delta t)^2}{2} (\partial_t a - \partial_x (\partial_h (uh) \partial_t h)) \\ &= h_0 + \Delta t (a - \partial_x (hu)) + \frac{(\Delta t)^2}{2} (\partial_t a - \partial_x (u \partial_t h)) \\ &= h_0 + \Delta t (a - \partial_x (hu)) + \frac{(\Delta t)^2}{2} (\partial_t a - \partial_x (u(a - \partial_x (uh)))) \end{aligned}$$

or

$$\frac{1}{\Delta t} (h_1 - h_0) = a - \partial_x (hu) - \frac{\Delta t}{2} \partial_x (u(a - \partial_x (uh))) + \frac{\Delta t}{2} \partial_t a \quad (2.39)$$

All the terms of the right-hand side of (2.39) refer to time step 0. I get the implicit theta method if I evaluate the right-hand side at both 1 and 0 and weight with Θ .

After a partial integration we get a correction term

$$< \frac{1}{2} \Delta t \partial_x u, a - \partial_x (uh) >$$

2.10 Third order implicit Taylor Galerkin (1HD)

A better justification for evaluation at both time step 1 and 0 comes from writing

$$h_1 = h_0 + \Delta t \partial_t h_0 + \frac{(\Delta t)^2}{2} \partial_{tt}^2 h_0,$$

$$h_0 = h_1 - \Delta t \partial_t h_1 + \frac{(\Delta t)^2}{2} \partial_{tt}^2 h_1,$$

adding

$$2(h_1 - h_0) = \Delta t (\partial_t h_0 + \partial_t h_1) + \frac{(\Delta t)^2}{2} (\partial_{tt}^2 h_0 - \partial_{tt}^2 h_1)$$

and simplifying gives

$$\frac{1}{\Delta t} (h_1 - h_0) = \frac{1}{2} (\partial_t h_0 + \partial_t h_1) + \frac{\Delta t}{4} (\partial_{tt}^2 h_0 - \partial_{tt}^2 h_1) \quad (2.40)$$

Now use

$$\partial_t h = a - \partial_x(hu)$$

and

$$\begin{aligned} \partial_{tt}^2 h &= \partial_t a - \partial_{tx}^2(hu) \\ &= \partial_t a - \partial_t(h\partial_x u + u\partial_x h) \\ &= \partial_t a - (\partial_x h\partial_t u + h\partial_{xt}^2 u + \partial_t h\partial_x u + u\partial_{xt}^2 h) \\ &= \partial_t a - (\partial_x h\partial_t u + h\partial_{xt}^2 u + \partial_x u(a - \partial_x(hu)) + u\partial_x(a - \partial_x(hu))) \\ &= \partial_t a - (\partial_x h\partial_t u + h\partial_{xt}^2 u + \partial_x u(a - \partial_x(hu)) + u\partial_x(a - \partial_x(hu))) \\ &= \partial_t a - \partial_x h\partial_t u - h\partial_{xt}^2 u - \partial_x(u(a - \partial_x(hu))) \end{aligned}$$

or

$$\partial_{tt}^2 h = \partial_t a - \partial_x(h\partial_t u + ua - u\partial_x(hu)) \quad (2.41)$$

Inserting (2.41) into (2.3) gives

$$\begin{aligned} 0 &= Lh \\ &= \frac{1}{\Delta t} (h_1 - h_0) \\ &\quad - \frac{1}{2} (a_0 - \partial_x(q_{x0}) + a_1 - \partial_x(q_{x1})) \\ &\quad - \frac{\Delta t}{4} (\partial_t a_0 - \partial_x(h_0\partial_t u_0 + u_0(a_0 - \partial_x(q_{x0}))) - \partial_t a_1 + \partial_x(h_1\partial_t u_1 + u_1(a_1 - \partial_x(q_{x1})))) \end{aligned}$$

Galerkin

$$\langle Lh_1 | N_p \rangle = 0$$

with $u_1 = N_q u_q$, etc. I used partial integration to get rid of second order spatial derivatives

$$\begin{aligned} 0 &= \langle Lu | N_q \rangle \\ &= \frac{1}{\Delta t} \int (h_1 - h_0) N_q dx \\ &\quad - \frac{1}{2} \int (a_0 - \partial_x(q_{x0}) + a_1 - \partial_x(q_{x1})) N_q dx \\ &\quad - \frac{\Delta t}{4} \int (\partial_t a_0 - \partial_t a_1) N_q dx \\ &\quad - \frac{\Delta t}{4} \int (h_0\partial_t u_0 + u_0 a_0 - u_0 \partial_x(q_{x0})) - h_1\partial_t u_1 - u_1 a_1 + u_1 \partial_x(q_{x1})) \partial_x N_q dx \\ &\quad - \frac{\Delta t}{4} (-h_0\partial_t u_0 - u_0(a_0 - \partial_x(q_{x0})) + h_1\partial_t u_1 + u_1(a_1 - \partial_x(q_{x1}))) N_q \Big|_{x_l}^{x_r} \end{aligned}$$

The unknown is h_1

$$\begin{aligned}
& \int (h_1 + \frac{\Delta t}{2} \partial_x(q_{x1})) N_q dx \\
& + \frac{(\Delta t)^2}{4} \int (h_1 \partial_t u_1 - u_1 \partial_x(q_{x1})) \partial_x N_q dx + \frac{(\Delta t)^2}{4} (u_1 \partial_x(q_{x1}) - h_1 \partial_t u_1) N_q|_{x_l}^{x_r} \\
& = \int (h_0 + \frac{\Delta t}{2} (a_1 + a_0 - \partial_x(q_{x0}))) N_q dx \\
& + \frac{(\Delta t)^2}{4} \int \partial_t(a_0 + a_1) N_q dx \\
& + \frac{(\Delta t)^2}{4} \int (u_0 a_0 - u_1 a_1 + h_0 \partial_t u_0 - u_0 \partial_x(q_{x0})) \partial_x N_q dx \\
& + \frac{(\Delta t)^2}{4} (u_1 a_1 - u_0 a_0 - h_0 \partial_t u_0 + u_0 \partial_x(q_{x0})) N_q|_{x_l}^{x_r}
\end{aligned}$$

Writing out the product terms

$$\begin{aligned}
& \int (h_1 + \frac{\Delta t}{2} (h_1 \partial_x u_1 + u_1 \partial_x h_1)) N_q dx \\
& + \frac{(\Delta t)^2}{4} \int (\partial_t u_1 h_1 - u_1 (h_1 \partial_x u_1 + u_1 \partial_x h_1)) \partial_x N_q dx \\
& + \frac{(\Delta t)^2}{4} (u_1 (h_1 \partial_x u_1 + u_1 \partial_x h_1) - h_1 \partial_t u_1) N_q|_{x_l}^{x_r} \\
& = \int (h_0 + \frac{\Delta t}{2} (a_1 + a_0 - (h_0 \partial_x u_0 + u_0 \partial_x h_0))) N_q dx \\
& + \frac{(\Delta t)^2}{4} \int \partial_t(a_0 + a_1) N_q dx \\
& + \frac{(\Delta t)^2}{4} \int (u_0 a_0 - u_1 a_1 + h_0 \partial_t u_0 - u_0 (h_0 \partial_x u_0 + u_0 \partial_x h_0)) \partial_x N_q dx \\
& + \frac{(\Delta t)^2}{4} (u_1 a_1 - u_0 a_0 - h_0 \partial_t u_0 + u_0 (h_0 \partial_x u_0 + u_0 \partial_x h_0)) N_q|_{x_l}^{x_r}
\end{aligned}$$

Taking this up to third order

$$\frac{1}{\Delta t} (h_1 - h_0) = \frac{1}{2} (\partial_t h_0 + \partial_t h_1) + \frac{\Delta t}{4} (\partial_{tt}^2 h_0 - \partial_{tt}^2 h_1) + \frac{(\Delta t)^2}{12} (\partial_{ttt}^3 h_0 + \partial_{ttt}^3 h_1)$$

is easy, if we simply approximate the time derivative in the third-order term through finite differences.

$$\begin{aligned}
\frac{1}{\Delta t} (h_1 - h_0) &= \frac{1}{2} (\partial_t h_0 + \partial_t h_1) + \frac{\Delta t}{4} (\partial_{tt}^2 h_0 - \partial_{tt}^2 h_1) + \frac{(\Delta t)^2}{12 \Delta t} (\partial_{tt}^2 (h_1 - h_0) + \partial_{tt}^2 (h_1 - h_0)) \\
&= \frac{1}{2} (\partial_t h_0 + \partial_t h_1) + \frac{\Delta t}{4} (\partial_{tt}^2 h_0 - \partial_{tt}^2 h_1) + \frac{\Delta t}{6} \partial_{tt}^2 (h_1 - h_0) \\
&= \frac{1}{2} (\partial_t h_0 + \partial_t h_1) + \frac{\Delta t}{12} \partial_{tt}^2 h_0 - \frac{\Delta t}{12} \partial_{tt}^2 h_1
\end{aligned}$$

The only thing that changes is the numerical factor of the second-order term. However this is now correct to third order.

Chapter 3

Constraints

In $\dot{U}a$ all essential boundary conditions, and various other constraints on the solution, are enforced using the Lagrange multiplier method. Any multi-linear constraints

$$\mathbf{L}\mathbf{x} - \mathbf{c} = 0$$

where \mathbf{x} are the unknowns for some \mathbf{L} and \mathbf{c} can be described.

3.1 Linear system with multi-linear constraints

For a quadratic minimisation problem

$$\min_{\mathbf{x}} I(\mathbf{x}) = \mathbf{x}^T \mathbf{A} \mathbf{x} - \mathbf{b} \mathbf{x}$$

subject to the linear set of constraints

$$\mathbf{L}\mathbf{x} - \mathbf{c} = 0$$

the system to be solved is

$$\begin{bmatrix} \mathbf{A} & \mathbf{L}^T \\ \mathbf{L} & 0 \end{bmatrix} \begin{bmatrix} \mathbf{x} \\ \boldsymbol{\lambda} \end{bmatrix} = \begin{bmatrix} \mathbf{b} \\ \mathbf{c} \end{bmatrix}$$

3.2 Non-linear system with non-linear constraints

If we both have a non-linear minimisation problem

$$\min_{\mathbf{x}} I(\mathbf{x})$$

and non-linear set of constraints $\mathbf{l}(\mathbf{x}) = 0$, we minimise

$$I(\mathbf{x}) + \boldsymbol{\lambda}^T \mathbf{l}(\mathbf{x}),$$

A stable equilibrium point is a minimum with respect to \mathbf{x} and a maximum with respect to $\boldsymbol{\lambda}$.

Setting the derivatives with respect to \mathbf{x} and $\boldsymbol{\lambda}$ to zero leads to

$$\begin{aligned} \partial_{\mathbf{x}} I(\mathbf{x}) + \boldsymbol{\lambda}^T \partial_{\mathbf{x}} \mathbf{l}(\mathbf{x}) &= 0 \\ \mathbf{l}(\mathbf{x}) &= \mathbf{0} \end{aligned}$$

If this non-linear system is again solved using Newton-Raphson method then we use first-order Taylor expansion and write

$$\begin{aligned} \partial_{\mathbf{x}} I(\mathbf{x}) &= \partial_{\mathbf{x}} I_0 + \boldsymbol{\lambda}_0^T \partial_{\mathbf{x}} \mathbf{l}_0 + \partial_{\mathbf{x}\mathbf{x}}^2 I_0 \Delta \mathbf{x} + \boldsymbol{\lambda}_0 \partial_{\mathbf{x}\mathbf{x}} \mathbf{l}_0 \Delta \mathbf{x} + \partial_{\mathbf{x}} \mathbf{l}_0^T \Delta \boldsymbol{\lambda} \\ \mathbf{l}(\mathbf{x}) &= \mathbf{l}(x_0) + \partial_{\mathbf{x}} \mathbf{l} \Delta \mathbf{x} \end{aligned}$$

and repeatedly solve

$$\begin{bmatrix} \mathbf{H} + \partial_{\mathbf{x}\mathbf{x}}^2 \mathbf{l} \boldsymbol{\lambda}_0 & \partial_{\mathbf{x}} \mathbf{l}^T \\ \partial_{\mathbf{x}} \mathbf{l} & \mathbf{0} \end{bmatrix} \begin{bmatrix} \Delta \mathbf{x} \\ \Delta \boldsymbol{\lambda} \end{bmatrix} = \begin{bmatrix} -\partial_{\mathbf{x}} I_0 - \partial_{\mathbf{x}} \mathbf{l}^T \boldsymbol{\lambda}_0 \\ -\mathbf{l}_0 \end{bmatrix} \quad (3.1)$$

where again $\mathbf{H} = \partial_{\mathbf{x}\mathbf{x}}I$ is the Hessian matrix.

If the constraints are linear, we can write

$$\mathbf{l} = \mathbf{L}\mathbf{x} - \mathbf{c} = \mathbf{0}$$

in which case

$$\partial_{\mathbf{x}}\mathbf{l} = \mathbf{L} \quad \text{and} \quad \partial_{\mathbf{x}\mathbf{x}}^2\mathbf{l} = \mathbf{0}$$

and the system to be solved is

$$\begin{bmatrix} \mathbf{H} & \mathbf{L}^T \\ \mathbf{L} & \mathbf{0} \end{bmatrix} \begin{bmatrix} \mathbf{x} \\ \boldsymbol{\lambda} \end{bmatrix} = \begin{bmatrix} -\partial_{\mathbf{x}}I_0 - \partial_{\mathbf{x}}\mathbf{l}^T \boldsymbol{\lambda}_0 \\ -\mathbf{L}\mathbf{x}_0 + \mathbf{c} \end{bmatrix}$$

3.3 FE formulation of the Newton-Raphson method with multi-linear constraints

The constraints $\mathbf{l} = \mathbf{0}$ imply

$$\langle \mathbf{l}(x), \phi_q(x) \rangle = 0$$

or if we write

We need to solve

$$\langle f(u), N_p \rangle = 0 \quad \text{subject to} \quad g_q(u) = 0 \quad \text{where} \quad q = 1 \dots N$$

Assume there is a scalar $I(u)$ such that $\langle f(u), N_p \rangle$ is derivative of I with respect to u_p , then minimising

$$I(u) + \langle \boldsymbol{\lambda}, g(u) \rangle$$

gives, with $u = N_p u_p$ and $\boldsymbol{\lambda} = N_p \boldsymbol{\lambda}_p$

$$\langle f(u), N_q \rangle + \langle \boldsymbol{\lambda}, \partial g / \partial u N_q \rangle = 0 \quad (3.2)$$

$$\langle N_q, g(u) \rangle = 0 \quad (3.3)$$

In general, the $\langle f(u), N_q \rangle$ system can be expected to be non-linear. Assuming for the moment that the constraints $g(u) = 0$ are linear ($g_q = A_{qr}u_r - b_q = 0$), and that the resulting system is solved using the Newton-Raphson method, gives

$$\langle f(u_0), N_q \rangle + \langle \partial f / \partial u \Delta u, N_q \rangle + \langle \boldsymbol{\lambda}, \partial g / \partial u N_q \rangle = 0 \quad (3.4)$$

$$\langle N_q, \partial g / \partial u (u_0 + \Delta u) - b \rangle = 0 \quad (3.5)$$

or

$$\langle f(u_0), N_q \rangle + \langle \partial f / \partial u N_p, N_q \rangle \Delta u_p + \langle N_p, \partial g / \partial u N_q \rangle \lambda_p = 0 \quad (3.6)$$

$$\langle N_q, \partial g / \partial u N_p \rangle u_{0p} + \langle N_q, \partial g / \partial u N_p \rangle \Delta u_p - \langle N_q, N_p \rangle b_p = 0 \quad (3.7)$$

$$\begin{bmatrix} \mathbf{K} & \mathbf{L}^T \\ \mathbf{L} & \mathbf{0} \end{bmatrix} \begin{bmatrix} \Delta \mathbf{u} \\ \boldsymbol{\lambda} \end{bmatrix} = \begin{bmatrix} -\mathbf{f}(\mathbf{u}_0) \\ \mathbf{S}\mathbf{b} - \mathbf{L}\mathbf{u}_0 \end{bmatrix}$$

where

$$f_q := \langle f(u_0), N_q \rangle$$

and

$$S_{qp} := \langle N_q, N_p \rangle$$

and

$$L_{pq} = \langle N_p, \partial g / \partial u N_q \rangle$$

or if $\boldsymbol{\lambda} = \Delta \boldsymbol{\lambda} + \boldsymbol{\lambda}_0$,

$$\begin{bmatrix} \mathbf{K} & \mathbf{L}^T \\ \mathbf{L} & \mathbf{0} \end{bmatrix} \begin{bmatrix} \Delta \mathbf{u} \\ \Delta \boldsymbol{\lambda} \end{bmatrix} = \begin{bmatrix} -\mathbf{f}(\mathbf{u}_0) - \mathbf{L}^T \boldsymbol{\lambda}_0 \\ \mathbf{S}\mathbf{b} - \mathbf{L}\mathbf{u}_0 \end{bmatrix}$$

If the constraints can be written as $\mathbf{A}\mathbf{u} - \mathbf{b} = \mathbf{0}$ then

$$L_{pq} = \langle N_p, A_{pq} N_q \rangle$$

(no summation implied). If the form functions are delta functions then, $\mathbf{S} = \mathbf{1}$ and $\mathbf{L} = \mathbf{A}$.

3.4 Thickness-positivity constraint

The thickness constraint is

$$h = h_q N_q \geq 0.$$

In the active set method we identify the nodes where $h_i < 0$. The nodes become members of the ‘active set’ \mathcal{A} . Using the Lagrange method, we introduce as many Lagrange parameters as there are elements ($N_{\mathcal{A}}$) in \mathcal{A} and write

$$\lambda = \lambda_p M_p$$

where $p \in \mathcal{A}$. A thickness constrains is

$$h_q = h',$$

where h' is the min allowed ice thickness, usually set to zero.

The Lagrange term is

$$< \lambda, h_q N_q > - < \lambda, h' N_q >$$

(no summation over q), or

$$< M_p \lambda_p, h_q N_q > - < M_p \lambda_p, h' N_q >$$

Note that M_p are the Lagrange shape functions. These could be the same as those used for other fields, but in that case the sum only goes over a sub-set of the FE domain’s shape functions. In principle one could use different shape functions for the λ variables, for example delta functions, i.e.

$$\lambda = \sum_{p=1}^{N_p} \lambda_p \delta(x_p, y_p)$$

where (x_p, y_p) are the coordinates of the nodes in the active set

Taking the derivative with respect to λ and h , gives

$$\sum_{q=1}^{N_h} < M_p, N_q > h_q = 0 \quad (3.8)$$

$$\sum_{p=1}^{N_{\mathcal{A}}} < M_p, N_q > \lambda_p = 0 \quad (3.9)$$

where

$$\begin{bmatrix} \mathbf{H} & \mathbf{L}^T \\ \mathbf{L} & \mathbf{0} \end{bmatrix} \begin{bmatrix} \Delta h \\ \Delta \lambda \end{bmatrix} = \begin{bmatrix} -f(h_0) - \mathbf{L} \lambda_0 \\ -\mathbf{L} h_0 \end{bmatrix}$$

If $M_p = N_p$, i.e. the Lagrange form-functions are the same as used for other fields, then $\mathbf{L} = < N_q, N_p >$ is a subset of the mass matrix. The Lagrange matrix $\mathbf{L} = < N_q, N_p >$ has as many lines as there are thickness constraints, and as many columns as the total number of thickness nodes. It can most easily be formed from the mass matrix by sub-selecting those lines corresponding to constrained thickness nodes.

3.4.1 Thickness barrier

$$I = \gamma_h \lambda_h e^{-(h-h_{\min})/\lambda_h}$$

$$\partial_h I = -\gamma_h e^{-(h-h_{\min})/\lambda_h}$$

$$\partial_{hh}^2 I = \frac{\gamma_h}{\lambda_h} e^{-(h-h_{\min})/\lambda_h}$$

If the problem were self-adjoint then this amounts to adding a term to the prognostic equations, i.e.

$$\partial_t h + \nabla_{xy} \cdot \mathbf{q}_h + \partial_h I = a$$

or

$$\partial_t h + \nabla_{xy} \cdot \mathbf{q}_{xy} = a - \partial_h I$$

which shows that the method is equivalent to adding a fictitious mass-balance term. For $\gamma_h = 1$, $\lambda = 1$, $h_{\min} = 0$ and $h = 100$ the numerical value is about 10^{-44} and about 10^{-5} for $h = 1$.

If $a = 0$ and $\mathbf{v}_h = 0$

$$\partial_t h = -\partial_h I = \gamma_h e^{-(h-h_{\min})/\lambda_h}$$

and γ_h has the units length per time and can be thought of as the fictitious mass balance at $h = h_{\min}$.

Solving

$$\partial_t h + \nabla_{xy} \cdot \mathbf{q}_h - \gamma_h e^{-(h-h_{\min})/\lambda_h} = a$$

implicitly using NR with respect to h where

$$h_{n+1} = h_{n+1}^i + \Delta h$$

is h at time step $n + 1$ and i is the NR iteration number, gives

$$\frac{1}{\Delta t}(\Delta h + h_{n+1}^i - h_n) - \frac{1}{2} \left(\gamma_h e^{-(h_{n+1}^i - h_{\min})/\lambda_h} - \frac{\gamma_h}{\lambda_h} e^{-(h_{n+1}^i - h_{\min})/\lambda_h} \Delta h + \gamma_h e^{-(h_n - h_{\min})/\lambda_h} \right) = 0$$

where I have omitted writing the flux and the accumulation terms, i.e.

$$\left(\frac{1}{\Delta t} + \frac{1}{2} \frac{\gamma_h}{\lambda_h} e^{-(h_{n+1}^i - h_{\min})/\lambda_h} \right) \Delta h = -\frac{1}{\Delta t}(h_{n+1}^i - h_n) + \frac{\gamma_h}{2} \left(e^{-(h_{n+1}^i - h_{\min})/\lambda_h} + \gamma_h e^{-(h_n - h_{\min})/\lambda_h} \right)$$

Chapter 4

Solving the non-linear system

4.1 Convergence criteria

The system to be solved is

$$\mathbf{R}(\mathbf{x}) = \mathbf{0} \quad (4.1)$$

subject to the (linear) constraints

$$\mathbf{l} = \mathbf{L}\mathbf{x} - \mathbf{c} = \mathbf{0} \quad (4.2)$$

Introducing Lagrange parameters we solve (4.1) and (4.2) using the Newton-Raphson method

$$\begin{bmatrix} \mathbf{H} & \mathbf{L}^T \\ \mathbf{L} & \mathbf{0} \end{bmatrix} \begin{bmatrix} \Delta\mathbf{x} \\ \Delta\boldsymbol{\lambda} \end{bmatrix} = \begin{bmatrix} -\mathbf{R}(\mathbf{x}_0) - \mathbf{L}^T \boldsymbol{\lambda}_0 \\ \mathbf{c} - \mathbf{L}\mathbf{x}_0 \end{bmatrix} \quad (4.3)$$

If the Newton-Raphson method converges then $\Delta\mathbf{x} \rightarrow 0$ and $\Delta\boldsymbol{\lambda} \rightarrow 0$. This suggests defining an error criteria in terms of the norm of $\Delta\mathbf{x}$ and $\Delta\boldsymbol{\lambda}$, i.e. in terms of the size of the update to the solution. Another convergence criteria is the norm of the solution error, Writing the residual vector \mathbf{R} as

$$\mathbf{R} = \mathbf{T}(\mathbf{x}) - \mathbf{F}$$

where \mathbf{T} and \mathbf{F} are the internal and the external nodal forces, respectively facilitates the definition of a normalised residual error as

$$r = \sqrt{\frac{(\mathbf{R} + \mathbf{L}^T \boldsymbol{\lambda})^T (\mathbf{R} + \mathbf{L}^T \boldsymbol{\lambda}) + (\mathbf{c} - \mathbf{L}\mathbf{x})^T (\mathbf{c} - \mathbf{L}\mathbf{x})}{\mathbf{F}^T \mathbf{F}}} \quad (4.4)$$

At start of an iteration using as start values $\mathbf{x} = \mathbf{0}$ and $\boldsymbol{\lambda} = \mathbf{0}$, the internal forces are all zero, $\mathbf{T} = \mathbf{0}$, and hence $r = 1$. The iteration is continued until r drops below a prescribed tolerance.

The primary convergence criteria in \hat{U} is based on the size of the residuals. However, one can also define tolerances on $\Delta\mathbf{x}$.

If the boundary conditions are linear then $\mathbf{L}\mathbf{x} - \mathbf{c} = \mathbf{0}$ at any iteration step, and the corresponding term in the residual can be omitted. It is always tested internally in \hat{U} that this condition is indeed fulfilled at the end of the non-linear iteration procedure.

4.2 Line search

For increased robustness the NR method is combined with a line-search. We write

$$\begin{aligned} \mathbf{x}_{\text{new}} &= \mathbf{x}_{\text{old}} + \gamma \Delta\mathbf{x} \\ \boldsymbol{\lambda}_{\text{new}} &= \boldsymbol{\lambda}_{\text{old}} + \gamma \Delta\boldsymbol{\lambda}. \end{aligned}$$

A ‘full’ Newton step corresponds to $\gamma = 1$. If r , as given by Eq. (4.4), is not reduced in the full Newton step, or if the reduction in r is not considered to be sufficiently large, r is minimised as a function of γ using a line-search algorithm. Using (4.4) one finds that

$$\left. \frac{\partial r}{\partial \gamma} \right|_{\gamma=0} = -r(0)$$

Therefore from $r(0)$ not only the error at the start of the step is known but also the variation of the error with respect to the step γ .

Once the system (4.3) has been solved, the normalised residual square error r can be calculated for $\gamma = 1$. If the reduction in r at the full Newton-Raphson step, i.e. $\gamma = 1$ is not judged to be sufficiently large, for example if $r(1)/r(0) > 0.5$, a parabolic fit to $r(\gamma)$ can be constructed given $r(0)$, $r(1)$, and the slope at $\gamma = 0$. (The cost of calculating r is small as it can be done without having to solve (4.3) again.)

Chapter 5

Inverse modelling

We have a forward model

$$F(u(p), p) = 0$$

where p are model parameters and u the state variable. We consider the problem of minimising an objective function J with respect to p . Typically the objective function J can be thought of as a sum of two terms

$$J(u, p) = I(u) + R(p)$$

where I is a misfit term and R a regularisation term.

When inverting for the (distributed) model parameter p we refer to it as the control variable to distinguish it from any other model parameters.

5.1 Objective functions

The data misfit is the distance between model output and measurements and it could, for example, be measured as

$$I(f) = \frac{1}{2\mathcal{A}} \|f\|^2 = \frac{1}{2\mathcal{A}} \iint f(x) \gamma(x, x') f(x') dx dx' \quad (5.1)$$

where $\gamma(x, x')$ is the covariance kernel and \mathcal{A} is the domain area. Defined in this manner, I is dimensionless. In the particular case of uncorrelated fields $\gamma(x, x') = c \delta(x - x')$

$$I = \frac{1}{2} \int (f(x)/e(x))^2 dx \quad (5.2)$$

where $e(x) = 1/\sqrt{c}$ are the data errors.

If \tilde{u} denotes estimates of u then a typical misfit term might be on the form

$$I = \frac{1}{2\mathcal{A}} \|u - \tilde{u}\|^2 = \frac{1}{2\mathcal{A}} \int ((u - \tilde{u})/e_u)^2 dA$$

where e_u are measurement errors.

Table 5.1: Notation used in inverse modelling

$F(u(p), p) = 0$	forward model
p	control variables
p_{prior}	a priori estimates of model parameters
u_{meas}	estimates of the state variable (measurements)
J	objective function ($J = I + R$)
R	regularisation term
I	misfit term
u	state variable
K	covariance matrix

In Bayesian context the regularisation term has the same form as $I(f)$ and is a measure of the distance between the system state and the a prior. In a discrete form the misfit term could, for example, be written as

$$R = (\mathbf{p} - \hat{\mathbf{p}})^T \mathbf{K}^{-1} (\mathbf{p} - \hat{\mathbf{p}})$$

where \mathbf{K} is a covariance matrix, \mathbf{p} the model parameters, and $\hat{\mathbf{p}}$ the a prior estimates of those model parameters. Apart from often having only a very limited knowledge of the covariance matrix \mathbf{K} , problems with this formulation can, for example, arise if the inverse of \mathbf{K} is not sparse. Practical reasons often influence the form of regularisation term. A pragmatic approach is to select a differential operator having a sparse representation. Preferable such a sparse operator is also the inverse of a ‘reasonable’ covariance matrix.

As a regularisation term we consider

$$R(f) = \|Lf\|^2$$

where L is a differential operator related to the inverse of the a prior covariance for p . For example using the Helmholtz equation leads to

$$R = \frac{\gamma_1^2}{2\mathcal{A}} \int \left((\nabla(p - \hat{p}))^2 + \gamma_2^2 (p - \hat{p})^2 \right) dA \quad (5.3)$$

where p are the model parameters for which we are inverting for (system state, model parameters) and \hat{p} the prior. The corresponding covariance kernel is the real part of

$$g(r) = \frac{i}{2\pi\gamma_1^2} H_0^{(1)}(i\gamma_2|r|) \quad (5.4)$$

where $H^{(1)}$ is a Hankel function, which is a monotonically decreasing function. For R to be dimensionless

$$\begin{aligned} [\gamma_1^2 \gamma_2^2] &= [p]^{-2} \\ [\gamma_1^2] &= [p]^{-2} [l] \end{aligned}$$

where l denotes length, so for example

$$\begin{aligned} [\gamma_1] &= [l] [p]^{-1} \\ [\gamma_2] &= [l]^{-1} \end{aligned}$$

5.2 Misfit functions in \mathcal{U}_a

Currently the misfit function I has the form

$$I = I_u + I_v$$

or

$$\begin{aligned} I &= \frac{1}{2\mathcal{A}} \int ((u - u_{\text{meas}})/u_{\text{error}})^2 dA \\ &\quad + \frac{1}{2\mathcal{A}} \int ((v - v_{\text{meas}})/v_{\text{error}})^2 dA \end{aligned}$$

where u and v are the horizontal velocity components, and

$$\mathcal{A} = \int dA$$

is the total area.

5.2.1 ...not yet, but soon, was there, then disappeared, may come again...

Currently the rate-of-elevation change is not included in the cost function. This may soon be added, in which case

$$I = I_u + I_v + I_{\dot{h}}$$

or

$$\begin{aligned} I = & \frac{1}{2\mathcal{A}} \int ((u - u_{\text{meas}})/u_{\text{error}})^2 dA \\ & + \frac{1}{2\mathcal{A}} \int ((v - v_{\text{meas}})/v_{\text{error}})^2 dA \\ & + \frac{1}{2\mathcal{A}} \int ((\dot{h} - \dot{h}_{\text{meas}})/\dot{h}_{\text{error}})^2 dA \end{aligned}$$

where u and v are the horizontal velocity components, and \dot{h} is the rate of thickness change.

The rate of elevation change is calculated as

$$\dot{h} = -(a - \partial_x(uh) - \partial_y(vh))$$

This will be solved in diagnostic mode, and in the adjoint method the gradient of the cost function with respect to the state variable \mathbf{v} acquires an additional term:

$$2I = \|u - \tilde{u}\|^2 + \|\dot{h}_d - (a - \partial_x(uh))\|^2$$

or

$$I_{\dot{h}} = \frac{1}{2\mathcal{A}} \int (a - \partial_x(uh) - \partial_y(vh) - \dot{h}_{\text{meas}})^2 dx dy$$

and

$$\delta_u I_{\dot{h}} = \frac{1}{\mathcal{A}} \int (a - \partial_x(uh) - \partial_y(vh) - \dot{h}_{\text{meas}}) \partial_x(h \delta u) dx dy$$

5.3 Regularisation in $\dot{U}a$

In $\dot{U}a$ the regularisation can be done either using a Bayesian or Tikhonov regularisation.

Using the Bayesian motivated approach the regularisation term has the form

$$R = (\mathbf{p} - \hat{\mathbf{p}})^T \mathbf{K}_{pp}^{-1} (\mathbf{p} - \hat{\mathbf{p}}) \quad (5.5)$$

where \mathbf{K} is the a priori covariance. The Bayesian approach has a clear statistical interpretation, but requires an inversion of the covariance matrix \mathbf{K} which can be impractical for large problems.

Alternatively, one can use Tikhonov regularisation where

$$R = \frac{1}{2\mathcal{A}} \int \left(\gamma_s^2 (\nabla(p - \hat{p}))^2 + \gamma_a^2 (p - \hat{p})^2 \right) dA \quad (5.6)$$

with the s and a subscripts being mnemonics for slope and amplitude, respectively.¹

The units of γ_a and γ_s are

$$\begin{aligned} [\gamma_a] &= \frac{1}{[p]} \\ [\gamma_s] &= \frac{[l]}{[p]} \end{aligned}$$

¹If we think of (5.6) in terms of (5.3) and (5.4) then

$$\begin{aligned} \gamma_s &= \gamma_1 \\ \gamma_a &= \gamma_1 \gamma_2 \end{aligned}$$

The inversion can be done directly with respect to the variable p , or with respect to the logarithm of the variable, i.e. $\log_{10} p$. If done with respect to the logarithm of p , the Tikhonov regularisation term has the form

$$\begin{aligned} R &= \frac{1}{2\mathcal{A}} \int \left(\gamma_s^2 (\nabla (\log_{10}(p) - \log_{10}(\hat{p})))^2 + \gamma_a^2 (\log_{10}(p) - \log_{10}(\hat{p}))^2 \right) dA \\ &= \frac{1}{2\mathcal{A}} \int \left(\gamma_s^2 (\nabla \log_{10}(p/\hat{p}))^2 + \gamma_a^2 \log_{10}^2(p/\hat{p}) \right) dA \end{aligned} \quad (5.7)$$

Now γ_a is dimensionless and the dimension of γ_s is length, i.e.

$$\begin{aligned} [\gamma_a] &= [] \\ [\gamma_s] &= [l] \end{aligned}$$

The Tikhonov is a commonly used and a practical approach. As shown in section (5.5) the Tikhonov approach leads to

$$R = \frac{1}{2} (\mathbf{p} - \hat{\mathbf{p}})^T (\gamma_a^2 \mathbf{M} + \gamma_s^2 (\mathbf{D}_x + \mathbf{D}_y)) (\mathbf{p} - \hat{\mathbf{p}}) \quad (5.8)$$

$$= \frac{1}{2} (\mathbf{p} - \hat{\mathbf{p}})^T \mathbf{K}_{pp}^{-1} (\mathbf{p} - \hat{\mathbf{p}}) \quad (5.9)$$

where

$$\mathbf{K}_{pp}^{-1} = \frac{1}{2} (\gamma_a^2 \mathbf{M} + \gamma_s^2 (\mathbf{D}_x + \mathbf{D}_y)) \quad (5.10)$$

and \mathbf{M} is the mass matrix and \mathbf{D}_x and \mathbf{D}_y the stiffness matrices. Hence, with the Tikhonov approach we are calculating directly the inverse of the covariance matrix \mathbf{K}_{pp} given by (5.10) and this inverse will be sparse.

5.4 Calculation of the gradient of the objective function with the adjoint method

The adjoint method is a simple trick to speed up the calculation of gradients of the objective function with respect to the control variables.

Assume I want to solve the minimisation problem

$$\min_{p \in P, u \in U} J(u(p))$$

subject to forward model

$$F(u(p), p) = 0.$$

The objective function is

$$J : U \times P \rightarrow \mathbb{R}$$

and the forward model, i.e. the state equation

$$F : U \times P \rightarrow \mathbb{W}$$

the model control parameter space P (also referred to as control space), the state space U and the image space W are Banach spaces.

We want to determine the sensitivity of cost function J with respect to the (distributed) control parameter p .

We define the Lagrange function

$$\mathcal{L} = U \times V \times W^* \rightarrow \mathbb{R}$$

as

$$\mathcal{L}(u(p), p, \lambda) = J(u(p), p) + \langle \lambda \mid F(u(p), p) \rangle_{W^*, W} \quad (5.11)$$

The adjoint variable λ is in the dual of the image space \mathbb{W} .

Equation (5.11) provides no constraints on the adjoint variable λ because the second term is always equal to zero for any value of p . Therefore

$$\mathcal{L}(u(p), p, \lambda) = J(u(p), p)$$

and

$$D\mathcal{L}(p)[\phi] = DJ(p)[\phi] = \langle \nabla_p \mathcal{L} \mid \phi \rangle$$

Also note that

$$d_p F = \partial_p F + \partial_u F d_p u = 0.$$

Introducing

$$j(u) = J(u(p), p) = \mathcal{L}(u(p), p, \lambda)$$

we have

$$j'(p) = (\partial u / \partial p)^* \partial \mathcal{L}(u(p), p, \lambda) / \partial u + \partial \mathcal{L}(u(p), p, \lambda) / \partial p \quad (5.12)$$

Now we chose λ such that

$$\partial \mathcal{L}(u(p), p, \lambda) / \partial u = 0$$

Hence λ must be a solution to

$$\partial J(u(p), p) / \partial u + (\partial F / \partial u)^* \lambda = 0$$

and then the direction derivative $j'(p)$ is

$$j'(p) = \partial J(u(p), p) / \partial p + (\partial F / \partial p)^* \lambda \quad (5.13)$$

Eq. (5.12) can also be written as

$$\langle j'(p), \phi \rangle_{P^*, P} = \langle \partial_u \mathcal{L}, \partial_p u \phi \rangle_{U^*, U} + \langle \partial_p \mathcal{L}, \phi \rangle_{P^*, P}$$

and the directional derivative is then

$$\langle \partial_u \mathcal{L}, \partial_p u \phi \rangle_{U^*, U} = 0$$

for all ϕ .

Another approach: Differentiating Eq. (5.11) with respect to the control variable p we obtain

$$\begin{aligned} DJ(p)[\phi] &= D\mathcal{L}(p)[\phi] = \langle \partial_u J \mid d_p u \phi \rangle + \langle \partial_p J, \phi \rangle + \langle \lambda \mid \partial_u F d_p u \phi \rangle + \langle \lambda \mid \partial_p F \phi \rangle + \langle d_p \lambda \mid F \rangle \\ &= \langle \partial_u J \mid d_p u \phi \rangle + \langle (\partial_u F)^* \lambda \mid d_p u \phi \rangle + \langle \lambda \mid \partial_p F \phi \rangle + \langle \partial_p J, \phi \rangle \\ &= \langle \partial_u J + (\partial_u F)^* \lambda \mid d_p u \phi \rangle + \langle \lambda \mid \partial_p F \phi \rangle + \langle \partial_p J, \phi \rangle \end{aligned}$$

We now use the freedom that λ has not been specified and now determine λ by specifying

$$\langle \partial_u J + (\partial_u F)^* \lambda \mid \phi \rangle = 0$$

and therefore

$$DJ(p)[\phi] = \langle (\partial_p F)^* \lambda + \partial_p J \mid \phi \rangle \quad (5.14)$$

which is identical to (5.13).

This now gives us three-step method for calculating the directional gradient of object function J with respect to p :

1. Solve the state equation, i.e. the forward problem

$$\langle F(u(p), p) \mid \phi \rangle_{W^*, W} = 0$$

for the state variable u .

This, in general, is a non-linear problem that can be solved iteratively using the Newton-Raphson system, i.e.

$$\langle \partial_u F \Delta u \mid \phi \rangle = - \langle F(u) \mid \phi \rangle$$

and can be written in discrete form as

$$\mathbf{K} \Delta \mathbf{u} = \mathbf{b}$$

where

$$[\mathbf{K}]_{pq} = \langle \partial_{u_p} F, \phi_q \rangle$$

and

$$[\mathbf{b}]_q = - \langle F(u), \phi_q \rangle$$

2. Solve the adjoint problem for $\langle \partial_u J + (\partial_u F)^* \lambda \mid \phi \rangle = 0$ for $\lambda \in W^*$, i.e.

$$\langle (\partial_u F)^* \lambda \mid \phi \rangle_{U^*, U} = - \langle \partial_u J \mid \phi \rangle_{U^*, U}$$

for the adjoint variable λ . If the forward tangential model $(\partial_u F)$ is self adjoint, this involves solving

$$K\lambda = b$$

3. Calculate the directional derivative of J as

$$\langle j'(p), \phi \rangle_{P^*, P} = \langle (\partial_p F)^* \lambda + \partial_p J \mid \phi \rangle_{P^*, P}$$

$$DJ(p)[\phi] = \langle (\partial_p F)^* \lambda + \partial_p J \mid \phi \rangle_{P^*, P}$$

In discrete form the derivative can be evaluated as

$$j'(p) = P\lambda + Q$$

where

$$[P]_{rs} = \langle \partial_{p_r} F, \phi_s \rangle$$

and

$$[Q]_r = \langle \partial_p J, \phi_r \rangle$$

However, $\langle (\partial_p F)^* \lambda \rangle$ can usually be evaluated directly within the assembly loop without the need of ever forming the matrix P . Furthermore, the forward model is solved in a weak form and this often involves some manipulations of the $\langle \lambda, F \rangle$ term in Eq. (5.11).

Usually only the regularisation term is an explicit function of the control variable, i.e.

$$J(F(p, u(p)), p) = I(F(p, u(p))) + R(p)$$

and therefore

$$\partial_p J = \partial_p R$$

The adjoint variable λ is the gradient of the objective function with respect to the state variable u

$$\lambda = \nabla_u J$$

In the adjoint method we need to calculate a number of derivatives. These are:

1. The derivative of the forward model with respect to the state variable, i.e. $\partial_u F$
2. The derivative of the objective function with respect to the state variable, i.e. $\partial_u J$
3. The derivative of the forward model with respect to the control variable, i.e. $\partial_p F$.

5.5 Evaluating objective functions and their directional derivatives

If ϕ_i are the basis functions then

$$[M]_{pq} = \langle \phi_p, \phi_q \rangle$$

is the mass matrix (also known as the Gramian matrix), and

$$\begin{aligned} [D_x]_{pq} &= \langle \nabla_x \phi_p, \nabla_x \phi_q \rangle \\ [D_y]_{pq} &= \langle \nabla_y \phi_p, \nabla_y \phi_q \rangle \end{aligned}$$

the stiffness matrices.

For

$$I = \frac{1}{2} \|f\|^2 = \frac{1}{2} \int f(x, y) f(x, y) \, dx dy \quad (5.15)$$

and

$$f(x, y) = f_i \phi_i(x, y)$$

we find

$$\begin{aligned} I &= \frac{1}{2} \|f\|^2 \\ &= \frac{1}{2} \langle f, f \rangle \\ &= \frac{1}{2} \langle f_p \phi_p, f_q \phi_q \rangle \\ &= \frac{1}{2} f_p \langle \phi_p, \phi_q \rangle f_q \\ &= \frac{1}{2} f_p M_{pq} f_q \end{aligned}$$

or

$$I = \frac{1}{2} \mathbf{f} \mathbf{M} \mathbf{f}$$

where

$$[\mathbf{f}]_i = f_i$$

The directional derivative is

$$\begin{aligned} DI(f, \phi_q) &= \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} I(f + \epsilon \phi_q) \\ &= \frac{1}{2} \frac{d}{d\epsilon} \langle f + \epsilon \phi_q, f + \epsilon \phi_q \rangle \\ &= \langle f, \phi_q \rangle \\ &= \langle f_p \phi_p, \phi_q \rangle \\ &= \langle \phi_q, \phi_p \rangle f_p \\ &= M_{qp} f_p \\ &= \mathbf{M} \mathbf{f} \end{aligned}$$

or

$$DI(f, \phi_q) = [\mathbf{M} \mathbf{f}]_q$$

The p component of the directional derivative represents the (linear) rate-of-change in I as the value of f is perturbed by $\epsilon \phi_p$.

We can also write

$$DI(f, \phi_q) = \langle f_p \phi_p, \phi_q \rangle$$

and therefore by the definition of a gradient as

$$\frac{d}{d\epsilon} J(f + \epsilon \delta f)|_{\epsilon=0} = \langle \text{grad} J(f), \delta f \rangle$$

the gradient of I in Eq. (5.15) is f (as it of course should be).

Similarly if

$$I = \frac{1}{2} \langle \nabla f, \nabla f \rangle = \frac{1}{2} (\langle \partial_x f, \partial_x f \rangle + \langle \partial_y f, \partial_y f \rangle)$$

then

$$\begin{aligned} I_x &= \frac{1}{2} \langle \partial_x f, \partial_x f \rangle \\ &= \frac{1}{2} \langle f_p \partial_x \phi_p, f_q \partial_x \phi_q \rangle \\ &= \frac{1}{2} f_p \langle \partial_x \phi_p, \partial_x \phi_q \rangle f_q \\ &= \frac{1}{2} \mathbf{f} \mathbf{D}_x \mathbf{f} \end{aligned}$$

and

$$\begin{aligned}
DI_x(f, \phi_p) &= \frac{1}{2} \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} \langle \partial_x(f + \epsilon \phi_p), \partial_x(f + \epsilon \phi_p) \rangle \\
&= \langle \partial_x f, \partial_x \phi_p \rangle \\
&= \langle f_q \partial_x \phi_q, \partial_x \phi_p \rangle \\
&= \langle \partial_x \phi_p, \partial_x \phi_q \rangle f_q \\
&= \mathbf{D}_x \mathbf{f}
\end{aligned}$$

or

$$DI_x(f, \phi_p) = \mathbf{D}_x \mathbf{f}$$

Summarising, if we have a regularisation term on the form

$$R = \frac{1}{2} \|f\|_{L^2(\Omega)}^2 + \frac{1}{2} \|\nabla f\|_{L^2(\Omega)}^2$$

it can be evaluated knowing the mass and the stiffness matrices as

$$\begin{aligned}
R &= \frac{1}{2} \|f\|_{L^2(\Omega)}^2 + \frac{1}{2} \|\nabla f\|_{L^2(\Omega)}^2 \\
&= \frac{1}{2} \mathbf{f}^T \mathbf{M} \mathbf{f} + \frac{1}{2} \mathbf{f}^T (\mathbf{D}_x + \mathbf{D}_y) \mathbf{f}
\end{aligned}$$

and direction derivative is

$$\frac{d}{d\epsilon} I(f + \epsilon \phi_p) = [\mathbf{M} \mathbf{f} + (\mathbf{D}_x + \mathbf{D}_y) \mathbf{f}]_p$$

And the regularisation term (5.3) can be evaluated similarly as

$$\begin{aligned}
R &= \frac{\gamma_1^2}{2\mathcal{A}} \int \left((\nabla(p - \hat{p}))^2 + \gamma_2^2 (p - \hat{p})^2 \right) dA \\
&= \frac{\gamma_1^2}{2\mathcal{A}} \left((\mathbf{p} - \hat{\mathbf{p}})^T \mathbf{D} (\mathbf{p} - \hat{\mathbf{p}}) + \gamma_2^2 (\mathbf{p} - \hat{\mathbf{p}})^T \mathbf{M} (\mathbf{p} - \hat{\mathbf{p}}) \right) \\
&= \frac{\gamma_1^2}{2\mathcal{A}} \left((\mathbf{p} - \hat{\mathbf{p}})^T (\mathbf{D} + \gamma_2^2 \mathbf{M}) (\mathbf{p} - \hat{\mathbf{p}}) \right)
\end{aligned}$$

and the directional derivative is

$$d_{\mathbf{p}} R = \frac{\gamma_1^2}{\mathcal{A}} (\mathbf{D} + \gamma_2^2 \mathbf{M}) (\mathbf{p} - \hat{\mathbf{p}})$$

5.6 Gradients of objective functions with respect to model parameters

5.6.1 Gradient calculation in 1HD with respect to C

As an example we consider the calculation of the gradient of the objective function J with respect to slipperiness.

$$\mathbf{t}_b = \mathcal{H}(h - h_f) C^{-1/m} |\mathbf{v}_b|^{1/m-1} \mathbf{v}_b$$

We need to evaluate

$$\begin{aligned}
DJ(C)[\phi] &= \langle (\partial_C F)^* \lambda + \partial_C J, \phi \rangle \\
&= \langle (\partial_C \mathbf{t}_b)^* \lambda + \partial_C J, \phi \rangle
\end{aligned}$$

giving

$$DJ(C)[\phi] = \langle \frac{1}{m} \mathcal{H}(h - h_f) C^{-1/m-1} |\mathbf{v}_b|^{1/m-1} \mathbf{v}_b \lambda, \phi \rangle + \langle \partial_C J, \phi \rangle$$

In the above listed expression one needs to form a sum between \mathbf{v}_b and λ for each value of C . The adjoint variable λ is a solution of the adjoint equation and can be considered as a vector variables with x and y components similarly to \mathbf{v} .

5.6.2 Gradient calculation in 1HD with respect to A

The directional derivative can be calculated (see Eq. 5.13) as

$$j'(A) = \partial J(u(p), p) / \partial A + (\partial F / \partial A)^* \lambda \quad (5.16)$$

Focusing on the second term

$$\begin{aligned} (\partial_A F)^* \lambda &= \langle \partial_A \left(2\partial_x (A^{-1/n} h |\partial_x u|^{(1-n)/n} \partial_x u) - t_{bx} - \frac{1}{2} g \partial_x (\rho h^2 - \rho_o d^2) + g \mathcal{H}(h - h_f)(\rho h - \rho_o H^+) \partial_x B \right), \lambda \rangle \\ &= - \langle 2\partial_A \left(A^{-1/n} h |\partial_x u|^{(1-n)/n} \partial_x u \right), \partial_x \lambda \rangle \\ &= - \langle \frac{2}{n} A^{-1/n-1} h |\partial_x u|^{(1-n)/n} \partial_x u \delta A, \partial_x \lambda \rangle \end{aligned}$$

where we have omitted writing the boundary term assuming that λ is set to zero along the boundary (or periodic boundary conditions applied for periodic domains.)

If both A and λ are expanded in the same way. i.e.

$$\begin{aligned} A &= A_P \phi_p(x, y) \\ \lambda &= \lambda_q \phi_q(x, y) \end{aligned}$$

then

$$\begin{aligned} (\partial F p A)^* \lambda &= \langle \frac{2}{n} A^{-1/n-1} h |\partial_x u|^{(1-n)/n} \partial_x u \delta A, \partial_x \lambda \rangle \\ &= \langle \frac{2}{n} A^{-1/n-1} h |\partial_x u|^{(1-n)/n} \partial_x u \phi_p, \lambda_q \partial_x \phi_q \rangle \\ &= \langle \frac{2}{n} A^{-1/n-1} h |\partial_x u|^{(1-n)/n} \partial_x u \phi_p, \partial_x \phi_q \rangle \lambda_q \end{aligned}$$

or

$$(\partial F / \partial A)^* \lambda = \mathbf{K} \lambda$$

where

$$\mathbf{K} = (\partial F / \partial A)^*$$

is

$$K_{pq} = \langle \frac{2}{n} A^{-1/n-1} h |\partial_x u|^{(1-n)/n} \partial_x u \phi_p, \lambda_q \partial_x \phi_q \rangle \lambda_q$$

however it is more efficient to calculate this matrix-vector product directly without ever forming the matrix as

$$\mathbf{K} \lambda = \langle \frac{2}{n} A^{-1/n-1} h |\partial_x u|^{(1-n)/n} \partial_x u \phi_p, \partial_x \lambda \rangle$$

5.7 Inverting for log p

To avoid issues with a strictly positive parameter p becoming negative in the course of the inversion we can make a change of variables writing

$$p = 10^\gamma = e^{\gamma \ln(10)}$$

or

$$\log_{10} p = \gamma$$

We have

$$\begin{aligned} \frac{\partial J}{\partial \gamma} &= \frac{\partial J}{\partial p} \frac{\partial C}{\partial \gamma} \\ &= \frac{\partial J}{\partial p} \ln(10) 10^\gamma \\ &= \ln(10) p \frac{\partial J}{\partial p} \end{aligned}$$

showing that the change of variables causes a rescaling of the gradient,

5.8 The form of the adjoint equations for Bayesian approach using Gaussian statistics

We anticipate using a Bayesian approach assuming Gaussian statistics and therefore that the cost function might be on the form

$$\begin{aligned} \min_p I(u(p)) &= \langle u - \hat{u} \mid K_u^{-1} \mid u - \hat{u} \rangle + \langle p - \hat{p} \mid K_P^{-1} \mid p - \hat{p} \rangle \\ &= \langle K_u^{-T/2}(u - \hat{u}) \mid K_u^{-1/2}(u - \hat{u}) \rangle + \langle K_P^{-T/2}(p - \hat{p}) \mid K_P^{-1/2}(p - \hat{p}) \rangle \end{aligned}$$

where K is a covariance matrix (and therefore positive definite).

Therefore

$$\begin{aligned} d_p I &= d_p \mathcal{L} = \langle K_u^{-1/2}(u - \hat{u}) \mid d_p u \rangle + \langle \lambda \mid \partial_u F d_p u + \partial_p F \rangle + \langle \partial_p \lambda \mid F \rangle \\ &= \langle K_u^{-1/2}(u - \hat{u}) \mid d_p u \rangle + \langle \lambda \mid \partial_u F d_p u + \partial_p F \rangle \\ &= \langle K_u^{-1/2}(u - \hat{u}) + \lambda(\partial_u F)^* \mid d_p u \rangle + \langle \lambda \mid \partial_p F \rangle \end{aligned}$$

where we have omitted the $\langle p - \hat{p} \mid K_P^{-1} \mid p - \hat{p} \rangle$ term for the time being. We now use the flexibility of λ not having been specified and require that

$$\langle K_u^{-1/2}(u - \hat{u}) + \lambda(\partial_u F)^* \mid \delta p \rangle = 0$$

and therefore

$$d_p I = \langle \lambda \mid \partial_p F \rangle$$

For a cost function on the form

$$\min_p I(u(p)) = \langle u - \hat{u} \mid K_u^{-1} \mid u - \hat{u} \rangle + \langle p - \hat{p} \mid K_P^{-1} \mid p - \hat{p} \rangle$$

we would arrive at

$$d_p I = \langle \lambda \mid \partial_p F \rangle + \langle K_P^{-1/2}(p - \hat{p}) \mid \delta p \rangle$$

One might as why we don't just calculate the cost gradient as

$$d_p = \langle K_u^{-1/2}(u - \hat{u}) \mid d_p u \rangle$$

But this would require calculating $d_p u$ which is a pain in the neck.

5.9 Adjoint equations (Bayesian case with constraints on vertical velocity)

We want to minimise a cost function \tilde{J} on the form

$$\tilde{J}(u, v, w, p) = I(u, v, w) + F(p)$$

where I is a data discrepancy functional, and R a regularisation term

As a misfit function we use

$$I = I_u + I_v + I_o,$$

where each term has the form

$$I_u = \langle C_{uu}^{-1/2}u - \hat{u}, C_{uu}^{-1/2}u - \hat{u} \rangle$$

with $C_{uu}^{-1/2}$ being an error covariance matrix.

The regularisation term has the form

$$R = \langle C_{pp}^{-1/2}p, C_{pp}^{-1/2}p \rangle$$

We minimise \tilde{J} subject to the conditions

$$F(u(p), v(p), p) = 0$$

and

$$w_s = f(u, v, h, b)$$

where r are the diagnostic equations, f is a function giving the vertical surface velocity w_o as a function of the variables of the diagnostic equations, and where p stands for some control variable (distributed model parameter) such as the basal slipperiness C or the rate factor A . We therefore consider the extended cost function

$$J(u, v, w, \lambda, \mu, p) = I(u, v, w) + F(p) + \langle \lambda \mid F(u(p), v(p), p) \rangle + \langle \mu \mid w - f(u, v, h, b) \rangle$$

where λ and μ are Lagrange multipliers.

The directional derivative of J with respect to λ in the direction of $\delta\lambda$ is defined as

$$\frac{d}{d\epsilon} J(\lambda + \epsilon \delta\lambda) \big|_{\epsilon=0}$$

and is denoted by $\delta J(\lambda, \delta\lambda)$

The directional derivatives of J are

$$\begin{aligned} \delta J(\lambda, \delta\lambda) &= \delta_\lambda J = \langle \delta\lambda, F(u, v, p) \rangle \\ J(\mu, \delta\mu) &= \langle \delta\mu, w - f(u, v, h, b) \rangle \\ J(u, \delta u) &= \langle C_{uu}^{-1/2}(u - \hat{u}, C^{-1/2}\delta u) \rangle + \langle \lambda, \nabla_u r \delta u \rangle - \langle \mu, \nabla_u f \delta u \rangle \end{aligned}$$

5.10 Prognostic equations are formally self-adjoint

Define the inner product

$$r = \langle f_x, \lambda \rangle + \langle f_y, \mu \rangle$$

$$\begin{aligned} f_x &= \partial_x(h\eta(4\partial_x u + 2\partial_y v)) + \partial_y(h\eta(\partial_y u + \partial_x v)) - \mathcal{H}(h - h_f)t_{bx} \\ &\quad - \frac{1}{2}g\partial_x(\rho h^2 - \rho_o d^2) + g\mathcal{H}(h - h_f)(\rho h - \rho_o H^+)\partial_x B \end{aligned} \quad (5.17)$$

$$\begin{aligned} f_y &= \partial_y(h\eta(4\partial_y v + 2\partial_x u)) + \partial_x(h\eta(\partial_x v + \partial_y u)) - \mathcal{H}(h - h_f)t_{by} \\ &\quad - \frac{1}{2}g\partial_y(\rho h^2 - \rho_o d^2) + g\mathcal{H}(h - h_f)(\rho h - \rho_o H^+)\partial_y B \end{aligned} \quad (5.18)$$

or

$$\begin{aligned} r &= \iint \left\{ (\partial_x(h\eta(4\partial_x u + 2\partial_y v)) + \partial_y(h\eta(\partial_y u + \partial_x v))) - \mathcal{H}(h - h_f)t_{bx} \right. \\ &\quad \left. - \frac{1}{2}g\partial_x(\rho h^2 - \rho_o d^2) + g\mathcal{H}(h - h_f)(\rho h - \rho_o H^+)\partial_x B \right\} \lambda \, dx \, dy \\ &\quad + \iint \left\{ (\partial_y(h\eta(4\partial_y v + 2\partial_x u)) + \partial_x(h\eta(\partial_x v + \partial_y u))) - \mathcal{H}(h - h_f)t_{by} \right. \\ &\quad \left. - \frac{1}{2}g\partial_y(\rho h^2 - \rho_o d^2) + g\mathcal{H}(h - h_f)(\rho h - \rho_o H^+)\partial_y B \right\} \mu \, dx \, dy \end{aligned}$$

The use of Green's theorem gives

$$\begin{aligned} r &= - \iint_{\Omega} \left\{ h\eta(4\partial_x u + 2\partial_y v)\partial_x \lambda + h\eta(\partial_y u + \partial_x v)\partial_y \lambda + \mathcal{H}(h - h_f)\beta^2 u \lambda \right. \\ &\quad \left. - \frac{1}{2}g(\rho h^2 - \rho_o d^2)\partial_x \lambda + \lambda g\mathcal{H}(h - h_f)(\rho h - \rho_o H^+)\partial_x B \right\} dx \, dy \\ &\quad + \oint_{\Gamma} (h\eta(4\partial_x u + 2\partial_y v)\lambda n_x + h\eta(\partial_y u + \partial_x v)\lambda n_y - \frac{1}{2}g(\rho h^2 - \rho_o d^2)\lambda n_x) \, d\Gamma \\ &\quad - \iint_{\Omega} \left\{ h\eta(4\partial_y v + 2\partial_x u)\partial_y \mu + h\eta(\partial_x v + \partial_y u)\partial_x \mu + \mathcal{H}(h - h_f)\beta^2 v \right. \\ &\quad \left. - \frac{1}{2}g(\rho h^2 - \rho_o d^2)\partial_y \mu + \mu g\mathcal{H}(h - h_f)(\rho h - \rho_o H^+)\partial_y B \right\} dx \, dy \\ &\quad + \oint_{\Gamma} (h\eta(4\partial_y v + 2\partial_x u)\mu n_y + h\eta(\partial_x v + \partial_y u)\mu n_x - \frac{1}{2}g(\rho h^2 - \rho_o d^2)\mu n_y) \, d\Gamma \end{aligned}$$

and a second use of Green's theorem gives after some rearrangements

$$\begin{aligned}
r = & \iint_{\Omega} \{ \partial_x (h\eta(4\partial_x\lambda + 2\partial_y\mu)) u + \partial_y (h\eta(\partial_y\lambda + \partial_x\mu)) u - \mathcal{H}(h - h_f)\beta^2 u \lambda \\
& + \frac{1}{2}g(\rho h^2 - \rho_o d^2)\partial_x\lambda - \lambda g \mathcal{H}(h - h_f)(\rho h - \rho_o H^+)\partial_x B \} dx dy \\
& + \oint_{\Gamma} (h\eta(4\partial_x u + 2\partial_y v) \lambda n_x + h\eta(\partial_y u + \partial_x v) \lambda n_y - \frac{1}{2}g(\rho h^2 - \rho_o d^2) \lambda n_x) d\Gamma \\
& + \iint_{\Omega} \{ \partial_y (h\eta(4\partial_x\mu + 2\partial_y\lambda)) v + \partial_x (h\eta(\partial_x\mu + \partial_y\lambda)) v - \mathcal{H}(h - h_f)\beta^2 v \lambda \\
& + \frac{1}{2}cdg(\rho h^2 - \rho_o d^2)\partial_y\mu - \mu g \mathcal{H}(h - h_f)(\rho h - \rho_o H^+)\partial_x B \} dx dy \\
& + \oint_{\Gamma} (h\eta(4\partial_y v + 2\partial_x u) \mu n_y + h\eta(\partial_x v + \partial_y u) \mu n_x - \frac{1}{2}g(\rho h^2 - \rho_o d^2) \mu n_y) d\Gamma \\
& - \oint_{\Omega} (u h\eta(4\partial_x\lambda + 2\partial_y\mu)n_x + u h\eta(\partial_y\lambda + \partial_x\mu)n_y) d\Gamma \\
& - \oint_{\Omega} (v h\eta(4\partial_y\mu + 2\partial_x\lambda)n_y + v h\eta(\partial_x\mu + \partial_y\lambda)n_x) d\Gamma
\end{aligned}$$

The adjoint approach is based on the use of the adjoint of the linearised/tangent forward model around the converged solution of non-linear forward model. The non-linear forward model is

$$F(\mathbf{u}) = \mathbf{0}$$

and the tangent model is the directional derivative of the forward model in the direction $\delta\mathbf{u}$.

$$K = D\mathbf{F}(\mathbf{u})[\delta\mathbf{u}]$$

Often this is simply be written as

$$K = \frac{\partial \mathbf{F}}{\partial \mathbf{u}}$$

Define

$$< \mathbf{L} \delta U, \Lambda > = < U, \mathbf{L} \Lambda >$$

where $U = (\delta u, \delta v)^T$ and \mathbf{L} is the operator acting on U as given by the system (1.48) and (1.49).

$$\begin{aligned}
< \mathbf{L} \delta u, \Lambda > = & \iint_{\Omega} \{ (\partial_x (h\eta(4\partial_x\delta u + 2\partial_y\delta v)) + \partial_y (h\eta(\partial_y\delta u + \partial_x\delta v))) \lambda \\
& + (\partial_y (h\eta(4\partial_y\delta v + 2\partial_x\delta u)) + \partial_x (h\eta(\partial_x\delta v + \partial_y\delta u))) \mu \} dx dy \\
= & - \iint_{\Omega} \{ h\eta(4\partial_x\delta u + 2\partial_y\delta v)\partial_x\lambda + h\eta(\partial_y\delta u + \partial_x\delta v)\partial_y\lambda \\
& h\eta(4\partial_y\delta v + 2\partial_x\delta u)\partial_y\mu + h\eta(\partial_x\delta v + \partial_y\delta u)\partial_x\mu \} dx dy \\
& + \oint_{\Gamma} \{ h\eta(4\partial_x\delta u + 2\partial_y\delta v) \lambda n_x + h\eta(\partial_y\delta u + \partial_x\delta v) \lambda n_y \\
& + h\eta(4\partial_y\delta v + 2\partial_x\delta u) \mu n_y + h\eta(\partial_x\delta v + \partial_y\delta u) \mu n_x \} d\Gamma \\
= & \iint_{\Omega} \{ (\partial_x (h\eta(4\partial_x\lambda + 2\partial_y\mu)) + \partial_y (h\eta(\partial_y\lambda + \partial_x\mu))) \delta u \\
& (\partial_y (h\eta(4\partial_x\mu + 2\partial_y\lambda)) + \partial_x (h\eta(\partial_x\mu + \partial_y\lambda))) \delta v \} dx dy \\
& + \oint_{\Gamma} \{ (h\eta(4\partial_x\delta u + 2\partial_y\delta v) n_x + h\eta(\partial_y\delta u + \partial_x\delta v) n_y) \lambda \\
& + (h\eta(4\partial_y\delta v + 2\partial_x\delta u) n_y + h\eta(\partial_x\delta v + \partial_y\delta u) n_x) \mu \} d\Gamma \\
& - \oint_{\Gamma} (h\eta(4\partial_x\lambda + 2\partial_y\mu)n_x + h\eta(\partial_y\lambda + \partial_x\mu)n_y)\delta u d\Gamma \\
& - \oint_{\Gamma} (h\eta(4\partial_y\mu + 2\partial_x\lambda)n_y + h\eta(\partial_x\mu + \partial_y\lambda)n_x)\delta v d\Gamma
\end{aligned}$$

Once the forward model has been solved the velocity field fulfills given the BCs to a high degree of accuracy. The boundary conditions on the δ fields follow from above

5.11 Covariance kernels

$$F(f) = \int \int f(x) \kappa(x, x') f(x') dx dx'$$

where κ is the covariance kernel.

Multipole expansion. Isotropic, stationary, and translation invariance

$$\kappa(x, x') = \kappa(|x - x'|)$$

$$e^{-|x-x_i|^2/4T} = \sum_{n_1, n_2=0}^{\infty} \Theta_{n_1 n_2}(x - c) \dots$$

Chapter 6

Further technical implementation details

6.1 Only the (fully) floating condition as a natural boundary condition

Here I am ignoring possible gradients in density and the treatment of the boundary term only includes the fully floated case as a natural condition.

Note that

$$\begin{aligned}\rho gh \partial_x s &= \rho gh \partial_x s + \frac{1}{2} \varrho g \partial_x h^2 - \rho gh (1 - \rho/\rho_o) \partial_x h \\ &= \frac{1}{2} \varrho g \partial_x h^2 + \rho gh \partial_x (s - S - (1 - \rho/\rho_o) h)\end{aligned}$$

hence

$$\rho gh \partial_x s = \frac{1}{2} \varrho g \partial_x h^2 + \rho gh \partial_x s' \quad (6.1)$$

with

$$s' := s - S - (1 - \rho/\rho_o) h$$

and

$$\varrho = \rho(1 - \rho/\rho_o),$$

The field equations can therefore also be written as

$$\begin{aligned}\partial_x(4h\eta\partial_x u + 2h\eta\partial_y v) + \partial_y(h\eta(\partial_x v + \partial_y u)) - \beta^2 u &= \rho gh(\partial_x s' \cos \alpha - \sin \alpha) + \frac{1}{2} \varrho g \cos \alpha \partial_x h^2, \\ \partial_y(4h\eta\partial_y v + 2h\eta\partial_x u) + \partial_x(h\eta(\partial_y u + \partial_x v)) - \beta^2 v &= \rho gh\partial_y s' \cos \alpha + \frac{1}{2} \varrho g \cos \alpha \partial_y h^2,\end{aligned}$$

6.1.1 Remark

To see that the right-hands sides of (1.45) and (6.1) i.e.

$$\begin{aligned}\rho gh \partial_x s &= \frac{1}{2} \varrho g \partial_x h^2 + \rho gh \partial_x s' \\ &= \frac{1}{2} g \partial_x (\rho h^2 - \rho_o d^2) + g(\rho h - \rho_o d) \partial_x b\end{aligned}$$

are equal (ignoring spatial gradients in density) we consider the three cases:

1. Fully floating: In that case $s' = 0$ and $\rho h = \rho_o d$ and both sides are equal.
2. Fully grounded: We have $d = 0$

$$\begin{aligned}\frac{1}{2} g \partial_x (\rho h^2 - \rho_o d^2) + g(\rho h - \rho_o d) \partial_x b &= \frac{1}{2} g \rho \partial_x h^2 + g \rho h \partial_x b \\ &= \rho gh \partial_x s\end{aligned}$$

and

$$\begin{aligned}\frac{1}{2}\varrho g \partial_x h^2 + \rho g h \partial_x s' &= \frac{1}{2}\varrho g \partial_x h^2 + \rho g h \partial_x (s - S - (1 - \rho/\rho_o)h) \\ &= \rho g h \partial_x s\end{aligned}$$

3. Partly floating:

$$\begin{aligned}\frac{1}{2}g\partial_x(\rho h^2 - \rho_o d^2) + g(\rho h - \rho_o d)\partial_x b &= \rho g h \partial_x h - \rho_o g d \partial_x d + g(\rho h - \rho_o d)\partial_x b \\ &= \rho g h \partial_x s - \rho_o g d \partial_x d - \rho_o g d \partial_x b \\ &= \rho g h \partial_x s - \rho_o g d (\partial_x d + \partial_x b) \\ &= \rho g h \partial_x s - \rho_o g d (\partial_x S - \partial_x b + \partial_x b) \\ &= \rho g h \partial_x s\end{aligned}$$

6.1.2 FE formulation

x direction

$$\int_{\Omega} (\partial_x (4h\eta \partial_x u + 2h\eta \partial_y v) - \frac{1}{2}\varrho g \cos \alpha \partial_x h^2 + \partial_y (h\eta (\partial_x v + \partial_y u)) - t_{bx} - \rho g h (\partial_x s' \cos \alpha - \sin \alpha)) N \, dx \, dy = 0$$

with Weertman type Neumann BC on Γ_2

$$(4h\eta \partial_x u + 2h\eta \partial_y v) n_x + \eta h (\partial_x v + \partial_y u) n_y = \frac{1}{2}\rho(1 - \rho/\rho_o)gh^2 n_x$$

Green's theorem used to get rid of second derivatives gives

$$\begin{aligned}- \int_{\Omega} ((4h\eta \partial_x u + 2h\eta \partial_y v) \partial_x N - \frac{1}{2}\varrho g \cos \alpha h^2 \partial_x N + h\eta (\partial_x v + \partial_y u) \partial_y N) \, dx \, dy \\ - \int_{\Omega} (t_{bx} + \rho g h (\partial_x s' \cos \alpha - \sin \alpha)) N \, dx \, dy \\ + \int_{\Gamma} ((4h\eta \partial_x u + 2h\eta \partial_y v - \frac{1}{2}\varrho g \cos \alpha h^2) n_x + h\eta (\partial_x v + \partial_y u) n_y) N \, d\Gamma = 0\end{aligned}$$

If the von Neumann boundary condition is of Weertman type, the boundary integral along Γ_2 is equal to zero (for $\alpha = 0$), and zero on the remaining part of the boundary if we set the weight functions to zero and determine the values of the unknowns using Dirichlet boundary conditions.

We are left with

$$\begin{aligned}- \int_{\Omega} (h\eta (4\partial_x u + 2\partial_y v) \partial_x N + h\eta (\partial_x v + \partial_y u) \partial_y N) \, dx \, dy - \int_{\Omega} t_{bx} N \, dx \, dy \\ = \rho g \int_{\Omega} h ((\partial_x s - (1 - \rho/\rho_o)\partial_x h) \cos \alpha - \sin \alpha) N \, dx \, dy - \frac{1}{2}\varrho g \cos \alpha \int_{\Omega} h^2 \partial_x N \, dx \, dy\end{aligned}$$

y direction

$$\int_{\Omega} \partial_y ((4h\eta \partial_y v + 2h\eta \partial_x u) - \frac{1}{2}\varrho g \cos \alpha \partial_y h^2 + \partial_x (h\eta (\partial_y u + \partial_x v)) - t_{by} - \rho g h \partial_y s' \cos \alpha) N \, dx \, dy$$

with Weertman type boundary condition

$$\eta h (\partial_x v + \partial_y u) n_x + (4h\eta \partial_y v + 2h\eta \partial_x u) n_y = \frac{1}{2}\rho(1 - \rho/\rho_o)gh^2 n_y$$

We have

$$\begin{aligned}- \int_{\Omega} ((4h\eta \partial_y v + 2h\eta \partial_x u) \partial_y N - \frac{1}{2}\varrho g \cos \alpha h^2 \partial_y N + h\eta (\partial_y u + \partial_x v) \partial_x N) \, dx \, dy \\ - \int_{\Omega} (t_{by} + \rho g h \partial_y s' \cos \alpha) N \, dx \, dy \\ + \int_{\Gamma} ((4h\eta \partial_y v + 2h\eta \partial_x u - \frac{1}{2}\varrho g \cos \alpha h^2) n_y + \eta h (\partial_y u + \partial_x v) n_x) N \, d\Gamma = 0\end{aligned}\tag{6.2}$$

Again we can ignore the boundary integral as it is identically equal to zero.

$$\begin{aligned}
& - \int_{\Omega} ((4h\eta\partial_y v + 2h\eta\partial_x u)\partial_y N + h\eta(\partial_y u + \partial_x v)\partial_x N) dx dy - \int_{\Omega} t_{by} N dx dy \\
& = \rho g \cos \alpha \int_{\Omega} h(\partial_y s - (1 - \rho/\rho_o)\partial_y h) N dx dy - \frac{1}{2} \rho g \cos \alpha \int_{\Omega} h^2 \partial_y N dx dy
\end{aligned} \tag{6.3}$$

6.1.3 2HD FE diagnostic equation written in terms of h (suitable for fully coupled approach)

Where the ice is afloat, $s - S = (1 - \rho/\rho_o)h$ and $s' = 0$, hence

$$s' = \begin{cases} s - S - (1 - \rho/\rho_o)h, & \text{if } h > h_f \\ 0, & \text{if } h \leq h_f \end{cases}$$

i.e.

$$s' := \mathcal{H}(h - h_f)(s - S - (1 - \rho/\rho_o)h)$$

where

$$h_f := (S - B)\rho_o/\rho$$

We can also write s' as

$$\begin{aligned}
s' &= \mathcal{H}(h - h_f)(s - S - (1 - \rho/\rho_o)h) \\
&= \mathcal{H}(h - h_f)(h\rho/\rho_o + s - S - h) \\
&= \mathcal{H}(h - h_f)(h\rho/\rho_o + h + b - S - h) \\
&= \mathcal{H}(h - h_f)(h\rho/\rho_o + h + B - S - h) \\
&= \mathcal{H}(h - h_f)(h\rho/\rho_o + B - S) \\
&= \mathcal{H}(h - h_f)(h\rho/\rho_o - H)
\end{aligned}$$

i.e.

$$s'(x) = \mathcal{H}(h - h_f)(\rho/\rho_o h - H) \tag{6.4}$$

where we used the fact that $b = B$ whenever $\mathcal{H}(h - h_f) = 1$. This expression for s' is needed for linearisation around h .

The field equations can therefore be written as before, i.e. as

$$\begin{aligned}
\partial_x(4h\eta\partial_x u + 2h\eta\partial_y v) + \partial_y(h\eta(\partial_x v + \partial_y u)) - \beta^2 u &= \rho g h(\partial_x s' \cos \alpha - \sin \alpha) + \frac{1}{2} \rho g \cos \alpha \partial_x h^2, \\
\partial_y(4h\eta\partial_y v + 2h\eta\partial_x u) + \partial_x(h\eta(\partial_y u + \partial_x v)) - \beta^2 v &= \rho g h \partial_y s' \cos \alpha + \frac{1}{2} \rho g \cos \alpha \partial_y h^2,
\end{aligned}$$

but the linearisation with respect to h needed in a fully coupled approach requires (6.4).

The FE formulation for the prognostic equation is the θ method, i.e.

$$R_p^h = \int_{\Omega} \left\{ \frac{1}{\Delta t} (h_1 - h_0) + \theta \partial_x (u_1 h_1) + (1 - \theta) \partial_x (u_0 h_0) + \theta \partial_y (v_1 h_1) + (1 - \theta) \partial_y (u_0 h_0) - a \right\} N_p dx dy = 0 \tag{6.5}$$

where $0 \leq \theta \leq 1$.

$$a := a_s + a_b$$

6.2 Element integrals

$$x = x_p N_P(\xi, \eta)$$

For 3-node element:

Nodal u displacement vector of a particular element

$$u_e := \begin{pmatrix} u1 \\ u2 \\ u3 \end{pmatrix}$$

$$\text{fun} := \begin{pmatrix} N_1(\xi, \eta) \\ N_2(\xi, \eta) \\ N_3(\xi, \eta) \end{pmatrix}$$

$$u(x, y) = u_e^T \text{fun}$$

$$\text{der} = \begin{pmatrix} \frac{\partial N_1}{\partial \xi} & \frac{\partial N_2}{\partial \xi} & \frac{\partial N_3}{\partial \xi} \\ \frac{\partial N_1}{\partial \eta} & \frac{\partial N_2}{\partial \eta} & \frac{\partial N_3}{\partial \eta} \end{pmatrix}$$

$$\text{coo} = \begin{pmatrix} x_1 & y_1 \\ x_2 & y_2 \\ x_3 & y_4 \end{pmatrix}$$

$$\mathbf{J} = \begin{pmatrix} \frac{\partial x}{\partial \xi} & \frac{\partial y}{\partial \xi} \\ \frac{\partial x}{\partial \eta} & \frac{\partial y}{\partial \eta} \end{pmatrix}$$

$$\mathbf{J}^{-1} = \begin{pmatrix} \frac{\partial \xi}{\partial x} & \frac{\partial \eta}{\partial x} \\ \frac{\partial \xi}{\partial y} & \frac{\partial \eta}{\partial y} \end{pmatrix}$$

$$\begin{pmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{pmatrix} = \mathbf{J} \begin{pmatrix} \frac{\partial}{\partial x} \\ \frac{\partial}{\partial y} \end{pmatrix}$$

$$\begin{pmatrix} \frac{\partial}{\partial x} \\ \frac{\partial}{\partial y} \end{pmatrix} = \mathbf{J}^{-1} \begin{pmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{pmatrix}$$

$$\mathbf{J} = \begin{pmatrix} \frac{\partial N_1}{\partial \xi} & \frac{\partial N_2}{\partial \xi} & \frac{\partial N_3}{\partial \xi} \\ \frac{\partial N_1}{\partial \eta} & \frac{\partial N_2}{\partial \eta} & \frac{\partial N_3}{\partial \eta} \end{pmatrix} \begin{pmatrix} x_1 & y_1 \\ x_2 & y_2 \\ x_3 & y_4 \end{pmatrix}$$

$$\mathbf{J} = \text{der coo}$$

$$\mathbf{D} = \begin{pmatrix} \frac{\partial N_1}{\partial x} & \frac{\partial N_2}{\partial x} & \frac{\partial N_3}{\partial x} \\ \frac{\partial N_1}{\partial y} & \frac{\partial N_2}{\partial y} & \frac{\partial N_3}{\partial y} \end{pmatrix} = \mathbf{J}^{-1} \begin{pmatrix} \frac{\partial N_1}{\partial \xi} & \frac{\partial N_2}{\partial \xi} & \frac{\partial N_3}{\partial \xi} \\ \frac{\partial N_1}{\partial \eta} & \frac{\partial N_2}{\partial \eta} & \frac{\partial N_3}{\partial \eta} \end{pmatrix}$$

$$\mathbf{D} = \mathbf{J}^{-1} \text{der}$$

Change of integral, example:

$$\int_{A_e} u_p N_p(x, y) N_q(x, y) dx dy = \int_{\Delta} u_p N_p(\xi, \eta) N_q(\xi, \eta) \det \mathbf{J} d\xi d\eta$$

Example:

$$\int \partial_x u \partial_x N dx dy = \left(\int D_{1p} D_{1q} |\mathbf{J}| d\eta d\xi \right) u_p$$

6.3 Edge integrals

We have integrals on the form

$$\int_{\Gamma} u(x, y) N(x, y) n_x d\Gamma$$

with

$$\mathbf{n} = \begin{pmatrix} n_x \\ n_y \end{pmatrix}$$

If the boundary is parameterised such that $(x, y) = (x(\gamma), y(\gamma))$ as γ goes from 0 to 1 then

$$\mathbf{n} = \frac{1}{\sqrt{(\partial_{\gamma}x)^2 + (\partial_{\gamma}y)^2}} \begin{pmatrix} -\partial_{\gamma}y \\ \partial_{\gamma}x \end{pmatrix}$$

and

$$d\Gamma = \sqrt{(\partial_{\gamma}x)^2 + (\partial_{\gamma}y)^2} d\gamma$$

and therefore

$$\mathbf{n} d\Gamma = \begin{pmatrix} -\partial_{\gamma}y \\ \partial_{\gamma}x \end{pmatrix} d\gamma$$

Hence

$$\int_{\Gamma} u(x, y) N(x, y) n_x d\Gamma = - \int_0^1 u(x(\gamma), y(\gamma)) N(x(\gamma), y(\gamma)) \partial_{\gamma}y d\gamma$$

6.3.1 Edge 12

For edge 12, $\eta = 0$. I parameterise it as $(\xi, \eta) = (1 - \gamma, 0)$ as this takes me from node 1 to node 2 in clockwise direction (that is how I order the nodes, most FE do it the other way around)

$$x = x_P N_P(1 - \gamma, 0) \quad \text{and} \quad y = y_P N_P(1 - \gamma, 0)$$

The normal is

$$\mathbf{n} d\Gamma = \begin{pmatrix} -\partial_{\gamma}y \\ \partial_{\gamma}x \end{pmatrix} d\gamma$$

and

$$\begin{aligned} \frac{1}{\partial_{\gamma}} &= \frac{1}{\partial_{\xi}} \frac{\partial \xi}{\partial \gamma} + \frac{1}{\partial_{\eta}} \frac{\partial \eta}{\partial \gamma} \\ &= -\frac{1}{\partial_{\xi}} \end{aligned}$$

and therefore

$$\partial_{\gamma}y = -\partial_{\xi}y = -J_{12}$$

and

$$\mathbf{n} d\Gamma = \begin{pmatrix} \partial_{\xi}y \\ -\partial_{\xi}x \end{pmatrix} d\gamma = \begin{pmatrix} J_{12} \\ -J_{11} \end{pmatrix} d\gamma$$

or simply

$$\mathbf{n} d\Gamma = \begin{pmatrix} y_q \partial_{\xi} N_q(\xi, 0) \\ -x_q \partial_{\xi} N_q(\xi, 0) \end{pmatrix} d\gamma$$

For the linear triangle, for example, I get

$$\begin{aligned} x &= x_1 \xi + x_2(1 - \xi) + x_3 \eta \\ &= x_1(1 - \gamma) + x_2(1 - (1 - \gamma)) + x_3 0 \\ &= x_1(1 - \gamma) + x_2 \gamma \end{aligned}$$

$$y = y_1(1 - \gamma) + y_2 \gamma$$

and

$$\partial_{\gamma}x = -x_1 + x_2$$

$$\partial_{\gamma}y = -y_1 + y_2$$

and a normal

$$\mathbf{n} d\Gamma = \begin{pmatrix} y_1 - y_2 \\ x_2 - x_1 \end{pmatrix} d\gamma$$

6.3.2 Edge 23

For edge 23, $\xi = 0$. I parameterise the edge as $(\xi, \eta) = (0, \gamma)$, this takes me from node 2 to 3 as γ varies from 0 to 1.

$$\mathbf{n} d\Gamma = \begin{pmatrix} -\partial_\gamma y \\ \partial_\gamma x \end{pmatrix} d\gamma$$

$$\partial_\gamma = \partial_\eta$$

and therefore

$$\mathbf{n} d\Gamma = \begin{pmatrix} -\partial_\eta y \\ \partial_\eta x \end{pmatrix} = \begin{pmatrix} -J_{22} \\ J_{21} \end{pmatrix} d\gamma$$

For the linear triangle, for example, I get

$$x = x_1 0 + x_2(1 - \gamma) + x_3 \gamma$$

$$y = y_1 0 + y_2(1 - \gamma) + y_3 \gamma$$

and

$$\partial_\gamma x = -x_2 + x_3$$

$$\partial_\gamma y = -y_2 + y_3$$

and a normal

$$\mathbf{n} d\Gamma = \begin{pmatrix} y_2 - y_3 \\ x_3 - x_2 \end{pmatrix} d\gamma$$

6.3.3 Edge 32

For edge 32 is parameterised as $(\xi, \eta) = (\gamma, 1 - \gamma)$, and

$$\mathbf{n} d\Gamma = \begin{pmatrix} -\partial_\gamma y \\ \partial_\gamma x \end{pmatrix} d\gamma$$

and

$$\begin{aligned} \frac{1}{\partial_\gamma} &= \frac{1}{\partial_\xi} \frac{\partial \xi}{\partial \gamma} + \frac{1}{\partial_\eta} \frac{\partial \eta}{\partial \gamma} \\ &= \frac{1}{\partial_\xi} - \frac{1}{\partial_\eta} \end{aligned}$$

and therefore

$$\mathbf{n} d\Gamma = \begin{pmatrix} -y_q(\partial_\xi N_q - \partial_\eta N_q) \\ x_q(\partial_\xi N_q - \partial_\eta N_q) \end{pmatrix} d\gamma = \begin{pmatrix} J_{22} - J_{12} \\ J_{11} - J_{21} \end{pmatrix} d\gamma$$

For the linear triangle, for example, I get

$$x = x_1 \gamma + x_2(1 - \gamma - (1 - \gamma)) + x_3(1 - \gamma)$$

or

$$x = x_1 \gamma + x_3(1 - \gamma)$$

$$y = y_1 \gamma + y_3(1 - \gamma)$$

and

$$\partial_\gamma x = x_1 - x_3$$

$$\partial_\gamma y = y_1 - y_3$$

and a normal

$$\mathbf{n} d\Gamma = \begin{pmatrix} y_3 - y_1 \\ x_1 - x_3 \end{pmatrix} d\gamma$$

6.4 Various directional derivatives

6.4.1 Directional derivative of draft with respect to ice thickness

For implicit forward time integration with respect to h using the NR method, various directional derivatives with respect to h must be calculated.

Using Eq. (1.30) we find that the directional derivative of the draft d with respect to h is

$$\begin{aligned} Dd(h)[\Delta h] &= \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} d(h + \epsilon \Delta h) \\ &= \mathcal{H}(h_f - h) \rho \Delta h / \rho_o - \rho h \delta(h_f - h) \Delta h / \rho_o + \mathcal{H}(H) H \delta(h - h_f) \Delta h \\ &= \frac{\rho}{\rho_o} \mathcal{H}(h_f - h) \Delta h + \delta(h - h_f) (H - \frac{\rho}{\rho_o} h) \Delta h \end{aligned}$$

When integrated the second term in the above expression integrates to zero, because where $h = h_f$ we have $H = \rho h / \rho_o$, hence¹

$$Dd(h)[\Delta h] = \frac{\rho}{\rho_o} \mathcal{H}(h_f - h) \Delta h. \quad (6.6)$$

Using Eq. (6.6) the directional derivative of

$$D\left(\frac{1}{2}g(\rho h^2 - \rho_o d^2)\right)[\Delta h]$$

with respect to perturbation in h is found to be

$$\begin{aligned} D\left(\frac{1}{2}g(\rho h^2 - \rho_o d^2)\right)[\Delta h] &= g(\rho(h \Delta h - \rho_o d \frac{\rho}{\rho_o} \mathcal{H}(h_f - h) \Delta h)) \\ &= \rho g(h - \mathcal{H}(h_f - h)d) \Delta h \end{aligned}$$

The directional derivative of $g(\rho h - \rho_o d) \partial_x b$ with respect to h is found to be

$$D(g(\rho h - \rho_o d) \partial_x b)[\Delta h] = \rho g \mathcal{H}(h - h_f) \partial_x B \Delta h. \quad (6.7)$$

To see this first notice that using Eq. (1.30)

$$\begin{aligned} g(\rho h - \rho_o d) \partial_x b &= g(\rho h - \mathcal{H}(h_f - h) \rho h - \rho_o \mathcal{H}(H) \mathcal{H}(h - h_f) H) \partial_x b \\ &= g(\rho h - (1 - \mathcal{H}(h - h_f)) \rho h - \rho_o \mathcal{H}(H) \mathcal{H}(h - h_f) H) \partial_x b \\ &= g(\rho h + \mathcal{H}(h - h_f)(\rho h - \rho_o H^+) - \rho h) \partial_x b \\ &= g \mathcal{H}(h - h_f)(\rho h - \rho_o H^+) \partial_x b \\ &= g \mathcal{H}(h - h_f)(\rho h - \rho_o H^+) \partial_x B. \end{aligned}$$

Using the above expression we now calculate the directional derivative of $g(\rho h - \rho_o d) \partial_x b$ with respect to h and find

$$\begin{aligned} D(g(\rho h - \rho_o d) \partial_x b)[\Delta h] &= \mathcal{H}(h_f - h) \rho g \partial_x B \Delta h + g \delta(h - h_f)(\rho h - \rho_o H^+) \partial_x B \Delta h \\ &= (\rho \mathcal{H}(h - h_f) + \delta(h - h_f)(\rho h - \rho_o H^+)) g \partial_x B \Delta h \\ &= \rho g \mathcal{H}(h - h_f) \partial_x B \Delta h \end{aligned}$$

where the last step is correct when the expression is evaluated under an integral, thus demonstrating the correctness of Eq. (6.7).

(Not sure where to put this, but keep it here for the time being.) The lower ice surface is related to thickness through

$$b = \mathcal{H}(h - h_f) B + \mathcal{H}(h_f - h)(S - \rho h / \rho_o)$$

and therefore

$$\partial_h b = \delta(h - h_f) B - \delta(h_f - h)(S - \rho h / \rho_o) - \mathcal{H}(h_f - h) \rho / \rho_o$$

¹This argument does not hold if the Heaviside function and the Dirac delta functions are approximated. In that case the full expression must be used. Important for getting quadratic convergence in NR.

and

$$\partial_x b = \partial_h b \partial_x h = \delta(h - h_f) B \partial_x h - \delta(h_f - h) (S - \rho h / \rho_o) \partial_x h - \mathcal{H}(h_f - h) \rho \partial_x h / \rho_o$$

and assuming

$$\frac{\partial^2 b}{\partial h \partial x} = \frac{\partial^2 b}{\partial x \partial h}$$

If $f(x)$ is a test function

$$\partial_x \int \delta(x) \partial_x f(x) dx = -\partial_x \int \mathcal{H}(x) f(x) dx = -f(0) - \int \mathcal{H}(x) \partial_x f(x) dx$$

6.4.2 Linearisation of the 2HD forward problem needed for the adjoint method

For the adjoint method we need

$$\mathbf{K}^{xC} \Delta C_q := -D\mathbf{F}_x(\mathbf{u}_1^i, \mathbf{v}_1^i, \mathbf{h}_1^i)[\Delta C_q]$$

Here ΔC_q is the nodal value itself, the perturbation in C is $\Delta C_q N_q$ (no summation).

$$[\mathbf{K}^{xC}]_{pq} = \frac{1}{m} \int_{\Omega} \mathcal{H}(h - h_f) C^{-1/m-1} |\mathbf{v}_{xy}|^{1/m-1} u N_p N_q dx dy \quad (6.8)$$

and

$$[\mathbf{K}^{yC}]_{pq} = \frac{1}{m} \int_{\Omega} \mathcal{H}(h - h_f) C^{-1/m-1} |\mathbf{v}_{xy}|^{1/m-1} v N_p N_q dx dy \quad (6.9)$$

where

$$\mathbf{v}_{xy} = \begin{pmatrix} u \\ v \end{pmatrix}$$

and

$$\beta^2 = C^{-1/m} |\mathbf{v}_{xy}|^{1/m-1}$$

$$\mathbf{K}^C = \begin{bmatrix} \mathbf{K}^{xC} \\ \mathbf{K}^{yC} \end{bmatrix}$$

is $2N \times n$ where N are degrees of freedom.

If the cost function I is calculated using FE type inner product, then the gradient of the cost function is then given by

$$\begin{aligned} \frac{\partial I}{\partial C_q} &= [\mathbf{K}^C]_{pq} \lambda_p \\ &= \frac{1}{m} \int_{\Omega} \mathcal{H}(h - h_f) C^{-1/m-1} |\mathbf{v}_{xy}|^{1/m-1} (u\lambda + v\mu) N_q dx dy \end{aligned}$$

If I is calculated as a discrete sum over values then

$$\frac{\partial I}{\partial C_q} = \frac{1}{m} \mathcal{H}(h - h_f) C^{-1/m-1} |\mathbf{v}_{xy}|^{1/m-1} (u\lambda + v\mu)$$

Note: Perturbing on particular nodal value in $C = C_r N_r$ can be written as

$$(C_r + \Delta C \delta_{rp}) N_r$$

$$C = C_r N_r, \Delta C = \Delta \hat{C} N_q \text{ with } \Delta \hat{C} = \Delta C_q$$

$$\begin{aligned} (C + \Delta C)^m &= (C_r N_r + \Delta \hat{C} N_q)^m \\ &= (C_r N_r + \Delta \hat{C} N_q (C_j N_j) / (C_i N_i))^m \\ &= (C_r N_r)^m (1 + \Delta \hat{C} N_q / (C_i N_i))^m \\ &\approx (C_r N_r)^m (1 + m \Delta \hat{C} N_q / (C_i N_i)) \\ &= (C_s N_s)^m + m (C_s N_s)^{m-1} \Delta \hat{C} N_q \\ &= (C_s N_s)^m + m (C_s N_s)^{m-1} \Delta C \end{aligned}$$

I can write the perturbation as

$$K\Delta\hat{C} = m(C_s N_s)^{m-1} N_q \Delta\hat{C}$$

where

$$K = m(C_s N_s)^{m-1} N_q$$

6.5 FE formulation and linearisation for the 1HD Problem

6.5.1 Field equations and boundary conditions (1HD)

$$4\partial_x(h\eta\partial_x u) - \beta^2 u = \rho gh(\partial_x s \cos \alpha - \sin \alpha)$$

$$\beta^2 = C^{-1/m} |u|^{1/m-1}$$

with the sliding law written on the form

$$u = C |t_{bx}|^{m-1} t_{bx}$$

We have

$$t_{bx} = C^{-1/m} |u|^{1/m-1} u.$$

Glen's flow law is

$$\dot{\epsilon}_{ij} = A\tau^{n-1}\tau_{ij},$$

where

$$\tau = \sqrt{\tau_{ij}\tau_{ij}/2}$$

The flow law can also be written as

$$\tau_{ij} = A^{-1/n} \dot{\epsilon}^{(1-n)/n} \dot{\epsilon}_{ij},$$

where

$$\dot{\epsilon} = \sqrt{(\partial_x u)^2} = |\partial_x u|$$

If we write

$$\tau_{ij} = 2\eta\dot{\epsilon}_{ij}$$

then η is the effective viscosity given by

$$\eta = \frac{1}{2} A^{-1/n} \dot{\epsilon}^{(1-n)/n} = \frac{1}{2} A^{-1/n} |\partial_x u|^{(1-n)/n}$$

6.6 Linearisation of field equations (1HD)

In the non-linear case using Newton's method I need to linearise the equation.

Linearised with respect to u by writing $u = \bar{u} + \Delta u$, $\eta = \bar{\eta} + \partial_u \eta \Delta u$, and $\beta^2 = \bar{\beta}^2 + \partial_u(\beta^2) \Delta u$

$$4\partial_x(h(\bar{\eta} + \partial_u \eta \Delta u)\partial_x(\bar{u} + \Delta u)) - (\bar{\beta}^2 + \partial_u \beta^2(u)[\Delta u])(\bar{u} + \Delta u) = \rho gh(\partial_x s \cos \alpha - \sin \alpha)$$

or

$$4\partial_x(h(\bar{\eta}\partial_x \bar{u} + \partial_x \bar{u} D\eta(u)[\Delta u] + \bar{\eta}\partial_x \Delta u)) - \bar{\beta}^2 \bar{u} - \bar{\beta}^2 \Delta u - \bar{u} D\beta^2(u)[\Delta u] = \rho gh(\partial_x s \cos \alpha - \sin \alpha) \quad (6.10)$$

where

$$\eta = \frac{1}{2} A^{-1/n} \dot{\epsilon}^{(1-n)/n} = \frac{1}{2} A^{-1/n} |\partial_x u|^{(1-n)/n}$$

$$D\eta(u)[\Delta u] = \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} \eta(u + \epsilon \Delta u) \quad (6.11)$$

$$\begin{aligned} &= \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} \frac{1}{2} A^{-1/n} |\partial_x(u + \epsilon \Delta u)|^{(1-n)/n} \\ &\stackrel{\partial_x u > 0}{=} \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} \frac{1}{2} A^{-1/n} (\partial_x u + \epsilon \partial_x \Delta u)^{(1-n)/n} \\ &= \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} \frac{1}{2} A^{-1/n} ((\partial_x u)^{(1-n)/n} + (\partial_x u)^{(1-2n)/n} \frac{1-n}{n} \epsilon \partial_x \Delta u + \dots) \\ &= \frac{1}{2} A^{-1/n} ((\partial_x u)^{(1-2n)/n} \frac{1-n}{n} \partial_x \Delta u) \\ &= \frac{1-n}{2n} A^{-1/n} (\partial_x u)^{(1-2n)/n} \partial_x \Delta u \end{aligned} \quad (6.12)$$

Doing the same for $\partial_x u < 0$ shows that

$$D\eta(u)[\Delta u] = \frac{1-n}{2n} A^{-1/n} |\partial_x u|^{(1-2n)/n} \text{sign}(\partial_x u) \partial_x \Delta u$$

(old result)

$$\partial_u \eta = \frac{1-n}{2n} A^{-1/n} |\partial_x u|^{1/n-2} \text{sign}(\partial_x u)$$

The directional derivative of β^2 is

$$\begin{aligned} D\beta(u)[\Delta u] &= (1/m - 1) C^{-1/m} |u|^{(1-3m)/m} u \Delta u \\ &= (1/m - 1) C^{-1/m} |u|^{(1-2m)/m} \text{sign}(u) \Delta u \end{aligned}$$

old result

$$\partial_u \beta^2 = (1/m - 1) C^{-1/m} |u|^{1/m-2} \text{sign}(u)$$

Inserting into (6.10) gives

$$4\partial_x(h(\bar{\eta}\partial_x \bar{u} + \bar{u}E\partial_x \Delta u + \bar{\eta}\partial_x \Delta u)) - \bar{\beta}^2 \bar{u} - \bar{\beta}^2 \Delta u - \bar{u}B\Delta u = \rho gh(\partial_x s \cos \alpha - \sin \alpha)$$

where

$$B = (1/m - 1) C^{-1/m} |u|^{(1-2m)/m} \text{sign}(u)$$

and

$$E = \frac{1-n}{2n} A^{-1/n} |\partial_x u|^{(1-2n)/n} \text{sign}(\partial_x u)$$

which can be written on the form

$$4\partial_x(h(\bar{\eta} + \bar{u}E)\partial_x \Delta u) - (\bar{\beta}^2 + \bar{u}B)\Delta u = \rho gh(\partial_x s \cos \alpha - \sin \alpha) - 4\partial_x(h\bar{\eta}\partial_x \bar{u}) + \bar{\beta}^2 \bar{u}$$

6.6.1 Newton Rapson

$$4\partial_x(\eta h \partial_x u) - \mathcal{H}(h - h_f) \beta^2 u = \frac{1}{2} g \cos \alpha \rho (1 - \rho/\rho_o) \partial_x h^2 + \rho gh \cos \alpha \mathcal{H}(h - h_f) \partial_x s' - \rho gh \sin \alpha, \quad (6.13)$$

$$s'(x) = \mathcal{H}(h - h_f)(\rho/\rho_o h - H)$$

where

$$H = S - B,$$

or as

$$4\partial_x(\eta h \partial_x u) - \mathcal{H}(h - h_f) \beta^2 u = \frac{\partial g}{2} \partial_x h^2 \cos \alpha + \rho gh \mathcal{H}(h - h_f)(\rho/\rho_o \partial_x h - \partial_x H) \cos \alpha - \rho gh \sin \alpha \quad (6.14)$$

The FE formulation is

$$\begin{aligned} R_p^u &= \int \{4\eta h \partial_x u \partial_x N_p + \mathcal{H}(h - h_f) \beta^2 u N_p \\ &\quad - \frac{\rho g}{2} h^2 \partial_x N \cos \alpha + \rho g h \mathcal{H}(h - h_f) (\rho / \rho_o \partial_x h - \partial_x H) N_p \cos \alpha - \rho g h N_p \sin \alpha\} dx \\ &\quad - h(4\eta \partial_x u - \rho g h / 2) u N_p|_{x_0}^{x_l} = 0 \end{aligned} \quad (6.15)$$

where

$$u = N_p u_p$$

If all von Neumann BC are of Weertman type, the boundary term is zero because at the calving front

$$8\eta \partial_x u = \rho g h.$$

I also have

$$\partial_t h + \partial_x(uh) = a \quad (6.16)$$

where

$$a = a_s + a_b$$

The FE formulation used is the θ method, i.e.

$$R_p^h = \int \left\{ \frac{1}{\Delta t} (h_1 - h_0) + \theta \partial_x(u_1 h_1) + (1 - \theta) \partial_x(u_0 h_0) - a_s - a_b \right\} N_p dx = 0 \quad (6.17)$$

where $0 \leq \theta \leq 1$.

I go from time $t = t_0$ to time $t = t_1$ where $t_1 > t_0$, and I assume that the values for u at h at $t = t_0$ (i.e. u_0 and h_0) are known. I iteratively solve for corrections to the values at time step $t = t_1$

$$u_1 = \bar{u}_1 + \Delta u$$

$$h_1 = \bar{h}_1 + \Delta h$$

using the Newton-Raphson method.

For Newton-Raphson I need to take the directional derivative of this equation with respect to u and h ,

$$K(\Delta u, \Delta v)^T = \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} \mathbf{R}(\mathbf{v} + \epsilon \Delta \mathbf{v}, \mathbf{h} + \epsilon \Delta \mathbf{h})$$

where I have now ordered the discrete values of u and h into a vector

$$D(\mathcal{H}(h - h_f) \beta^2(u))[\Delta h] = \delta(\bar{h} - h_f) \beta^2(\bar{u}) \Delta h$$

Directional derivative of right-hand term with respect to h .

$$\begin{aligned} D\{\rho g h \partial_x s'\}[\Delta h] &= D\{\rho g h \partial_x (\mathcal{H}(h - h_f) (\rho h / \rho_o - H))\}[\Delta h] \\ &= \lim_{\epsilon \rightarrow 0} \partial_\epsilon (\rho g (h + \epsilon \Delta h) \partial_x (\mathcal{H}(h + \epsilon \Delta h - h_f) (\rho (h + \epsilon \Delta h) / \rho_o - H))) \\ &= \rho g \partial_x (\mathcal{H}(h - h_f) (\rho h / \rho_o - H)) \Delta h \\ &\quad + \rho g h \partial_x (\mathcal{H}(h - h_f) \rho \Delta h / \rho_o) \\ &\quad + \rho g h \partial_x (\delta(h - h_f) \Delta h (\rho h / \rho_o - H)) \quad (= 0) \\ &= \rho g \mathcal{H}(h - h_f) (\rho \partial_x h / \rho_o - \partial_x H) \Delta h \\ &\quad + \frac{\rho^2}{\rho_o} g h \mathcal{H}(h - h_f) \partial_x \Delta h \\ &\quad + \frac{\rho^2}{\rho_o} \delta(h - h_f) h \partial_x h \Delta h \end{aligned}$$

$$\begin{aligned} [Kuh]_{pq} \Delta h_q &= DR_p^u(u, h)[\Delta h_q] = \int \{4\bar{\eta} \partial_x \bar{u} \partial_x N_p \\ &\quad + \delta(\bar{h} - h_f) \bar{\beta}^2 \bar{u} N_p \\ &\quad - \rho g \bar{h} \partial_x N_p \cos \alpha \\ &\quad + \rho g \mathcal{H}(\bar{h} - h_f) (\rho / \rho_o \partial_x \bar{h} - \partial_x H) N_p \cos \alpha \\ &\quad + \rho g \bar{h} \delta(\bar{h} - h_f) (\rho / \rho_o \partial_x \bar{h} - \partial_x H) N_p \cos \alpha \\ &\quad + \rho g \bar{h} \mathcal{H}(\bar{h} - h_f) \rho / \rho_o N_p \cos \alpha \partial_x \\ &\quad - \rho g N_p \sin \alpha\} \Delta h_q dx \end{aligned}$$

$$\begin{aligned}
[Kuu]_{pq}\Delta u_q &= DR_p^u(u, h)[\Delta u_q] = \int \{4\bar{h}\partial_u\eta(\bar{u})\partial_x\bar{u}\partial_x N_p\partial_x \\
&\quad + 4\eta(\bar{u})\bar{h}\partial_x N_p\partial_x \\
&\quad + \mathcal{H}(h - h_f)\partial_u\beta^2(\bar{u})N_p \\
&\quad + \mathcal{H}(h - h_f)\beta^2(\bar{u})N_p\} \Delta u_q dx
\end{aligned}$$

Linearising (6.17) gives

$$\int \{(\Delta h + \bar{h} - h_0)/\Delta t + \theta\partial_x((\bar{u} + \Delta u)(\bar{h} + \Delta h)) + (1 - \theta)\partial_x(u_0 h_0) - a_s - a_b\} N_p = 0$$

or

$$\begin{aligned}
0 &= \int \{\Delta h/\Delta t + \theta\partial_x((\bar{u}\Delta h + \bar{h}\Delta u))\} N_p dx + \\
&\quad \int \{(\bar{h} - h_0)/\Delta t + \theta\partial_x(\bar{u}\bar{h}) + (1 - \theta)\partial_x(u_0 h_0) - a_s - a_b\} N_p dx
\end{aligned} \tag{6.18}$$

or

$$[Khu]_{pq}\Delta u_q = \theta(\partial_x\bar{h} + \bar{h}\partial_x)\Delta u_q N_p$$

$$[Khh]_{pq} = (1/\Delta t + \theta(\partial_x\bar{u}N_q + \bar{u}\partial_x N_q)) N_p$$

$$\begin{bmatrix} Kuu & Kuh \\ Khu & Khh \end{bmatrix} \begin{bmatrix} \Delta v \\ \Delta h \end{bmatrix} = \begin{bmatrix} \mathbf{R}^u \\ \mathbf{R}^h \end{bmatrix} \tag{6.19}$$

where

$$\mathbf{r}^h = \mathbf{T}^h - \mathbf{F}^h$$

where \mathbf{T} and \mathbf{F} are the internal and external nodal forces, respectively, and \mathbf{R} is the residual or out-of-balance nodal forces.

$$F^h = - \int \{a_s + a_b - (\bar{h} - h_0)/\Delta t\} N_p dx$$

and

$$T^h = \int \{\theta\partial_x(\bar{u}\bar{h}) + (1 - \theta)\partial_x(u_0 h_0)\} N_p dx$$

$$T_p^u = \int \{4\eta h\partial_x u\partial_x N_p + \mathcal{H}(h - h_f)\beta^2 u N_p\} N_p dx$$

$$F_p^u = \int \left\{ \frac{\rho g}{2} h^2 \partial_x N + \rho g h \mathcal{H}(h - h_f) (\rho/\rho_o \partial_x h - \partial_x H) N_p \right\} dx$$

6.6.2 Connection to Picard iteration

If instead of writing

$$4\partial_x(h(\bar{\eta}\partial_x\bar{u} + \partial_u\eta\partial_x\bar{u}\Delta u + \bar{\eta}\partial_x\Delta u)) - (\bar{\beta}^2\bar{u} + (\bar{\beta}^2 + \partial_u\beta^2\bar{u})\Delta u) = \rho gh(\partial_x s \cos \alpha - \sin \alpha)$$

we ignore the dependency of η and β^2 on u we get

$$4\partial_x(h(\bar{\eta}\partial_x\bar{u} + \bar{\eta}\partial_x\Delta u)) - (\bar{\beta}^2\bar{u} + \bar{\beta}^2\Delta u) = \rho gh(\partial_x s \cos \alpha - \sin \alpha)$$

or simply

$$4\partial_x(h\bar{\eta}\partial_x(\bar{u} + \Delta u)) - \bar{\beta}^2(\bar{u} + \Delta u) = \rho gh(\partial_x s \cos \alpha - \sin \alpha)$$

which can be solved directly for $\bar{u} + \Delta u$. This is the Picard iteration, i.e. an incomplete Newton iteration.

6.7 Linearisation in 2HD

6.7.1 Drag-term linearisation (2HD)

Basal drag is generally a function of basal sliding velocity and flotation conditions, i.e.

$$\mathbf{t}_b = f(h, h_f, \mathbf{v})$$

Using Weertman sliding law, basal drag is

$$\mathbf{t}_b = \mathcal{H}(h - h_f) \beta^2 \mathbf{v}_b$$

or

$$\begin{pmatrix} t_{xb} \\ t_{xy} \end{pmatrix} = \mathcal{H}(h - h_f) \beta^2 \begin{pmatrix} u \\ v \end{pmatrix} \quad (6.20)$$

with

$$\beta^2 = C^{-1/m} |\mathbf{v}_b|^{1/m-1}$$

therefore

$$\tau_b = \tau_b(u, v, h)$$

We therefore need to linearise \mathbf{t}_b with respect to u , v , and h .

We start by linearising β^2 with respect to u and v obtaining²

$$\begin{aligned} \beta(u + \epsilon \Delta u, v + \epsilon \Delta v) &= C^{-1/m} ((u + \epsilon \Delta u)^2 + (v + \epsilon \Delta v)^2)^{(1/m-1)/2} \\ &= C^{-1/m} (u^2 + v^2 + 2\epsilon(u \Delta u + v \Delta v))^{(1/m-1)/2} \\ &= C^{-1/m} ((u^2 + v^2)^{(1/m-1)/2} + (1/m-1)(u^2 + v^2)^{(1/m-1)/2-1} \epsilon(u \Delta u + v \Delta v)) \\ &= \beta^2(u, v) + C^{-1/m} (1/m-1)(u^2 + v^2)^{(1/m-3)/2} \epsilon(u \Delta u + v \Delta v) \end{aligned}$$

The directional derivative is defined as

$$D\beta(\mathbf{v})[\Delta \mathbf{v}_{xy}] = \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} \beta^2(u + \epsilon \Delta u, v + \epsilon \Delta v)$$

and we arrive at³

$$D\beta(\mathbf{v})[\Delta u, \Delta v] = (1/m-1) C^{-1/m} |\mathbf{v}|^{(1-3m)/m} (u \Delta u, v \Delta v)$$

and the directional derivatives of \mathbf{t}_b with respect to u and v are therefore

$$\begin{aligned} D\mathbf{t}_{xb}[\Delta u, \Delta v] &= \beta^2 \Delta u + D\beta^2[\Delta u] u + D\beta^2[\Delta v] u \\ &= \beta^2 \Delta u + (1/m-1) C^{-1/m} |\mathbf{v}|^{(1-3m)/m} (u^2 \Delta u + uv \Delta v) \\ D\mathbf{t}_{yb}[\Delta u, \Delta v] &= \beta^2 \Delta v + (1/m-1) C^{-1/m} |\mathbf{v}|^{(1-3m)/m} (v^2 \Delta v + uv \Delta u) \end{aligned}$$

which can also be written on the form

$$\begin{pmatrix} \Delta t_{xb} \\ \Delta t_{xy} \end{pmatrix} = \mathcal{H}(h - h_f) \begin{pmatrix} \beta^2 + \mathcal{D} u^2 & \mathcal{D} uv \\ \mathcal{D} uv & \beta^2 + \mathcal{D} v^2 \end{pmatrix} \begin{pmatrix} \Delta u \\ \Delta v \end{pmatrix} \quad (6.21)$$

where we have now added back the $\mathcal{H}(h - h_f)$ factor, and where

$$\mathcal{D} := (1/m-1) C^{-1/m} |\mathbf{v}|^{(1-3m)/m}$$

Note that because of the non-linearity of the sliding law, the basal drag term in x direction depends on both u and v and this is reflected in the directional derivatives above.

²If $y \ll x$ then $(x+y)^m \approx x^m + m x^{m-1} y$.

³In 1d we get

$$\begin{aligned} D\beta(\mathbf{v})[\Delta \mathbf{v}] &= (1/m-1) C^{-1/m} |u|^{(1-3m)/m} u \Delta u \\ &= (1/m-1) C^{-1/m} |u|^{(1-2m)/m} \text{sign}(u) \Delta u \end{aligned}$$

In a fully implicit treatment we also must include the effect of grounding/un-grounding the basal drag term, or

$$\begin{aligned}
 Dt_{xb}[\Delta h] &= \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} t_{xb}(h + \epsilon \Delta h) \\
 &= \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} (\mathcal{H}(h + \epsilon \Delta h - h_f) \beta^2 u) \\
 &= \lim_{\epsilon \rightarrow 0} \delta(h + \epsilon \Delta h - h_f) \Delta h \beta^2 u \\
 &= \delta(h - h_f) \beta^2 u \Delta h
 \end{aligned}$$

and therefore

$$\begin{pmatrix} \Delta t_{bx} \\ \Delta t_{by} \end{pmatrix} = \begin{pmatrix} \mathcal{H}(h - h_f)(\beta^2 + \mathcal{D} u^2) & \mathcal{H}(h - h_f) \mathcal{D} uv & \delta(h - h_f) \beta^2 u \\ \mathcal{H}(h - h_f) \mathcal{D} uv & \mathcal{H}(h - h_f)(\beta^2 + \mathcal{D} v^2) & \delta(h - h_f) \beta^2 v \end{pmatrix} \begin{pmatrix} \Delta u \\ \Delta v \\ \Delta h \end{pmatrix} \quad (6.22)$$

where again

$$\mathcal{D} := (1/m - 1) C^{-1/m} |\mathbf{v}|^{(1-3m)/m}$$

Ocean drag term

We add an ocean drag term over the floating section of the form

$$\mathbf{t}_b^o = \mathcal{H}(h_f - h) \beta_o^2 (\mathbf{v}_b - \mathbf{v}_o)$$

with

$$\beta_o^2(u, v) = C_o^{-1/m_o} |\mathbf{v}_b - \mathbf{v}_o|^{1/m_o - 1}$$

The total drag is a sum of that due to basal sliding and ocean currents.

$$\mathbf{t}_b = \mathcal{H}(h - h_f) \beta^2 \mathbf{v} + \mathcal{H}(h_f - h) \beta_o^2 (\mathbf{v} - \mathbf{v}_o)$$

So

$$\begin{aligned}
 t_{bx}^o &= \mathcal{H}(h_f - h) C_o^{-1/m_o} ((u - u_o)^2 + (v - v_o)^2)^{(1-m)/2m} (u - u_o) \\
 t_{yx}^o &= \mathcal{H}(h_f - h) C_o^{-1/m_o} ((u - u_o)^2 + (v - v_o)^2)^{(1-m)/2m} (v - v_o)
 \end{aligned}$$

and hence

$$\begin{aligned}
 Dt_{bx}^o[\Delta u] &= \mathcal{H}(h_f - h) C_o^{-1/m_o} \left(|\mathbf{v}_b - \mathbf{v}_o|^{(1-m_o)/m_o} + (1/m - 1) |\mathbf{v}_b - \mathbf{v}_o|^{(1-3m_o)/m_o} (u - u_o)^2 \right) \Delta u \\
 Dt_{bx}^o[\Delta v] &= \mathcal{H}(h_f - h) C_o^{-1/m_o} (1/m - 1) |\mathbf{v}_b - \mathbf{v}_o|^{(1-3m_o)/m_o} (v - v_o) (u - u_o) \Delta v \\
 Dt_{by}^o[\Delta u] &= \mathcal{H}(h_f - h) C_o^{-1/m_o} (1/m - 1) |\mathbf{v}_b - \mathbf{v}_o|^{(1-3m_o)/m_o} (v - v_o) (u - u_o) \Delta u \\
 Dt_{by}^o[\Delta v] &= \mathcal{H}(h_f - h) C_o^{-1/m_o} \left(|\mathbf{v}_b - \mathbf{v}_o|^{(1-m_o)/m_o} + (1/m - 1) |\mathbf{v}_b - \mathbf{v}_o|^{(1-3m_o)/m_o} (v - v_o)^2 \right) \Delta v \\
 Dt_{bx}^o[\Delta h] &= -\delta(h_f - h) C_o^{-1/m_o} |\mathbf{v}_b - \mathbf{v}_o|^{(1-m_o)/m_o} (u - u_o) \Delta h \\
 Dt_{by}^o[\Delta h] &= -\delta(h_f - h) C_o^{-1/m_o} |\mathbf{v}_b - \mathbf{v}_o|^{(1-m_o)/m_o} (v - v_o) \Delta h
 \end{aligned}$$

6.7.2 Flow law linearisation (2HD)

Using Glen's flow law, deviatoric stresses are related to strain rates through Eq. (1.18), i.e.

$$\tau_{ij} = A^{-1/n} \dot{\epsilon}^{(1-n)/n} \dot{\epsilon}_{ij},$$

where

$$\dot{\epsilon} = \sqrt{\dot{\epsilon}_{ij} \dot{\epsilon}_{ij} / 2}$$

which in the Shallow Ice Stream Approximation takes the form

$$\dot{\epsilon} = \sqrt{(\dot{\epsilon}_{xx})^2 + (\dot{\epsilon}_{yy})^2 + \dot{\epsilon}_{xx} \dot{\epsilon}_{yy} + (\dot{\epsilon}_{xy})^2} \quad (6.23)$$

$$= ((\partial_x u)^2 + (\partial_y v)^2 + \partial_x u \partial_y v + (\partial_x v + \partial_y u)^2/4)^{1/2}. \quad (6.24)$$

If we write

$$\tau_{ij} = 2\eta \dot{\epsilon}_{ij}$$

where η is the effective viscosity given by

$$\eta = \frac{1}{2} A^{-1/n} \dot{\epsilon}^{(1-n)/n}$$

then, in the Shallow Ice Stream Approximation, η is

$$\eta = \frac{1}{2} A^{-1/n} ((\partial_x u)^2 + (\partial_y v)^2 + \partial_x u \partial_y v + (\partial_x v + \partial_y u)^2/4)^{(1-n)/2n}.$$

More generally we can express the stresses as a function of velocities as

$$\tau_{ij} = f(u_q).$$

We now need to linearise each of the stress components with respect to the unknown velocity components u and v velocities. We start with τ_{xx} where we have

$$\tau_{xx} = A^{-1/n} \dot{\epsilon}^{(1-n)/n} \dot{\epsilon}_{xx} \quad (6.25)$$

$$= A^{-1/n} \dot{\epsilon}^{(1-n)/n} \partial_x u \quad (6.26)$$

and we linearise

$$D\tau_{xx}[u; \Delta u] = \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} \tau_{xx}(u; \Delta u) \quad (6.27)$$

$$= \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} (2\eta \partial_x u) \quad (6.28)$$

$$= 2\eta \partial_x \Delta u + (2D\eta[\Delta u]) \partial_x u \quad (6.29)$$

$$\eta = \frac{1}{2} A^{-1/n} ((\partial_x u)^2 + (\partial_y v)^2 + \partial_x u \partial_y v + (\partial_x v + \partial_y u)^2/4)^{(1-n)/2n}$$

$$\begin{aligned} & (\partial_x(u + \Delta u))^2 + (\partial_y(v + \Delta v))^2 + \partial_x(u + \Delta u) \partial_y(v + \Delta v) + (\partial_x(v + \Delta v) + \partial_y(u + \Delta u))^2/4 \\ &= (\partial_x u)^2 + 2\partial_x u \partial_x \Delta u \\ &+ (\partial_y v)^2 + 2\partial_y v \partial_y \Delta v \\ &+ (\partial_x u + \partial_x \Delta u)(\partial_y v + \partial_y \Delta v) \\ &+ (\partial_x v + \partial_x \Delta v + \partial_y u + \partial_y \Delta u)^2/4 \\ &= (\partial_x u)^2 + 2\partial_x u \partial_x \Delta u \\ &+ (\partial_y v)^2 + 2\partial_y v \partial_y \Delta v \\ &+ \partial_x u \partial_y v + \partial_x u \partial_y \Delta v + \partial_y v \partial_x \Delta u \\ &+ (\partial_x v + \partial_y u)/4 + (\partial_x v + \partial_y u)(\partial_x \Delta v + \partial_y \Delta u)/2 \\ &= e^2 + \delta e^2 \end{aligned}$$

where

$$\begin{aligned} e^2 &= (\partial_x u)^2 + (\partial_y v)^2 + \partial_x u \partial_y v + (\partial_x v + \partial_y u)/4 \\ &= \dot{\epsilon}_{xx}^2 + \dot{\epsilon}_{yy}^2 + \dot{\epsilon}_{xx} \dot{\epsilon}_{yy} + \dot{\epsilon}_{xy}^2 \end{aligned}$$

$$\delta e^2 = 2\partial_x u \partial_x \Delta u + 2\partial_y v \partial_y \Delta v + \partial_x u \partial_y \Delta v + \partial_y v \partial_x \Delta u + (\partial_x v + \partial_y u)(\partial_x \Delta v + \partial_y \Delta u)/2$$

or

$$\begin{aligned}\delta e^2 &= (2\partial_x u + \partial_y v) \partial_x \Delta u + (2\partial_y v + \partial_x u) \partial_y \Delta v + \frac{1}{2}(\partial_x v + \partial_y u) (\partial_x \Delta v + \partial_y \Delta u) \\ &= (2\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy}) \partial_x \Delta u + (2\dot{\epsilon}_{yy} + \dot{\epsilon}_{xx}) \partial_y \Delta v + \dot{\epsilon}_{xy} (\partial_x \Delta v + \partial_y \Delta u)\end{aligned}$$

The directional derivative of η is

$$\begin{aligned}D\eta(u, v)[\Delta u, \Delta v] &= E((2\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy}) \partial_x \Delta u + (2\dot{\epsilon}_{yy} + \dot{\epsilon}_{xx}) \partial_y \Delta v + \dot{\epsilon}_{xy} (\partial_x \Delta v + \partial_y \Delta u)) \\ &= E((2\partial_x u + \partial_y v) \partial_x \Delta u + (2\partial_y v + \partial_x u) \partial_y \Delta v + \frac{1}{2}(\partial_x v + \partial_y u) (\partial_x \Delta v + \partial_y \Delta u))\end{aligned}$$

where

$$E := \frac{1-n}{4n} A^{-1/n} e^{(1-3n)/n}$$

which I can also write as

$$D\eta(u, v)[\Delta u, \Delta v] = d_u \eta \Delta u + d_v \eta \Delta v \quad (6.30)$$

where

$$d_u \eta = E((2\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy}) \partial_x + \dot{\epsilon}_{xy} \partial_y)$$

and

$$d_v \eta = E((2\dot{\epsilon}_{yy} + \dot{\epsilon}_{xx}) \partial_y + \dot{\epsilon}_{xy} \partial_x)$$

where

$$E := \frac{1-n}{4n} A^{-1/n} e^{(1-3n)/n}$$

or as

$$D\eta(u, v)[\Delta u, \Delta v] = d_{xu} \partial_x \Delta u + d_{yu} \partial_y \Delta u + d_{yv} \partial_y \Delta v + d_{xv} \partial_x \Delta v \quad (6.31)$$

where

$$d_{xu} = E(2\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy})$$

and

$$d_{yu} = E \dot{\epsilon}_{xy}$$

and

$$d_{yv} = E(2\dot{\epsilon}_{yy} + \dot{\epsilon}_{xx})$$

and

$$d_{xv} = E \dot{\epsilon}_{xy}$$

$$\begin{pmatrix} D\eta_x \\ D\eta_y \end{pmatrix} = \begin{pmatrix} E(2\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy})\partial_x & E\dot{\epsilon}_{xy}\partial_y & 0 \\ E\dot{\epsilon}_{xy}\partial_x & E(\dot{\epsilon}_{xx} + 2\dot{\epsilon}_{yy})\partial_y & 0 \end{pmatrix} \begin{pmatrix} \Delta u \\ \Delta v \\ \Delta h \end{pmatrix} \quad (6.32)$$

In the 1d case we get

$$\frac{1-n}{4n} A^{-1/n} |\partial_x u|^{(1-3n)/n} 2\partial_x u \partial_x \Delta u = \frac{1-n}{2n} A^{-1/n} |\partial_x u|^{(1-2n)/n} \text{sign}(\partial_x u) \partial_x \Delta u$$

6.7.3 Field-equation linearisation

linearising

$$\begin{aligned}\partial_x(4h\eta\partial_x u + 2h\eta\partial_y v) + \partial_y(h\eta(\partial_x v + \partial_y u)) - \beta^2 u \\ = \frac{1}{2}\varrho g \cos \alpha \partial_x h^2 + \rho gh(\partial_x s' \cos \alpha - \sin \alpha)\end{aligned}$$

gives

$$\begin{aligned}\partial_x(4h(\eta\partial_x u + D\eta\partial_x u + \eta\partial_x \Delta u) + 2h(\eta\partial_y v + D\eta\partial_y v + \eta\partial_y \Delta v)) \\ + \partial_y(h(\eta\partial_x v + D\eta\partial_x v + \eta\partial_x \Delta v) + h(\eta\partial_y u + D\eta\partial_y u + \eta\partial_y \Delta u)) \\ - (\beta^2 u + D\beta^2 u + \beta^2 \Delta u) \\ = \frac{1}{2}\varrho g \cos \alpha \partial_x h^2 + \rho gh(\partial_x s' \cos \alpha - \sin \alpha)\end{aligned}$$

or

$$\begin{aligned}
& \partial_x(4h(D\eta \partial_x u + \eta \partial_x \Delta u) + 2h(D\eta \partial_y v + \eta \partial_y \Delta v)) \\
& + \partial_y(h(D\eta \partial_x v + \eta \partial_x \Delta v) + h(D\eta \partial_y u + \eta \partial_y \Delta u)) \\
& - (D\beta^2 u + \beta^2 \Delta u) \\
& = \frac{1}{2} \rho g \cos \alpha \partial_x h^2 + \rho g h (\partial_x s' \cos \alpha - \sin \alpha) - \partial_x(4h\eta(\partial_x u + \partial_y v)) - \partial_y(h\eta(\partial_x v + \partial_y u)) + \beta^2 u
\end{aligned}$$

after having done the partial integration I get within the integral

$$\begin{aligned}
& (4h(D\eta \partial_x u + \eta \partial_x \Delta u) + 2h(D\eta \partial_y v + \eta \partial_y \Delta v)) \partial_x N_i \\
& + h((D\eta \partial_x v + \eta \partial_x \Delta v) + h(D\eta \partial_y u + \eta \partial_y \Delta u)) \partial_y N_i \\
& + (D\beta^2 u + \beta^2 \Delta u) N_i \\
& = + \frac{1}{2} \rho g \cos \alpha \partial_x h^2 \partial_x N_i - \rho g h (\partial_x s' \cos \alpha - \sin \alpha) N_i - 4h\eta(\partial_x u + 2\partial_y v) \partial_x N_i - h\eta(\partial_x v + \partial_y u) \partial_y N_i - \beta^2 u N_i
\end{aligned} \tag{6.33}$$

Inserting (6.30)

$$\begin{aligned}
& (4h(\eta \partial_x \Delta u + \partial_x u(d_u \eta \Delta u + d_v \eta \Delta v)) + 2h(\eta \partial_y \Delta v + \partial_y v(d_u \eta \Delta u + d_v \eta \Delta v))) \partial_x N_i \\
& + (h(\eta \partial_x \Delta v + \partial_x v(d_u \eta \Delta u + d_v \eta \Delta v)) + h(\eta \partial_y \Delta u + \partial_y u(d_u \eta \Delta u + d_v \eta \Delta v))) \partial_y N_i \\
& + (\beta^2 \Delta u + u(d_u \beta^2 \Delta u + d_v \beta^2 \Delta v)) N_i \\
& = + \frac{1}{2} \rho g \cos \alpha \partial_x h^2 \partial_x N_i - \rho g h (\partial_x s' \cos \alpha - \sin \alpha) N_i - 4h\eta(\partial_x u + 2\partial_y v) \partial_x N_i - h\eta(\partial_x v + \partial_y u) \partial_y N_i - \beta^2 u N_i
\end{aligned}$$

I take all coefficients in front of Δu and put them in the 11 part of the matrix, and everything in front of Δv and put that in the 12 part of the matrix.

To make this clear insert (6.31) into (6.34)

$$\begin{aligned}
& 4h(\eta \partial_x \Delta u + \partial_x u(d_{xu} \partial_x \Delta u + d_{yu} \partial_y \Delta u + d_{yv} \partial_y \Delta v + d_{xv} \partial_x \Delta v)) \partial_x N_i \\
& + 2h(\eta \partial_y \Delta v + \partial_y v(d_{xu} \partial_x \Delta u + d_{yu} \partial_y \Delta u + d_{yv} \partial_y \Delta v + d_{xv} \partial_x \Delta v)) \partial_x N_i \\
& + h(\eta \partial_x \Delta v + \partial_x v(d_{xu} \partial_x \Delta u + d_{yu} \partial_y \Delta u + d_{yv} \partial_y \Delta v + d_{xv} \partial_x \Delta v)) \partial_y N_i \\
& + h(\eta \partial_y \Delta u + \partial_y u(d_{xu} \partial_x \Delta u + d_{yu} \partial_y \Delta u + d_{yv} \partial_y \Delta v + d_{xv} \partial_x \Delta v)) \partial_y N_i \\
& + (\beta^2 \Delta u + u(d_u \beta^2 \Delta u + d_v \beta^2 \Delta v)) N_i \\
& = + \frac{1}{2} \rho g \cos \alpha \partial_x h^2 \partial_x N_i - \rho g h (\partial_x s' \cos \alpha - \sin \alpha) N_i - 4h\eta(\partial_x u + 2\partial_y v) \partial_x N_i - h\eta(\partial_x v + \partial_y u) \partial_y N_i - \beta^2 u N_i
\end{aligned}$$

We have $\Delta u = N_j \Delta u_j$ and $\Delta v = N_j \delta v_j$

$$\begin{pmatrix} K_{11} & K_{12} \\ K_{21} & K_{22} \end{pmatrix} \begin{pmatrix} \Delta u_j \\ \Delta v_j \end{pmatrix}$$

$$\begin{aligned}
[K_{12}]_{ij} &= h\eta \partial_x N_j \partial_y N_i \\
&+ 4h\partial_x u d_{yv} \partial_y N_j \partial_x N_i + 4h\partial_x u d_{xv} \partial_x N_j \partial_x N_i \\
&+ 2h\partial_y v d_{yv} \partial_y N_j \partial_x N_i + 2h\partial_y v d_{xv} \partial_x N_j \partial_x N_i \\
&+ h\partial_x v d_{yv} \partial_y N_j \partial_y N_i + h\partial_x v d_{xv} \partial_x N_j \partial_y N_i \\
&+ h\partial_y u d_{yv} \partial_y N_j \partial_y N_i + h\partial_y u d_{xv} \partial_x N_j \partial_y N_i \\
&= h\eta \partial_x N_j \partial_y N_i \\
&+ (4h\partial_x u d_{xv} + 2h\partial_y v d_{xv}) \partial_x N_j \partial_x N_i \\
&+ (h\partial_x v d_{yv} + h\partial_y u d_{yv}) \partial_y N_j \partial_y N_i \\
&+ (h\partial_x v d_{xv} + h\partial_y u d_{xv}) \partial_x N_j \partial_y N_i \\
&+ (4h\partial_x u d_{yv} + 2h\partial_y v d_{yv}) \partial_y N_j \partial_x N_i \\
&= h\eta \partial_x N_j \partial_y N_i \\
&+ 2Eh(2\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy})\dot{\epsilon}_{xy} \partial_x N_j \partial_x N_i \\
&+ Eh2\dot{\epsilon}_{xy}(2\dot{\epsilon}_{yy} + \dot{\epsilon}_{xx}) \partial_y N_j \partial_y N_i \\
&+ Eh2\dot{\epsilon}_{xy}\dot{\epsilon}_{xy} \partial_x N_j \partial_y N_i \\
&+ 2Eh(2\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy})(2\dot{\epsilon}_{yy} + \dot{\epsilon}_{xx}) \partial_y N_j \partial_x N_i
\end{aligned}$$

If we swap u and v and x and y and then i and j (transpose) we get the same matrix, hence

$$K_{12} = K'_{21}$$

$$\begin{aligned}
[K_{11}]_{ij} &= 4h(\eta \partial_x N_j + \partial_x u(d_{xu} \partial_x N_j + d_{yu} \partial_y N_j)) \partial_x N_i \\
&\quad + 2h \partial_y v(d_{xu} \partial_x N_j + d_{yu} \partial_y N_j) \partial_x N_i \\
&\quad + h(\partial_x v(d_{xu} \partial_x N_j + d_{yu} \partial_y N_j) \partial_y N_i \\
&\quad + h(\eta \partial_y N_j + \partial_y u(d_{xu} \partial_x N_j + d_{yu} \partial_y N_j)) \partial_y N_i \\
&= h(4\eta + (4\partial_x u + 2\partial_y v)d_{xu}) \partial_x N_j \partial_x N_i \\
&\quad + h(\eta + (\partial_y u + \partial_x v)d_{yu}) \partial_y N_j \partial_y N_i \\
&\quad + h(4\partial_x u + 2\partial_y v)d_{yu} \partial_y N_j \partial_x N_i \\
&\quad + h(\partial_x v + \partial_y u)d_{xu} \partial_x N_j \partial_y N_i \\
&= h(4\eta + E2(2\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy})(2\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy})) \partial_x N_j \partial_x N_i \\
&\quad + h(\eta + 2E\dot{\epsilon}_{xy}) \partial_y N_j \partial_y N_i \\
&\quad + Eh2(2\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy})\dot{\epsilon}_{xy} \partial_y N_j \partial_x N_i \\
&\quad + Eh2\dot{\epsilon}_{xy}(2\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy}) \partial_x N_j \partial_y N_i \\
&= 4h\eta \partial_x N_j \partial_x N_i + h\eta \partial_y N_j \partial_y N_i \\
&\quad + 2hE(2\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy})^2 \partial_x N_j \partial_x N_i \\
&\quad + 2Eh\dot{\epsilon}_{xy}^2 \partial_y N_j \partial_y N_i \\
&\quad + 2Eh(2\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy})\dot{\epsilon}_{xy}(\partial_y N_j \partial_x N_i + \partial_x N_j \partial_y N_i)
\end{aligned}$$

so K_{11} and K_{22} are symmetrical.

One might expect that the $u d_u \beta^2 \Delta v$ makes the matrix unsymmetrical, but in fact

$$u d_v \beta^2 = u(1/m - 1)C^{-1/m}|\mathbf{v}|^{(1-3m)/m}v = v(1/m - 1)C^{-1/m}|\mathbf{v}|^{(1-3m)/m}u = v d_u \beta^2$$

so the contributions to 12 and 21 are equal, and hence this term does not give rise to an unsymmetrical matrix.

The tangent matrix K is symmetrical for non-linear flow including both the non-linear effects of β^2 and η .

Note: In 1D I get

$$\begin{aligned}
&4h(D\eta \partial_x u + \eta \partial_x \Delta u) \partial_x N_i + (D\beta^2 u + \beta^2 \Delta u) N_i \\
&= +\frac{1}{2} \rho g \cos \alpha \partial_x h^2 \partial_x N_i - \rho g h(\partial_x s' \cos \alpha - \sin \alpha) N_i - 4h\eta \partial_x u \partial_x N - \beta^2 u N
\end{aligned}$$

6.8 Weak form

x direction

$$\int_{\Omega} (\partial_x (4h\eta \partial_x u + 2h\eta \partial_y v) + \partial_y (h\eta (\partial_x v + \partial_y u)) - t_{bx} - \rho g h(\partial_x s \cos \alpha - \sin \alpha)) N dx dy = 0$$

with von Neumann BC on Γ_2

$$(4h\eta \partial_x u + 2h\eta \partial_y v) n_x + \eta h(\partial_x v + \partial_y u) n_y = \frac{1}{2} \rho g h(h - H) n_x$$

and

$$\eta h(\partial_x v + \partial_y u) n_x + (4h\eta \partial_y v + 2\eta h \partial_x u) n_y = \frac{1}{2} \rho g h(h - H) n_y$$

Green's theorem used to get rid of second derivatives gives

$$\begin{aligned}
& - \int_{\Omega} ((4h\eta \partial_x u + 2h\eta \partial_y v) \partial_x N + h\eta (\partial_x v + \partial_y u) \partial_y w) dx dy \\
& - \int_{\Omega} (t_{bx} + \rho g h(\partial_x s \cos \alpha - \sin \alpha) N) dx dy + \int_{\Gamma} ((4h\eta \partial_x u + 2h\eta \partial_y v) n_x + h\eta (\partial_x v + \partial_y u) n_y) N d\Gamma = 0
\end{aligned}$$

Using the BC we have

$$\begin{aligned} & - \int_{\Omega} ((4h\eta\partial_x u + 2h\eta\partial_y v)\partial_x N + h\eta(\partial_x v + \partial_y u)\partial_y w) dx dy \\ & - \int_{\Omega} (t_{bx} + \rho gh(\partial_x s \cos \alpha - \sin \alpha))N dx dy + \int_{\Gamma_2} \frac{1}{2} g\rho(1 - \rho/\rho_o)h^2 n_x N d\Gamma = 0 \end{aligned}$$

y direction

$$\int_{\Omega} \partial_y ((4h\eta\partial_y v + 2h\eta\partial_x u) + \partial_x (h\eta(\partial_y u + \partial_x v)) - t_{by} - \rho gh\partial_y s \cos \alpha)N dx dy$$

Green's

$$\begin{aligned} & - \int_{\Omega} ((4h\eta\partial_y v + 2h\eta\partial_x u)\partial_y N + h\eta(\partial_y u + \partial_x v)\partial_x w) dx dy \\ & - \int_{\Omega} (t_{by} + \rho gh\partial_y s \cos \alpha)N dx dy \\ & + \int_{\Gamma} ((4h\eta\partial_y v + 2h\eta\partial_x u)n_y + \eta h(\partial_y u + \partial_x v)n_x)N d\Gamma \end{aligned} \quad (6.35)$$

the von Neumann BC is

$$\eta h(\partial_x v + \partial_y u)n_x + (4h\eta\partial_y v + 2h\eta\partial_x u)n_y = \frac{1}{2}\rho gh(h - H)n_y$$

hence

$$\begin{aligned} & - \int_{\Omega} ((4h\eta\partial_y v + 2h\eta\partial_x u)\partial_y N + h\eta(\partial_y u + \partial_x v)\partial_x N) dx dy \\ & - \int_{\Omega} (t_{by} + \rho gh\partial_y s \cos \alpha)N dx dy + \int_{\Gamma_2} \frac{1}{2} g\rho(1 - \rho/\rho_o)h^2 n_y w d\Gamma = 0 \end{aligned}$$

Ice shelf

$$\int_{\Omega} (\partial_x (4h\eta\partial_x u + 2h\eta\partial_y v) + \partial_y (h\eta(\partial_x v + \partial_y u)) - t_{bx} - \frac{1}{2}\rho(1 - \rho/\rho_o)g\partial_x h^2 N) dx dy = 0$$

On Γ_2 we write the von Neumann BC as

$$(4h\eta\partial_x u + 2h\eta\partial_y v)n_x + \eta h(\partial_x v + \partial_y u)n_y = \frac{1}{2}\rho(1 - \rho/\rho_o)gh^2 n_x$$

and

$$\eta(\partial_x v + \partial_y u)n_x + (4h\eta\partial_y v + 2h\eta\partial_x u)n_y = \frac{1}{2}\rho(1 - \rho/\rho_o)gh^2 n_y$$

we consider the term

$$\int_{\Omega} (\partial_x (4h\eta\partial_x u + 2h\eta\partial_y v) + \partial_y (h\eta(\partial_x v + \partial_y u)) - \frac{1}{2}\rho(1 - \rho/\rho_o)g\partial_x h^2)N dx dy = \quad (6.36)$$

$$- \int_{\Omega} (4h\eta\partial_x u + 2h\eta\partial_y v + h\eta(\partial_x v + \partial_y u) - \frac{1}{2}\rho(1 - \rho/\rho_o)gh^2)\partial_x N dx dy \quad (6.37)$$

$$+ \int_{\Gamma} ((4h\eta\partial_x u + 2h\eta\partial_y v)n_x + h\eta(\partial_x v + \partial_y u)n_y - \frac{1}{2}\rho(1 - \rho/\rho_o)gh^2 n_x)N d\Gamma \quad (6.38)$$

Along Γ_2 , the path integral disappears and along Γ_1 we set $w^* = 0$, hence

$$- \int_{\Omega} (4h\eta\partial_x u + 2h\eta\partial_y v + h\eta(\partial_x v + \partial_y u) - \frac{1}{2}\rho(1 - \rho/\rho_o)gh^2)\partial_x w - t_{bx}N) dx dy = 0 \quad (6.39)$$

6.9 Thoughts about ice shelf von Neumann BC

6.9.1 1d case

Field equation:

$$4\partial_x(\eta h \partial_x u) - t_x - \rho g h \partial_x s \cos \alpha + \rho g h \sin \alpha = 0$$

Boundary condition

$$4\eta \partial_x u = \frac{1}{2} \rho g (1 - \rho/\rho_o) h \quad (6.40)$$

which for $\eta = \frac{1}{2} A^{-1/n} |\partial_x u|^{(1-n)/n}$ can also be written as

$$\partial_x u = A(\rho g f/4)^n$$

where $f = (1 - \rho/\rho_o)h(x_c)$, and x_c is the location of the calving front. We write the field equation as

$$4\partial_x(\eta h \partial_x u) - \beta^2 u - \rho g h \partial_x (s' + (1 - \rho/\rho_o)h) \cos \alpha + \rho g h \sin \alpha = 0$$

with

$$s' := f - (1 - \rho/\rho_o)h = s - S - (1 - \rho/\rho_o)h$$

or as

$$4\partial_x(\eta h \partial_x u) - \frac{1}{2} g \cos \alpha \rho (1 - \rho/\rho_o) \partial_x h^2 - \rho g h \cos \alpha \partial_x s' - \beta^2 u + \rho g h \sin \alpha = 0,$$

using $\partial_x S = 0$. Here S is the elevation of sea level (usually the coordinate system would be defined so that $S = 0$), and s the surface elevation of the upper ice surface.

When deriving the weak form we do integration by terms on the first two terms

$$\begin{aligned} & \int (4\partial_x(\eta h \partial_x u) - \frac{1}{2} g \cos \alpha \rho (1 - \rho/\rho_o) \partial_x h^2 - \rho g h \cos \alpha \partial_x s' - t_x + \rho g h \sin \alpha) N \, dx \\ &= (4\eta h \partial_x u - \frac{1}{2} g \cos \alpha \rho (1 - \rho/\rho_o) h^2) N \Big|_{x_1}^{x_2} \\ &- \int (4\eta h \partial_x u - \frac{1}{2} g \cos \alpha \rho (1 - \rho/\rho_o) h^2) \partial_x N \, dx \\ &- \int (\rho g \cos \alpha h \partial_x s' + t_x - \rho g h \sin \alpha) N \, dx \end{aligned}$$

The neat thing about this formulation is that for the usual BC at the ice-shelf edge, the ‘boundary integral term’ is zero.

The quantity s' is the difference between the actual surface altitude above sea level, and the surface altitude above sea level if floating. On a floating ice shelf s' is equal to zero everywhere.

If all von Neumann boundary conditions are of the type (6.40) we only have to solve

$$\int ((-4\eta h \partial_x u + \frac{1}{2} g \cos \alpha \rho (1 - \rho/\rho_o) h^2) \partial_x w - (\rho g \cos \alpha h \partial_x s' + t_x - \rho g h \sin \alpha) N) \, dx = 0$$

with

$$s' = s - S - (1 - \rho/\rho_o)h$$

or

$$- \int 4\eta h \partial_x u \, \partial_x N \, dx - \int t_x N \, dx = \rho g \cos \alpha \int h \partial_x s' N \, dx - \frac{1}{2} \rho g \cos \alpha (1 - \rho/\rho_o) \int h^2 \partial_x N \, dx - \rho g \sin \alpha \int h N \, dx$$

Part II

Glacier mechanics

Here I've put some rather random bits related to glacier mechanics. The selection is based both on what many *Ua* users might find useful, but also reflects somewhat the topics I've covered in previous lectures that I've given on glacier mechanics, especially lectures given at Caltech in 2014.

Chapter 7

An ice shelf in one horizontal dimension (1HD).

We consider an ice shelf in one horizontal dimension (1HD) under plain-strain conditions

$$\dot{\epsilon}_{yy} = 0,$$

and in addition we assume $v = 0$, and that all other transverse gradients to be zero as well. This ice shelf is laterally confined, i.e. it can not spread out in y direction, but there is no friction along the sides. It is free to deform in x direction only.

The vertical stress is, as always in the SSA approximation, given by

$$\sigma_{zz} = -\rho g(s - z), \quad (7.1)$$

where s is the upper surface. The traction at the lower surface, where $z = b$, must equal the ocean pressure giving

$$\rho(s - b) = \rho_o(S - b),$$

from which various other floating relationships follow:

$$h = \rho_o d / \rho = \frac{s - S}{1 - \rho / \rho_o} = \frac{\rho_o}{\rho} (S - b), \quad (7.2)$$

$$b = \frac{\rho s - \rho_o S}{\rho - \rho_o} = S - \frac{\rho}{\rho_o} h, \quad (7.3)$$

$$s = S + (1 - \rho / \rho_o) h = (1 - \rho_o / \rho) b + \frac{\rho_o}{\rho} S, \quad (7.4)$$

$$f = (1 - \rho / \rho_o) h. \quad (7.5)$$

Note that we write $d = S - b$ for the ice shelf draft, $f = S - s$ for the freeboard, and $h = s - b$ and $H = S - B$.

The SSTREAM/SSA equation to be solved is in this case simply

$$4\partial_x(h\eta\partial_x u) = \rho g(1 - \rho / \rho_o) h \partial_x h, \quad (7.6)$$

where the effective viscosity η is given by

$$\eta = \frac{1}{2} A^{-1/n} |\partial_x u|^{(1-n)/n}.$$

Eq. (7.6) is the vertical integrated form of the momentum equation in x direction, and reflects the equilibrium of vertically integrated forces in the horizontal direction. The equation can also be written in terms of the horizontal deviatoric stress τ_{xx} as

$$2\partial_x(h\tau_{xx}) = \rho g(1 - \rho / \rho_o) h \partial_x h, \quad (7.7)$$

or as

$$\partial_x(h\tau_{xx}) = \frac{1}{4} g \rho \partial_x h^2, \quad (7.8)$$

where

$$\varrho = \rho(1 - \rho/\rho_o).$$

If ϱ is independent of x (an assumption that has already been made in the derivation of Eq. 7.6) then we can integrate (7.8) on both sides, and we find that

$$\tau_{xx} = \frac{1}{4}g\varrho h + K, \quad (7.9)$$

where K is independent of x .

Note that the freeboard $f = s - S$ is equal to

$$f = (1 - \rho/\rho_o)h,$$

so we can also write (7.9) as,

$$\tau_{xx} = \frac{1}{4}g\rho f + K. \quad (7.10)$$

The integration constant K follows from the boundary conditions, and we will show below that $K = 0$.

Eq. (7.10) shows that the horizontal deviatoric stresses are directly proportional to the freeboard. In some ways $g\rho f/4$ is the ‘driving stress’ that drives the deformation of the ice shelf, and it plays a similar role to the driving stress $\rho gh \partial_x s$ in the SIA.

Knowing τ_{xx} and σ_{zz} , we can now determine the pressure p from

$$p = \tau_{zz} - \sigma_{zz} = -\tau_{xx} - \sigma_{zz}, \quad (7.11)$$

using the incompressibility condition $\dot{\epsilon}_{xx} + \dot{\epsilon}_{zz} = 0$ and the flow law. The horizontal stress σ_{xx} is then

$$\sigma_{xx} = \tau_{xx} - p = \tau_{xx} - (-\tau_{xx} - \sigma_{zz}) = 2\tau_{xx} + \sigma_{zz}, \quad (7.12)$$

Once we have determined all stresses, we can determine the deformation using the Glen-Steinemann flow law

$$\dot{\epsilon}_{ij} = A\tau^{n-1}\tau_{ij}, \quad (7.13)$$

where A and n are some rheological parameters, and τ is the second invariant of the deviatoric stress tensor given by

$$\tau = (\tau_{pq}\tau_{pq}/2)^{1/2}. \quad (7.14)$$

7.1 Boundary condition at the calving front

Across the (vertical) interface between ice and ocean the traction must be continuous, i.e.

$$(\sigma_{\text{ice shelf}} - \sigma_{\text{ocean}})\hat{\mathbf{n}} = 0,$$

where $\hat{\mathbf{n}}$ is a unit normal vector pointing horizontally outwards from the calving front. The position of the calving front is given by $x = x_c$.

We ignore bending forces at the calving front and only require continuity of integrated values. Hence we require

$$\underbrace{\int_b^s \sigma_{xx} dz}_{\text{ice shelf}} = \underbrace{\int_b^S \sigma_{xx} dx}_{\text{ocean}} \quad (7.15)$$

The vertically integrated force of the ocean on the ice at $x = x_c$ is given by the integral

$$\underbrace{\int_b^s \sigma_{xx} dx}_{\text{ocean}} = - \int_b^S \rho_o g (S - z) dz = -\frac{1}{2}\rho_o g (S - b)^2, \quad (7.16)$$

where S is the ocean surface.

This integrated force must equal the vertical integrated horizontal stress within the ice shelf along the calving front given by the integral

$$\int_b^s \sigma_{xx} dz,$$

at $x = x_c$. We know σ_{zz} within the ice shelf from Eq. (10.51). Using Eq. (7.12) to express σ_{xx} in terms of σ_{zz} and σ_{xx} , we find

$$\begin{aligned}
 \underbrace{\int_b^s \sigma_{xx} dz}_{\text{ice shelf}} &= \int_b^s (2\tau_{xx} + \sigma_{zz}) dz \\
 &= 2h\tau_{xx} + \rho g \int_b^s (z - s) dz \\
 &= 2h\tau_{xx} + \rho g ((s^2 - b^2)/2 - s(s - b)) \\
 &= 2h\tau_{xx} - \frac{1}{2}\rho gh^2,
 \end{aligned} \tag{7.17}$$

where we have made use of the fact that τ_{xx} is independent of z . Hence, equality of vertical integrated horizontal stresses at the calving front requires

$$2h\tau_{xx} - \frac{1}{2}\rho gh^2 = -\frac{1}{2}\rho_o g(S - b)^2, \tag{7.18}$$

Using Eq. (7.18) together with the floating condition (10.81) we find

$$\begin{aligned}
 2h\tau_{xx} &= \frac{1}{2}\rho gh^2 - \frac{1}{2}\rho_o g(S - b)^2 \\
 &= \frac{1}{2}\rho gh^2 - \frac{1}{2}\rho_o gh(S - b)\frac{\rho}{\rho_o} \\
 &= \frac{1}{2}\rho gh(h - S + b) \\
 &= \frac{1}{2}\rho gh(s - S)
 \end{aligned}$$

or

$$\tau_{xx} = \frac{1}{4}\rho gf, \tag{7.19}$$

at the calving front where $x = x_c$

Comparing the above boundary condition, valid at the calving front, with expression (7.9), valid anywhere within the ice shelf, we find that integration constant K in Eq. (7.9) is equal to zero, and therefore

$$\tau_{xx} = \frac{1}{4}\rho gf,$$

everywhere.

Note that the (horizontal) boundary condition (7.19) is now identical to the expression for horizontal deviatoric stresses valid everywhere within the ice shelf. Physically this implies that at any given location within the ice shelf, the stresses are identical to stresses imposed by the calving-front boundary condition at that location. In other words, if we were to cut off the ice shelf at any given location, thereby forming a new calving front, the stresses at that newly formed calving front will not be affected. In this respect, the ice shelf downstream of a given point is ‘passive’ and does not affect the flow in upstream direction. In particular, the stresses in the ice shelf downstream from the grounding line do not affect the stresses in the ice shelf at the grounding line.

7.2 The SSA as an expression of horizontal force balance.

Inserting the Glen-Steinemann flow law directly into the boundary condition (7.19) gives

$$A^{-1/n} |\partial_x u|^{(1-n)/n} \partial_x u = \frac{1}{4}\rho gf$$

Since $(s - S) \geq 0$, both τ_{xx} and $\partial_x u$ are positive, and Eq. (7.19) can be written on the form

$$\dot{\epsilon}_{xx} = \partial_x u = A(\rho gf/4)^n, \tag{7.20}$$

at $x = x_c$. Further versions of Eq. (7.19) are

$$2A^{-1/n} h |\partial_x u|^{(1-n)/n} \partial_x u = \frac{1}{2}\rho g(1 - \rho/\rho_o)h^2 \tag{7.21}$$

and

$$8\eta h \partial_x u - \rho g(1 - \rho/\rho_o)h^2 = 0, \quad (7.22)$$

at $x = x_c$.

Note that if we differentiate Eq. (7.22) with respect to x , we find

$$4\partial_x(\eta h \partial_x u) = \rho g(1 - \rho/\rho_o)h \partial_x h. \quad (7.23)$$

which is formally identical to Eq. (7.6) valid for any x . We arrived at (7.23) by considering the vertical integrated force balance along the calving front using 1) the fact that the effective stress τ_{xx} is independent of z , and 2) that the vertical stress component σ_{zz} is equal to the weight of the ice (Eq. 10.51). By simply making these two assumptions, rather than deriving these facts through scaling analysis as we have done, we would have been able to derive Eq. (7.6) in a fairly simple manner.

7.3 Stresses and strains within a one-dimensional plane-strain ice shelf

We now know that

$$\tau_{xx} = \frac{1}{4}\rho g f, \quad (7.24)$$

everywhere within a 1HD ice shelf.

The only non-zero terms of the deviatoric stress tensor are τ_{xx} and τ_{zz} . From (10.28) we find that

$$\tau = |\tau_{xx}| = \frac{1}{4}\rho g f.$$

and using the Glen-Steinmann flow law that

$$\dot{\epsilon}_{xx} = \partial_x u = A(\rho g f/4)^n, \quad (7.25)$$

or alternatively using (7.5)

$$\dot{\epsilon}_{xx} = \partial_x u = A(\varrho g h/4)^n, \quad (7.26)$$

where

$$\varrho = \rho(1 - \rho/\rho_o).$$

The horizontal stress component can then be calculated as

$$\sigma_{xx} = 2\tau_{xx} + \sigma_{zz} = \frac{1}{2}\rho g f + (z - s)\rho g \quad (7.27)$$

where we have used Eq. (10.51). As can be seen σ_{xx} varies linearly with depth. Along the upper surface where $z = s$, $\sigma_{xx} = \rho g f/2$ and is positive, and along the lower surface, where $z = b$, $\sigma_{xx} = -\rho(1 - \rho/\rho_o)g h/2$, and is negative.

The pressure is given by

$$p = \tau_{zz} - \sigma_{zz} = -\frac{1}{4}\rho g f + (s - z)\rho g.$$

Note that the pressure is not hydrostatic.

We write the transverse stress component as

$$\sigma_{yy} = \tau_{yy} - p = \tau_{yy} + \sigma_{zz} - \tau_{zz}.$$

The plane strain conditions implies $\tau_{yy} = 0$ and incompressibility $\tau_{zz} = -\tau_{xx}$, hence

$$\sigma_{yy} = \sigma_{zz} + \tau_{xx} = (z - s)\rho g + \rho g f/4.$$

7.4 Shear stress

The shear stress τ_{xz} is a first-order quantity (but its vertical derivative enters the equilibrium equations at zeroth order). From

$$\partial_x \sigma_{xx} + \partial_z \sigma_{xz} = 0.$$

we find

$$\partial_z \sigma_{xz} = -\partial_x \sigma_{xx} = \frac{1}{2} \rho g \partial_x s, \quad (7.28)$$

which shows that σ_{xz} also varies linearly with depth and that

$$\tau_{xz} = \frac{1}{2} \rho g \partial_x s z + K, \quad (7.29)$$

where K is an integration constant.

The boundary condition at the upper surface ($z = s$) is

$$\tau_{xz}(z = s) = \sigma_{xx}(z = s) \partial_x s$$

giving

$$\tau_{xz}(z = s) = \frac{1}{2} \rho g s \partial_x s. \quad (7.30)$$

using (7.27). Similarly the boundary condition at $z = b$ gives,

$$\begin{aligned} \tau_{xz}(z = b) &= \rho g h \partial_x b + \sigma_{xx}(z = b) \partial_x b, \\ &= \rho g h \partial_x b + \frac{1}{2} \rho g f \partial_x b - \rho g h \partial_x b, \\ &= \frac{1}{2} \rho g f \partial_x b. \end{aligned} \quad (7.31)$$

From Eq. (7.30) we find can now determine K in (7.29) and find

$$\tau_{xz} = \frac{1}{2} \rho g \partial_x s (z - S). \quad (7.32)$$

It remains to be seen if this expression is consistent with the other boundary condition (7.31) at the lower surface. Inserting $z = b$ into (7.32) gives

$$\tau_{xz} = \frac{1}{2} \rho g \partial_x s (b - S). \quad (7.33)$$

Using the floating condition one can show that

$$\partial_x s (b - S) = \partial_x b (s - S),$$

and therefore that (7.32) fulfills the lower boundary condition also.

From (7.32) we see that σ_{xz}/τ_{xz} is $O(\delta)$ as expected. In contrast to the other stress terms listed above, the shear stress τ_{xz} is a first-order quantity.

Summarising, the stress tensor in an ice shelf where all transverse gradients are zero ($\partial/\partial y = 0$) and $S = 0$, is given by

$$\boldsymbol{\sigma} = -\rho g \begin{pmatrix} s/2 - z & 0 & -\frac{1}{2} z \partial_x s \\ 0 & 3s/4 - z & 0 \\ -\frac{1}{2} z \partial_x s & 0 & s - z \end{pmatrix}, \quad (7.34)$$

and the pressure by

$$p = \rho g (3s/4 - z),$$

and deviatoric stress tensor

$$\boldsymbol{\tau} = \boldsymbol{\sigma} + \mathbf{1}p,$$

is

$$\boldsymbol{\tau} = \rho g \begin{pmatrix} s/4 & 0 & \frac{1}{2} z \partial_x s \\ 0 & 0 & 0 \\ \frac{1}{2} z \partial_x s & 0 & -s/4 \end{pmatrix}. \quad (7.35)$$

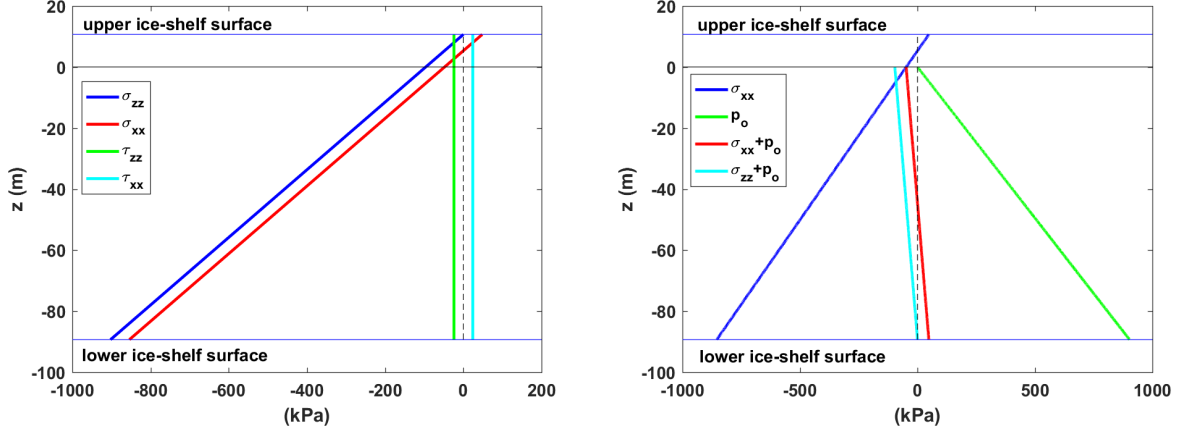


Figure 7.1: Left: Stresses within a one-dimensional ice shelf. Horizontal Cauchy stresses are positive at the surface and negative below $z = d/2$ where d is the ice shelf draft. Horizontal and vertical deviatoric stresses are independent of depth, and horizontal deviatoric stresses positive while vertical deviatoric stresses are negative. Parameters: $\rho = 910 \text{ kg m}^{-3}$, $\rho_o = 1030 \text{ kg m}^{-3}$, $h = 100 \text{ m}$.

Note that the sign of the horizontal stress components (σ_{xx} and σ_{yy}) changes with depth. At the surface ($z = s$) horizontal stresses are positive, at the base ($z = b$) they are negative. The longitudinal horizontal stresses (σ_{xx}) are only positive (compression) for $z > s/2$ (see Fig. 7.1).

The stresses in the shelf given by Eq. (7.34) are valid everywhere within the ice shelf, in particular the stresses in the ice shelf at the grounding line are also given by Eq. (7.34). As is evident from Eq. (7.34) the stresses, and therefore also the strain rates, are at each location functions of local surface slope and local ice thickness only.

Note that it has here been assumed that $S = 0$, in which case $s = f$, so one could replace s with the freeboard (f) in the above expressions for the stresses.

The ocean pressure, p_o is

$$p_o = \rho_o(S - z)$$

for $z < S$, and therefore

$$\sigma_{xx} + p_o = \rho g(s/2 - z) + \rho_o z$$

for $S = 0$. For $z = -\frac{1}{2} \frac{\rho}{\rho_o} h$ the sum of horizontal stresses and ocean pressure is zero (Fig. 7.1).

7.5 Steady-state geometry of a 1HD plane-strain ice sheet

We will now derive an analytical expressions for steady-state geometry and the velocity of a 1HD plane-strain ice sheet. The surface mass-balance is assumed to be constant. This surface mass balance (a) can be thought of as the sum of the mass fluxes along the upper (a_s) and the lower surface (a_b), i.e. $a = a_s + a_b$.

From Eq. (7.26) we have

$$\partial_x u = A(\rho g h/4)^n. \quad (7.36)$$

In a steady state, mass continuity requires

$$\partial_x(uh) = a, \quad (7.37)$$

where we have used that the ice flux q is $q = uh$. Assuming constant accumulation rate we can integrate Eq. (7.37) giving

$$u(x)h(x) - q_{gl} = a(x - x_{gl}) \quad (7.38)$$

where x_{gl} is the grounding line position, and $q_{gl} = q(x_{gl})$ is the flux at the grounding line. We define the origin of the x coordinates so that $x_{gl} = 0$.

We also have from Eq. (7.37)

$$h \partial_x u + u \partial_x h = a \quad (7.39)$$

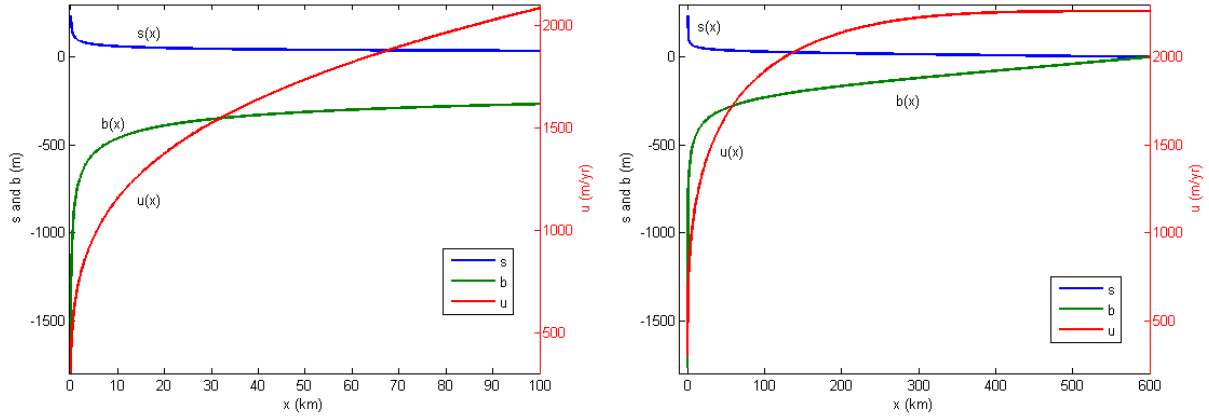


Figure 7.2: Analytical ice shelf profile. The left hand figure is for an accumulation of $a = 0.3 \text{ m a}^{-1}$, while the figure on the right was made for $a = -1 \text{ m a}^{-1}$. All other parameters are the same in both cases. Parameters: $A = 1.14 \times 10^{-9} \text{ kPa}^{-3} \text{ a}^{-1}$, $n = 3$, $h_{gl} = 2000 \text{ m}$, $u_{gl} = 300 \text{ m a}^{-1}$, $\rho = 910 \text{ kg m}^{-3}$, $\rho_o = 1030 \text{ kg m}^{-3}$. The value for A corresponds to an ice temperature of about -10 degrees Celsius.

Replacing u in (7.39) using (7.38) and inserting (7.36) for $\partial_x u$, gives

$$Ah(\varrho gh/4)^n + ((ax + q_{gl})/h)\partial_x h = a, \quad (7.40)$$

which we write as

$$\gamma h^{n+2} + (ax + q_{gl})d_x h = ah, \quad (7.41)$$

with

$$\gamma = A(\varrho g/4)^n.$$

Separating variables

$$\frac{dh}{ah - \gamma h^{n+2}} = \frac{dx}{ax + q_{gl}}, \quad (7.42)$$

integrating both sides and simplifying gives

$$h = \left(\frac{1}{a} \left(\gamma + \frac{K}{(q + ax)^{n+1}} \right) \right)^{-1/(n+1)}, \quad (7.43)$$

where K is an integration constant.

We determine K by specifying the thickness at the grounding line, i.e.

$$h(x_{gl} = 0) = h_{gl},$$

and using $q_{gl} = h_{gl}u_{gl}$ which gives

$$K = q_{gl}^{n+1}(a/h_{gl}^{n+1} - \gamma).$$

The solution is shown in Fig. 7.2. Possibly the most striking aspect of the solution is how quickly the thickness decreases downstream from the grounding line.

Now that the ice geometry has been determined, the ice velocity can be calculated directly from

$$u = (ax + q_{gl})/h.$$

For $a > 0$, the ice sheet is infinitely long and approaches asymptotically the thickness $h(x \rightarrow +\infty) = (a/\gamma)^{1/(n+1)}$. For $a < 0$, the ice shelf has a finite length l given by $l = -q_{gl}/a$.

The special case $a = 0$ is not covered by the above equations. One finds that for $a = 0$ the thickness distribution is given by

$$h = (h_{gl}^{-(n+1)} + \gamma(n+1)x/q_{gl})^{-1/(n+1)}. \quad (7.44)$$

The above solution describes an ice shelf, with a given ice thickness $h = h_{gl}$ at the grounding line, that spreads out in 1HD without any addition or removal of mass. In the limit $x \rightarrow +\infty$ the ice thickness is zero.

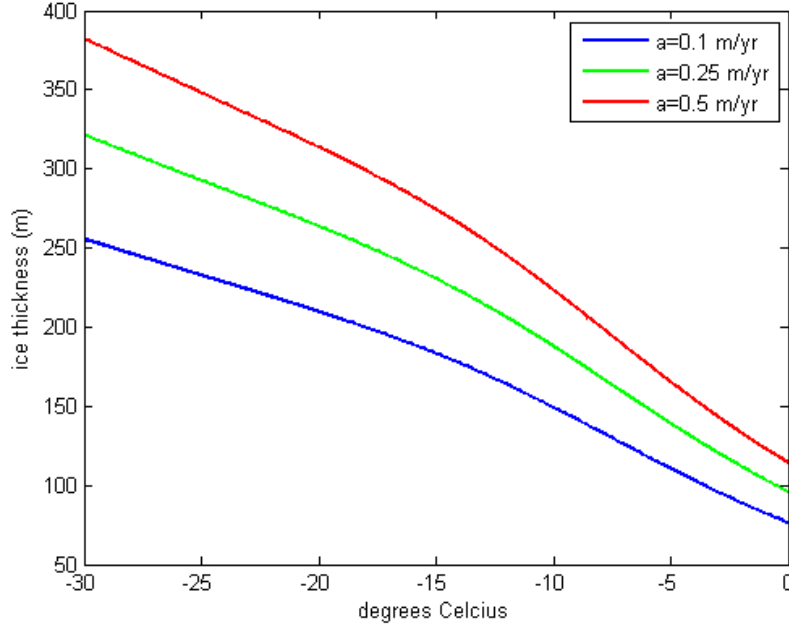


Figure 7.3: Steady-state ice shelf thickness as a function of englacial temperature and surface accumulation. Parameters: $n = 3$, $\rho = 910 \text{ kg m}^{-3}$, $\rho_o = 1030 \text{ kg m}^{-3}$.

Note that in the above analysis we fixed the flux at the grounding line to q_{gl} . We then determined the ice geometry and velocities down-stream of the grounding line. We are here not in a position to calculate the flux at the grounding line for a given ice thickness (or as a function of any other aspects of the ice geometry that might affect the flux). For a given ice thickness $h = h_{gl}$, and a given ocean bathymetry ($B(x)$), we can however always determine possible positions of the grounding line from the floating condition $\rho h = \rho_o H$, where $H = S - B$ with S the ocean surface and B the ocean bed.

Also note that there are two further possible solutions to the differential equation (7.41). There is the (trivial) solution $h = 0$ and also the somewhat more interesting solution $h = (a/\gamma)^{1/(n+1)}$. For $a < 0$ this solution can be discarded (negative thickness). However, if $a > 0$ this solution represents an ice shelf with a constant ice thickness that is regenerated through snow fall at the same rate that it spreads out. The ice thickness of such a steady-state ice shelf is shown in Fig. 7.3 as a function of temperature and surface mass balance.

Another interesting fact is that the ice shelf thickness can *increase* with distance downstream from the grounding line. This happens whenever

$$h_{gl} < (a/\gamma)^{1/(n+1)}.$$

This can be seen from an inspection of the solution shown above, or by writing

$$uh = ax + q_{gl},$$

differentiating and inserting Eq. (7.26), giving

$$u\partial_x h + hA(\rho gh/4)^n = a,$$

and then using $u = (ax + q_{gl})/h$ and solving for $\partial_x h$ to arrive at

$$\partial_x h = \frac{a - \gamma h^{n+1}}{(ax + q_{gl})/h},$$

showing that $\partial_x h$ is positive for $h < (a/\gamma)^{1/(n+1)}$.

7.6 Side-drag dominated ice shelf

We are here interested in the limiting case when all the driving stress is balanced by side drag alone, i.e. the term $\partial_x(h\tau_{xx})$ now dropped. This is the opposite limit to the one we considered above where

$\partial_x(h\tau_{xx})$ was the dominating term and was $\partial_y(h\tau_{xy})$ was ignored.

The shallow-ice stream (SSTREAM/SSA/Shelfy) equations are

$$\partial_x(h(2\tau_{xx} + \tau_{yy})) + \partial_y(h\tau_{xy}) - t_{bx} = \rho gh \partial_x s \quad (7.45)$$

$$\partial_y(h(2\tau_{yy} + \tau_{xx})) + \partial_x(h\tau_{xy}) - t_{by} = \rho gh \partial_y s \quad (7.46)$$

Over the floating section $t_{bx} = t_{by} = 0$. We assume

$$\begin{aligned} \dot{\epsilon}_{yy} &= \tau_{yy} = 0, \\ \partial_y \tau_{xx} &= 0, \\ \partial_y s &= \partial_y h = 0, \end{aligned}$$

and ignore longitudinal stretching in the momentum balance, hence Eqs. (7.45) and (7.46) become

$$\partial_y(h\tau_{xy}) = \rho gh \partial_x h, \quad (7.47)$$

$$\partial_x(h\tau_{xy}) = 0. \quad (7.48)$$

We integrate (7.47) with respect to y from the center-line $y = 0$ to the left-hand margin $y = w$, where the total width of the ice shelf is $2w$, i.e.

$$\tau_m = -\rho gw \partial_x h,$$

where we τ_m is a (positive) shear stress at the margin, i.e. $\tau_m = -\tau_{xy}(y = w)$, and we have used that $\tau_{xy}(y = 0) = 0$. We consider the plastic case when τ_m is a yield stress and independent of ice velocity. This simplification allows us to *calculate the ice thickness directly* as

$$h = h_c - \frac{\tau_m}{\rho gw}(x - x_c),$$

where h_c is the ice thickness at the calving front $x = x_c$. The ice thickness is now a linear function of distance x . For a given location, x_c , of the grounding line, the grounding line will simply be located where the ice draft reaches the ocean floor. In this particular case the mass balance upstream of the grounding line has no impact on the position of the grounding line.

Eq. (7.48) remains correct if evaluated along the centre line where $\tau_{xy} = 0$. However, it is unclear if and how this equation can be consistent with the assumption of constant side shear stress and variable ice thickness. But if the ice behaves, as we assume, as a plastic material, then this poses not problem.

Similar to the case where a plastic formulation for basal sliding is used, the geometry is determined directly from the yield stress.¹ The steady-state velocity can then be calculated from the ice thickness distribution using

$$h \partial_x u + u \partial_x h = 0,$$

giving

$$(\lambda - (x - x_c)) \partial_x u - u = 0,$$

where

$$\lambda = \frac{\rho gw h_c}{\tau_m}.$$

The lengthscale λ is $\rho gh_c / \tau_m$ times the half-width of the ice shelf. I'm guessing a reasonable estimate is $O(\rho gh_c / \tau_m) = 1$ so λ might be similar to the width.

¹For a plastic symmetrical ice sheet on a flat bed

$$\rho gh \partial_x h = \tau_b$$

where τ_b is here the basal yield stress and therefore $\partial_x h^2 = \frac{2\tau_b}{\rho g}$ and hence

$$h^2 = \frac{2\tau_b}{\rho g} |x - x_c|$$

Chapter 8

Simple 1d solution for an icestream

8.1 Problem definition:

Uniform ice thickness h on a constant sloping bed with slope α . The calving front position is at $x = l$. The calving front can be either grounded or floating, and d $0 \leq d < \rho h / \rho_o$.

$$4\partial_x(h\eta\partial_x u) - \beta^2 u = \rho g h \partial_x s$$

with

$$\eta = \frac{1}{2} A^{-1/n} |\partial_x u|^{(1-n)/n}$$

$$\beta^2 = C^{-1/m} |u|^{(1-m)/m}$$

Boundary conditions:

$$u = C \rho g h \alpha \quad \text{at} \quad x = 0 \quad (8.1)$$

$$\tau_{xx} = \frac{1}{4h} g(\rho h^2 - \rho_o d^2) \quad \text{at} \quad x = l \quad (8.2)$$

Boundary condition (8.2) can also be written as

$$\partial_x u|_{x=l} = A \left(\frac{g(\rho h^2 - \rho_o d^2)}{4h} \right)^n$$

8.2 Solution:

The non-linear case is

$$\frac{2hA^{-1/n}}{n} (\partial_x u)^{1/n-1} \partial_{xx}^2 u - C^{-1/m} u^{1/m} = -\rho g h \alpha$$

which I'm not sure if can be solved.

However the linear case

$$\partial_{xx}^2 u - \kappa^2 u = -\frac{A\tau}{2h},$$

has the general solution

$$u = c_1 e^{\kappa x} + c_2 e^{-\kappa x} + C\tau,$$

with

$$\kappa^2 = \frac{A}{2hC},$$

and

$$\tau = \rho g h \alpha$$

BCs (8.1) and (8.2) give

$$c_1 + c_2 + C\tau = C\tau$$

$$c_1 \kappa e^{\kappa l} - c_2 \kappa e^{-\kappa l} = K$$

where

$$K = A \frac{g(\rho h^2 - \rho_o d^2)}{4h}.$$

Hence

$$u = C\tau + \frac{K \sinh \kappa x}{\kappa \cosh \kappa l}.$$

Chapter 9

Grounding-line dynamics

Here some basic aspects of grounding-line dynamics are summarized. This is all well-known stuff from the literature.

9.1 Ice-Shelf Buttressing

Ice-shelf buttressing is defined by the impact of the ice shelf on the stress at the grounding line. If the vertically integrated horizontal stress state is unaffected by the ice shelf — i.e. if removing the ice shelf does not affect the state of stress at the grounding line — the ice-shelf provides no mechanical support to the grounding line beside that of the ocean, and there is no ice-shelf buttressing.

It is sometimes convenient to define a *buttressing parameter* θ as

$$\theta = \frac{N}{\frac{1}{2}\varrho gh}$$

where

$$N = \hat{\mathbf{n}}_h^T \cdot (\mathbf{R}\hat{\mathbf{n}}_h) \quad (9.1)$$

and

$$\varrho = \rho(1 - \rho/\rho_o),$$

and where $\hat{\mathbf{n}}_h$ is a normal vector pointing horizontally outwards from the grounding line. Buttressing is the difference between the normal stress at the grounding line with and without an iceshelf.

In the particular case of a floating ice shelf, the field equations Eq. (10.60) can be written as

$$\nabla_{xy}^T \cdot (h \mathbf{R}) = \frac{1}{2} \nabla_{xy}^T (\varrho g \rho h^2),$$

Using the divergence theorem we find

$$\oint (\mathbf{R} \cdot \hat{\mathbf{n}}_h - \frac{1}{2} \varrho g \rho h \hat{\mathbf{n}}_h) d\Gamma = 0 \quad (9.2)$$

The integrand is identical to the (point wise) expression of the force balance (10.95) at the calving front of a freely floating ice shelf. We can split this path integral into a 1) section along the grounding line, 2) section along the margins, and 3) section along the calving front. If the margins do not contribute, the contribution along the grounding line is equal to that of the calving front. Hence, unbuttressed uniformly-wide ice shelves are passive and don't provide any buttressing.

From Eq. (9.2) it follows that $\theta = 1$ implies no ice-shelf buttressing. This can be taken a bit further by defining normal and tangential buttressing numbers, but the principle is the same: If the ice-shelf does not affect the state of stress along the grounding line, the ice-shelf provides no buttressing.

9.2 Kinematic expression for GL migration

At the grounding line we have

$$\rho h = \rho_o H.$$



Figure 9.1: Geometrical variables: Glacier surface (s), glacier bed (b), ocean surface (S), ocean floor (B), glacier thickness ($h = s - b$), ocean depth ($H = S - B$), glacier draft ($d = S - b$), glacier freeboard ($f = s - S$).

or simply

$$h = h_f$$

At any given time t , this condition must always be fulfilled at the grounding line. Note that $H = S - B$ is independent of time, whereas $h = s - b$ is a function of time and space, i.e.

$$\begin{aligned} h &= h(x, t) \\ H &= H(x) \end{aligned}$$

We consider the rate-of-change as the grounding line is followed, i.e.

$$\left. \frac{dh_{gl}}{dt} \right|_{x=x_{gl}} = \frac{\rho_o}{\rho} \left. \frac{dH_{gl}}{dt} \right|_{x=x_{gl}}, \quad (9.3)$$

where

$$h_{gl} = h_{gl}(t, x_{gl}(t)),$$

and

$$H_{gl} = H_{gl}(x_{gl}(t)),$$

and therefore

$$\frac{dh_{gl}}{dt} = \frac{\partial h_{gl}}{\partial t} + \frac{\partial h_{gl}}{\partial x_{gl}} \frac{\partial x_{gl}}{\partial t}, \quad (9.4)$$

and

$$\frac{dH_{gl}}{dt} = \frac{\partial H}{\partial x_{gl}} \frac{\partial x_{gl}}{\partial t}. \quad (9.5)$$

Inserting (9.4) and (9.5) into (9.3) gives

$$\partial_t h + \dot{x}_{gl} \partial_x h = \frac{\rho_o}{\rho} \dot{x}_{gl} \partial_x H$$

or

$$\dot{x}_{gl} = \frac{\partial_t h}{\frac{\rho_o}{\rho} \partial_x H - \partial_x h} \quad (9.6)$$

$$= \frac{\partial_t h}{\partial_x (h_f - h)} \quad (9.7)$$

where we now have skipped writing the index gl . All derivatives in (9.6) are local derivatives (but it is to be understood that all quantities must be evaluated at the current position of the grounding line).

Eqs. (9.6) and (9.7) are kinematic relationships, and we refer to this equation (9.7) as the *kinematic grounding-line equation*. It contains no additional physics beside that of the floating condition.

At the grounding line $h_f - h = 0$, and with increasing distance directly downstream of the grounding line $h_f - h$ must increase, and hence $\partial_x(h_f - h) \geq 0$. Therefore the grounding line must advance whenever $\partial_t h > 0$.

One issue with using (9.6) is that the gradient in thickness h is typically discontinuous across the grounding line.

9.3 Geometrical grounding-line migration

Changing the sea level (S) causes a shift in the position of the grounding line. This shift is only related to geometrical factors such as ice thickness ($h = s - b$) and ocean depth bedrock ($H = S - B$). To distinguish this shift in grounding-line position from ice-dynamical effects, we refer to this shift as at ‘geometrical grounding-line migration’. This horizontal shift in grounding line position due to time dependent changes in the height of the ocean surface can be determined as follows.

Ocean height (S) changes with time as

$$S(t) = \bar{S} + \Delta S(t),$$

but not spatially (i.e. $\partial_x S(t) = \partial_x \bar{S} = \partial_x \Delta S = 0$)

For $S = \bar{S}$ the grounding line is at $x = x_{gl}$ and

$$\rho(s(x_{gl}) - b(x_{gl})) = \rho_o(\bar{S} - B(x_{gl})). \quad (9.8)$$

For a given perturbation, ΔS , in ocean height, the grounding line moves by some distance ΔL in either up or down-stream direction. At this new grounding line position the floating condition must again hold, and we have

$$\rho(s(x_{gl} + \Delta L) - b(x_{gl} + \Delta L)) = \rho_o(\bar{S} + \Delta S - B(x_{gl} + \Delta L))$$

or

$$\rho(s(x_{gl}) + \partial_x s \Delta L - b(x_{gl}) - \partial_x b \Delta L) = \rho_o(\bar{S} + \Delta S - B(x_{gl}) - \partial_x B \Delta L)$$

(For notational simplicity we have not indicated in the above equation that the derivatives are to be evaluated on both sides of the grounding line and that they are in fact directional derivatives in the up and down-stream directions.) Using (9.8) gives

$$\rho(\partial_x s - \partial_x b) \Delta L = \rho_o(\Delta S - \partial_x B \Delta L)$$

or

$$\Delta L^{+/-} = \frac{\rho_o \Delta S}{\rho(\partial_x s^{+/-} - \partial_x b^{+/-}) + \rho_o \partial_x B^{+/-}} \quad (9.9)$$

Eq. (9.9) is valid even if the derivatives are not constant across the location of grounding line (x_{gl}) at mean tide ($\Delta S = 0$). We have indicated this by adding the superscript $+/-$. Here ΔS is positive for a high tide, and negative for a low tide. At low tide the gradients downstream of x_{gl} are to be used (minus sign), and at high tide the gradients upstream of x_{gl} (positive sign).

If $\partial_x B$ is the same on both sides of the x_{gl} , then it follows from (9.9) that the shifts in grounding line position at high and low tides are only equal in magnitude provided $\partial_x h$ is the same on both sides of $x = x_{gl}$ as well. In other words, for a constant bed slope, a tidally-induced grounding line migration is symmetrical if, and only if, the thickness gradient does not change across the grounding line. Conversely, if the thickness gradients are not equal on both sides of the grounding line, the grounding line movement will always be asymmetrical with respect to the tidal cycle.

Such an asymmetrical grounding line migration takes place if, for example, the upper surface gradient ($\partial_x s$) is constant across the grounding line. In that case the thickness gradient cannot be equal on both sides of $x = x_{gl}$ as well. There is then a break in thickness gradient at $x = x_{gl}$ given by

$$\partial_x h = \begin{cases} \partial_x h^+ = \partial_x s - \partial_x B & \text{for } x \leq x_{gl} \\ \partial_x h^- = \partial_x s / (1 - \rho/\rho_o) & \text{for } x \geq x_{gl} \end{cases}$$

(There is no need to use the superscripts $+/-$ with $\partial_x s$ and $\partial_x B$ in the above equation because here we are assuming that those derivatives are continuous across the grounding line.) When this expression for

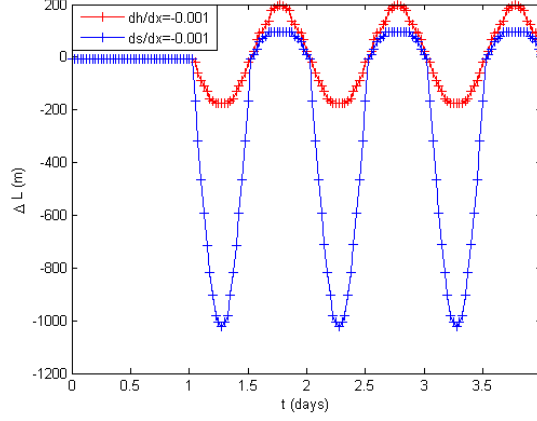


Figure 9.2: Example of grounding line migration in response to tidal forcing using the hydrostatic assumption. The curves were calculated using the flow model $\acute{U}a$ which is a vertically integrated flow model that calculates grounding line positions using the hydrostatic assumption. The red curve was calculated for a constant surface slope of $ds/dx = -0.001$ and blue curve for a constant thickness gradient of $dh/dx = -0.001$. In both cases the bedrock gradient as $dB/dx = -0.01$. The tidal amplitude was 2 m and the tidal period 1 d. To suppress the effects of ice flow the flow parameters were set to values that made the ice effectively rigid and basal sliding was enforced to be close to zero.

the break in thickness gradient is inserted in (9.9) we find that the migration distance ΔL^- at a low tide (when $S = \bar{S} - \Delta L$), is given by

$$\Delta L^- = -\frac{\Delta S}{\frac{\rho/\rho_o}{1-\rho/\rho_o}\partial_x s + \partial_x B},$$

and at a high tide by

$$\Delta L^+ = \frac{\Delta S/(1 - \rho/\rho_o)}{\frac{\rho/\rho_o}{1-\rho/\rho_o}\partial_x s + \partial_x B},$$

and that

$$|\Delta L^-| = (1 - \rho/\rho_o)|\Delta L^+|.$$

In the case of a constant surface gradient, the upstream grounding-line shift at high tide is therefore about 9 times as large as the downstream shift at low tide.

In general we expect neither the thickness nor the surface gradient to be constant across the grounding line. Since the migration is only symmetrical in the particular case of a constant thickness gradient across the grounding line, we expect an asymmetrical grounding line migration in response to tides to be the general rule rather than an exception.

An example of transient hydrostatic grounding-line migration in response to tides is shown in Fig. 9.2. The migration was calculated using the flow model $\acute{U}a$. This model, as do most commonly used flow model in glaciology, assumes that the grounding line is always exactly where the hydrostatic floating condition $\rho h = \rho_o(S - b)$ is met. The modelled grounding line displacements are in a good agreement with those calculated using Eq. (9.9). For example, in the case of constant surface slope the modelled values are $\Delta L^- = 105$ m and $\Delta L^+ = -1013$ while those based on Eq. (9.9) are $\Delta L^- = 109$ m and $\Delta L^+ = -1020$. These differences of a few meters are considerably smaller than the spatial dimension of 85 m of the smallest element of the mesh used in this particular run by the FE-model $\acute{U}a$.

9.4 Flux at the grounding line

Upstream from the grounding line

$$2\partial_x \left(hA^{-1/n} |\partial_x u|^{1/n-1} \partial_x u \right) - C^{-1/m} |u|^{1/m-1} u = \rho g h \partial_x s \quad (9.10)$$

In terms of stresses this equation can also be written as

$$2\partial_x (h\tau_{xx}) - t_{bx} = \rho g h \partial_x s \quad (9.11)$$

Boundary conditions at the grounding line where $x = x_{gl}$ are

$$\partial_x u = A(\varrho gh/4)^n \quad (9.12)$$

$$h = \rho_o H / \rho. \quad (9.13)$$

Note that boundary condition (9.12) can be rearranged as

$$2A^{-1/n}(\partial_x u)^n = \frac{1}{2}\varrho gh$$

If we insert the above expression into (9.10), assuming $\partial_x u > 0$, then we arrive at

$$\frac{1}{2}\varrho g \partial_x h^2 - t_{bx} = \rho gh \partial_x s$$

If we now assume that $\partial_x s \approx \partial_x h$ upstream of the grounding line, for example by setting $s = B + h$ with $\partial_x B = 0$ for $x > x_{gl}$, we obtain

$$[\frac{1}{2}\varrho g \partial_x h^2] - [t_{bx}] = [\frac{1}{2}\rho gh \partial_x h^2] \quad (9.14)$$

where the brackets are used to indicate that we are here simply comparing sizes of terms, and where we have writtin $h \partial_x h = \frac{1}{2} \partial_x h^2$

Since $\varrho \approx \rho/10$ is it clear that the first term on the left-hand side of (9.14) is small compared to the right-hand side, and that the right-hand side must therefore be approximately balanced by the second term on the left-hand side of (9.14), i.e.

$$[t_{bx}] = [-\frac{1}{2}\rho gh \partial_x h^2]. \quad (9.15)$$

Note that the key assumption here is that $\partial_x s \approx \partial_x h$ for $x < x_{gl}$.

Downstream of the grounding line, flotation implies that $\partial_x s = \varrho \partial_x h / \rho$ (see Eq. 10.80) and if, for example, $\partial_x s \approx \varrho \partial_x h / \rho$, upstream of the grounding line we instead of (9.14) arrive at

$$[\frac{1}{2}\varrho gh^2] - [t_{bx}] = [\frac{1}{2}\varrho g \partial_x h^2] \quad (9.16)$$

and clearly now it is the first term on the left-hand side that balances the right-hand side.

We will now derive an approximation for the flux at the grounding line as a function of (local) ice thickness, and start by making the assumption that $\partial_x s \approx \partial_x h$ in which case as we have seen

$$t_{bx} \approx -\rho gh \partial_x s,$$

i.e. that the second term on the left-hand sides of (9.10) and (9.11) is now approximately balanced by their respective right-hand sides. Hence, using Weertman sliding law we have

$$u = C(-\rho gh \partial_x h)^m, \quad (9.17)$$

where we have anticipated that $\partial_x h$ will be strictly negative.

In a steady state

$$\partial_x (uh) = a, \quad (9.18)$$

which allows us to write

$$\partial_x h = (a - h \partial_x u) / u. \quad (9.19)$$

Inserting the boundary condition (9.12) into (9.19) gives

$$\partial_x h = (a - h A (\varrho gh/4)^n) / u, \quad (9.20)$$

and then inserting (9.20) into (9.17) and assuming that $a \ll h A (\varrho gh/4)^n$ gives

$$u = C (\rho gh h A (\rho g \delta gh/4)^n / u)^m. \quad (9.21)$$

or

$$u^{m+1} = 4^{-nm} C A^m (g\rho)^{m+nm} \delta^{nm} h^{nm+2m},$$

where δ is defined as

$$\delta := 1 - \rho/\rho_o$$

The ice flux $q = uh$ at the grounding line is therefore

$$q = [4^{-nm} C A^m (g\rho)^{m+nm} (1 - \rho/\rho_o)^{nm}]^{1/(m+1)} h^{(nm+3m+1)/(m+1)}, \quad (9.22)$$

where¹ h is the thickness at the grounding line, i.e.

$$h = h_{gl} = \rho_o H / \rho.$$

The relationship between flux relationship (9.22) is identical to that of Schoof's 'B' model. We arrived at this flux relation ship by assuming that velocity at the grounding line is given by the SIA relationship, and that $\partial_x u$ at the grounding line is given by the boundary condition (9.12) for horizontal strain rates. In addition we assumed steady-state conditions and that $a \ll h \partial_x u$.

Note that, as Eq. (9.21) shows, it is possible to express the velocity at the grounding line as a function of the ice thickness h alone, i.e. without any reference to the surface slope $\partial_x h$. This is possible because the surface slope at the grounding line is related to thickness through Eq. (9.20). We were able to use the boundary condition (9.12), the mass conservation equation (9.18), and the simplified momentum equation (9.17) to arrive at Eq. (9.21), giving velocity at the grounding-line as a function of thickness alone.

9.5 Balance between terms on both side of the grounding line

(What follows is basically a slightly different framing of the argument in section 9.4 used to show that the basal shear traction will balance the driving stress upstream of the grounding line, provided $\partial_x s \approx \partial_x h$.)

Field equation and boundary condition at the grounding line written in terms of velocity are

$$2A^{-1/n} \partial_x \left(h |\partial_x u|^{(1-n)/n} \partial_x u \right) - \mathcal{H}(h - h_f) C^{-1/m} |u|^{(1-m)/m} u = \rho g h \partial_x s \quad (9.23)$$

$$2A^{-1/n} h |\partial_x u|^{(1-n)/n} \partial_x u = \frac{1}{2} \varrho g h^2 \quad \text{at } x = x_{gl} \quad (9.24)$$

$$h = h_f \quad \text{at } x = x_{gl} \quad (9.25)$$

where \mathcal{H} is the Heaviside step function, and where

$$h_f = \rho_o H / \rho$$

and where $\varrho = \rho(1 - \rho/\rho_o)$.

Inserting (9.24) into (9.23) and assuming that over the grounded area $|\partial_x s| \gg |\partial_x b|$, and therefore that $\partial_x h = \partial_x s - \partial_x b \approx \partial_x s$, we arrive at

$$\frac{1}{2} \varrho g \partial_x (h^2) - \mathcal{H}(h - h_f) C^{-1/m} |u|^{(1-m)/m} u = \rho g h \partial_x h$$

or

$$\frac{1}{2} \varrho g \partial_x h^2 - \mathcal{H}(h - h_f) C^{-1/m} |u|^{(1-m)/m} u = \frac{1}{2} \rho g \partial_x h^2$$

which immediately shows that the first term on the left hand side is $\varrho/\rho = \rho(1 - \rho/\rho_o)/\rho = (1 - \rho/\rho_o) \approx 0.1$ the size of the right-hand side.

Downstream of the grounding line $s = (1 - \rho_o/\rho)h$ and therefore $\partial_x s = \delta \partial_x h \approx 0.1 \partial_x h$, or $\partial_x s \ll \partial_x h$ and this reversal of the relative sizes of the upper and lower slopes ensures that we now also have the right balance downstream of the grounding line, where the first term on the left-hand side now equals the right-hand side.

¹For $q = \rho u h$ and keeping the θ term we have

$$q = \rho \left(4^{-n} C^{1/m} A (\rho g)^{n+1} \delta^n \right)^{m/(1+m)} \theta^{nm/(1+m)} h^{(nm+3m+1)/(1+m)}$$

9.6 GL scalings (Schoof)

Field equations written in terms of velocity and stresses, respectively, are

$$2A^{-1/n} \partial_x \left(h |\partial_x u|^{(1-n)/n} \partial_x u \right) - \mathcal{H}(h - h_f) C^{-1/m} |u|^{(1-m)/m} u = \rho g h \partial_x s \quad (9.1)$$

$$2\partial_x (h \tau_{xx}) - \tau_b = \rho g h \partial_x s \quad (9.2)$$

where \mathcal{H} is the Heaviside step function. Boundary conditions at $x = x_{gl}$ are

$$2A^{-1/n} h |\partial_x u|^{(1-n)/n} \partial_x u = \frac{1}{2} \varrho g h^2 \quad (9.3)$$

$$h = h_f \quad (9.4)$$

where

$$h_f = \rho_o H / \rho$$

and where $\varrho = \rho(1 - \rho/\rho_o)$. The above model is only valid for $[z]/[x] \ll 1$ and $u_d/u_b \ll 1$, where u_d is the deformational velocity and u_b the sliding velocity.

Scalings: With $[u]$ and $[x]$ as scales for the horizontal velocity and the span of the ice sheet, the kinematic boundary condition suggests

$$\frac{[u][z]}{[x]} = [a] \quad \text{and} \quad [t] = \frac{[x]}{[u]}$$

We set a scale for C by writing

$$[u]^{1/m} / [C]^{1/m} = \rho g [z][z]/[x]$$

i.e. we balance basal shear stress with the driving stress. We introduce

$$\epsilon = \frac{\tau_{xx}}{\sigma_{xx}} = \frac{A^{-1/n} ([u]/[x])^{1/n}}{2\rho g [z]} \quad (9.5)$$

and will consider the case $\epsilon \ll 1$, and we also define

$$\delta = 1 - \rho/\rho_o \quad (9.6)$$

where $\delta \approx 0.1$.

Inserting these scalings into field equation (9.1) and the boundary condition (9.3) gives

$$4\epsilon \partial_x \left(h |\partial_x u|^{(1-n)/n} \partial_x u \right) - |u|^{(1-m)/m} u = h \partial_x s \quad \text{for } x < x_{gl} \quad (9.7)$$

$$|\partial_x u|^{(1-n)/n} \partial_x u = \frac{\delta h}{8\epsilon} \quad \text{at } x = x_{gl} \quad (9.8)$$

Summarizing the scaled momentum equations are

$$4\epsilon \partial_x \left(h |\partial_x u|^{(1-n)/n} \partial_x u \right) - |u|^{(1-m)/m} u = h \partial_x s \quad \text{for } x < x_{gl} \quad (9.9)$$

$$|\partial_x u|^{(1-n)/n} \partial_x u = \frac{\delta h}{8\epsilon} \quad \text{at } x = x_{gl} \quad (9.10)$$

Now we consider the scaled momentum equation (9.7) in the vicinity of the grounding line. As we approach $x \rightarrow x_{gl}$ from the upstream side we expect the first term on the left-hand side of (9.7) to be given by (9.8). Inserting (9.8) into (9.7) gives

$$\delta h \partial_x h - |u|^{(1-m)/m} u = h \partial_x s, \quad (9.11)$$

all quantities evaluated at the grounding line.

Now consider the right-hand side of the above equation. We always have

$$\partial_x s = \partial_x (h + b),$$

Upstream of the grounding line we do not expect $\partial_x b$ to be related (at least not in some simple way) to $\partial_x h$. Consider the case $\partial_x b = 0$ while $\partial_x h$ takes some finite value, then equation (9.2) is

$$\delta h \partial_x h - |u|^{(1-m)/m} u = h \partial_x h, \quad (9.12)$$

and since $\delta \ll 1$ (in fact $\delta \approx 0.1$) the second term on the left-hand side approximately balances the right-hand side. We therefore have an approximate balance between basal stress and driving stress.

In physical terms the situation down-stream of the grounding line is clear. There the first term on the left-hand side must balance the term on the right hand side. Here our formulation gives the right balance because since $\partial_x b$ and $\partial_x h$ are related by the floating condition

$$\partial_x s = \partial_x (h + b) = \partial_x (h - (1 - \rho/\rho_o)h) = \delta \partial_x h,$$

and when inserted into

$$\frac{1}{2} \delta \partial_x (hh) = \delta h \partial_x h, \quad (9.13)$$

we arrive, which is the right balance.

Summarizing, if h and b are related through the floating condition, the balance is always between the first lhs term and the rhs, but the balance is between the second rhs term and the lhs if

$$|\partial_x b| \ll |\partial_x h|$$

In the particular case $\partial_x b = 0$ the balance is always between the basal shear stress term and the driving stress.

9.7 Grounding-line instability

For a steady state with the grounding line located at $x = x_{gl}$, mass conservation requires

$$\gamma x_{gl} + q(x_{gl}) = 0$$

where we have assumed that the ice divide is at $x = 0$ and the surface mass balance is γ . We assume the surface mass balance is spatially constant and positive, i.e. $\gamma > 0$. Clearly if $\gamma < 0$, no ice sheet is possible.

Perturb x_{gl} by Δx

$$x'_{gl} = x_{gl} + \Delta x$$

If

$$\gamma \Delta x - \partial_x q \Delta x < 0$$

then more ice flows across the grounding line than is added over the new surface Δx upstream of the grounding line. The volume of the (grounded) ice sheet must decrease with time and the grounding line must retreat towards the original steady state. (Note that the derivative of q with respect to x is the derivative of q following the grounding-line position.)

Hence, if

$$\partial_x q > \gamma$$

then the grounding line is stable, but

$$\begin{aligned} \frac{\partial q}{\partial x} &= \frac{\partial q}{\partial h} \frac{\partial h}{\partial x} \\ &= \frac{\partial q}{\partial h} \frac{\rho_o}{\rho} \frac{\partial (S - B)}{\partial x} \\ &= - \frac{\rho_o}{\rho} \frac{\partial q}{\partial h} \frac{\partial B}{\partial x} \end{aligned}$$

For a prograde bed, $\partial_x B < 0$, and therefore we must have $\partial_h q > 0$ for the grounding line to be stable. Conversely, for a retrograde bed where $\partial_x B > 0$ we must have $\partial_h q < 0$ for the grounding line to be stable.

Chapter 10

Shallow Ice Stream Approximation (SSTREAM/SSA)

10.1 Field equations and boundary conditions

The field equations are

$$v_{i,i} = 0 \quad (\text{mass}) \quad (10.1)$$

$$\sigma_{ki,k} + \rho b_i = 0 \quad (\text{linear momentum}) \quad (10.2)$$

$$\sigma_{ij} - \sigma_{ji} = 0 \quad (\text{angular momentum}) \quad (10.3)$$

where v_i are the components of the velocity vector, σ_{ij} the components of the full stress tensor (i.e. the Cauchy stress tensor), and ρ the ice density.

In addition we have the kinematic boundary conditions

$$\frac{\partial s}{\partial t} + u \frac{\partial s}{\partial x} + v \frac{\partial s}{\partial y} - w = a \quad (10.4)$$

valid at the surface $z = s(x, y)$, where a is the accumulation rate, and we have used u , v , and w to denote the x , y , and z components of the velocity vector, respectively. There is a corresponding equation valid at the glacier sole. At the glacier sole both the accumulation rate and the rate of elevation can often be ignored. The kinematic boundary condition is then usually referred to as the ‘no-penetration condition’.

The relation between strain rates and stresses is taken to be

$$\dot{\epsilon}_{ij} = A(T) \tau^{n-1} \tau_{ij}. \quad (10.5)$$

where A is the rate factor and n the stress exponent. Furthermore, τ_{ij} are the deviatoric stress components

$$\tau_{ij} = \sigma_{ij} - \delta_{ij} \sigma_{kk}/3,$$

$\dot{\epsilon}_{ij}$ are the components of the deformation rate tensor (the stretching tensor), and τ is *effective stress* (the square root of the (negative of the) second invariant of the deviatoric stress tensor), i.e.

$$\tau = \sqrt{\tau_{ij} \tau_{ij} / 2}.$$

Eq. (10.5) is the well-known Glen-Steinemann law (Steinemann, 1954, 1958a,b; Glen, 1955). Outside of glaciology it is better known as the Norton-Hoff rheology model, or simply as power-law rheology. An increasingly popular alternative description of ice rheology can be found in Goldsby and Kohlstedt (2001).

In the situation when the glacier slides over its bed various theoretical arguments show that we have a mixed-type basal boundary condition to consider of the type

$$\mathbf{t}_b = \mathbf{f}(\mathbf{v}_b)$$

where \mathbf{f} is some function, \mathbf{t}_b is the basal stress vector given by

$$\mathbf{t}_b = \sigma \hat{\mathbf{n}} - (\hat{\mathbf{n}}^T \cdot \boldsymbol{\sigma} \hat{\mathbf{n}}) \hat{\mathbf{n}},$$

Third, on ice streams and ice shelves vertical shearing is small compared to longitudinal stretching, and vertical shear stresses small compared to horizontal deviatoric stresses. In particular, one can expect that horizontal strain rates are ‘balanced’ by the horizontal deviatoric stresses.¹ What is meant by ‘balancing’ different quantities will become clear in what follows.

Motivated by these observations we now consider the case of an ice stream with horizontal length scale $[x]$ and vertical length scale $[z]$ where the shallow ice approximation $[z]/[x] = \delta \ll 1$ holds, and write

$$(x, y, z) = [x](x^*, y^*, \delta z^*).$$

where the asterisks denote scaled dimensionless variables.

For the mass conservation equation ($v_{i,i} = 0$) to be invariant we scale the velocity as

$$(u, v, w) = [u](u^*, v^*, \delta w^*). \quad (10.7)$$

If we furthermore require the kinematic boundary condition at the surface

$$\partial_t s + u \partial_x s + v \partial_y s - w = a,$$

where s is the surface to be invariant under the scalings we must have

$$a = \delta [u] a^*,$$

where a is the accumulation rate. Thus the scale for a is $[a] = \delta [u] = [w]$, which seems reasonable as we can expect the vertical velocity to scale with accumulation rate for small surface slopes. We also find, using the same invariant requirement for the surface kinematic boundary condition, that the time must be scaled as

$$t = [x][u]^{-1} t^*.$$

For $[x] \sim 1000$ km, and a vertical dimension of 100 m to 1 km, we have δ in the range of 0.001 to 0.01. Horizontal velocities can be expected to be on the order of a 100 m a^{-1} and w around 0.1 to 1 m a^{-1} , giving the same range of δ . The time scale $[t] = [x][u]^{-1}$ is therefore on the order of 1 to 10 ka.

We assume that the velocity is of same order across the whole ice thickness. In particular we assume that the horizontal components of the basal sliding velocity (u_b and v_b) are of the same order as the surface velocity, i.e.

$$(u_b, v_b) = [u](u_b^*, v_b^*) \quad (10.8)$$

We are considering a situation where the vertical shear components are small compared to all other stress components. A set of scalings for the stresses which reflects this situation is

$$\begin{aligned} &(\sigma_{xx}, \sigma_{yy}, \sigma_{zz}, \tau_{xy}, \tau_{xz}, \tau_{yz}) \\ &= [\sigma](\sigma_{xx}^*, \sigma_{yy}^*, \sigma_{zz}^*, \tau_{xy}^*, \delta \tau_{xz}^*, \delta \tau_{yz}^*). \end{aligned} \quad (10.9)$$

Same scale is used for the pressure, that is $p = [\sigma] p^*$. For the time being, we do not specify how the scale $[\sigma]$ relates to other variables entering the problem.

Note that we are assuming a ratio between vertical and horizontal dimensions equal to that of the vertical and horizontal deviatoric stresses, so for example

$$\delta = \frac{[z]}{[x]} = \frac{[\tau_{xz}]}{[\tau_{xx}]}.$$

In other words, the *aspect ratio*, $[z]/[x]$, is the same as the *stress ratio*, $[\tau_{xz}]/[\tau_{xx}]$.

10.2.2 Scaling the equations

The analysis is done in a coordinate system which is tilted forward in x direction by the angle α . The equilibrium equations are

$$\begin{aligned} \partial_x \sigma_{xx} + \partial_y \tau_{xy} + \partial_z \tau_{xz} &= -\rho g \sin \alpha, \\ \partial_x \tau_{xy} + \partial_y \sigma_{yy} + \partial_z \sigma_{yz} &= 0, \\ \partial_x \tau_{xz} + \partial_y \sigma_{yz} + \partial_z \sigma_{zz} &= \rho g \cos \alpha. \end{aligned}$$

¹Note that this situation contracts sharply with what is found on most alpine glaciers, ice sheets and ice caps, where rates of ice deformation due to shearing dominate horizontal strain rates (except for the top most layer). In this case normal deviatoric stresses are small compared to the shear stress and the normal stress field close to being isotropic.

The above listed scalings give

$$[\sigma][x]^{-1}\partial_x^*\sigma_{xx}^* + [\sigma][x]^{-1}\partial_{y^*}\tau_{xy}^* + [\sigma]\delta[x]^{-1}\delta^{-1}\partial_{z^*}\tau_{xz}^* = -\rho g \sin \alpha,$$

$$[\sigma][x]^{-1}\partial_x^*\tau_{xy}^* + [\sigma][x]^{-1}\partial_{y^*}\sigma_{yy}^* + [\sigma]\delta[x]^{-1}\delta^{-1}\partial_{z^*}\sigma_{yz}^* = 0,$$

$$[\sigma][x]^{-1}\delta\partial_x^*\tau_{xz}^* + [\sigma][x]^{-1}\delta\partial_{y^*}\sigma_{yz}^* + [\sigma][x]^{-1}\delta^{-1}\partial_{z^*}\sigma_{zz}^* = \rho g \cos \alpha,$$

which can be written as

$$\partial_x^*\sigma_{xx}^* + \partial_{y^*}\tau_{xy}^* + \partial_{z^*}\tau_{xz}^* = -\rho g[x][\sigma]^{-1} \sin \alpha, \quad (10.10)$$

$$\partial_x^*\tau_{xy}^* + \partial_{y^*}\sigma_{yy}^* + \partial_{z^*}\sigma_{yz}^* = 0, \quad (10.11)$$

$$\delta^2\partial_x^*\tau_{xz}^* + \delta^2\partial_{y^*}\sigma_{yz}^* + \partial_{z^*}\sigma_{zz}^* = \rho g\delta[x][\sigma]^{-1} \cos \alpha, \quad (10.12)$$

If we now fix the scale $[\sigma]$ for the stresses as

$$[\sigma] = \rho g[z] = \rho g\delta[x], \quad (10.13)$$

we arrive at

$$\partial_x^*\sigma_{xx}^* + \partial_{y^*}\tau_{xy}^* + \partial_{z^*}\tau_{xz}^* = -\delta^{-1} \sin \alpha, \quad (10.14)$$

$$\partial_x^*\tau_{xy}^* + \partial_{y^*}\sigma_{yy}^* + \partial_{z^*}\sigma_{yz}^* = 0, \quad (10.15)$$

$$\delta^2\partial_x^*\tau_{xz}^* + \delta^2\partial_{y^*}\sigma_{yz}^* + \partial_{z^*}\sigma_{zz}^* = \cos \alpha. \quad (10.16)$$

For the two terms on the right-hand side of the above set of equations to be of order unity we must furthermore require

$$\alpha = O(\delta),$$

i.e. the tilt angle α of the coordinate system must be small. Hence, the angle α is not arbitrary. (As we will see below, and as is to be expected, the shear stress τ_{xz} scales with $\rho g[z] \sin \alpha$ so for it to be small in comparison to the stress scale $\rho g[z]$, α must be small.)

Note that the stress scale must be $[\sigma] = \rho g[z] = \rho g\delta[x]$ for the right-hand side term in Eq. (10.12) to be of order unity for $\alpha = 0$. We could have defined this to be the stress scale from the outset, but doing so would have obscured the fact that this stress scale is required for the vertical gradient of the vertical stresses (i.e. $\partial_z\sigma_{zz}$) to be balanced by the vertical component of the body force (i.e. $-\rho g$) for $\alpha = 0$. Furthermore, note that had we defined the stress scale to be the product of thickness and mean slope, i.e. $[\sigma] = \rho g[z][z]/[x] = \rho g\delta[x]\delta[x]/[x] = \rho g[x]\delta^2$, the right-hand term in Eq. (10.16) would have been on the order of δ^{-1} for $\alpha = 0$, with no term on the left-hand side of that equation to match that term.

If we only consider terms of zeroth order and drop terms of order δ and higher, we arrive at a reduced system where the horizontal gradients of the vertical shear stresses are omitted from the equilibrium equations. There are no first-order terms in the scaled equilibrium equations ((10.14) to (10.16)), and the resulting reduced system is therefore correct to second order. Despite no first-order terms appearing in the scaled equilibrium equations, it does of course not follow that none of the quantities entering these equations are of first order. The vertical shear stresses, τ_{xz} and τ_{yz} , are, for example, of first order.

Sliding law

We assume a power-law relationship between basal shear stress

$$\mathbf{t}_b = \boldsymbol{\sigma} \hat{\mathbf{n}} - (\hat{\mathbf{n}}^T \cdot \boldsymbol{\sigma} \hat{\mathbf{n}}) \hat{\mathbf{n}},$$

and basal velocity

$$\mathbf{v}_b = \mathbf{v} - (\hat{\mathbf{n}}^T \cdot \mathbf{v}) \hat{\mathbf{n}}.$$

or

$$\mathbf{v}_b = c |\mathbf{t}_b|^{m-1} \mathbf{t}_b, \quad (10.17)$$

where $\hat{\mathbf{n}}$ being a unit normal vector to the bed pointing into the ice. The scaling of the basal sliding law is done in Appendix A. We find that the components of the scaled basal shear stress vector are given by

$$t_{bx} = [\sigma](\delta\partial_x^*b^*(\sigma_{zz}^* - \sigma_{xx}^*) - \delta\partial_{y^*}b^*\tau_{xy}^* + \delta\tau_{xz}^*) + O(\delta^3), \quad (10.18)$$

$$t_{by} = [\sigma](\delta\partial_{y^*}b^*(\sigma_{zz}^* - \sigma_{yy}^*) - \delta\partial_x^*b^*\tau_{xy}^* + \delta\sigma_{yz}^*) + O(\delta^3), \quad (10.19)$$

$$t_{bz} = [\sigma](\delta^2((\sigma_{zz}^* - \sigma_{xx}^*)(\partial_x^*b^*)^2 + (\sigma_{zz}^* - \sigma_{yy}^*)(\partial_{y^*}b^*)^2 - 2\tau_{xy}^*\partial_x^*b^*\partial_{y^*}b^* + \tau_{xz}^*\partial_x^*b^* + \sigma_{yz}^*\partial_{y^*}b^*)) + O(\delta^4).$$

We see from the above listed equations that the x and y components of the shear stresses vector are of order δ , and that therefore the length of that vector is also of order δ , i.e.

$$|\mathbf{t}_b| = O(\delta). \quad (10.21)$$

Hence

$$\begin{aligned} |\mathbf{t}_b| &= [|\mathbf{t}_b|] |\mathbf{t}_b^*| \\ &= \delta [\sigma] |\mathbf{t}_b^*|. \end{aligned}$$

or

$$[|\mathbf{t}_b|] = \delta [\sigma]$$

The scaled basal sliding velocity (\mathbf{v}_b) is

$$\mathbf{v}_b = [u] \begin{pmatrix} u_b^* + O(\delta^2) \\ v_b^* + O(\delta^2) \\ \delta u_b^* \partial_{x^*} b^* + \delta v_b^* \partial_{y^*} b^* + O(\delta^3) \end{pmatrix} \quad (10.22)$$

We have not yet specified the scale $[c]$ for the parameter c in the sliding law but we have already introduced scales for the velocity and the stresses, so by inserting (10.7), (10.13), and (10.24) into (10.17) we arrive at

$$\begin{aligned} [u] u_b^* &= c |\mathbf{t}_b|^{m-1} |\mathbf{t}_b^*|^{m-1} [t_{bx}] t_{bx}^* \\ &= c \delta^{m-1} [\sigma]^{m-1} |\mathbf{t}_b^*|^{m-1} \delta [\sigma] t_{bx}^* \\ &= c \delta^m [\sigma]^m |\mathbf{t}_b^*|^{m-1} t_{bx}^* \end{aligned}$$

or

$$u_b^* = c \delta^m [\sigma]^m [u]^{-1} |\mathbf{t}_b^*|^{m-1} t_{bx}^*. \quad (10.23)$$

If we want the the sliding velocity to be balanced by the basal shear stress, terms on both side of Eq. (10.23) must be of same order, hence

$$c \delta^m [\sigma]^m [u]^{-1} = O(1).$$

If we write

$$c = [c] c^*, \quad (10.24)$$

then

$$[c] = \delta^{-m} [u] [\sigma]^{-m}. \quad (10.25)$$

Eq. (10.25) shows that c is of the order δ^{-m} . In this sense the slipperiness (c) must be ‘large’ for the theory to be consistent.

The product $c[\sigma]^m$ is the (typical) basal sliding velocity, while $[u]$ is the (typical) surface velocity. Hence, Eq. (10.25) simply reflects the condition that for ‘most’ of the forward motion to be due to basal sliding, the basal slipperiness c must be ‘large’. As an example, if $\partial_x b = \partial_y b = 0$ we find that

$$u^* = c^* \tau_{xz}^{*m}.$$

In principle we could have observed right at the beginning that defining the vertical shear stress components to be of $O(\delta)$ and $u = O(1)$ implies $c = O([u][\sigma]^{-m} \delta^{-m})$ for a basal sliding law of the form $u_b = c \tau_b^m$ if u_b is to be of same order as u .

For the z component of the sliding law obtain using Eq. (10.20) and Eq. (10.22)

$$\begin{aligned} \delta u^* \partial_{x^*} b^* + \delta v^* \partial_{y^*} b^* &= c |\mathbf{t}_b|^{m-1} \\ &[\sigma] (\delta^2 ((\sigma_{zz}^* - \sigma_{xx}^*) (\partial_{x^*} b^*)^2 + (\sigma_{zz}^* - \sigma_{yy}^*) (\partial_{y^*} b^*)^2 - 2 \tau_{xy}^* \partial_{x^*} b^* \partial_{y^*} b^* + \tau_{xz}^* \partial_{x^*} b^* + \sigma_{yz}^* \partial_{y^*} b^*)). \end{aligned} \quad (10.26)$$

Note that the sum of the two terms on the left-hand-side as given by Eqs. (10.18) and (10.19) gives the left-hand side of (10.26), so these equations are consistent. Note furthermore that the vertical component w does not enter the sliding law The vertical component must be calculated from the basal kinematic boundary condition ($u \partial_x b + v \partial_y b - w = 0$).

Flow law

The flow law can be either written as

$$\dot{\epsilon}_{ij} = A(T) \tau^{n-1} \tau_{ij}, \quad (10.27)$$

or alternatively as

$$\tau_{ij} = A^{-1/n} \dot{\epsilon}^{(1-n)/n} \dot{\epsilon}_{ij},$$

where $\dot{\epsilon} = \sqrt{\dot{\epsilon}_{ij}\dot{\epsilon}_{ij}/2}$ is the *effective strain rate*, and T the englacial temperature.

We have so far not discussed the scalings for the normal deviatoric stresses τ_{xx}^* , τ_{yy}^* , and τ_{zz}^* . These scalings follow directly from the fact that we decided above to scale both the normal stresses (σ_{xx} , σ_{yy} , σ_{zz}) and the pressure with $[\sigma]$. Because $\tau_{xx} = \sigma_{xx} + p = [\sigma](\sigma_{xx}^* + p^*) = [\sigma]\tau_{xx}^*$ and similarly for the other components we have

$$(\tau_{xx}, \tau_{yy}, \tau_{zz}) = [\sigma](\tau_{xx}^*, \tau_{yy}^*, \tau_{zz}^*).$$

The square root of the second invariant of the deviatoric stress tensor, or what glaciologist usually refer to as the *effective stress*, is thus

$$\tau = [\sigma] \sqrt{(\tau_{xx}^{*2} + \tau_{yy}^{*2} + \tau_{zz}^{*2})/2 + \tau_{xy}^{*2} + \delta^2(\tau_{xz}^{*2} + \tau_{yz}^{*2})}, \quad (10.28)$$

and therefore

$$[\tau] = [\sigma] \quad (10.29)$$

and

$$\tau = [\sigma] \tau^*$$

where

$$\tau^* = \sqrt{(\tau_{xx}^{*2} + \tau_{yy}^{*2} + \tau_{zz}^{*2})/2 + \tau_{xy}^{*2} + \delta^2(\tau_{xz}^{*2} + \tau_{yz}^{*2})}.$$

The effective stress τ is of order unity.

Using the flow law and the incompressibility condition we find

$$0 = v_{i,i} = \dot{\epsilon}_{ii} = \tau_{ii}$$

and

$$\tau_{zz}^2 = (\tau_{xx} + \tau_{yy})^2$$

which can be used to eliminate τ_{zz} from Eq. (10.28).

We now look at the relation between the individual components of the stretching tensor (the strain rates) and the deviatoric stress tensor. We find, for example, that

$$\dot{\epsilon}_{xx} = A \tau^{n-1} \tau_{xx},$$

is in scaled variables

$$[u][x]^{-1} \dot{\epsilon}_{xx}^* = [\sigma]^n A \tau^{*n-1} \tau_{xx}^*.$$

or

$$\dot{\epsilon}_{xx}^* = A[\sigma]^n [x][u]^{-1} \tau^{*n-1} \tau_{xx}^*. \quad (10.30)$$

where

$$\dot{\epsilon}_{xx}^* = \partial_{x^*} u^*$$

If we want the horizontal strain rates ($\dot{\epsilon}_{xx}$, $\dot{\epsilon}_{xy}$, and $\dot{\epsilon}_{yy}$) to be balanced by the corresponding horizontal deviatoric stresses, we must require that both sides of Eq. (10.30) are of same order implying

$$A[\sigma]^n [x][u]^{-1} = O(1).$$

Writing

$$A = [A] A^*,$$

therefore leads to

$$[A] = [u][x]^{-1} [\sigma]^{-n}. \quad (10.31)$$

Next we look at

$$\dot{\epsilon}_{xz} = A \tau^{n-1} \tau_{xz},$$

and find that this gives

$$[u][x]^{-1}(\delta^{-1}\partial_z^*u^* + \delta\partial_{x^*}w^*) = \delta[\sigma]^n A\tau^{*n-1}\tau_{xz}^*,$$

which we can also write as

$$\begin{aligned}\partial_z^*u^* + \delta^2\partial_{x^*}w^* &= \delta^2[x][\sigma]^n[u]^{-1}A\tau^{*n-1}\tau_{xz}^* \\ &= \delta^2A^*\tau^{*n-1}\tau_{xz}^*,\end{aligned}$$

Note that here we are using the scale for A given by (10.31). We must equate terms of same order, and hence find that

$$\partial_z^*u^* = O(\delta^2),$$

and

$$\partial_{x^*}w^* = [x][\sigma]^n[u]^{-1}A\tau^{*n-1}\tau_{xz}^*.$$

We have now reached the important conclusion that the *horizontal velocity component u is independent of depth to second order*. Same argument shows that the other horizontal component v is also independent of depth. Thus, to second order the horizontal velocity components u and v are both independent of z .

We have shown that consistency with the scalings used for stresses requires $\partial_z u$ to be $O(\delta^2)$. Note that we have NOT shown vertical shearing ($\dot{\epsilon}_{xz}$) to be zero. Both $\partial_x w$ and τ_{xz} enter the field equations as first order terms.

The incompressibility conditions states that

$$\partial_{x^*}u^* + \partial_{y^*}v^* + \partial_{z^*}w^* = 0.$$

Differentiating with respect to z and assuming that the order of differentiation can be changed gives

$$\partial_{x^*z^*}^2u^* + \partial_{y^*z^*}^2v^* + \partial_{z^*z^*}^2w^* = 0.$$

From which using $\partial_{z^*}u^* = \partial_{z^*}v^* = O(\delta^2)$ it follows that

$$\partial_{z^*z^*}^2w^* = O(\delta^2)$$

Hence, *to second order $\dot{\epsilon}_{zz}$ is independent of depth and the vertical velocity varies linearly with depth*.

It turns out to be more convenient working with the flow law in the form

$$\tau_{ij} = 2\eta\dot{\epsilon}_{ij}$$

where η is the *effective viscosity* defined as

$$\eta = \frac{1}{2}A^{-1/n}\dot{\epsilon}^{(1-n)/n}.$$

Toward this end we determine the effective strain rate

$$\begin{aligned}\dot{\epsilon} &= \sqrt{(\dot{\epsilon}_{xx}^2 + \dot{\epsilon}_{yy}^2 + \dot{\epsilon}_{zz}^2)/2 + \dot{\epsilon}_{xy}^2 + \dot{\epsilon}_{xz}^2 + \dot{\epsilon}_{yz}^2} \\ &= \sqrt{(\dot{\epsilon}_{xx}^2 + \dot{\epsilon}_{yy}^2 + (\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy})^2)/2 + \dot{\epsilon}_{xy}^2 + \dot{\epsilon}_{xz}^2 + \dot{\epsilon}_{yz}^2}.\end{aligned}$$

Inserting $\dot{\epsilon}_{ij} = (v_{i,j} + v_{j,i})/2$ and using the fact that $\partial_z u = O(\delta^2)$ and $\partial_z v = O(\delta^2)$ we find

$$\begin{aligned}\dot{\epsilon} &= [u][x]^{-1}((\partial_{x^*}u^*)^2 + (\partial_{y^*}v^*)^2 + \partial_{x^*}u^*\partial_{y^*}v^* + (\partial_{x^*}v^* + \partial_{y^*}u^*)^2/4 \\ &\quad + (\delta\partial_{x^*}w^* + O(\delta^2))^2/4 + (\delta\partial_{y^*}w^* + O(\delta^2))^2/4)^{1/2} \\ &= [u][x]^{-1}\sqrt{(\partial_{x^*}u^*)^2 + (\partial_{y^*}v^*)^2 + \partial_{x^*}u^*\partial_{y^*}v^* + (\partial_{x^*}v^* + \partial_{y^*}u^*)^2/4 + O(\delta^2)},\end{aligned}$$

or

$$\dot{\epsilon} = \sqrt{(\partial_x u)^2 + (\partial_y v)^2 + \partial_x u \partial_y v + (\partial_x v + \partial_y u)^2/4 + O(\delta^2)}. \quad (10.32)$$

Slip ratio

We can write the horizontal velocity component u as the sum

$$u = u_b + u_d$$

where u_d is the *deformational velocity*, and u_b the basal sliding velocity. The ratio

$$\gamma := \frac{u_b}{u_d}$$

between the basal sliding velocity and the deformational velocity is the *slip ratio*. Using the relationship for the basal velocity for a uniformly inclined slab with ice thickness h , i.e.

$$u_d = \frac{2A}{n+1} \tau^{n-1} \tau_{xz} h,$$

we find

$$[u_d] = [A][\tau]^{n-1}[\tau_{xy}][z] \quad (10.33)$$

$$= [u][x]^{-1}[\sigma]^{-n}[\sigma]^{n-1}\delta[\sigma]\delta[x] \quad (10.34)$$

$$= \delta^2[u], \quad (10.35)$$

where we have used Eqs. (10.21), (10.25), (10.29), and (10.31). Since $[u_b] = [u]$, we have

$$\gamma = \frac{[u_b]}{[u_d]} = O(\delta^{-2}). \quad (10.36)$$

Note that the we did not specify from the outset that the sliding velocity had to be large as compared to the deformational velocity, so (10.36) is a result rather than an assumption. It is worthwhile to think about how we arrived at the conclusion that the slip ratio is of order δ^{-2} . By assuming that the horizontal deviatoric stresses are large compared to vertical shear stresses (see Eq. 10.9), and by balancing the horizontal strain rates with the horizontal deviatoric stresses (see Eq. 10.30), we arrived at a scale for the rate factor A (see Eq. 10.31). We furthermore assumed that the basal sliding velocity was of the same order as the surface velocity (see Eq. 10.8), i.e. of order unity. We then found the basal stress to be of order δ (see Eq. 10.21), and by requiring a balance between the basal sliding velocity and the basal stress implied by the sliding law, we arrived at a scale for the basal slipperiness c (see Eq. 10.25). It then follows, as shown above, that the slip ratio is of order δ^{-2} .

We also have

$$\frac{[c]}{[A]} = \frac{\delta^{-m}[u][\sigma]^{-m}}{[u][x]^{-1}[\sigma]^{-n}} = \delta^{-m}[x][\sigma]^{n-m}.$$

which puts constraints on the numerical value of c with respect to that of A .

Implications of different balances for the slip ratio

When finding the scale $[u_d]$ (see (10.34)) we used the fact that the effective stress (τ) is of order unity (see Eq. 10.29). In the absence of any significant horizontal deviatoric stresses however, the effective stress would be of order δ and $[u_d]/[u] = O(\delta^{n+1})$. If the ice is not subjected to horizontal deviatoric stresses of order unity, we can still balance horizontal strain rates with the horizontal deviatoric stresses as we did above to arrive at $[A]$. However, in that case it seems more logical to balance the vertical shear strain rates and the vertical shear stresses to arrive at a (different) scale for A .

Furthermore, had we not assumed the aspect ratio and the stress ratio to be equal, but instead written

$$\frac{[z]}{[x]} = \varepsilon \quad \text{and} \quad \frac{[\tau_{xz}]}{[\tau_{xx}]} = \delta,$$

with ε now being the aspect ratio, and δ as ‘stress’ ratio, we would have found that

$$\begin{aligned} [u_d] &= [A][\sigma]^{n-1}\delta[\sigma][z] \\ &= [u][\sigma]^{-n}[x]^{-1}[\sigma]^{n-1}\delta[\sigma]\varepsilon[x] \\ &= [u]\delta\varepsilon, \end{aligned}$$

i.e.

$$\frac{[u_d]}{[u]} = \varepsilon \delta.$$

Remember that the scaling originally introduced for velocity was in terms of the basal sliding velocity, and that therefore in fact

$$\frac{[u_d]}{[u_b]} = \varepsilon \delta,$$

which is the inverse of the slip ratio. We see that for any given aspect ratio ε , the slip ratio $([u_b]/[u_d])$ becomes small as the stress ratio δ goes to infinity, corresponding to the situation where horizontal deviatoric stresses are small compared to vertical shear stresses.

10.3 The SSTREAM (zeroth-order) equations

Now that we have scaled all equations we can collect terms to the desired order and go back to dimensional quantities.

Note that in the field equations and all the boundary conditions, first order terms are all identically equal to zero. Although we only collect zeroth order terms, the first order correction is zero and the theory is therefore correct to second order in δ .

10.3.1 Boundary conditions

The stress conditions at the surface $\boldsymbol{\sigma} \hat{\mathbf{n}} = \mathbf{0}$ gives to second order

$$-\sigma_{xx} \partial_x s - \tau_{xy} \partial_y s + \tau_{xz} = 0, \quad (10.37)$$

$$-\tau_{xy} \partial_x s - \sigma_{yy} \partial_y s + \tau_{yz} = 0, \quad (10.38)$$

$$\sigma_{zz} = 0, \quad (10.39)$$

for $z = s(x, y)$. And

$$u_b = c |t_b|^{m-1} t_{bx}, \quad (10.40)$$

$$v_b = c |t_b|^{m-1} t_{by}, \quad (10.41)$$

where

$$t_{bx} = \partial_x b (\sigma_{zz} - \sigma_{xx}) - \partial_y b \tau_{xy} + \tau_{xz}, \quad (10.42)$$

$$t_{by} = \partial_y b (\sigma_{zz} - \sigma_{yy}) - \partial_x b \tau_{xy} + \tau_{yz}, \quad (10.43)$$

for $z = b(x, y)$.

We could use the z component of the sliding law to calculate w_b . But since the sliding law is fully consistent with the no-penetration condition (see for example Eq. 10.26), it is easier to determine w_b as a function of u_b and v_b and the bed geometry directly using the no-penetration condition.

10.3.2 Field equations

To zeroth order we obtain from Eq. (10.14) to (10.16)

$$\partial_x \sigma_{xx} + \partial_y \tau_{xy} + \partial_z \tau_{xz} = -\rho g \sin \alpha \quad (10.44)$$

$$\partial_x \tau_{xy} + \partial_y \sigma_{yy} + \partial_z \tau_{yz} = 0, \quad (10.45)$$

$$\partial_z \sigma_{zz} = \rho g \cos \alpha. \quad (10.46)$$

Using $\tau_{ij} = 2\eta \dot{\epsilon}_{ij}$ and $\sigma_{ij} = \tau_{ij} - p \delta_{ij}$ we can also write this system as

$$-\partial_x p + 2\partial_x (\eta \dot{\epsilon}_{xx}) + 2\partial_y (\eta \dot{\epsilon}_{xy}) + 2\partial_z (\eta \dot{\epsilon}_{xz}) = -\rho g \sin \alpha, \quad (10.47)$$

$$-\partial_y p + 2\partial_x (\eta \dot{\epsilon}_{xy}) + 2\partial_y (\eta \dot{\epsilon}_{yy}) + 2\partial_z (\eta \dot{\epsilon}_{yz}) = 0, \quad (10.48)$$

$$-\partial_z p + 2\partial_z (\eta \dot{\epsilon}_{zz}) = \rho g \cos \alpha. \quad (10.49)$$

10.3.3 Vertical integration

We start by considering Eq. (10.46). Integrating from z to $z = s(x, y)$ gives

$$\sigma_{zz}(s) - \sigma_{zz}(z) = -(s - z)\rho g \cos \alpha. \quad (10.50)$$

From (10.39) we find $\sigma_{zz}(s) = 0$ so that

$$\sigma_{zz} = (z - s)\rho g \cos \alpha. \quad (10.51)$$

We now integrate Eq. (10.44) over the depth and use Leibniz' rule

$$\partial_x \int_{b(x)}^{s(x)} f(x, z) dz = \int_{b(x)}^{s(x)} \partial_x f(x, z) dz + f(x, s) \partial_x s - f(x, b) \partial_x b$$

to interchange the order of integration and differentiation, and find

$$\begin{aligned} -\rho g(s - b) \sin \alpha &= \partial_x \int_b^s \sigma_{xx} dz + \partial_y \int_b^s \tau_{xy} dz \\ &\quad - \sigma_{xx}(s) \partial_x s - \tau_{xy}(s) \partial_y s + \tau_{xz}(s) \\ &\quad + \sigma_{xx}(b) \partial_x b + \tau_{xy}(b) \partial_y b - \tau_{xz}(b). \end{aligned}$$

Note that we did not have to specify how τ_{xz} varies across the depth. Because of boundary condition (10.37) the second line is equal to zero. Using (10.42) we find that the third line can be written as $-t_{bx} + \partial_x b \sigma_{zz}(b)$ so that

$$-\rho g(s - b) \sin \alpha = \partial_x \int_b^s \sigma_{xx} dz + \partial_y \int_b^s \tau_{xy} dz - t_{bx} + \partial_x b \sigma_{zz}(b).$$

Since $p = \tau_{zz} - \sigma_{zz}$ and $\tau_{xx} + \tau_{yy} + \tau_{zz} = 0$ because ice is incompressible, we find that σ_{xx} can be written as

$$\begin{aligned} \sigma_{xx} &= \tau_{xx} - p \\ &= \tau_{xx} - \tau_{zz} + \sigma_{zz} \\ &= \tau_{xx} - (-\tau_{xx} - \tau_{yy}) + \sigma_{zz} \\ &= 2\tau_{xx} + \tau_{yy} + \sigma_{zz}. \end{aligned} \quad (10.52)$$

Because u and v are independent of depth it follows that τ_{xy} , τ_{xx} , and τ_{yy} are also all independent of depth. The corresponding vertical integrals are therefore simple to evaluate and we obtain

$$-\rho g h \sin \alpha = \partial_x \int_b^s \sigma_{zz} dz + \partial_x (h(2\tau_{xx} + \tau_{yy})) + \partial_y (h\tau_{xy}) - t_{bx} + \partial_x b \sigma_{zz}(b), \quad (10.53)$$

where $h = s - b$ is the ice thickness. We have already determined σ_{zz} (see Eq. (10.51)) and find that

$$\begin{aligned} \partial_x \int_b^s \sigma_{zz} dz &= \partial_x \int_b^s (z - s) \rho g \cos \alpha dz \\ &= \partial_x \left(-\frac{1}{2} (s - b)^2 \rho g \cos \alpha \right) \\ &= -(s - b) \rho g \cos \alpha (\partial_x s - \partial_x b) \\ &= \sigma_{zz}(b) (\partial_x s - \partial_x b), \end{aligned}$$

which when inserted into Eq. (10.53) gives

$$\partial_x (h(2\tau_{xx} + \tau_{yy})) + \partial_y (h\tau_{xy}) - t_{bx} = \partial_x s h \rho g \cos \alpha - \rho g h \sin \alpha.$$

We can express this result in terms of the components of the velocity vector using $\tau_{ij} = \eta(v_{i,j} + v_{j,i})$ and find that

$$\partial_x (4h\eta \partial_x u + 2h\eta \partial_y v) + \partial_y (h\eta (\partial_x v + \partial_y u)) - t_{bx} = \rho g h (\partial_x s \cos \alpha - \sin \alpha), \quad (10.54)$$

$$\partial_y (4h\eta \partial_y v + 2h\eta \partial_x u) + \partial_x (h\eta (\partial_y u + \partial_x v)) - t_{by} = \rho g h \partial_y s \cos \alpha, \quad (10.55)$$

where we have added the results for the y direction which follow in an identical manner.² The effective viscosity is

$$\eta = \frac{1}{2} A^{-1/n} \dot{\epsilon}^{(1-n)/n},$$

where

$$\dot{\epsilon} = \sqrt{(\partial_x u)^2 + (\partial_y v)^2 + \partial_x u \partial_y v + (\partial_x v + \partial_y u)^2/4}.$$

10.3.4 Tensor of resisting stresses

In most modeling work the coordinate system is not tilted. For $\alpha = 0$ the vertically integrated form of the momentum equation in x and y directions is

$$\partial_x(h(2\tau_{xx} + \tau_{yy})) + \partial_y(h\tau_{xy}) - t_{bx} = \rho g h \partial_x s, \quad (10.58)$$

$$\partial_y(h(2\tau_{xx} + \tau_{yy})) + \partial_x(h\tau_{xy}) - t_{by} = \rho g h \partial_y s, \quad (10.59)$$

This system can be written in a more compact form as

$$\nabla_{xy}^T \cdot (h \mathbf{R}) - \mathbf{t}_{bh} = \rho g h \nabla_{xy}^T s, \quad (10.60)$$

where

$$\mathbf{R} = \begin{pmatrix} 2\tau_{xx} + \tau_{yy} & \tau_{xy} \\ \tau_{xy} & 2\tau_{yy} + \tau_{xx} \end{pmatrix}, \quad (10.61)$$

is sometimes referred to as the *resistive stress tensor*, and

$$\nabla_{xy} = (\partial_x, \partial_y)^T,$$

and

$$\mathbf{t}_{bh} = \begin{pmatrix} t_{bx} \\ t_{by} \end{pmatrix}.$$

Note that as Eq. (10.52) shows

$$2\tau_{xx} + \tau_{yy} = \sigma_{xx} - \sigma_{zz}, \quad (10.62)$$

$$2\tau_{yy} + \tau_{xx} = \sigma_{yy} - \sigma_{zz}, \quad (10.63)$$

and the resistive stress tensor can therefore be written as

$$\mathbf{R} = \begin{pmatrix} \sigma_{xx} - \sigma_{zz} & \tau_{xy} \\ \tau_{xy} & \sigma_{yy} - \sigma_{zz} \end{pmatrix}. \quad (10.64)$$

The resistive stress tensor is neither equal to the deviatoric stress tensor or the Cauchy stress tensor. In the Shallow Ice Stream approximation, the deviatoric and the resistive stress tensors are both independent of depth, whereas the Cauchy stress tensor is not.

10.3.5 Weertman sliding law

If we use Weertman sliding law the components of basal shear stress, t_{bx} and t_{by} can be written in terms of the basal velocity as

$$t_{bx} = c^{-1/m} |\mathbf{v}_b|^{1/m-1} u, \quad (10.65)$$

$$t_{by} = c^{-1/m} |\mathbf{v}_b|^{1/m-1} v. \quad (10.66)$$

The sliding law is sometimes written as

$$t_{bx} = \beta^2 u, \quad (10.67)$$

$$t_{by} = \beta^2 v, \quad (10.68)$$

²If ρ is a function of x and y then $\partial_x \int_b^s \sigma_{zz} dz = \sigma_{zz}(b) \partial_x h - \frac{1}{2} h^2 g \cos \alpha \partial_x \rho$ and we have

$$\partial_x(4h\eta\partial_x u + 2h\eta\partial_y v) + \partial_y(h\eta(\partial_x v + \partial_y u)) - t_{bx} = \rho g h (\partial_x s \cos \alpha - \sin \alpha) + \frac{1}{2} h^2 g \cos \alpha \partial_x \rho, \quad (10.56)$$

$$\partial_y(4h\eta\partial_y v + 2h\eta\partial_x u) + \partial_x(h\eta(\partial_y u + \partial_x v)) - t_{by} = \rho g h \partial_y s \cos \alpha + \frac{1}{2} h^2 g \cos \alpha \partial_y \rho, \quad (10.57)$$

where

$$\beta = c^{-1/2m} |\mathbf{v}_b|^{\frac{1-m}{2m}}.$$

in which case we can also write the field equations as

$$\partial_x(4h\eta\partial_x u + 2h\eta\partial_y v) + \partial_y(h\eta(\partial_x v + \partial_y u)) - \beta^2 u = \rho gh(\partial_x s \cos \alpha - \sin \alpha), \quad (10.69)$$

$$\partial_y(4h\eta\partial_y v + 2h\eta\partial_x u) + \partial_x(h\eta(\partial_y u + \partial_x v)) - \beta^2 v = \rho gh\partial_y s \cos \alpha. \quad (10.70)$$

We now have a system of two partial differential equations that, given appropriate boundary conditions, can be solved for the velocity components u and v . Remember that in general both η and β^2 are functions of the strain rates and the velocity, respectively. The system is therefore non-linear and if solved numerically, some sort of appropriate iterative algorithm (e.g. Newton-Raphson) must be used.

10.4 The shallow ice shelf approximation (SSHELF)

The scaling analysis shown above applies to ice shelves as well. The only change we have to make is setting the basal shear stress to zero. There is no reason to do the analysis in a tilted coordinate system so we also set $\alpha = 0$. Thus for floating ice shelves we have

$$\partial_x(4h\eta\partial_x u + 2h\eta\partial_y v) + \partial_y(h\eta(\partial_x v + \partial_y u)) = \rho gh \partial_x s, \quad (10.71)$$

$$\partial_y(4h\eta\partial_y v + 2h\eta\partial_x u) + \partial_x(h\eta(\partial_y u + \partial_x v)) = \rho gh \partial_y s, \quad (10.72)$$

No stresses act at the upper boundary $z = s(x, y)$, and therefore the stress boundary conditions at the upper surface are (to second order) simply

$$-\sigma_{xx}\partial_x s - \tau_{xy}\partial_y s + \tau_{xz} = 0, \quad (10.73)$$

$$-\tau_{xy}\partial_x s - \sigma_{yy}\partial_y s + \tau_{yz} = 0, \quad (10.74)$$

$$\sigma_{zz} = 0. \quad (10.75)$$

The stress boundary conditions at the lower boundary $z = b(x, y)$ are

$$-\sigma_{xx}\partial_x b - \tau_{xy}\partial_y b + \tau_{xz} = p_w \partial_x b, \quad (10.76)$$

$$-\tau_{xy}\partial_x b - \sigma_{yy}\partial_y b + \tau_{yz} = p_w \partial_y b, \quad (10.77)$$

$$\sigma_{zz} = -p_w, \quad (10.78)$$

where

$$p_w = \rho_w g(S - b),$$

is the ocean pressure acting on the lower boundary of the ice shelf, with ρ_w denoting the ocean density.

We know that

$$\sigma_{zz} = \rho g(z - s), \quad (10.79)$$

and the boundary condition (10.78) therefore implies that

$$\rho g(s - b) = \rho_w g(S - b),$$

or

$$\rho gh = \rho_w gd.$$

One can now work out various other floating relationships, and one finds that where the glacier is afloat the following relations hold:

$$h = \rho_w d / \rho = \frac{s - S}{1 - \rho / \rho_w} = \frac{\rho_w}{\rho} (S - b), \quad (10.80)$$

$$b = \frac{\rho s - \rho_w S}{\rho - \rho_w} = S - \frac{\rho}{\rho_w} h, \quad (10.81)$$

$$s = S + (1 - \rho / \rho_w) h = (1 - \rho_w / \rho) b + \frac{\rho_w}{\rho} S, \quad (10.82)$$

$$f = (1 - \rho / \rho_w) h. \quad (10.83)$$

Table 10.1: List of main geometrical variables. ($\mathcal{H}(x)$ is the Heaviside step function.)

symbol	definition
s	upper glacier surface
b	lower glacier surface
S	ocean surface
B	bedrock / ocean floor
$h := s - b$	glacier thickness
$H := S - B$	ocean depth (pos. or neg. depending on location)
$d := \mathcal{H}(H)(S - b)$	glacier draft (positive by definition)
$f := s - S$	freeboard (always positive)
$h_f := \rho_w H / \rho$	maximum ice thickness without grounding

If $\partial_x S = 0$, the slopes of the upper and the lower boundary are related through

$$b \partial_x s + s \partial_x b = S \partial_x h, \quad (10.84)$$

and also

$$\partial_x s = (1 - \rho / \rho_w) \partial_x h. \quad (10.85)$$

The maximum ice thickness that an ice shelf can have without grounding is

$$h_f := \rho_w H / \rho.$$

Where

$$h \geq h_f,$$

the ice is grounded.

Using (10.85) in (10.71) and (10.72) gives

$$\partial_x (4h\eta \partial_x u + 2h\eta \partial_y v) + \partial_y (h\eta (\partial_x v + \partial_y u)) = \rho g (1 - \rho / \rho_w) h \partial_x h, \quad (10.86)$$

$$\partial_y (4h\eta \partial_y v + 2h\eta \partial_x u) + \partial_x (h\eta (\partial_y u + \partial_x v)) = \rho g (1 - \rho / \rho_w) h \partial_y h. \quad (10.87)$$

Expressed in terms of stresses these equations read

$$\partial_x (h(2\tau_{xx} + \tau_{yy})) + \partial_y (h\tau_{xy}) = \rho g (1 - \rho / \rho_w) h \partial_x h, \quad (10.88)$$

$$\partial_y (h(2\tau_{yy} + \tau_{xx})) + \partial_x (h\tau_{xy}) = \rho g (1 - \rho / \rho_w) h \partial_y h. \quad (10.89)$$

Using the definition of the resisting stress tensor (see Eq. 10.61) the equations can be written on a compact form as

$$\nabla_{xy}^T \cdot (h \mathbf{R}) = \varrho g h \nabla_{xy} h,$$

where

$$\varrho = \rho (1 - \rho / \rho_w).$$

10.4.1 Boundary conditions at the calving front

At the calving front, Γ_s , we require balance of vertically integrated horizontal stresses, i.e.

$$\int_b^s \boldsymbol{\sigma} \hat{\mathbf{n}}_h = - \int_b^S p_w \hat{\mathbf{n}}_h \quad \text{on } \Gamma_c,$$

where p_w is the hydrostatic ocean pressure, and

$$\hat{\mathbf{n}}_h = (n_x, n_y, 0)^T, \quad (10.90)$$

is a unit normal pointing horizontally outward from the ice front. In x and y directions this stress condition is

$$\int_b^s (\sigma_{xx} n_x + \tau_{xy} n_y) dz = - \int_b^S p_w n_x dz \quad \text{on } \Gamma_c, \quad (10.91)$$

$$\int_b^S (\tau_{xy} n_x + \sigma_{yy} n_y) dz = - \int_b^S p_w n_y dz \quad \text{on } \Gamma_c. \quad (10.92)$$

If the draft d at the ice front is zero, i.e. if the ice front is fully grounded, then $S < b$, the right-hand sides of (10.91) and (10.92) are to be set to zero.

Because σ_{xx} can be written as

$$\sigma_{xx} = 2\tau_{xx} + \tau_{yy} + \sigma_{zz},$$

(see Eq. 10.52), and

$$\sigma_{zz} = -\rho g(s - z),$$

within the ice, we find that

$$\begin{aligned} \int_b^S \sigma_{xx} dz &= \int_b^S (2\tau_{xx} + \tau_{yy}) dz - \int_b^S \rho g(s - z) dz \\ &= h(2\tau_{xx} + \tau_{yy}) - \frac{\rho g}{2} h^2 \end{aligned}$$

The x component of the vertically integrated ocean pressure acting on the calving front is

$$\begin{aligned} - \int_b^S p_w n_x dz &= - \int_b^S \rho_w g(S - z) n_x dz \\ &= -\frac{1}{2} \rho_w g(S - b)^2 \\ &= -\frac{1}{2} \rho_w g d^2 \end{aligned}$$

Boundary conditions (10.91) and (10.92) can therefore be written as

$$h(2\tau_{xx} + \tau_{yy})n_x + h\tau_{xy}n_y = \frac{g}{2}(\rho h^2 - \rho_w d^2) n_x \quad (10.93)$$

$$h(2\tau_{yy} + \tau_{xx})n_y + h\tau_{xy}n_x = \frac{g}{2}(\rho h^2 - \rho_w d^2) n_y \quad (10.94)$$

or more compactly as

$$\mathbf{R} \hat{\mathbf{n}}_c = \frac{g}{2h}(\rho h^2 - \rho_w d^2) \hat{\mathbf{n}}_c \quad (10.95)$$

where

$$\hat{\mathbf{n}}_c = (n_x, n_y)^T, \quad (10.96)$$

is a unit normal to the calving front.

In arriving at (10.93) and (10.94) we have not specified any particular relationship between ice shelf thickness (h) and ice shelf draft (d). These boundary conditions therefore apply to both grounded and floating ice edges.

If the ice at the calving front is afloat, then h and d are related through the floating condition $\rho h = \rho_w d$. In that case boundary conditions (10.93) and (10.94) take the form

$$h(2\tau_{xx} + \tau_{yy})n_x + h\tau_{xy}n_y = \frac{1}{2}\varrho g h^2 n_x, \quad (10.97)$$

$$h(2\tau_{yy} + \tau_{xx})n_y + h\tau_{xy}n_x = \frac{1}{2}\varrho g h^2 n_y, \quad (10.98)$$

where

$$\varrho := \rho(1 - \rho/\rho_w),$$

or again more compactly using the resistive stress tensor as

$$\mathbf{R} \hat{\mathbf{n}}_c = \frac{1}{2}\varrho g h \hat{\mathbf{n}}_c. \quad (10.99)$$

On the other hand if the ice terminates on land then $d = 0$ and

$$h(2\tau_{xx} + \tau_{yy})n_x + h\tau_{xy}n_y = \frac{g}{2}\rho h^2 n_x, \quad (10.100)$$

$$h(2\tau_{yy} + \tau_{xx})n_y + h\tau_{xy}n_x = \frac{g}{2}\rho h^2 n_y. \quad (10.101)$$

or

$$\mathbf{R} \hat{\mathbf{n}}_c = \frac{1}{2} \rho g h \hat{\mathbf{n}}_c. \quad (10.102)$$

Written in terms of the velocity components the boundary conditions along a floating ice front are:

$$\eta h(4\partial_x u + 2\partial_y v)n_x + \eta h(\partial_x v + \partial_y u)n_y = \frac{\rho g h^2}{2} n_x, \quad (10.103)$$

$$\eta h(\partial_x v + \partial_y u)n_x + \eta h(4\partial_y v + 2\partial_x u)n_y = \frac{\rho g h^2}{2} n_y. \quad (10.104)$$

10.4.2 Ice Shelf Buttressing

The vertically integrated condition on stresses at the calving front is

$$\int_b^s \boldsymbol{\sigma}_h \hat{\mathbf{n}}_c dz = - \int_b^S p_w \hat{\mathbf{n}}_c dz \quad \text{on } \Gamma_c, \quad (10.105)$$

where the subscript h on the Cauchy stress tensor implies that we are only considering the horizontal stress components, that is

$$\boldsymbol{\sigma}_h = \begin{pmatrix} \sigma_{xx} & \tau_{xy} \\ \tau_{xy} & \sigma_{yy} \end{pmatrix}. \quad (10.106)$$

We denote the left-hand by \mathbf{t}_i (traction on ice side) and those of the right-hand side by \mathbf{t}_o (traction on ocean side), that is

$$\mathbf{t}_i = \int_b^s \boldsymbol{\sigma}_h \hat{\mathbf{n}}_c dz \quad \text{on } \Gamma_c$$

and

$$\mathbf{t}_o = - \int_b^S p_w \hat{\mathbf{n}}_c dz \quad \text{on } \Gamma_c,$$

and find as shown above that

$$\mathbf{t}_i = h \mathbf{R} \hat{\mathbf{n}}_c - \frac{1}{2} \rho g h^2 \hat{\mathbf{n}}_c$$

and

$$\mathbf{t}_o = - \frac{1}{2} \rho_w d^2 \hat{\mathbf{n}}_c = - \frac{1}{2} \frac{\rho}{\rho_w} \rho g h^2 \hat{\mathbf{n}}_c$$

For (10.105) to hold, i.e.

$$\mathbf{t}_i = \mathbf{t}_o$$

it follows that

$$\mathbf{R} \hat{\mathbf{n}}_c = \frac{1}{2} \rho g h \hat{\mathbf{n}}_c \quad (10.107)$$

Eq. (10.107) is a boundary condition for the resistive stress tensor valid along a floating calving front. Within an ice shelf, and along the grounding line, the resistive stress tensor will in general not fulfill this condition.

We can define ice-shelf buttressing as the impact of the ice shelf on the stress regime along the grounding line. Note that in the absence of an ice shelf, and assuming that the ice front at the grounding line is exactly at flotation, the ice front will be in a direct contact with the ocean. To quantify the impact of the ice shelf on stresses at the grounding line we must therefore compare the stresses along the grounding line to those caused by the ocean pressure. More specifically, if we denote the vertically integrated horizontal traction along the grounding with \mathbf{t}_{gl} , i.e.

$$\mathbf{t}_{gl} = \int_b^s \boldsymbol{\sigma}_h \hat{\mathbf{n}}_c dz \quad \text{on } \Gamma_{gl} \quad (10.108)$$

the buttressing, \mathbf{B} , is by definition

$$\mathbf{B} = \mathbf{t}_o - \mathbf{t}_{gl}. \quad (10.109)$$

The normal and tangential components of the buttressing vector \mathbf{B} are

$$B_n = \hat{\mathbf{n}}_{\text{gl}}^T \cdot (\mathbf{t}_o - \mathbf{t}_{\text{gl}}) \quad (10.110)$$

$$= -\frac{1}{2} \frac{\rho}{\rho_w} \rho g h^2 - h \hat{\mathbf{n}}_{\text{gl}} \cdot (\mathbf{R} \hat{\mathbf{n}}_c) + \frac{1}{2} \rho g h^2 \quad (10.111)$$

$$= \frac{1}{2} \rho g h^2 - h \hat{\mathbf{n}}_{\text{gl}}^T \cdot \mathbf{R} \hat{\mathbf{n}}_{\text{gl}} \quad (10.112)$$

and

$$B_t = \hat{\mathbf{m}}_{\text{gl}}^T \cdot (\mathbf{t}_o - \mathbf{t}_{\text{gl}}) \quad (10.113)$$

$$= 0 - h \hat{\mathbf{m}}_{\text{gl}}^T \cdot (\mathbf{R} \hat{\mathbf{n}}_{\text{gl}}) + 0 \quad (10.114)$$

$$= h \hat{\mathbf{m}}_{\text{gl}}^T \cdot (\mathbf{R} \hat{\mathbf{n}}_{\text{gl}}) \quad (10.115)$$

where $\hat{\mathbf{m}}_{\text{gl}}$ is a unit vector in the horizontal plane tangential to the grounding line and $\hat{\mathbf{m}}_{\text{gl}}^T \cdot \hat{\mathbf{n}}_{\text{gl}} = 0$.

If we want to non-dimensionalise expressions (10.112) and (10.115) then we can do so in a number of different ways. We could for example normalize with the vertically integrated ocean pressure $|\mathbf{t}_o| = \frac{1}{2} \rho_w g d^2$, or we could normalize using the magnitude of vertically integrated resistive stresses at a calving front, i.e. $h|\mathbf{R} \hat{\mathbf{n}}_c| = \frac{1}{2} \rho g h^2$. In ice shelves, and along grounding lines, horizontal deviatoric stresses are typically on the order of $\rho g h$ and much smaller than $\rho g d$ and we therefore opt for the second option and define dimensionless normal and tangential buttressing numbers as

$$K_N = \frac{\hat{\mathbf{n}}_{\text{gl}}^T \cdot (\mathbf{t}_o - \mathbf{t}_{\text{gl}})}{h \hat{\mathbf{n}}_c^T \cdot \mathbf{R} \hat{\mathbf{n}}_c} \quad (10.116)$$

$$= \frac{\frac{1}{2} \rho g h - \hat{\mathbf{n}}_{\text{gl}}^T \cdot \mathbf{R} \hat{\mathbf{n}}_{\text{gl}}}{\frac{1}{2} \rho g h} \quad (10.117)$$

$$= 1 - \frac{2 \hat{\mathbf{n}}_{\text{gl}}^T \cdot \mathbf{R} \hat{\mathbf{n}}_{\text{gl}}}{\rho g h} \quad (10.118)$$

and

$$K_T = \frac{\hat{\mathbf{m}}_{\text{gl}}^T \cdot (\mathbf{t}_o - \mathbf{t}_{\text{gl}})}{h \hat{\mathbf{n}}_c^T \cdot \mathbf{R} \hat{\mathbf{n}}_c} \quad (10.119)$$

$$= \frac{2 \hat{\mathbf{m}}_{\text{gl}}^T \cdot \mathbf{R} \hat{\mathbf{n}}_{\text{gl}}}{\rho g h}, \quad (10.120)$$

which agrees with Gudmundsson 2013.³

³If we normalize with the (absolute value) of the vertically integrated ocean pressure

$$T'_N = \frac{\frac{1}{2} \rho g h^2 - h \hat{\mathbf{n}}_c^T \cdot \mathbf{R} \hat{\mathbf{n}}_c}{\frac{1}{2} \rho_w g d^2},$$

then $T'_N = -1$ for a fully grounded calving front, whereas $T + N = -1/(\rho_w/\rho - 1)$. In general

$$T'_N = (\rho_w/\rho - 1)T_N,$$

so T_N is about 10 times larger than T'_N .

10.5 Scaling the sliding law

We start by scaling the unit normal and find

$$\begin{aligned}
 \hat{\mathbf{n}} &= \frac{1}{\sqrt{1 + (\partial_x b)^2 + (\partial_y b)^2}} \begin{pmatrix} -\partial_x b \\ -\partial_y b \\ 1 \end{pmatrix} \\
 &= \frac{1}{\sqrt{1 + \delta^2(\partial_{x^*} b^*)^2 + \delta^2(\partial_{y^*} b^*)^2}} \begin{pmatrix} -\delta\partial_{x^*} b^* \\ -\delta\partial_{y^*} b^* \\ 1 \end{pmatrix} \\
 &= (1 - \delta^2((\partial_{x^*} b^*)^2 + (\partial_{y^*} b^*)^2)/2 + O(\delta^4)) \begin{pmatrix} -\delta\partial_{x^*} b^* \\ -\delta\partial_{y^*} b^* \\ 1 \end{pmatrix} \\
 &= \begin{pmatrix} -\delta\partial_{x^*} b^* + O(\delta^3) \\ -\delta\partial_{y^*} b^* + O(\delta^3) \\ 1 - \delta^2((\partial_{x^*} b^*)^2 + (\partial_{y^*} b^*)^2)/2 + O(\delta^4) \end{pmatrix}. \tag{10.121}
 \end{aligned}$$

The stress tensor is

$$\boldsymbol{\sigma} = [\sigma] \begin{pmatrix} \sigma_{xx}^* & \tau_{xy}^* & \delta\tau_{xz}^* \\ \tau_{xy}^* & \sigma_{yy}^* & \delta\tau_{yz}^* \\ \delta\tau_{xz}^* & \delta\tau_{yz}^* & \sigma_{zz}^* \end{pmatrix},$$

and the product $\boldsymbol{\sigma}\hat{\mathbf{n}}$ is therefore

$$\begin{aligned}
 \boldsymbol{\sigma}\hat{\mathbf{n}} &= [\sigma] \begin{pmatrix} \sigma_{xx}^* & \tau_{xy}^* & \delta\tau_{xz}^* \\ \tau_{xy}^* & \sigma_{yy}^* & \delta\tau_{yz}^* \\ \delta\tau_{xz}^* & \delta\tau_{yz}^* & \sigma_{zz}^* \end{pmatrix} \begin{pmatrix} -\delta\partial_{x^*} b^* + O(\delta^3) \\ -\delta\partial_{y^*} b^* + O(\delta^3) \\ 1 - \delta^2((\partial_{x^*} b^*)^2 + (\partial_{y^*} b^*)^2)/2 + O(\delta^4) \end{pmatrix} \\
 &= [\sigma] \begin{pmatrix} -\delta\sigma_{xx}^* \partial_{x^*} b^* - \delta\tau_{xy}^* \partial_{y^*} b^* + \delta\tau_{xz}^* + O(\delta^3), \\ -\delta\tau_{xy}^* \partial_{x^*} b^* - \delta\sigma_{yy}^* \partial_{y^*} b^* + \delta\tau_{yz}^* + O(\delta^3), \\ -\delta^2\tau_{xz}^* \partial_{x^*} b^* - \delta^2\tau_{yz}^* \partial_{y^*} b^* + \sigma_{zz}^* (1 - \frac{1}{2}\delta^2((\partial_{x^*} b^*)^2 + (\partial_{y^*} b^*)^2) + O(\delta^4)) \end{pmatrix}, \tag{10.122}
 \end{aligned}$$

so that $\hat{\mathbf{n}}^T \cdot \boldsymbol{\sigma}^* \hat{\mathbf{n}}$ where $\boldsymbol{\sigma} = [\sigma]\boldsymbol{\sigma}^*$ is given by

$$\begin{aligned}
 \hat{\mathbf{n}}^T \cdot \boldsymbol{\sigma}^* \hat{\mathbf{n}} &= \delta^2 \partial_{x^*} b^* (\sigma_{xx}^* \partial_{x^*} b^* + \tau_{xy}^* \partial_{y^*} b^* - \tau_{xz}^*) + \delta^2 \partial_{y^*} b^* (\tau_{xy}^* \partial_{x^*} b^* + \sigma_{yy}^* \partial_{y^*} b^* - \tau_{yz}^*) \\
 &\quad - \delta^2 \tau_{xz}^* \partial_{x^*} b^* - \delta^2 \tau_{yz}^* \partial_{y^*} b^* + \sigma_{zz}^* (1 - \frac{1}{2}\delta^2((\partial_{x^*} b^*)^2 + (\partial_{y^*} b^*)^2) + O(\delta^4)),
 \end{aligned}$$

which, if we sort this according to order in δ , is

$$\begin{aligned}
 \hat{\mathbf{n}}^T \cdot \boldsymbol{\sigma}^* \hat{\mathbf{n}} &= \sigma_{zz}^* \\
 &\quad + \delta^2 \{ (\sigma_{xx}^* - \sigma_{zz}^*)(\partial_{x^*} b^*)^2 + (\sigma_{yy}^* - \sigma_{zz}^*)(\partial_{y^*} b^*)^2 + 2\tau_{xy}^* \partial_{x^*} b^* \partial_{y^*} b^* - 2\tau_{xz}^* \partial_{x^*} b^* - 2\tau_{yz}^* \partial_{y^*} b^* \} \\
 &\quad + O(\delta^4).
 \end{aligned}$$

It follows that the normal stress vector on the bed, $(\hat{\mathbf{n}}^T \cdot \boldsymbol{\sigma}\hat{\mathbf{n}})\hat{\mathbf{n}}$, is

$$\begin{aligned}
 (\hat{\mathbf{n}}^T \cdot \boldsymbol{\sigma}\hat{\mathbf{n}})\hat{\mathbf{n}} &= \\
 &\begin{pmatrix} -\delta\sigma_{zz}^* \partial_{x^*} b^* + O(\delta^3) \\ -\delta\sigma_{zz}^* \partial_{y^*} b^* + O(\delta^3) \\ \sigma_{zz}^* (1 - \delta^2((\partial_{x^*} b^*)^2 + (\partial_{y^*} b^*)^2)/2) + \delta^2((\sigma_{xx}^* - \sigma_{zz}^*)(\partial_{x^*} b^*)^2 + (\sigma_{yy}^* - \sigma_{zz}^*)(\partial_{y^*} b^*)^2 + 2\tau_{xy}^* \partial_{x^*} b^* \partial_{y^*} b^* - 2\tau_{xz}^* \partial_{x^*} b^* - 2\tau_{yz}^* \partial_{y^*} b^*) + O(\delta^4) \end{pmatrix}
 \end{aligned}$$

Predictably our insistence on keeping things up to third order is making things look a bit messy.

The shear stress vector (\mathbf{t}_b) can now easily be calculated and is found to be

$$\begin{aligned}
 \mathbf{t}_b &= \boldsymbol{\sigma}\hat{\mathbf{n}} - (\hat{\mathbf{n}}^T \cdot \boldsymbol{\sigma}\hat{\mathbf{n}})\hat{\mathbf{n}} \\
 &= [\sigma] \begin{pmatrix} \delta\partial_{x^*} b^* (\sigma_{zz}^* - \sigma_{xx}^*) - \delta\partial_{y^*} b^* \tau_{xy}^* + \delta\tau_{xz}^* + O(\delta^3) \\ \delta\partial_{y^*} b^* (\sigma_{zz}^* - \sigma_{yy}^*) - \delta\partial_{x^*} b^* \tau_{xy}^* + \delta\tau_{yz}^* + O(\delta^3) \\ \delta^2((\sigma_{zz}^* - \sigma_{xx}^*)(\partial_{x^*} b^*)^2 + (\sigma_{zz}^* - \sigma_{yy}^*)(\partial_{y^*} b^*)^2 - 2\tau_{xy}^* \partial_{x^*} b^* \partial_{y^*} b^* + \tau_{xz}^* \partial_{x^*} b^* + \tau_{yz}^* \partial_{y^*} b^*) + O(\delta^4) \end{pmatrix}.
 \end{aligned}$$

Hence, the components of the basal shear stress vector are $O(\delta)$ or less.

We now scale the basal velocity vector

$$\mathbf{v}_b = \mathbf{v} - (\hat{\mathbf{n}}^T \cdot \mathbf{v}) \hat{\mathbf{n}}$$

and find

$$\begin{aligned} \mathbf{v}_b &= [u] \begin{pmatrix} u^* \\ v^* \\ \delta w^* \end{pmatrix} \\ &\quad - [u] \begin{pmatrix} -\delta \partial_{x^*} b^* + O(\delta^3) \\ -\delta \partial_{y^*} b^* + O(\delta^3) \\ 1 - \delta^2 ((\partial_{x^*} b^*)^2 + (\partial_{y^*} b^*)^2)/2 + O(\delta^4) \end{pmatrix}^T \cdot \begin{pmatrix} u^* \\ v^* \\ \delta w^* \end{pmatrix} \begin{pmatrix} -\delta \partial_{x^*} b^* + O(\delta^3) \\ -\delta \partial_{y^*} b^* + O(\delta^3) \\ 1 - \delta^2 ((\partial_{x^*} b^*)^2 + (\partial_{y^*} b^*)^2)/2 + O(\delta^4) \end{pmatrix} \\ &= [u] \begin{pmatrix} u^* \\ v^* \\ \delta w^* \end{pmatrix} - [u] \begin{pmatrix} \delta^2 u^* (\partial_{x^*} b^*)^2 + \delta^2 v^* \partial_{y^*} b^* \partial_{x^*} b^* - \delta^2 \partial_{x^*} w^* + O(\delta^3) \\ \delta^2 u^* \partial_{x^*} b^* \partial_{y^*} b^* + \delta^2 v^* (\partial_{y^*} b^*)^2 - \delta^2 \partial_{y^*} w^* + O(\delta^3) \\ -\delta u^* \partial_{x^*} b^* - \delta v^* \partial_{x^*} b^* + \delta w^* + O(\delta^3) \end{pmatrix} \\ &= [u] \begin{pmatrix} u^* + O(\delta^2) \\ v^* + O(\delta^2) \\ \delta u^* \partial_{x^*} b^* + \delta v^* \partial_{y^*} b^* + O(\delta^3) \end{pmatrix}. \end{aligned}$$

Hence, to second order

$$\mathbf{v}_b = [u] \begin{pmatrix} u^* \\ v^* \\ \delta u^* \partial_{x^*} b^* + \delta v^* \partial_{y^*} b^* \end{pmatrix}.$$

Chapter 11

Perturbation solutions of the SSTREAM/SSA

11.1 Problem definition

We perform a small-amplitude perturbation analysis of the shallow ice stream (SSTREAM) equations. The discussion is limited to 1d along a flow line in which case the SSTREAM equations are

$$4\partial_x(h\eta\partial_x u) - (u/c)^{1/m} = \rho gh\partial_x s \cos \alpha - \rho gh \sin \alpha, \quad (11.1)$$

The horizontal velocity component (u) is constant across the depth, and the vertical velocity component (w) varies linearly with depth. In these equation s is the surface, h is ice thickness, η is the effective ice viscosity, and c is the basal slipperiness. The parameter m and the basal slipperiness c are parameters in the sliding law, We write the basal sliding law on the form

$$\mathbf{u}_b = c(x, y)|\mathbf{T}_b|^{m-1}\mathbf{T}_b. \quad (11.2)$$

where \mathbf{T}_b is the basal stress vector given by $\mathbf{T}_b = \sigma\hat{\mathbf{n}} - (\hat{\mathbf{n}}^T \cdot \sigma\hat{\mathbf{n}})\hat{\mathbf{n}}$, with $\hat{\mathbf{n}}$ being a unit normal vector to the bed pointing into the ice. The function $c(x)$ is referred to as the basal slipperiness.

For a linear viscous media ($n = 1$) and a non-linear sliding law (m arbitrary) this equation can be linearised and solved analytically using standard methods as follows. We write $f = \bar{f} + \Delta f$, where f stands for some relevant variable entering the problem, and look for a zeroth-order solution where \bar{f} is independent of x and y and time t , while the first-order field Δf is small but can be a function of space and time.

The perturbations in bedrock (Δb) and slipperiness (Δc) are step functions of time. They are applied at $t = 0$, i.e. for $t < 0$ we have $\Delta b = 0$ and $\Delta c = 0$ and for $t \geq 0$ both Δb and Δc are some constants. Using this history definition the solutions for the velocity field and the surface geometry become functions of time.

11.1.1 Bedrock perturbations

We start by considering the response to small perturbation in basal topography (Δb). Writing $h = \bar{h} + \Delta h$, $s = \bar{s} + \Delta s$, $b = \bar{b} + \Delta b$, where h is ice thickness, s surface topography, and b bedrock topography, and furthermore $u = \bar{u} + \Delta u$, $w = \Delta w$, where u and w are the x and z components of the velocity vector, and $c = \bar{c}$ where c is the basal slipperiness (see Eq. (11.2) and inserting into (11.1) and solving gives the zeroth-order solution

$$\bar{u} = \bar{c}\rho g\bar{h} \sin \alpha. \quad (11.3)$$

The zeroth-order solution represents a plug flow down an uniformly inclined plane.

The first-order field equations are

$$4\eta\bar{h}\partial_{xx}^2\Delta u - \gamma\Delta u = \rho g\bar{h} \cos \alpha \partial_x \Delta s - \rho g \sin \alpha \Delta h, \quad (11.4)$$

where

$$\gamma = \frac{\tau_d^{1-m}}{m\bar{c}}, \quad (11.5)$$

and

$$\tau_d = \rho g \bar{h} \sin \alpha, \quad (11.6)$$

is the driving stress.

The domain of the first-order solution is transformed to that of the zeroth-order problem. Let $f(z)$ be some function of the vertical coordinate z . We have

$$f = \bar{f} + \Delta f$$

where \bar{f} is the zeroth order approximation and Δf the first order perturbation. For $z = \bar{z} + \Delta z$ we write

$$\begin{aligned} f(z) &= \bar{f}(z) + \Delta f(z) \\ &= \bar{f}(\bar{z}) + \partial_z \bar{f}|_{z=\bar{z}}(\bar{z}) \Delta z + \Delta f(\bar{z}) \end{aligned}$$

where terms of second order have been ignored.

For the kinematic boundary condition at the surface

$$\partial_t s + u \partial_x s - w = 0$$

we, for example, get

$$\partial_t(\bar{s} + \Delta s) + (\bar{u} + \Delta u + \partial_z \bar{u}|_{z=\bar{s}} \Delta u) \partial_x(\bar{s} + \Delta s) - (\bar{w} + \Delta w + \partial_z \bar{w}|_{z=\bar{s}} \Delta s) = 0.$$

We have $\partial_z \bar{u} = 0$ and for the particular zeroth-order solution we are using (plug flow) we have $\partial_z \bar{w} = 0$. It follows that to first order the upper and lower boundary kinematic conditions are

$$\partial_t \Delta s + \bar{u} \partial_x \Delta s - \Delta w = 0, \quad (11.7)$$

and

$$\bar{u} \partial_x \Delta b - \Delta w = 0, \quad (11.8)$$

respectively. In (11.7) the surface mass-balance perturbation has been set to zero. The jump conditions for the stresses have already been using in the derivation of (11.1) and do not need to be considered further.

This system of equations is solved using standard Fourier and Laplace transform methods. All variables are Fourier transformed with respect to the spatial variables x and y and Laplace transformed with respect to the time variable t . The forward Fourier transform $f(k)$ of a function $f(x)$ is

$$f(k) = \int_{-\infty}^{+\infty} f(x) e^{ikx} dx, \quad (11.9)$$

where i is the imaginary unit. The forward Laplace transform $f(r)$ of a function $f(t)$ is

$$f(r) = \int_{0+}^{+\infty} f(t) e^{-rt} dt. \quad (11.10)$$

The Fourier and Laplace transforms of the first-order field Eq. (11.4) is

$$4\eta \bar{h} k^2 \Delta u + \gamma \Delta u = \rho g \sin \alpha (\Delta s - \Delta b) + ik \rho g \cos \alpha \bar{h} \Delta s, \quad (11.11)$$

The Fourier transformed mass-conservation equation is

$$-ik \Delta u + \partial_z \Delta w = 0. \quad (11.12)$$

The Fourier-Laplace transformed linearized kinematic boundary condition at the upper boundary (Eq. (11.7)) is

$$-ik \bar{u} \Delta s + r \Delta s - \Delta s(t=0) - \Delta w = 0, \quad (11.13)$$

and the one at the lower boundary is

$$-ik \bar{u} \Delta b - w = 0. \quad (11.14)$$

In addition we know that u is constant over the depth, so that

$$\Delta u = \Delta u(k, t) \quad (11.15)$$

and not $u = u(k, z, t)$. Furthermore, since w is a linear function of depth we have

$$\partial_z \Delta w = F(k, t) \quad (11.16)$$

where F is some function of k and t (but not of z).

Eqs. (11.11), (11.12), (11.13), (11.14) together with (11.15) and (11.16) can now be solved for Δu , Δw , and Δs as a function of Δb for $\Delta s(t = 0) = 0$. There are various ways of doing this. One can, for example, start by integrating Eq. (11.16) with respect to z giving

$$\Delta w(z) = (z - b)\partial_z \Delta w - ik\bar{u}\Delta b \quad (11.17)$$

where lower kinematic boundary condition (11.14) has been used to determine the integration constant. Then Δu can be eliminated from Eq. (11.12) using Eq. (11.11). Inserting the resulting expression for $\partial_z \Delta w$ into (11.17) and setting $z = \bar{s}$ gives Δw as a linear function of Δs and Δb . The upper boundary kinematic condition (11.13) can then be used to get rid of Δw giving Δs as a (linear) function of Δb . Doing these algebraic manipulations, one finds that the (complex) ratio between surface and bedrock amplitude $T_{sb} = \Delta s / \Delta b$ is given by

$$T_{sb}(k, r) = -\frac{ik(\bar{u} + \tau_d/\xi)}{r(r - p)}, \quad (11.18)$$

where

$$p = i/t_p - 1/t_r, \quad (11.19)$$

and the two timescales t_p (phase time scale) and t_r (relaxation time scale) are given by

$$t_p^{-1} = k(\bar{u} + \tau_d/\xi), \quad (11.20)$$

and

$$t_r^{-1} = \xi^{-1}k^2\tau_d\bar{h}\cot\alpha, \quad (11.21)$$

and furthermore

$$\xi = \gamma + 4\bar{h}k^2\eta. \quad (11.22)$$

The inverse Laplace transform is calculated using the Bromwich integral

$$f(t) = \frac{1}{2\pi i} \int_{\gamma - i\infty}^{\gamma + i\infty} e^{rt} f(r) dr. \quad (11.23)$$

We see from Eq. (11.18) that the $T_{sb}(k, r)$ transfer function has two poles, one at $r = 0$ and another one at $r = p$. The second pole is always in the left-half complex plane and the inverse Laplace transform can be evaluated by contour integration over a semicircle in the left hand plane. We find that

$$T_{sb}(k, l, t) = \frac{ik(\bar{u}\xi + \tau_d)}{p\xi}(e^{pt} - 1). \quad (11.24)$$

This transfer function describes the relation between surface and bedrock geometry, where $\Delta s(k, l, t) = T_{sb}(k, l, t)\Delta b(k, l)$. Transfer functions giving the perturbations in velocity can be calculated in the same manner.

Exercise: Calculate $T_{ss_0} := s(k, t)/s(k, t = 0)$ for $\Delta b = 0$ and show that

$$T_{ss_0} = e^{pt}. \quad (11.25)$$

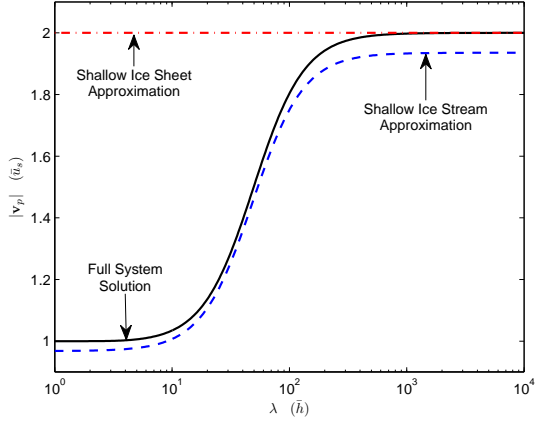


Figure 11.1: The phase speed ($|\mathbf{v}_p|$) as a function of wavelength for $\theta=0$. The dashed-dotted curve is based on the shallow-ice-sheet (SSHEET) approximation, the dashed one is based on the shallow-ice-stream (SSTREAM) approximation, and the solid one is a full-system (FS) solution. The surface slope is $\alpha=0.005$ and slip ratio $\bar{C}=30$ and $n=m=1$. The unit on the y axis is the mean surface velocity of the full-system solution ($\bar{u}=\bar{C}+1=31$).

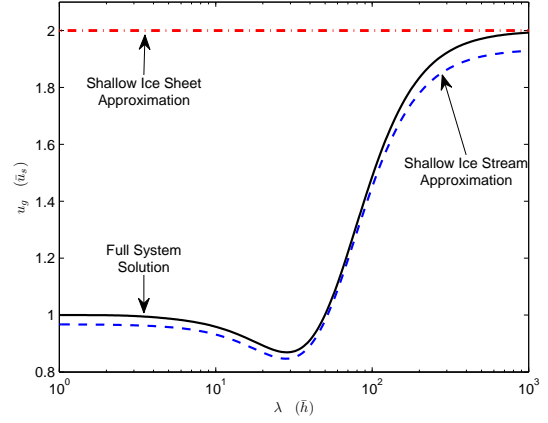


Figure 11.2: The x component of the group velocity (u_g) as a function of wavelength for $\theta=0$. Values of mean surface slope and slip ratio are 0.005 and 30, respectively, and $m=n=1$.

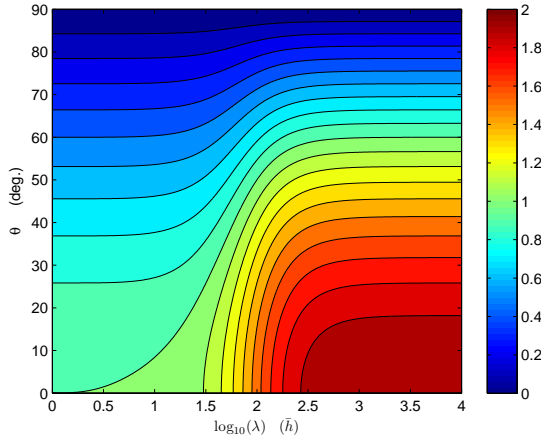


Figure 11.3: The phase speed ($|\mathbf{v}_p|$) of the full-system solution as a function of wavelength λ and orientation θ of the sinusoidal perturbations with respect to mean flow direction. The mean surface slope is $\alpha=0.002$ and the slip ratio is $\bar{C}=100$, and $n=m=1$. The plot has been normalised with the non-dimensional surface velocity $\bar{u}=\bar{C}+1=101$ of the full-system solution.

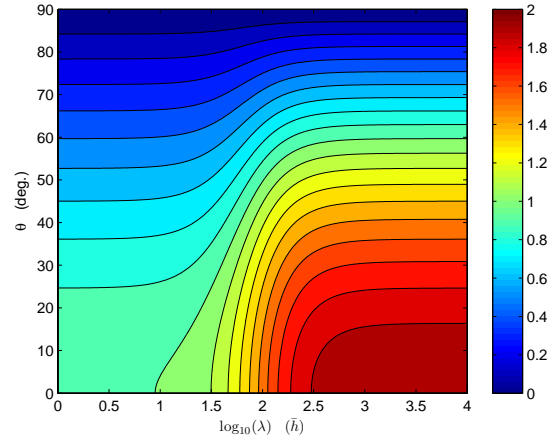


Figure 11.4: The shallow-ice-stream phase speed as a function of wavelength λ and orientation θ . As in Fig. 11.3a the mean surface slope is $\alpha=0.002$ and the slip ratio is $\bar{C}=100$, $n=m=1$, and the plot has been normalised with the non-dimensional surface velocity $\bar{u}=\bar{C}+1=101$ of the full-system solution.

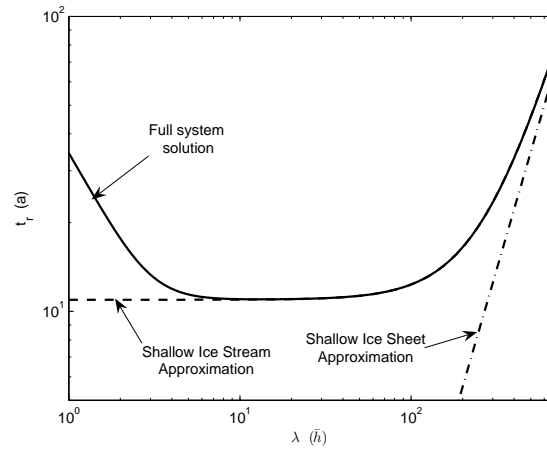


Figure 11.5: The relaxation time scale (t_r) as a function of wavelength λ . The wavelength is given in units of mean ice thickness (\bar{h}) and t_r is given in years. The mean surface slope is $\alpha=0.002$, the slip ratio is $\bar{C}=999$, and $n = m = 1$. For these values t_r is on the order of 10 years for a fairly wide range of wavelengths. Lowering the slip ratio will reduce the value of t_r . It follows that ice streams will react to sudden changes in basal properties or surface profile by a characteristic time scale of a few years.

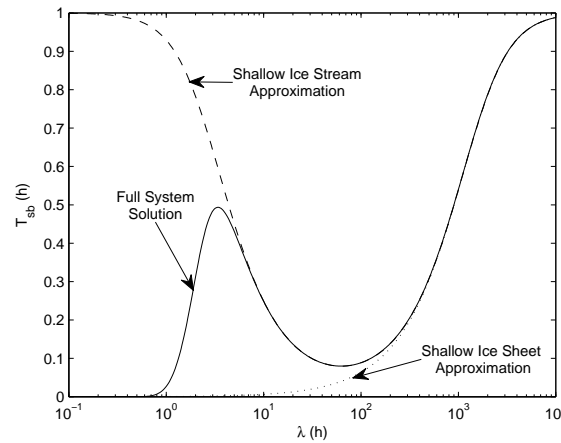


Figure 11.6: Steady-state response of surface topography (Δs) to a perturbation in bed topography (Δb). The surface slope is 0.002, the mean slip ratio $\bar{C}=100$, and $n = m = 1$. Transfer functions based on the shallow-ice-stream approximation (dashed line, Eq. 11.24), the shallow-ice-sheet approximation (dotted line), and a full system solution (solid line) are shown.

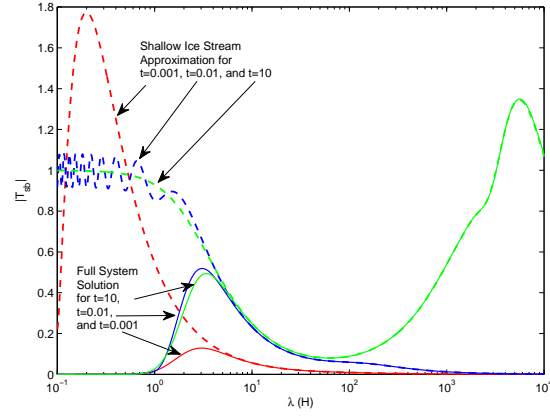


Figure 11.7: Transient surface topography response to a sinusoidal perturbation in bed topography applied at $t=0$. Shown are the amplitude ratios between surface and bed topography ($|T_{sb}|$) as a function of wavelength for $\alpha=0.002$, $\theta=0$, $\bar{C}=100$, and $n = m = 1$ for $t=0.001$ (red), $t=0.01$ (blue), and $t=10$ (green).

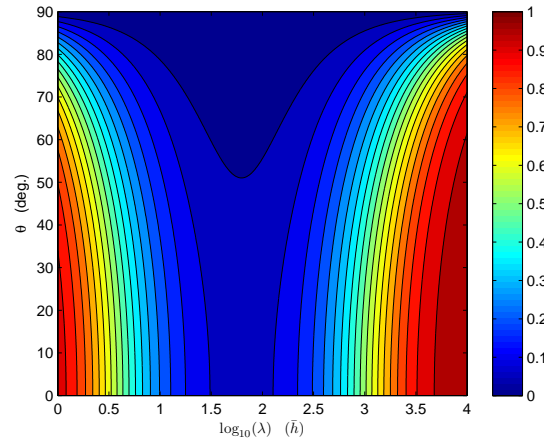


Figure 11.8: **(a)** The SSTREAM amplitude ratio ($|T_{sb}|$) between surface and bed topography (Eq. 11.24). Surface slope is 0.002, the slip ratio $\bar{C}=99$, and $n = m = 1$. λ is the wavelength of the sinusoidal bed topography perturbation and θ is the angle with respect to the x axis, with $\theta=0$ and $\theta=90$ corresponding to transverse and longitudinal undulations in bed topography, respectively.

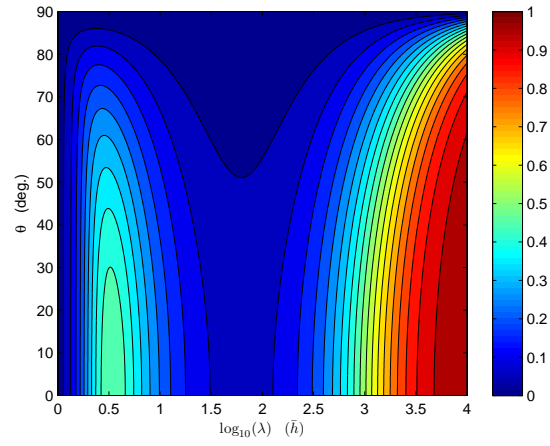


Figure 11.8: **(b)** The FS amplitude ratio between surface and bed topography ($|T_{sb}|$). The shape of the same transfer function for the same set of parameters based on the SSTREAM approximation is shown in Fig. 11.8a.

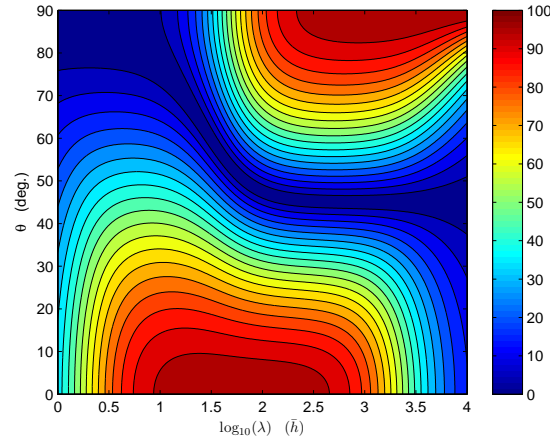


Figure 11.9: **(a)** The steady-state amplitude ratio ($|T_{ub}|$) between longitudinal surface velocity (Δu) and bed topography (Δb) in the shallow-ice-stream approximation. Surface slope is 0.002, the slip ratio is 99, and $n = m = 1$.

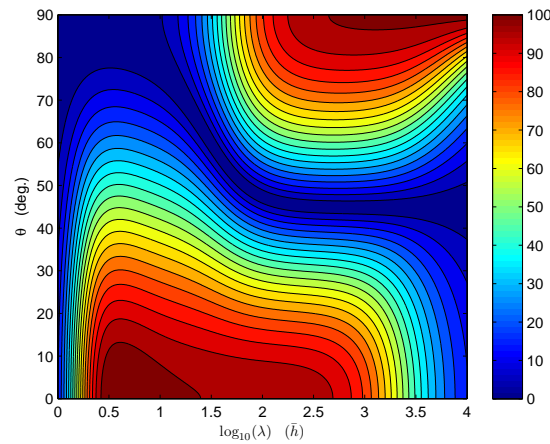


Figure 11.9: **(b)** The steady-state amplitude ratio ($|T_{ub}|$) between longitudinal surface velocity (Δu) and bed topography (Δb). The shape of the same transfer function for the same set of parameters, but based on the shallow-ice-stream approximation, is shown in Fig. 11.9a.

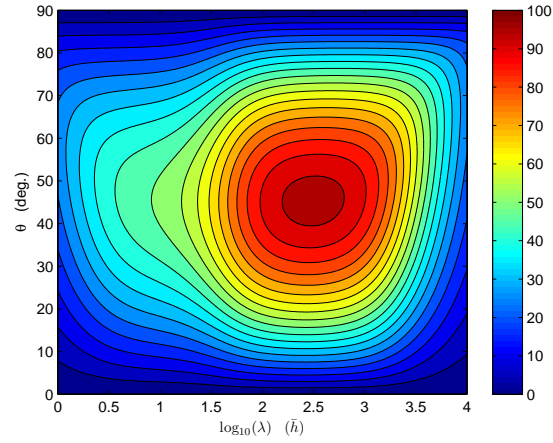


Figure 11.10: **(a)** The steady-state amplitude ratio ($|T_{vb}|$) between transverse velocity (Δv) and bed topography (Δb) in the shallow-ice-stream approximation. Surface slope is 0.002, the slip ratio is 99 and $n = m = 1$.

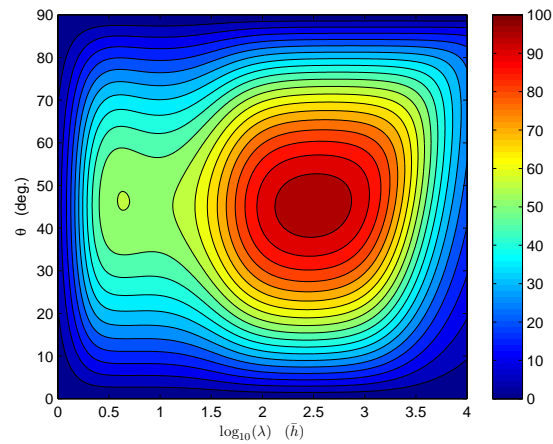


Figure 11.10: **(b)** The steady-state amplitude ratio ($|T_{vb}|$) between transverse velocity (Δv) and bed topography (Δb). The shape of the same transfer function for the same set of parameters, but based on the shallow-ice-stream approximation, is shown in Fig. 11.10a.

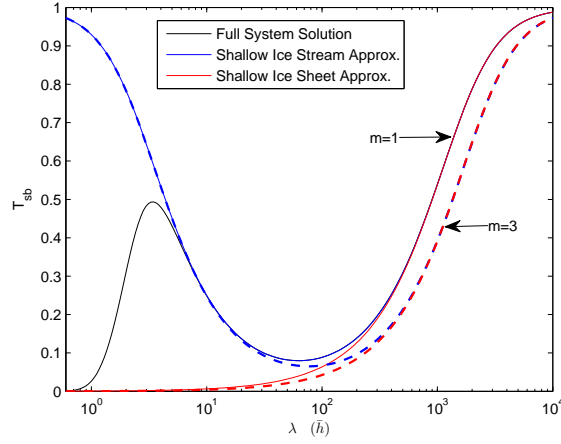


Figure 11.11: Steady-state response of surface topography to a perturbation in bed topography for linear and non-linear sliding. All curves are for linear medium ($n=1$). The solid lines are calculated for linear sliding ($m=1$) and the dashed lines for non-linear sliding ($m=3$). The red lines are SSHEET solutions, the blue ones are SSTREAM solutions, and the black line is a FS solution which is only available for $m=1$. Mean surface slope is 0.002 and slip ratio is equal to 100.

11.1.2 Slipperiness perturbations

We now consider the response to small perturbation in basal slipperiness. We write the slipperiness perturbation on the form

$$c(x) = \bar{c}(1 + \Delta c(x))$$

where Δc is the fractional perturbation in basal slipperiness. The total perturbation is $\bar{c}\Delta c$. As before we write $h = \bar{h} + \Delta h$, $s = \bar{s} + \Delta s$, $u = \bar{u} + \Delta u$, and $w = \Delta w$. Since there is no perturbation in basal topography we have $b = \bar{b}$ and $h = \bar{h} + \Delta s$.

For the zeroth-order problem we get the plug-flow solution as before

$$\bar{u} = \bar{c}\rho g \bar{h} \sin \alpha. \quad (11.26)$$

$$\partial_t \Delta s + \bar{u} \partial_x \Delta s - \Delta w = 0, \quad (11.27)$$

and

$$\bar{u} \partial_x \Delta b - \Delta w = 0, \quad (11.28)$$

To first order the upper and lower boundary kinematic conditions are

$$\partial_t \Delta s + \bar{u} \partial_x \Delta s - \Delta w = 0, \quad (11.29)$$

as before, while the basal boundary conditions are

$$\Delta w = 0, \quad (11.30)$$

In the field equation (11.1) we have, among other terms, the term $(u/c)^{1/m}$. For $u = \bar{u} + \Delta u$ and $c = \bar{c}(1 + \Delta c)$ we find

$$\left(\frac{u}{c}\right)^{1/m} = \left(\frac{\bar{u} + \Delta u}{\bar{c}(1 + \Delta c)}\right)^{1/m} = \bar{c}^{1/m} ((\bar{u} + \Delta u)(1 - \Delta c))^{1/m} = \left(\frac{\bar{u}}{\bar{c}}\right)^{1/m} - \gamma \bar{u} \Delta c + \gamma \Delta u$$

where

$$\gamma = \frac{1}{\bar{u}m} \left(\frac{\bar{u}}{\bar{c}}\right)^{1/m} = \frac{\tau_d^{1-m}}{m\bar{c}}$$

where

$$\gamma = \frac{\tau_d^{1-m}}{m\bar{c}}, \quad (11.31)$$

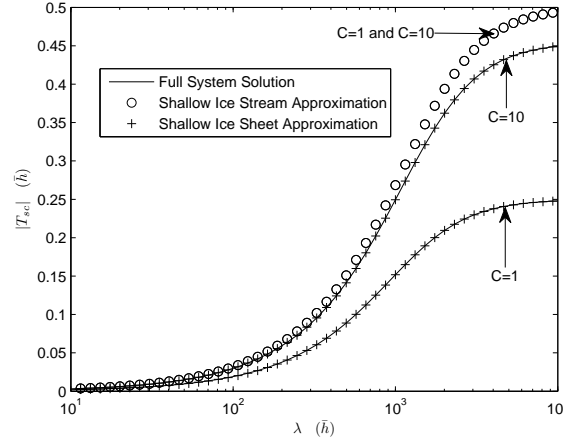


Figure 11.12: Steady-state response of surface topography to a basal slipperiness perturbation. Shown are FS (solid line), SSTREAM (circles), and SSHEET (crosses) transfer amplitudes for both $\bar{C}=1$ and $\bar{C}=10$. In the plot the SSTREAM curves for $\bar{C}=1$ and $\bar{C}=10$ are too similar to be distinguished. The surface slope is 0.002.

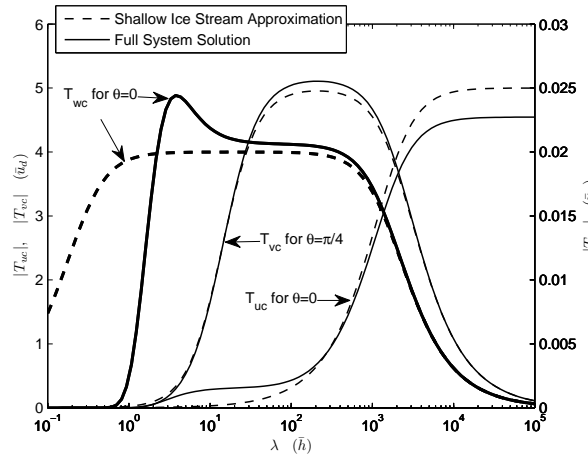


Figure 11.13: Steady-state response of surface longitudinal (u), transverse (v), and vertical (w) velocity components to a basal slipperiness perturbation. The surface slope is 0.002 and the slip ratio $\bar{C}=10$. The T_{uc} and T_{wc} amplitudes are calculated for slipperiness perturbations aligned transversely to the flow direction ($\theta=0$). For T_{vc} , $\theta=45$ degrees. Of the two y axis the scale to the left is for the horizontal velocity components (T_{uc} and T_{wc}), and the one to the right is the scale for T_{uc} .

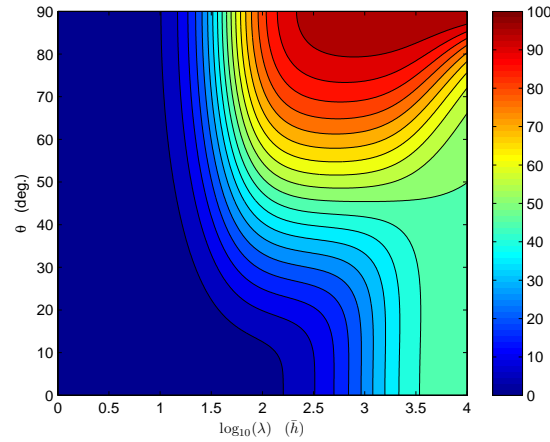


Figure 11.14: Steady-state response of the surface longitudinal (Δu) velocity component to a basal slipperiness perturbation in the shallow-ice-stream approximation. The surface slope is 0.002 and the slip ratio $\bar{C}=99$.

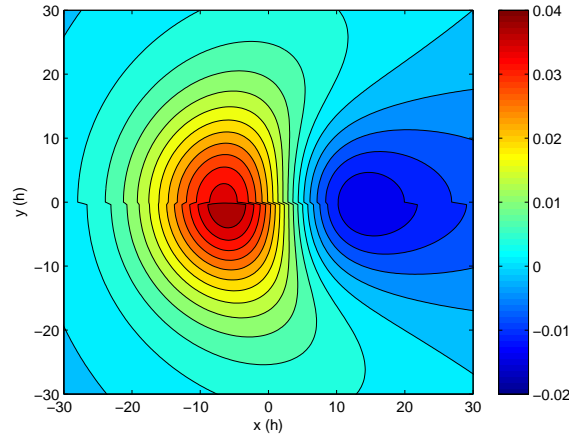


Figure 11.15: **(a)** Surface topography response to a flow over a Gaussian-shaped bedrock disturbance as given by a FS (lower half of figure) and a SSTREAM solution (upper half of figure). The mean flow direction is from left to right. Surface slope is 3 degrees and mean basal velocity equal to mean deformational velocity ($\bar{C}=1$). The spatial unit is one mean ice thickness (\bar{h}). The Gaussian-shaped bedrock disturbance has a width of $10 \bar{h}$ and its amplitude is $0.1 \bar{h}$. The problem definition is symmetrical about the x axis ($y=0$) and any deviations in the figure from this symmetry are due to differences in the FS and the SSTREAM solutions.

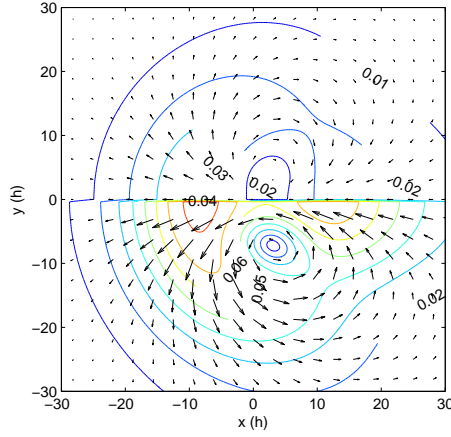


Figure 11.15: **(b)** Response in surface velocity to a Gaussian-shaped bedrock perturbation. All parameters are equal to those in Fig. 11.15a. The contour lines give horizontal speed and the vectors the horizontal velocities. The velocity unit is mean-deformational velocity (\bar{u}_d). The slip ratio is equal to one, and the mean surface velocity is $2\bar{u}_d$. The upper half of the figure is the SSTREAM solution and the lower half the corresponding FS solution.

and

$$\tau_d = \rho g \bar{h} \sin \alpha, \quad (11.32)$$

is the driving stress.

We then find that the first-order field equations are

$$4\eta \bar{h} \partial_{xx}^2 \Delta u - \gamma \Delta u = \rho g \bar{h} \cos \alpha \partial_x \Delta s - \rho g \sin \alpha \Delta h, \quad (11.33)$$

and these can be solved using standart Fourier and Laplace transformation methods as done above for the case of bedrock pertubations.

Part III

Appendices

Appendix A

Calculating vertical surface velocity

The sign convention for upper- and lower-surface mass balance is such that the kinematic boundary conditions at the upper and lower surfaces read, respectively,

$$\partial_t s + u \partial_x s + v \partial_y s - w_s = a_s, \quad (\text{A.1})$$

$$\partial_t b + u \partial_x b + v \partial_y b - w_b = -a_b, \quad (\text{A.2})$$

i.e. adding ice is always defined as positive surface mass balance.

Subtracting (A.2) from (A.1) gives

$$\partial_t h + u \partial_x h + v \partial_y h - w_s + w_b = a_s + a_b,$$

where it has been used that u does not change with depth. Now using (A.5) gives

$$\partial_t h + u \partial_x h + v \partial_y h + h(\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy}) = a_s + a_b,$$

or

$$\partial_t h + \partial_x(uh) + \partial_y(vh) = a_s + a_b, \quad (\text{A.3})$$

hence, in the flux-conservation equation both upper and lower mass balance terms have the same sign.

A.1 grounded part

On the grounded part $\partial_t s = \partial_t h$ and the kinematic boundary condition gives

$$w_s = \partial_t h + u \partial_x s + v \partial_y s - a_s, \quad (\text{A.4})$$

but this requires $\partial_t h$ to be known before we can calculate w_s . An alternative approach is to integrate the vertical strain rate $\dot{\epsilon}_{zz}$ over the thickness, use the mass conservation equation and the fact that horizontal strain rates do not change across the thickness, to arrive at

$$w_s = w_b - h(\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy}). \quad (\text{A.5})$$

We now calculate w_b from the kinematic boundary condition at the lower surface as

$$w_b = a_b + u \partial_x b + v \partial_y b,$$

and insert into (A.5) arriving at

$$w_s = a_b + u \partial_x b + v \partial_y b - h(\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy}), \quad (\text{A.6})$$

which represents a convenient way of calculation w_s once the horizontal velocity field has been determined.

A.2 floating part

Where the ice is afloat

$$s = S + (1 - \rho/\rho_o) h$$

i.e.

$$\partial_t s = (1 - \rho/\rho_o) \partial_t h$$

The kinematic boundary condition at the surface gives

$$w_s = \partial_t s + u \partial_x s + v \partial_y s - a_s$$

and therefore

$$w_s = (1 - \rho/\rho_o) \partial_t h + u \partial_x s + v \partial_y s - a_s \quad (\text{A.7})$$

If $\partial_t h$ is known (A.7) can be used to calculate w_s , otherwise we use (A.3) and find that

$$w_s = (1 - \rho/\rho_o) (a_s + a_b - \partial_x q_x - \partial_y q_y) + u \partial_x s + v \partial_y s - a_s \quad (\text{A.8})$$

An alternative way of calculating w_s is to insert the floating condition

$$s = (1 - \rho_o/\rho) b$$

into (A.1) and to use (A.2) to get rid of $\partial_t b$

$$(1 - \rho_o/\rho)(w_b - a_b - u \partial_x b - v \partial_y b) + (1 - \rho_o/\rho)(u \partial_x b + v \partial_y b) - w_s = a_s \quad (\text{A.9})$$

to arrive at the simple and intuitive expression

$$(1 - \rho_o/\rho)(w_b - a_b) = a_s + w_s \quad (\text{A.10})$$

and then to use the (A.5) to get rid of w_b leading to

$$w_s = -(1 - \rho/\rho_o) (h(\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy}) - a_b) - a_s \rho/\rho_o \quad (\text{A.11})$$

The above expression shows that adding ice to the surface ($a_s > 0$) causes neg. vertical surface velocity, as does horizontal divergent flow ($\dot{\epsilon}_{xx} + \dot{\epsilon}_{yy} > 0$), and basal melting ($a_b < 0$).

Appendix B

Integral theorem

If f and g are scalar functions then in x and y directions we have

$$\int_{\Omega} f \partial_x g \, d\Omega = - \int_{\Omega} \partial_x f g \, d\Omega + \oint_{\partial\Omega} f g n_x \, d\Gamma \quad (\text{B.1})$$

$$\int_{\Omega} f \partial_y g \, d\Omega = - \int_{\Omega} \partial_y f g \, d\Omega + \oint_{\partial\Omega} f g n_y \, d\Gamma \quad (\text{B.2})$$

If we write $g = g_x$ in the upper one and $g = g_y$ in the lower one, add them together and define $\mathbf{g} = (g_x, g_y)^T$ and $\hat{\mathbf{n}} = (n_x, n_y)^T$ then we arrive at

$$\int_{\Omega} f \nabla_{xy} \cdot \mathbf{g} \, d\Omega = - \int_{\Omega} \nabla_{xy} f \cdot \mathbf{g} \, d\Omega + \oint_{\partial\Omega} f \mathbf{g} \cdot \hat{\mathbf{n}} \, d\Gamma \quad (\text{B.3})$$

Appendix C

Definition of gradients in terms of directional derivatives and inner products

Sensitivities are directional derivatives. The directional derivative of the scalar function $J(p)$ in the direction ϕ is denoted by $Df(p)[\phi]$ and defined as

$$\begin{aligned} Df(p)[\phi] &= \lim_{\epsilon \rightarrow 0} \frac{d}{d\epsilon} f(p + \epsilon\phi) \\ &= \lim_{\epsilon \rightarrow 0} \frac{f(p + \epsilon\phi) - f(p)}{\epsilon} \end{aligned}$$

The directional derivative is sometimes written as $\delta f(p, \phi)$ or as $f'(p, \phi)$ i.e.

$$Df(p)[\phi] = f'(p, \phi) = \delta f(p, \phi)$$

are just different ways of writing the directional derivative.

We define the gradient through

$$Df(p)[\phi] = \langle \nabla J(p), \phi \rangle_H$$

where H is a Hilbert space and $f : H \rightarrow \mathbb{R}$. Here $\nabla J(p)$ is the gradient of J , and the expression above *defines* the gradient in terms of the (directional) derivative for a given inner product.

In other words, for a function $f : H \rightarrow \mathbb{R}$ the gradient is defined as the Riez-representation for the directional derivative $Df(p)[\phi]$ through

$$\langle \nabla f(p), \phi \rangle_H = Df(p)[\phi]$$

The directional derivative depends on the inner product \langle, \rangle_H and the gradient is not defined without specifying the inner product.

Example: Consider the case $H = \mathbb{R}^n$ with the inner product

$$\langle \mathbf{x}, \mathbf{y} \rangle = \mathbf{x}^T \mathbf{M} \mathbf{y}$$

where M is symmetric and positive definite (for example the mass matrix or any covariance matrix.)

The directional derivative is

$$Df(p)[\phi] = \frac{\partial f}{\partial p_q} \phi_q = \langle M^{-1} \partial f / \partial p_q, \phi_q \rangle$$

and therefore

$$[\nabla f]_p = [M^{-1}]_{pq} \partial f / \partial \phi_q$$

Had we instead used the Euclidean inner product as $\langle \mathbf{x}, \mathbf{y} \rangle = \mathbf{x}^T \mathbf{y}$ the corresponding Euclidean gradient would be

$$[\nabla_E f]_p = \partial f / \partial \phi_p \tag{C.1}$$

or

$$\nabla f = M^{-1} \nabla_E f \quad (\text{C.2})$$

where the subscript E denotes the Euclidean gradient. This distinction becomes important in the application of the adjoint method where we obtain a gradient that is dependent on the natural FE inner product

$$\begin{aligned} \langle f, g \rangle &= \int f g dA \\ &= \int f_p \phi_p g_q \phi_q dA \\ &= \mathbf{f}^T \mathbf{M} \mathbf{g} \end{aligned}$$

Hence in FE the dual pairing is

$$\langle f, g \rangle = \mathbf{f}^T \mathbf{M} \mathbf{g}$$

where f .

The adjoint L^* of a given operator L is defined as

$$\langle L^* f, g \rangle = \langle f, Lg \rangle$$

for any f and g .

If we are working in $H_1 = \mathbb{R}^{d_1}$ and the dual space is $H_2 = \mathbb{R}^{d_2}$ and

$$\langle f, g \rangle_{H_1, H_2} = \mathbf{f}^T \mathbf{g}$$

and

$$\langle L^* f, g \rangle_{H_1, H_2} = \langle f, Lg \rangle_{H_1, H_2}$$

and we denote by \mathbf{L} and \mathbf{L}^* the matrix representations of the continuous linear operators L and L^* , respectively, then

$$\mathbf{L}^* = \mathbf{L}^T$$

If, on the other hand, we have the dual pairings

$$\langle f, g \rangle_{H_1, H_2} = \mathbf{f}^T \mathbf{M} \mathbf{g}$$

where \mathbf{M} is a positive definite matrix, then

$$\mathbf{L}^* = \mathbf{M}^{-T} \mathbf{L}^T \mathbf{M}^T$$

as can be seen as follows

$$\begin{aligned} \langle \mathbf{M}^* f, g \rangle &= \langle f, Lg \rangle \\ &= \mathbf{f}^T \mathbf{M} (\mathbf{L} \mathbf{g}) \\ &= \mathbf{f}^T \mathbf{M} \mathbf{L} \mathbf{M}^{-1} \mathbf{M} \mathbf{g} \\ &= (\mathbf{M}^{-T} \mathbf{L}^T \mathbf{M}^T \mathbf{f})^T \mathbf{M} \mathbf{g} \\ &= \langle \mathbf{M}^{-T} \mathbf{L}^T \mathbf{M}^T \mathbf{f}, g \rangle \end{aligned}$$

We can generalise this a bit further and consider the case where the dual space has a different dimension with

$$\begin{aligned} \langle f, f \rangle_{H_1 H_1} &= \mathbf{f}^T \mathbf{M}_1 \mathbf{f} \\ \langle g, g \rangle_{H_2 H_2} &= \mathbf{g}^T \mathbf{M}_2 \mathbf{g} \end{aligned}$$

and find that

$$\mathbf{L}^* = \mathbf{M}_1^{-T} \mathbf{L}^T \mathbf{M}_2^T$$

Bibliography

- Baral, D. R. (1999). *Asymptotic theories of large scale motion, temperature and moisture distributions in land based polythermal ice shields and in floating ice shelves. A critical review and new developments*. PhD thesis, Technische Univeristaet Darmstadt.
- Glen, J. W. (1955). The creep of polycrystalline ice. *Proceedings of the Royal Society of London, Ser A*, 228(1175):519–538.
- Goldsby, D. L. and Kohlstedt, D. L. (2001). Superplastic deformation of ice: Experimental observations. *Journal of Geophysical Research*, 106(B6):11,017– 11,030.
- MacAyeal, D. R. (1989). Large-scale ice flow over a viscous basal sediment: Theory and application to Ice Stream B, Antarctica. *Journal of Geophysical Research*, 94(B4):4071–4078.
- Morland, L. W. (1984). Thermomechanical balances of ice-sheet flows. *Geophysical and Astrophysical Fluid Dynamics*, 29(3):237–266.
- Muszynski, I. and Birchfield, G. E. (1987). A coupled marine ice-stream-ice – shelf model. *Journal of Glaciology*, 33(113):3–14.
- Schoof, C. (2006). A variational approach to ice stream flow. *Journal of Fluid Mechanics*, 556:227–251.
- Steinemann, S. (1954). Results of preliminary experiments on the plasticity of ice crystals. *Journal of Glaciology*, 2:404–413.
- Steinemann, S. (1958a). Experimentelle Untersuchungen zur Plastizität von Eis. Geotechnische Serie Nr. 10, Beiträge zur Geologie der Schweiz.
- Steinemann, S. (1958b). Résultats expérimentaux sur la dynamique da la glace et leurs correlations avec le mouvement et la pétrographie des glaciers. *International Association of Scientific Hydrology*, 47:184–198.