

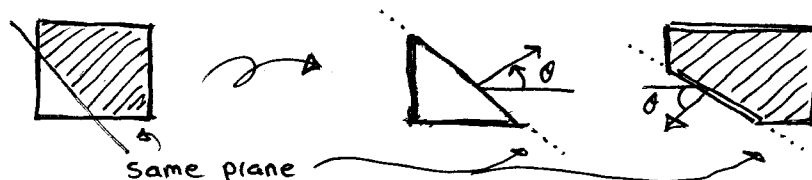
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Cont'd From previous example:

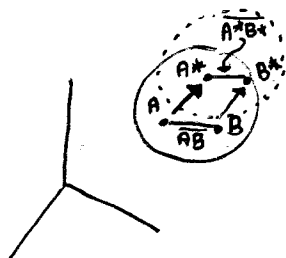
$$\begin{aligned}\sigma_1 &= 10 & \vec{N}_1 &= -0.6\vec{j} + 0.8\vec{k} \\ \sigma_2 &= 5 & \vec{N}_2 &= \vec{i} \\ \sigma_3 &= -15 & \vec{N}_3 &= 0.8\vec{i} + 0.6\vec{j}\end{aligned}$$

$$\text{For } \sigma_1 = 10, \quad \vec{N}_1 = 0.6\vec{j} - 0.8\vec{k}$$

either solution  
works - From  
same line.



## 2.6 Deformation



$$A(x, y, z)$$

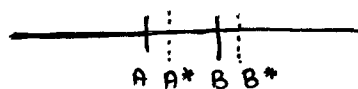
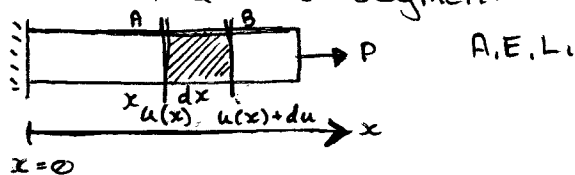
$$A^*(x^*, y^*, z^*)$$

$$\text{disp: } \vec{AA^*} = (u, v, w)$$

$$\Rightarrow \begin{cases} u = x^* - x \\ v = y^* - y \\ w = z^* - z \end{cases}$$

## 2.8 Small Displacement Theory

1° 1D: Strain of a line segment



line segment AB:  $dx$

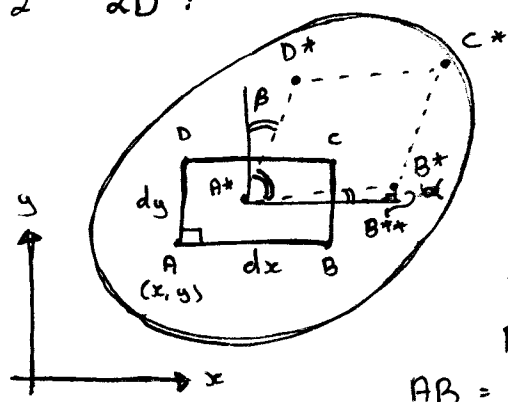
line segment  $A^*B^*$ :  $x + dx + u(x) + du - (x + u) = du + dx$

The normal strain of line segment AB:

$$\epsilon = \frac{A^*B^* - AB}{AB} = \frac{du + dx - dx}{dx} = \frac{du}{dx}$$

$$\boxed{\epsilon(x) = \frac{du(x)}{dx}}$$

2° 2D :



$$A \rightarrow A^* \\ (x, y) \rightarrow (x+u, y+v) \\ \begin{cases} u = u(x, y) \\ v = v(x, y) \end{cases}$$

$$A(x, y) \rightarrow A^*(x+u, y+v) \\ B(x+dx, y) \rightarrow B^*(x+dx+u_B, y+v_B)$$

$$AB = dx \\ A^*B^* = x+dx+u_B - (x+u) = u_B - u + dx$$

Normal strain of AB :

$$E_{xx} = \frac{A^*B^* - AB}{AB} = \frac{u_B - u + dx - dx}{dx} \\ = \frac{u_B - u}{dx}$$

$$\text{Here, } u = u(x, y) \\ u_B = u(x+dx, y) \\ = u(x, y) + \frac{du}{dx} \cdot dx + \dots$$

$$u_B - u = \frac{\partial u}{\partial x} dx \Rightarrow \frac{1}{dx} \cdot \left( \frac{\partial u}{\partial x} \right) dx$$

thus

$$E_{xx} = \frac{\partial u}{\partial x} ; E_{yy} = \frac{\partial v}{\partial y}$$

Elastic Shear Strain

$$E_{xy} = \frac{1}{2}(\alpha + \beta) = \left( \frac{1}{2} \left( \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) \right)$$

$$3^\circ \text{ 3D : } A(x, y, z) \rightarrow A^*(x+u, y+v, z+w)$$

$$\begin{cases} E_{xx} = \frac{\partial u}{\partial x} ; E_{yy} = \frac{\partial v}{\partial y} ; E_{zz} = \frac{\partial w}{\partial z} \\ E_{xy} = \frac{1}{2} \left( \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) ; E_{yz} = \frac{1}{2} \left( \frac{\partial v}{\partial z} + \frac{\partial w}{\partial y} \right) \\ E_{zx} = \frac{1}{2} \left( \frac{\partial w}{\partial x} + \frac{\partial u}{\partial z} \right) \end{cases}$$

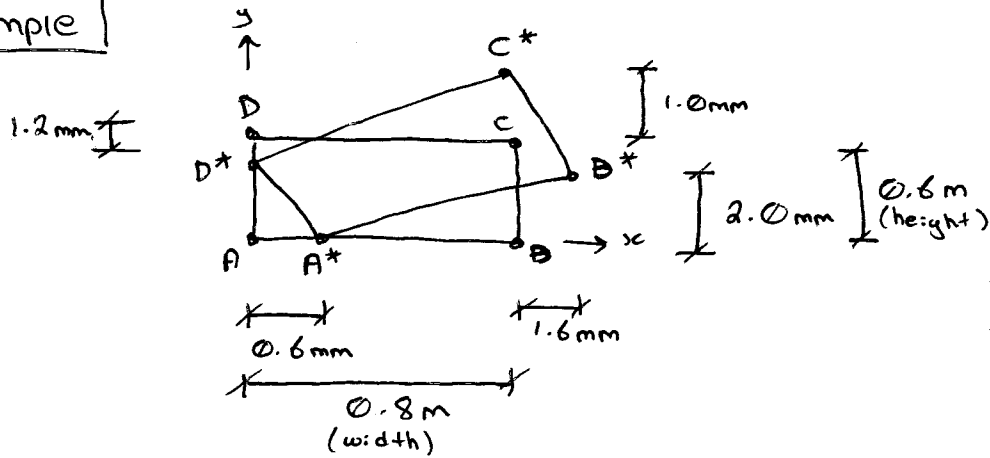
Strain compatibility rotations

$$\left\{ \begin{aligned} \frac{\partial^2 E_{yy}}{\partial x^2} + \frac{\partial^2 E_{xx}}{\partial y^2} &= 2 \frac{\partial^2 E_{xy}}{\partial x \partial y} \\ &\vdots \\ &\vdots \end{aligned} \right.$$

6 eqns

$$\frac{\partial^2 \epsilon_{zz}}{\partial x \partial y} + \frac{\partial^2 \epsilon_{xy}}{\partial z^2} = \frac{\partial^2 \epsilon_{yz}}{\partial z \partial x} + \frac{\partial^2 \epsilon_{zx}}{\partial z \partial y}$$

Example



Find strains at point A.

Solution : Assume

$$\begin{cases} u(x, y) = a_1 + b_1 x + c_1 y + d_1 xy \\ v(x, y) = a_2 + b_2 x + c_2 y + d_2 xy \end{cases}$$

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From previous Example:

$$u(x, y) :$$

$$\text{At A : } x=y=0, u_A = (0.6)(10^{-3}) = a_1 + 0 + 0 + 0$$

$$\text{At B : } x=0.8, y=0, u_B = 1.6 \times 10^{-3} = a_1 + 0.8b_1 + 0 + 0$$

$$\text{At D : } x=0, y=0.6, u_D = 0 = a_1 + 0 + 0.6c_1 + 0$$

$$\text{At C : } x=0.8, y=0.6, u_C = 0 = a_1 + 0.8b_1 + 0.6c_1 + (0.8)(0.6)d_1$$

$$a_1 = 0.6 \times 10^{-3}, b_1 = 1.25 \times 10^{-3}$$

$$c_1 = -1.0 \times 10^{-3}, d_1 = -2.083 \times 10^{-3}$$

$$\therefore u(x, y) = (0.6 + 1.25x - y - 2.083xy) \times 10^{-3}$$

$$\text{From } \Rightarrow (a_1 + b_1x + c_1y + d_1xy)$$

$$\text{Using } v_a = 0, v_b = 2 \times 10^{-3}, v_c = 1.0 \times 10^{-3}, v_d = -1.2 \times 10^{-3}$$

To Find  $a_2, b_2, c_2, d_2$  :

$$v(x, y) = (2.5x - 2y + 0.4167xy) \times 10^{-3}$$

Strains :

$$E_{xx} = \frac{\partial u}{\partial x} = (1.25 - 2.083y) \times 10^{-3}$$

$$E_{yy} = \frac{\partial v}{\partial y} = (-2 + 0.4167x) \times 10^{-3}$$

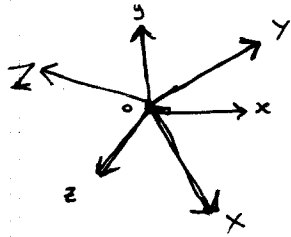
$$E_{xy} = \frac{1}{2} \left( \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) = \frac{1}{2} (-1 - 2.083x + 2.5 + 0.4167y) \times 10^{-3}$$

$$= (0.75 - 1.0415x + 0.20835y) \times 10^{-3}$$

$$\text{At A : } x=y=0$$

$$\begin{cases} E_{xx} = 1.25 \times 10^{-3} \\ E_{yy} = -2.0 \times 10^{-3} \\ E_{xy} = 0.75 \times 10^{-3} \end{cases}$$

## Stress Transformation



$$\epsilon_{xx}, \epsilon_{yy}, \epsilon_{zz}, \epsilon_{xy}, \epsilon_{xz}, \epsilon_{yz}$$

$$[S] = \begin{bmatrix} \epsilon_{xx} & \epsilon_{xy} & \epsilon_{xz} \\ \epsilon_{xy} & \epsilon_{yy} & \epsilon_{yz} \\ \epsilon_{xz} & \epsilon_{yz} & \epsilon_{zz} \end{bmatrix}_{3 \times 3}$$

$$X: \vec{N}_1 = l_1 \vec{i} + m_1 \vec{j} + n_1 \vec{k}$$

$$Y: \vec{N}_2 = l_2 \vec{i} + m_2 \vec{j} + n_2 \vec{k}$$

$$Z: \vec{N}_3 = l_3 \vec{i} + m_3 \vec{j} + n_3 \vec{k}$$

$$\begin{cases} \vec{N}_1 \cdot \vec{N}_1 = \vec{N}_2 \cdot \vec{N}_2 = \vec{N}_3 \cdot \vec{N}_3 = 1 \\ \vec{N}_1 \cdot \vec{N}_2 = \vec{N}_2 \cdot \vec{N}_3 = \vec{N}_1 \cdot \vec{N}_3 = 0 \end{cases}$$

$$\Rightarrow \vec{N}_p \cdot \vec{N}_q = \delta_{pq} = \begin{cases} 1 & p=q \\ 0 & p \neq q \end{cases}$$

$$p, q = 1, 2, 3$$

In the "New" XYZ :

$$\begin{bmatrix} \epsilon_{xx} & \epsilon_{xy} & \epsilon_{xz} \\ \epsilon_{yx} & \epsilon_{yy} & \epsilon_{yz} \\ \epsilon_{zx} & \epsilon_{zy} & \epsilon_{zz} \end{bmatrix}$$

Normal Strain

$$\epsilon_{xx} = l_1^2 \epsilon_{xx} + m_1^2 \epsilon_{yy} + n_1^2 \epsilon_{zz} + 2l_1 m_1 \epsilon_{xy} + 2l_1 n_1 \epsilon_{xz} + 2m_1 n_1 \epsilon_{yz}$$

Shear strain

$$\epsilon_{xy} = l_1 l_2 \epsilon_{xx} + m_1 m_2 \epsilon_{yy} + n_1 n_2 \epsilon_{zz} + (l_1 m_2 + l_2 m_1) \epsilon_{xy} + \dots$$

$$\dots + (m_1 n_2 + m_2 n_1) \epsilon_{yz} + (l_1 n_2 + l_2 n_1) \epsilon_{xz}$$

Principal Strains

\* Three mutually perpendicular line segments at one point will remain perpendicular after deformation

The three normal strains — principal strains

Principal strains :

$$\det \begin{bmatrix} \epsilon_{xx} - \epsilon & \epsilon_{xy} & \epsilon_{xz} \\ \epsilon_{yx} & \epsilon_{yy} - \epsilon & \epsilon_{yz} \\ \epsilon_{zx} & \epsilon_{zy} & \epsilon_{zz} - \epsilon \end{bmatrix} = 0$$

$$\epsilon^3 - \bar{I}_1 \epsilon^2 + \bar{I}_2 \epsilon - \bar{I}_3 = 0$$

Here  $\bar{I}_1 = \epsilon_{xx} + \epsilon_{yy} + \epsilon_{zz}$

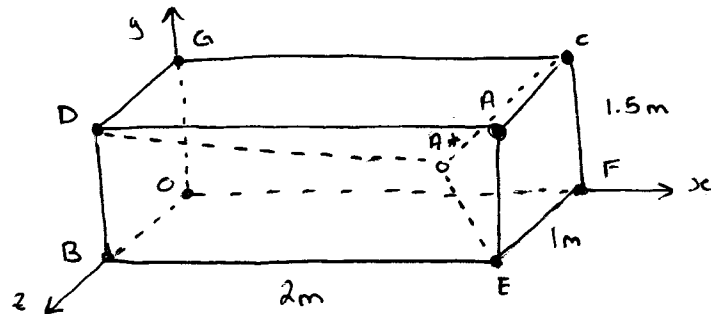
$$\bar{I}_2 = \epsilon_{xx}\epsilon_{yy} + \epsilon_{xx}\epsilon_{zz} + \epsilon_{yy}\epsilon_{zz} - \epsilon_{xy}^2 - \epsilon_{zy}^2 - \epsilon_{xz}^2$$

$$\bar{I}_3 = \begin{vmatrix} \epsilon_{xx} & \epsilon_{xy} & \epsilon_{xz} \\ \epsilon_{yx} & \epsilon_{yy} & \epsilon_{yz} \\ \epsilon_{zx} & \epsilon_{zy} & \epsilon_{zz} \end{vmatrix}$$

Principal direction :

$$\begin{cases} (\epsilon_{xx} - \epsilon)l + \epsilon_{xy}m + \epsilon_{xz}n = 0 \\ \epsilon_{yx}l + (\epsilon_{yy} - \epsilon)m + \epsilon_{yz}n = 0 \\ \epsilon_{zx}l + \epsilon_{zy}m + (\epsilon_{zz} - \epsilon)n = 0 \end{cases}$$

Example :



$$A(2, 1.5, 1) \rightarrow A^*(1.4985, 1.4988, 1.00009)$$

Find 1° the strain components at A

2° the normal strain in the direction of line AB

3° the shear strain for lines AB and AC

4° the principal strains at A

Solution :

$$u(x, y, z) = \cancel{a_1} + \cancel{b_1}x + \cancel{c_1}y + \cancel{d_1}z + \cancel{e_1}xy + \cancel{f_1}yz + \cancel{g_1}zx + h_1xyz$$

$$\rightarrow \begin{cases} u(x, y, z) = h_1 xyz \\ v(x, y, z) = h_2 xyz \\ w(x, y, z) = h_3 xyz \end{cases}$$

$$\text{At } A : u_A = 1.4985 - 2 = -0.0015$$

$$\rightarrow -0.0015 = h_1(2)(1.5)(1) \Rightarrow h_1 = -0.0005$$

$$v_A = 1.4988 - 1.5 = -0.0012$$

$$\Rightarrow -0.0012 = h_2 (2)(1.5)(1)$$

$$h_2 = -0.0004$$

$$w_A = 1.0009 - 1 = 0.0009 = h_3 (2)(1.5)(1)$$

$$h_3 = 0.0003$$

$$\Rightarrow \begin{cases} u = -0.0005 x y z = -500 \mu x y z \\ v = -0.0004 x y z = -400 \mu x y z \\ w = 0.0003 x y z = 300 \mu x y z \end{cases}$$

$$(1 \mu = 10^{-6})$$

1° Strains at A :

$$\begin{aligned} \epsilon_{xx} &= \frac{\partial u}{\partial x} = -500 \mu y z = -500 \mu (1.5)(1) \\ &= -750 \mu \end{aligned}$$

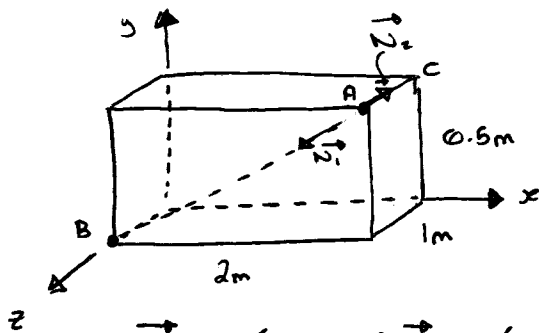
$$\begin{aligned} \epsilon_{yy} &= \frac{\partial v}{\partial y} = -400 \mu x z = -400 \mu (2)(1) \\ &= -800 \mu \end{aligned}$$

$$\begin{aligned} \epsilon_{zz} &= \frac{\partial w}{\partial z} = 300 x y = 300 \mu (2)(1.5) \\ &= 900 \mu \end{aligned}$$

$$\begin{aligned} \epsilon_{xy} &= \frac{1}{2} \left( \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) = \frac{1}{2} (-500 \mu x z - 400 \mu y z) \\ &= -800 \mu \end{aligned}$$

$$\epsilon_{xz} = -525 \mu \quad \epsilon_{yz} = -300 \mu$$

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$$A(2, 1.5, 1)$$

$$B(0, 0, 1)$$

$$\vec{AB} = (x_B - x_A)\vec{i} + (y_B - y_A)\vec{j} + (z_B - z_A)\vec{k}$$

$$\vec{AB} = -2\vec{i} - 1.5\vec{j}$$

$$|\vec{AB}| = \sqrt{(-2)^2 + (-1.5)^2 + 0^2} = 2.5$$

the unit vector in AB:

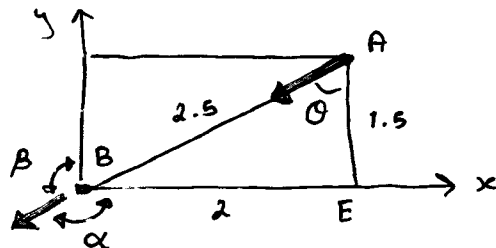
$$\vec{N}_1 = \frac{\vec{AB}}{|\vec{AB}|} = \frac{-2\vec{i} - 1.5\vec{j}}{2.5} = -0.8\vec{i} - 0.6\vec{j}$$

$$\therefore l_1 = -0.8, m_1 = -0.6, n_1 = 0$$

Normal strain along the line segment AB:

$$\begin{aligned} E_{xx} &= l_1^2 E_{xx} + m_1^2 E_{yy} + n_1^2 E_{zz} + 2l_1 m_1 E_{xy} + 2l_1 n_1 E_{xz} + 2m_1 n_1 E_{yz} \\ &= (-0.8)^2 (-750\mu) + (-0.6)^2 (-800\mu) + 2(-0.8)(-0.6)(-800\mu) \\ &= -1536\mu \end{aligned}$$

OTHER  
METHOD



$$l_1 = \cos \alpha$$

$$m_1 = \cos \beta$$

$$n_1 = \cos \gamma$$

$$\triangle ABE; \tan \theta = \frac{BE}{AE} = \frac{2}{1.5}$$

$$\theta = 53.13^\circ$$

$$\therefore \alpha = 90^\circ + \theta = 143.13^\circ$$

$$\beta = 180^\circ - \theta = 136.87^\circ$$

$$\gamma = 90^\circ$$

$$\Rightarrow l_1 = \cos \alpha = -0.8$$

$$m_1 = \cos \beta = -0.6$$

$$n_1 = \cos \gamma = 0$$



3° Shear Strain of the angle BAC ( $= 90^\circ$ )

$$\vec{N}_2 = l_2 \vec{i} + m_2 \vec{j} + n_2 \vec{k}$$

is a unit vector in AC

$$\vec{N}_2 = -\vec{n}$$

$$l_2 = m_2 = 0, \quad n_2 = -1 \quad (\text{where } l_1 = -0.8, m_1 = -0.6, n_1 = 0)$$

$$\therefore \epsilon_{xy} = l_1 l_2 \epsilon_{xx} + m_1 m_2 \epsilon_{yy} + n_1 n_2 \epsilon_{zz} + (l_1 m_2 + l_2 m_1) \epsilon_{xy} + (l_1 n_2 + l_2 n_1) \epsilon_{xz} \\ \dots + (n_1 m_2 + n_2 m_1) \epsilon_{yz}$$

$$\Rightarrow \epsilon_{xy} = l_1 n_2 \epsilon_{zx} + m_1 n_2 \epsilon_{yz}$$

$$\Rightarrow (-0.8)(-1)(-525 \mu) + (-0.6)(-1)(-300 \mu)$$

$$= -600 \mu$$

the angle change

$$\gamma_{xy} = 2\epsilon_{xy} = -1200 \mu$$

4° Principal Strains

$$\bar{I}_1 = \epsilon_{xx} + \epsilon_{yy} + \epsilon_{zz}$$

$$= (-750 \mu) + (-800 \mu) + (900 \mu)$$

$$= -650 \mu$$

$$\bar{I}_2 = \epsilon_{xx} \epsilon_{yy} + \epsilon_{yy} \epsilon_{zz} + \epsilon_{xx} \epsilon_{zz} - \epsilon_{xy}^2 - \epsilon_{yz}^2 - \epsilon_{xz}^2$$

$$= (\dots)$$

$$= -1.800625 \times 10^{-6} \mu^2$$

$$\bar{I}_3 = \begin{vmatrix} \epsilon_{xx} & \epsilon_{xy} & \epsilon_{xz} \\ \epsilon_{yx} & \epsilon_{yy} & \epsilon_{yz} \\ \epsilon_{zx} & \epsilon_{zy} & \epsilon_{zz} \end{vmatrix} \Rightarrow \begin{vmatrix} -750 \mu & -800 \mu & -525 \mu \\ -800 \mu & -800 \mu & -300 \mu \\ -525 \mu & -300 \mu & 900 \mu \end{vmatrix}$$

$$= 0$$

$$\Rightarrow \epsilon^3 - \bar{I}_1 \epsilon^2 + \bar{I}_2 \epsilon - \bar{I}_3 = 0$$

$$\Rightarrow \epsilon^3 + 650 \mu \epsilon^2 - 1.800625 (10^{-6}) \mu^2 \epsilon = 0$$

$$\text{Let } E = \mu \bar{E}$$

$$\Rightarrow \mu^3 \bar{E}^3 + 650 \mu \cdot \mu^2 \bar{E}^2 - 1.800625(10^6) \mu^2 \cdot \mu \bar{E} = 0$$

$$\Rightarrow \bar{E}^3 + 650 \bar{E}^2 - 1.800625(10^6) \bar{E} = 0$$

$$\Rightarrow \bar{E}_1 = 1055.67$$

$$\bar{E}_2 = 0$$

$$\bar{E}_3 = -1705.67$$

$$\Rightarrow E_1 = 1055.67 \mu, \quad E_2 = 0, \quad E_3 = -1705.67 \mu$$