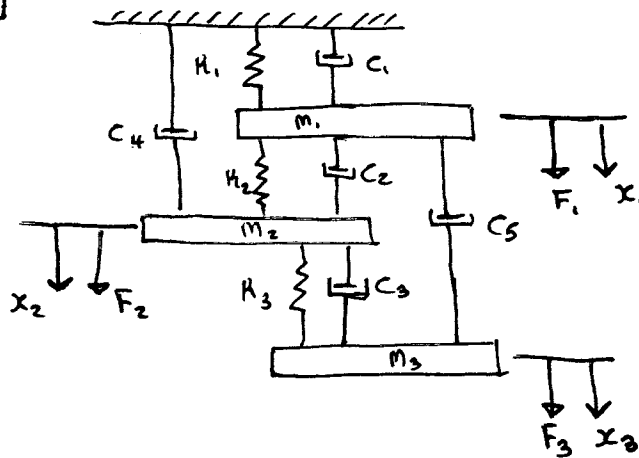


Modal summation

Example:



Find the eq'n's of motion and calculate the forced response.

Solution:

Kinetic energy : $T = (1/2)m_1\dot{x}_1^2 + (1/2)m_2\dot{x}_2^2 + (1/2)m_3\dot{x}_3^2$

Potential energy : $V = (1/2)H_1x_1^2 + (1/2)H_2(x_2 - x_1)^2 + (1/2)H_3(x_3 - x_2)^2$

Rayleigh's Formula : $R = (1/2)C_1\dot{x}_1^2 + (1/2)C_2(\dot{x}_2 - \dot{x}_1)^2 + (1/2)C_3(\dot{x}_3 - \dot{x}_2)^2 + \dots$
 $\dots (1/2)C_4\dot{x}_2^2 + (1/2)C_5(\dot{x}_3 - \dot{x}_0)^2$

The generalized force

$$\vec{Q} = \begin{Bmatrix} F_1 \\ F_2 \\ F_3 \end{Bmatrix}$$

Equations of Motion :

$$\left(\frac{d}{dt} \right) \left(\frac{\partial T}{\partial \dot{q}_i} \right) + \left(\frac{\partial V}{\partial q_i} \right) - \left(\frac{\partial T}{\partial q_i} \right) + \left(\frac{\partial R}{\partial \dot{q}_i} \right) = Q$$

where: $q_i = x_i$

then $\left(\frac{\partial V}{\partial q_1} \right) = H_1x_1 + H_2(x_1 - x_2)$
 $\left(\frac{\partial R}{\partial \dot{q}_1} \right) = C_1\dot{x}_1 + C_2(\dot{x}_1 - \dot{x}_2) + C_5(\dot{x}_1 - \dot{x}_0)$

$$\therefore m_1 \ddot{x}_1 + c_1 \dot{x}_1 + c_2(\dot{x}_1 - \dot{x}_2) + c_3(\dot{x}_1 - \dot{x}_3) + k_1 x_2 + k_2(x_1 - x_2) = F_1$$

• • • etc. For other equations

Matrix Form :

$$[M] = \begin{bmatrix} m_1 & 0 & 0 \\ 0 & m_2 & 0 \\ 0 & 0 & m_3 \end{bmatrix}$$

$$[C] = \begin{bmatrix} c_1 + c_2 + c_3 & -c_2 & -c_3 \\ -c_2 & c_2 + c_3 + c_4 & -c_3 \\ -c_3 & -c_3 & c_3 + c_5 \end{bmatrix}$$

$$[K] = \begin{bmatrix} k_1 + k_2 & -k_2 & 0 \\ -k_2 & k_2 + k_3 & -k_3 \\ 0 & -k_3 & k_3 \end{bmatrix}$$

Given:

$$m_1 = m_2 = m_3 = m$$

$$k_1 = k_2 = k_3 = k$$

$$c_4 = c_5 = 0 ; \boxed{\gamma_1 = \gamma_2 = \gamma_3 = 0.01}$$

$$F_1 = F_2 = F_3 = F_0 \cos(\omega t)$$

$$\hookrightarrow \omega = 1.75 \sqrt{k/m}$$

Step ①: The natural frequencies and modal shape (w/o damping)

$$[M] = m \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

$$[K] = k \begin{bmatrix} 2 & -1 & 0 \\ -1 & 2 & -1 \\ 0 & -1 & 1 \end{bmatrix}$$

$$\Rightarrow \left(-\omega^2 m \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix} + k \begin{bmatrix} 2 & -1 & 0 \\ -1 & 2 & -1 \\ 0 & -1 & 1 \end{bmatrix} \right) \vec{u} = 0$$

→ After solving:

$$\omega_1 = 0.44504 \times \sqrt{k/m}$$

$$\omega_2 = 1.2470 \times \sqrt{k/m}$$

$$\omega_3 = 1.8019 \times \sqrt{k/m}$$

$$\rightarrow \vec{u}_1 = \frac{1}{\sqrt{m}} \begin{Bmatrix} 0.32799 \\ 0.59101 \\ 0.73698 \end{Bmatrix}$$

$$\vec{u}_2 = \frac{1}{\sqrt{m}} \begin{Bmatrix} 0.73698 \\ 0.32799 \\ -0.59101 \end{Bmatrix}$$

$$\vec{u}_3 = \frac{1}{\sqrt{m}} \begin{Bmatrix} 0.59101 \\ -0.73698 \\ 0.32799 \end{Bmatrix}$$

Step ②: $\vec{U} = [\vec{u}_1 \ \vec{u}_2 \ \vec{u}_3]_{3 \times 3}$

Verify: $[\vec{U}]^T [\vec{M}] [\vec{U}] = [\vec{I}]$

$$[\vec{U}]^T [\vec{K}] [\vec{U}] = \text{diag}(\omega_1^2, \omega_2^2, \omega_3^2)$$

Define: $\vec{x} = [\vec{U}] \vec{q}$

The equations of motion

$$[\vec{M}] \ddot{\vec{x}} + [\vec{C}] \dot{\vec{x}} + [\vec{K}] \vec{x} = \vec{Q}$$

$$[\vec{M}] [\vec{U}] \ddot{\vec{q}} + [\vec{C}] [\vec{U}] \dot{\vec{q}} + [\vec{K}] [\vec{U}] \vec{q} = \vec{Q}$$

$$[\vec{U}]^T [\vec{M}] [\vec{U}] \ddot{\vec{q}} + [\vec{U}]^T [\vec{C}] [\vec{U}] \dot{\vec{q}} + [\vec{U}]^T [\vec{K}] [\vec{U}] \vec{q} = [\vec{U}]^T \vec{Q}$$

$$\rightarrow \ddot{\vec{q}} + [\vec{U}]^T [\vec{C}] [\vec{U}] \dot{\vec{q}} + \text{diag}(\omega_1^2, \omega_2^2, \omega_3^2) \vec{q} = [\vec{U}]^T \vec{Q}$$

Proportional damping:

$$[\vec{U}]^T [\vec{C}] [\vec{U}] = \text{diag}(2\zeta_1 \omega_1, 2\zeta_2 \omega_2, 2\zeta_3 \omega_3)$$

$$\begin{Bmatrix} Q_{10} \\ Q_{20} \\ Q_{30} \end{Bmatrix} = [\vec{U}]^T \vec{Q} = [\vec{U}]^T \begin{Bmatrix} F_1 \\ F_2 \\ F_3 \end{Bmatrix}$$

$$= [\vec{U}]^T F_0 \begin{Bmatrix} 1 \\ 1 \\ 1 \end{Bmatrix} \cos(\omega t) = \frac{F_0}{\sqrt{m}} \begin{Bmatrix} 1.6560 \\ 0.47395 \\ 0.18202 \end{Bmatrix} \cos(\omega t)$$

Equations in the Modal Coordinates

$$\begin{cases} \ddot{q}_1 + 2\zeta_1\omega_1\dot{q}_1 + \omega_1^2 q_1 = Q_{10} = (F_0/\sqrt{m})(1.6560)\cos(\omega t) \\ \ddot{q}_2 + 2\zeta_2\omega_2\dot{q}_2 + \omega_2^2 q_2 = Q_{20} = (F_0/\sqrt{m})(0.47395)\cos(\omega t) \\ \ddot{q}_3 + 2\zeta_3\omega_3\dot{q}_3 + \omega_3^2 q_3 = Q_{30} = (F_0/\sqrt{m})(0.18202)\cos(\omega t) \end{cases}$$

Step (3): Steady-state response of each modal coordinate

$$\boxed{q_i(t) = q_{i0} \cos(\omega t - \phi_i)} \quad ; \quad i = 1, 2, 3$$

and

$$q_{i0} = \frac{Q_{i0}}{\omega_i^2} \cdot \frac{1}{\sqrt{(1 - (\omega/\omega_i)^2)^2 + (2\zeta_i \omega/\omega_i)^2}}$$

$$\phi_i = \tan^{-1} \left(\frac{2\zeta_i(\omega/\omega_i)}{1 - (\omega/\omega_i)^2} \right)$$

$$i = 1 : \quad \frac{\omega}{\omega_1} = \frac{1.75 \sqrt{\text{K/m}}}{0.44504 \sqrt{\text{K/m}}} = 3.9322$$

$$q_{10} = 0.57811 (F_0 \sqrt{m} / \text{K})$$

$$\phi_1 = 3.1367$$

$$i = 2 : \quad \frac{\omega}{\omega_2} = \frac{1.75}{1.2470} = 1.4034$$

$$q_{20} = 1.0980$$

$$\phi_2 = 3.1187$$

$$i = 3 : \quad \frac{\omega}{\omega_3} = \frac{1.75}{1.809} = 0.97118$$

$$q_{30} = 8.4938$$

$$\phi_3 = 0.32941$$

$$\vec{x} = [U] \vec{q}$$

$$= [U] \begin{Bmatrix} q_1(t) \\ q_2(t) \\ q_3(t) \end{Bmatrix}$$

$$= [U] \begin{Bmatrix} q_{10} \cos(\omega t - \phi_1) \\ q_{20} \cos(\omega t - \phi_2) \\ q_{30} \cos(\omega t - \phi_3) \end{Bmatrix}$$

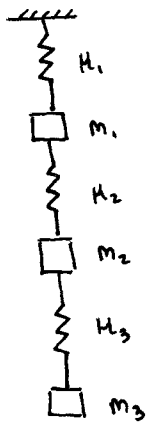
$$= \frac{1}{\sqrt{m}} \begin{bmatrix} 0.32799 & 0.73098 & 0.59101 \\ 0.59101 & 0.32799 & -0.73098 \\ 0.73098 & -0.59101 & 0.32799 \end{bmatrix} \begin{Bmatrix} 0.57811 \cos(\omega t - 3.1362) \\ 10.980 \cos(\omega t - 3.112) \\ 8.4930 \cos(\omega t - 0.32944) \end{Bmatrix} \cdot \frac{F_0 \sqrt{m}}{K}$$

$$= \frac{F_0}{K} \begin{Bmatrix} 3.7515 \cdot \cos \omega t + 1.6483 \sin(\omega t) \\ -6.6248 \cdot \cos \omega t - 2.0127 \sin(\omega t) \\ 2.9587 \cdot \cos \omega t + 0.88472 \sin(\omega t) \end{Bmatrix}$$

Determination of Natural Frequencies

Dunkerley's Formula :

the Fundamental natural Freq.



$$(-\omega^2 [M] + [K]) \vec{u} = 0$$

The Flexibility matrix

$$[A] = [K]^{-1}$$

$$[K] = [A]^{-1}$$

$$\rightarrow [A](-\omega^2 [M] + [K]) \vec{u} = 0$$

$$(-\omega^2 [A][M] + [A][K]) \vec{u} = 0$$

$$\rightarrow (-[A][M] + (1/\omega^2)[I]) \vec{u} = 0$$

Eigenvalues :

$$|- [A][M] + (1/\omega^2)[I]| = 0$$

$$[A] = \begin{bmatrix} a_{11} & a_{12} & a_{13} \\ a_{21} & a_{22} & a_{23} \\ a_{31} & a_{32} & a_{33} \end{bmatrix} \quad ; \quad [M] = \begin{bmatrix} m_1 & 0 & 0 \\ 0 & m_2 & 0 \\ 0 & 0 & m_3 \end{bmatrix}$$

$$[A][M] = \begin{bmatrix} a_{11}m_1 & a_{12}m_2 & a_{13}m_3 \\ a_{21}m_1 & a_{22}m_2 & a_{23}m_3 \\ a_{31}m_1 & a_{32}m_2 & a_{33}m_3 \end{bmatrix}$$

$$\Rightarrow \begin{vmatrix} a_{11}m_1 - (1/\omega^2) & a_{12}m_2 & a_{13}m_3 \\ a_{21}m_1 & a_{22}m_2 - (1/\omega^2) & a_{23}m_3 \\ a_{31}m_1 & a_{32}m_2 & a_{33}m_3 - (1/\omega^2) \end{vmatrix} = 0$$

$$\Rightarrow [a_{11}m_1 - (1/\omega^2)][a_{22}m_2 - (1/\omega^2)][a_{33}m_3 - (1/\omega^2)] \dots$$

$$\dots + a_{12}m_2 a_{23}m_3 + a_{31}m_1 + a_{21}m_1 a_{32}m_2 a_{13}m_3 \dots$$

$$\dots - a_{13}m_3 a_{31}m_1 (a_{22}m_2 - 1/\omega^2) \dots$$

$$\dots - a_{12}m_2 a_{21}m_1 (a_{33}m_3 - 1/\omega^2) \dots$$

$$\dots - a_{22}m_2 a_{32}m_2 (a_{11}m_1 - 1/\omega^2) = 0$$

Cubic eq. w.r.t. $1/\omega^2$

$$(1/\omega^2)^3 - (a_{11}m_1 + a_{22}m_2 + a_{33}m_3)(1/\omega^2)^2 + (\dots)(1/\omega^2) + (\dots) = 0$$

The three roots: $1/\omega_1^2$; $1/\omega_2^2$; $1/\omega_3^2$

$$1/\omega_1^2 + 1/\omega_2^2 + 1/\omega_3^2 = a_{11}m_1 + a_{22}m_2 + a_{33}m_3$$

$$(1/\omega^2 - 1/\omega_1^2)(1/\omega^2 - 1/\omega_2^2)(1/\omega^2 - 1/\omega_3^2) = 0$$

When $\omega_2 \gg \omega_1$, $\omega_3 \gg \omega_1$

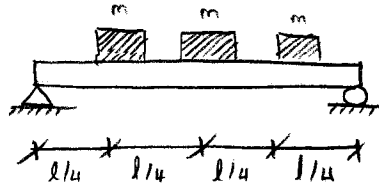
$$1/\omega_1^2 \approx 1/\omega_2^2 + 1/\omega_3^2 + 1/\omega_3^2 = a_{11}m_1 + a_{22}m_2 + a_{33}m_3$$

$$\therefore \omega_1 = \frac{1}{\sqrt{a_{11}m_1 + a_{22}m_2 + a_{33}m_3}}$$

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$$\frac{1}{\omega_1^2} + \frac{1}{\omega_2^2} + \dots + \frac{1}{\omega_n^2} = a_{11}m_1 + a_{22}m_2 + \dots + a_{nn}m_n$$

$$\frac{1}{\omega_1^2} \approx a_{11}m_1 + \dots + a_{nn}m_n$$

Example $EI = \text{const.}$

Estimate the Fundamental Frequency.

Solution: $a_{11} = a_{33} = \left(\frac{3}{256}\right)\left(\frac{l^3}{EI}\right)$ — by symmetry

$$a_{22} = \left(\frac{1}{48}\right)\left(\frac{l^3}{EI}\right)$$

$$\therefore \frac{1}{\omega_1^2} = a_{11}m_1 + a_{22}m_2 + a_{33}m_3$$

$$= \left(\frac{3}{256}\right)\left(\frac{l^3}{EI}\right)m_1 + \left(\frac{1}{48}\right)\left(\frac{l^3}{EI}\right)m_2 + \left(\frac{3}{256}\right)\left(\frac{l^3}{EI}\right)m_3$$

$$\frac{1}{\omega_1^2} = 0.04427 \left(\frac{l^3}{EI}\right)m$$

$$\omega_1 = 4.754 \sqrt{EI/ml^3}$$

The exact solution :

$$\omega_{1,\text{exact}} = 4.734 \sqrt{EI/ml^3} \quad ??? \quad (\text{should always be larger than } \omega_1)$$

$$\frac{1}{\omega_{1,d}^2} = \frac{1}{\omega_{1,\text{exact}}^2} + \frac{1}{\omega_2^2} + \dots + \frac{1}{\omega_n^2} > \frac{1}{\omega_{1,\text{exact}}^2}$$

Rayleigh's Method

$$[M]\ddot{\vec{x}} + [K]\vec{x} = 0$$

The motion :

$$\vec{x} = e^{i\omega t} \vec{u}$$

 ω : the natural freq. \vec{u} : the mode shape

$$\Rightarrow ([K] - \omega^2[M])\vec{u} = 0$$

$$\Rightarrow \vec{u}^T ([K] - \omega^2[M])\vec{u} = 0$$

$$\Rightarrow \vec{u}^T [K]\vec{u} - \omega^2 \vec{u}^T [M]\vec{u} = 0$$

$$\Rightarrow \omega^2 = \frac{\vec{u}^T [K]\vec{u}}{\vec{u}^T [M]\vec{u}}$$

Define : $R(\vec{x}) = \frac{\vec{x}^T [K]\vec{x}}{\vec{x}^T [M]\vec{x}}$

if \vec{x} is close to modal shape, then it approximates the natural frequency

$$T = (1/2) \dot{\vec{x}}^T [M] \dot{\vec{x}}$$

$$V = (1/2) \vec{x}^T [K] \vec{x}$$

→ Harmonic motion:

$$\vec{x} = \vec{X} e^{i\omega t}$$

$$\dot{\vec{x}} = \vec{X} \cdot i\omega e^{i\omega t}$$

→ The max kinetic energy:

$$T_{\max} = (1/2) \omega^2 \vec{X}^T [M] \vec{X}$$

→ The max potential energy:

$$V_{\max} = (1/2) \vec{X}^T [K] \vec{X}$$

Conservation:

$$T_{\max} = V_{\max}$$

$$(1/2) \omega^2 \vec{X}^T [M] \vec{X} = (1/2) \vec{X}^T [K] \vec{X}$$

$$\omega^2 = \frac{\vec{X}^T [K] \vec{X}}{\vec{X}^T [M] \vec{X}}$$

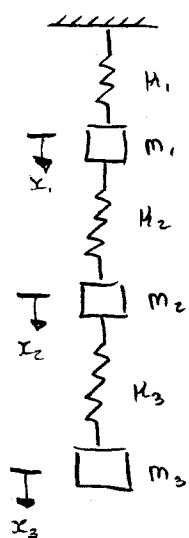
Verify:

$$\omega_1^2 \leq R(\vec{x}) \leq \omega_n^2$$

\vec{x} : the static deflection (disp.) of the system

Then $R(\vec{x})$: approximation of the Fundamental Freq.

Example



$$\text{Let } m_1 = m_2 = m_3 = m$$

$$k_1 = k_2 = k_3 = k$$

Estimate the Fundamental Freq. of the system.

$$\text{Solution: } [M] = \begin{bmatrix} m & 0 & 0 \\ 0 & m & 0 \\ 0 & 0 & m \end{bmatrix} = m \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

$$[K] = k \begin{bmatrix} 2 & -1 & 0 \\ -1 & 2 & -1 \\ 0 & -1 & 1 \end{bmatrix}$$

$$\text{Take } \vec{x} = \begin{Bmatrix} 1 \\ 2 \\ 3 \end{Bmatrix} = \begin{Bmatrix} x_1 \\ x_2 \\ x_3 \end{Bmatrix}$$

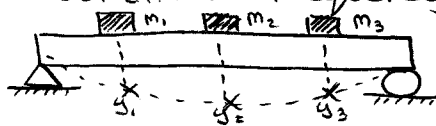
$$R(x) = \frac{\vec{x}^T [K] \vec{x}}{\vec{x}^T [M] \vec{x}} = \frac{(1, 2, 3) \begin{bmatrix} -2 & -1 & 0 \\ -1 & 2 & -1 \\ 0 & -1 & 1 \end{bmatrix} \begin{Bmatrix} 1 \\ 2 \\ 3 \end{Bmatrix}}{(1, 2, 3) \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix} \begin{Bmatrix} 1 \\ 2 \\ 3 \end{Bmatrix}} \frac{k}{m}$$

$$= 0.2143 \left(\frac{k}{m} \right)$$

$$\therefore \omega_1^2 \approx R(x) = 0.2143 \frac{k}{m}$$

$$\omega_1 = 0.4629 \sqrt{k/m} \quad (\omega_{1, \text{exact}} = 0.4450 \sqrt{k/m})$$

Fundamental Frequency of beams and shafts



beam/shaft : weightless

The max potential energy = the max strain energy
= the work done by all the forces

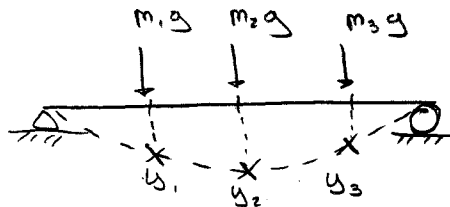
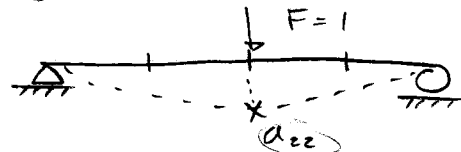
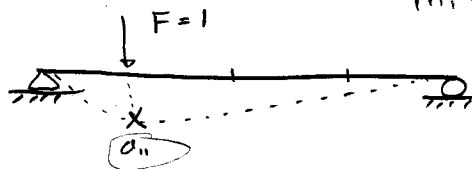
$$V_{\max} = (1/2)(m_1 g y_1 + m_2 g y_2 + m_3 g y_3)$$

$$T_{\max} = (1/2) \omega^2 (m_1 y_1^2 + m_2 y_2^2 + m_3 y_3^2)$$

$$\Rightarrow (1/2)(m_1 g y_1 + m_2 g y_2 + m_3 g y_3)$$

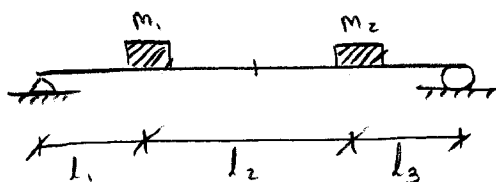
$$= (1/2) \omega^2 (m_1 y_1^2 + m_2 y_2^2 + m_3 y_3^2)$$

$$\Rightarrow \omega^2 = \frac{m_1 g y_1 + m_2 g y_2 + m_3 g y_3}{m_1 y_1^2 + m_2 y_2^2 + m_3 y_3^2}$$



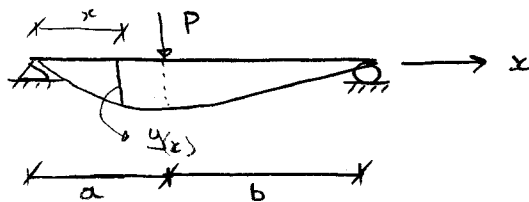
Example:

Estimate the Fundamental Freq. of the beam as shown.



where $l_1 = 1\text{ m}$; $l_2 = 3\text{ m}$; $l_3 = 2\text{ m}$
 $m_1 = 20\text{ kg}$; $m_2 = 50\text{ kg}$
 $EI = \text{const.}$

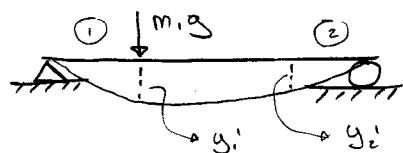
Solution :



The deflection :

$$y(x) = \begin{cases} \frac{Pbx}{6EI} (l^2 - b^2 - x^2) & ; 0 \leq x \leq a \\ -\frac{Pa(l-x)}{6EI} (a^2 + x^2 - 2lx) & ; a \leq x \leq b \end{cases}$$

→ The deflection due to $P = m_1 g$



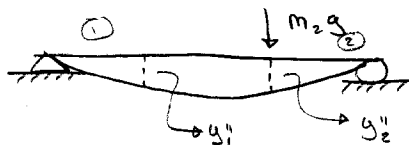
$$l = l_1 + l_2 + l_3 = 6\text{ m}$$

$$y_1' = \frac{Pbx}{6EI} (l^2 - b^2 - x^2) \Big|_{x=1} = \frac{(20 \times 9.81 \times 5)(1)}{6EI(6)} (6^2 - 5^2 - 1^2)$$

$$= \frac{272.5}{EI}$$

$$y_2' = -\frac{(20 \times 9.81)(1)(6-4)}{6EI(6)} (1^2 + 4^2 - 2 \times 6 \times 4) = \frac{337.9}{EI}$$

→ The deflection due to $P = m_2 g$



$$y_1'' = \frac{844.75}{EI}$$

$$y_2'' = \frac{1744.0}{EI}$$

The total displacement :

$$y_1 = y_1' + y_1'' = (272.5/EI) + (844.75/EI) = (1117.25/EI)$$

$$y_2 = y_2' + y_2'' = (337.9/EI) + (1744.0/EI) = (2081.9/EI)$$

$$\therefore \omega_1^2 = \frac{(m_1 y_1 + m_2 y_2) g}{m_1 y_1^2 + m_2 y_2^2}$$

$$\Rightarrow \omega_1 = 0.07166 \sqrt{EI}$$

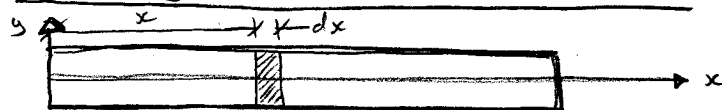
Dunkerley's Formula :

$$a_{11} = \frac{y_1''}{m_1 g} \quad ; \quad a_{22} = \frac{y_2''}{m_2 g}$$

$$\begin{aligned} 1/\omega_1^2 &= a_{11} m_1 + a_{22} m_2 \\ &= \frac{y_1''}{m_1 g} m_1 + \frac{y_2''}{m_2 g} m_2 \\ &= \frac{1}{9.81} \left(\frac{272.5}{EI} + \frac{1744.0}{EI} \right) \end{aligned}$$

$$\Rightarrow \omega_1 = 0.06974 \sqrt{EI}$$

Rayleigh's Method For a beam



mass density $\rho(x)$

area of cross-section $A(x)$

bending stiffness $EI(x)$

$$y(x, t) = y(x) e^{i\omega t}$$

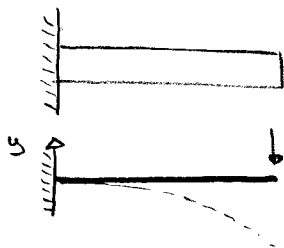
Max kinetic energy :

$$\begin{aligned} T_{\max} &= \left(\frac{1}{2} \right) \omega^2 \int_0^l \rho A dx \cdot y(x)^2 \\ &= \left(\frac{1}{2} \right) \omega^2 \int_0^l \rho A y^2 dx \end{aligned}$$

Max potential energy :

$$\begin{aligned} V_{\max} &= \left(\frac{1}{2} \right) \int_0^l EI (y'')^2 dx \\ (y'' = d^2 y / dx^2) \end{aligned}$$

$$\therefore \omega^2 = \frac{\int_0^l EI (y'')^2 dx}{\int_0^l \rho A y^2 dx}$$



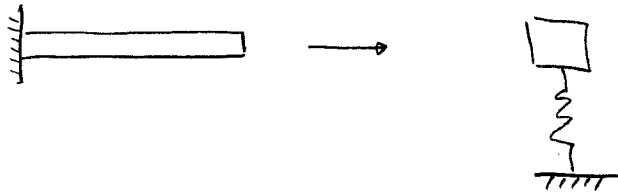
$\rho A, EI$ const.

$$y = \frac{-Px^2}{6EI} (3l - x)$$

$$\omega^2 = (140/11) \dots$$

$$\omega = 3.56753 \sqrt{EI/\rho A l^4}$$

$$\omega_{\text{exact}} = 3.5602 \sqrt{EI/\rho A l^4}$$



$$M = \frac{33}{140} \rho A l$$

$$k = \frac{3EI}{l^3}$$