

↳ Boresi / Schmidt

Mechanics

①

→ course outlines to be emailed out

→ Midterm Oct. 23rd (Tuesday) - 35%

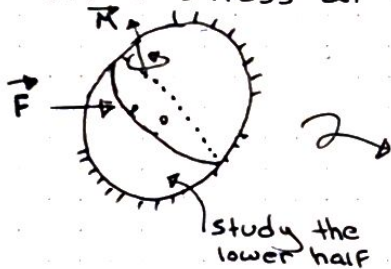
Final - 55%, Assignments - 10%

↳ Formula sheets provided

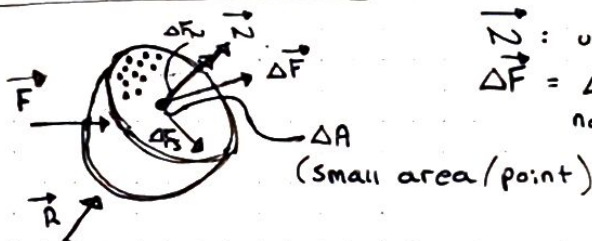
- Read Chapter 1 for review

→ Chapter 2 - Theories of Stress and Strain

7.1 → Stress at a point



Method of sections



\vec{n} : unit normal vector
 $\Delta \vec{F} = \Delta \vec{F}_n + \Delta \vec{F}_s$
 normal shear

Stress at the point:

$$\vec{\sigma} = \lim_{\Delta A \rightarrow 0} \Delta \vec{F} / \Delta A \quad (\text{stress vector})$$

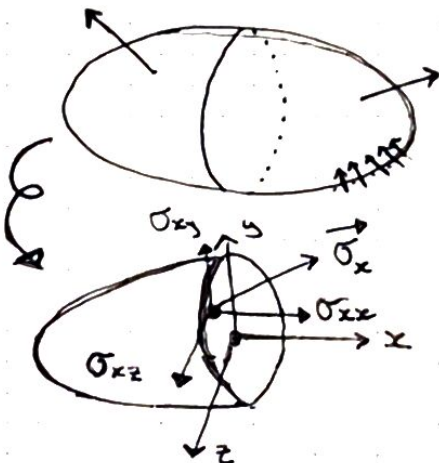
Normal stress:

$$\vec{\sigma}_n = \lim_{\Delta A \rightarrow 0} \Delta F_n / \Delta A$$

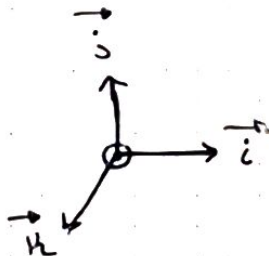
Shear stress:

$$\vec{\sigma}_s = \lim_{\Delta A \rightarrow 0} \Delta F_s / \Delta A$$

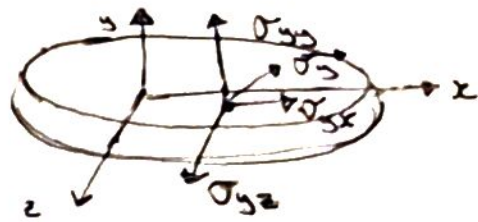
$$\Rightarrow \boxed{\vec{\sigma} = \vec{\sigma}_n + \vec{\sigma}_s}$$



$$\boxed{\vec{\sigma}_x = \sigma_{xx} \vec{i} + \sigma_{xy} \vec{j} + \sigma_{xz} \vec{k}}$$



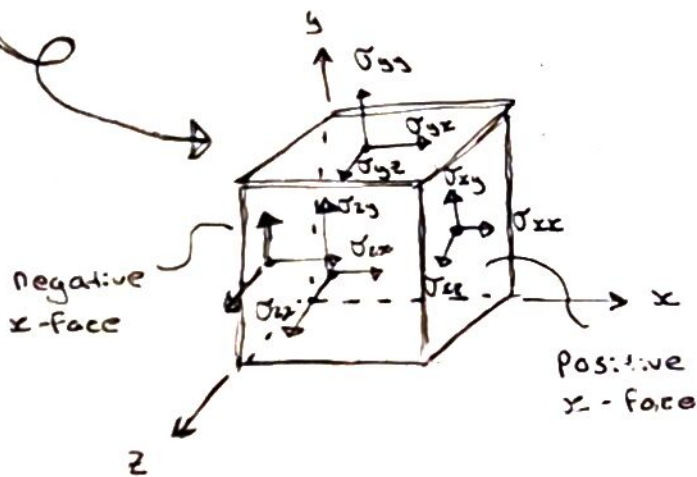
(2)



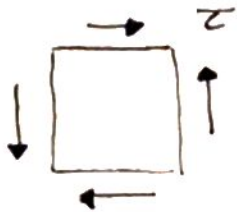
$$y: \vec{\sigma}_y = \sigma_{yx} \vec{i} + \sigma_{yy} \vec{j} + \sigma_{yz} \vec{k}$$

$$z: \vec{\sigma}_z = \sigma_{zx} \vec{i} + \sigma_{zy} \vec{j} + \sigma_{zz} \vec{k}$$

(When subscripts are the same - normal stress)
 ~ different - shear stress)



→ 9 stress components at a point



Pure shear

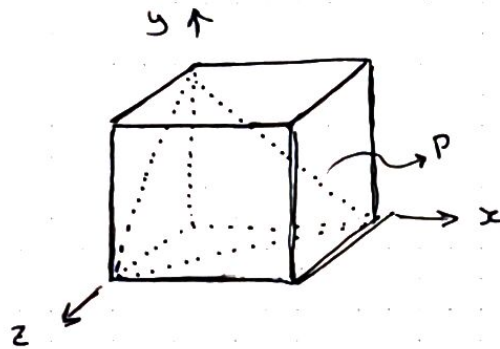
$$\sigma_{yx} = \sigma_{xy} ; \sigma_{yz} = \sigma_{zy} ; \sigma_{xz} = \sigma_{zx}$$

Only 6 of these components are independent

$$[T] = \begin{bmatrix} \sigma_{xx} & \sigma_{xy} & \sigma_{xz} \\ \sigma_{yx} & \sigma_{yy} & \sigma_{yz} \\ \sigma_{zx} & \sigma_{zy} & \sigma_{zz} \end{bmatrix}$$

Symmetric matrix

2.3 Symmetry of the stress array and stress on an arbitrarily oriented plane (3)



→ Given $\vec{\sigma}_x, \vec{\sigma}_y, \vec{\sigma}_z$

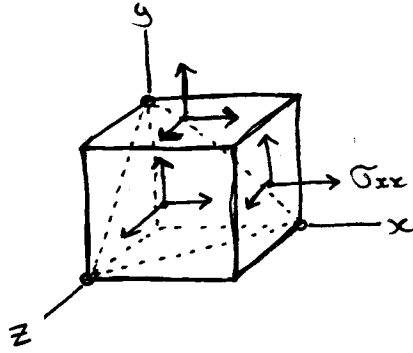
→ the normal of the inclined plane

$$\vec{N} = l\vec{i} + m\vec{j} + n\vec{k}$$

$$l^2 + m^2 + n^2 = 1$$

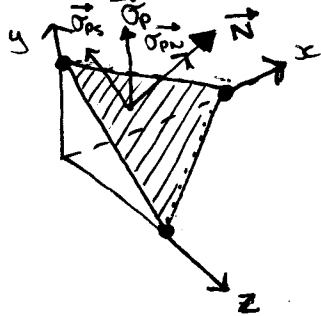
Find $\vec{\sigma}_p$ on the inclined plane

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$$\begin{aligned}\vec{\sigma}_x &= \sigma_{xx}\vec{i} + \sigma_{xy}\vec{j} + \sigma_{xz}\vec{k} \\ \vec{\sigma}_y &= \sigma_{yx}\vec{i} + \sigma_{yy}\vec{j} + \sigma_{yz}\vec{k} \\ \vec{\sigma}_z &= \sigma_{zx}\vec{i} + \sigma_{zy}\vec{j} + \sigma_{zz}\vec{k}\end{aligned}$$

Inclined plane :



unit normal vector

$$\vec{N} = l\vec{i} + m\vec{j} + n\vec{k}$$

$$l^2 + m^2 + n^2 = 1$$

stress vector $\vec{\sigma}_p = ?$

$$\vec{\sigma}_p = \sigma_{px}\vec{i} + \sigma_{py}\vec{j} + \sigma_{pz}\vec{k}$$

Equilibrium of the small volume :

$$\begin{cases} \sigma_{px} = l\sigma_{xx} + m\sigma_{xy} + n\sigma_{xz} \\ \sigma_{py} = l\sigma_{yx} + m\sigma_{yy} + n\sigma_{yz} \\ \sigma_{pz} = l\sigma_{zx} + m\sigma_{zy} + n\sigma_{zz} \end{cases}$$

$$\begin{Bmatrix} \sigma_{px} \\ \sigma_{py} \\ \sigma_{pz} \end{Bmatrix} = \begin{Bmatrix} \sigma_{xx} & \sigma_{xy} & \sigma_{xz} \\ \sigma_{yx} & \sigma_{yy} & \sigma_{yz} \\ \sigma_{zx} & \sigma_{zy} & \sigma_{zz} \end{Bmatrix} \begin{Bmatrix} l \\ m \\ n \end{Bmatrix}$$

Normal and shear stresses

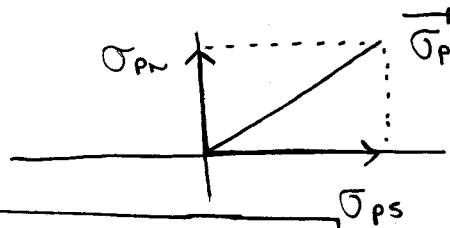
$$\begin{aligned}\sigma_{pn} &= \vec{\sigma}_p \cdot \vec{N} \\ &= (\sigma_{px}\vec{i} + \sigma_{py}\vec{j} + \sigma_{pz}\vec{k}) \cdot (l\vec{i} + m\vec{j} + n\vec{k}) \\ &= l\sigma_{px} + m\sigma_{py} + n\sigma_{pz} = \dots\end{aligned}$$

$$\sigma_{pn} = l^2\sigma_{xx} + m^2\sigma_{yy} + n^2\sigma_{zz} + 2lm\sigma_{xy} + 2mn\sigma_{yz} + 2nl\sigma_{zx}$$

$$\text{OR } \sigma_{pn} = (l \ m \ n) \begin{Bmatrix} \sigma_{px} \\ \sigma_{py} \\ \sigma_{pz} \end{Bmatrix}$$

$$\left(\vec{\sigma}_n = (l \ m \ n) \begin{bmatrix} \sigma_{xx} & \sigma_{xy} & \sigma_{xz} \\ \sigma_{yx} & \sigma_{yy} & \sigma_{yz} \\ \sigma_{zx} & \sigma_{zy} & \sigma_{zz} \end{bmatrix} \begin{Bmatrix} l \\ m \\ n \end{Bmatrix} \right)$$

Shear Stress :



$$\begin{aligned} \sigma_{PS} &= \sqrt{\sigma_P^2 - \sigma_{PN}^2} \\ &= \sqrt{\sigma_{Px}^2 - \sigma_{Py}^2 + \sigma_{Pz}^2 - \sigma_{PN}^2} \end{aligned}$$

Direction of Shear stress :

$$\vec{\sigma}_P = \sigma_{PN} \vec{N} + \sigma_{PS} \vec{S}$$

2.4 Transformation of Stress

Principal Stresses

xyz - old

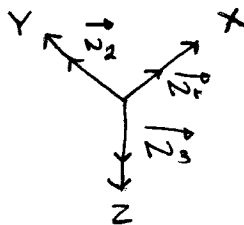
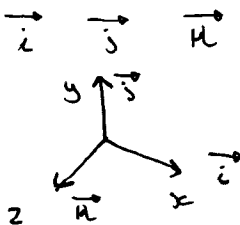
XYZ - new

one point

$$\begin{matrix} \sigma_{xx} & \sigma_{yy} & \sigma_{zz} \\ \sigma_{xy} & \sigma_{yz} & \sigma_{zx} \end{matrix}$$

$$\begin{matrix} \sigma_{xx} & \sigma_{xr} & \sigma_{zz} \\ \sigma_{xr} & \sigma_{rz} & \sigma_{zz} \end{matrix}$$

\Downarrow



Define :

$$\begin{cases} \vec{N}_1 = l_1 \vec{i} + m_1 \vec{j} + n_1 \vec{k} \\ \vec{N}_2 = l_2 \vec{i} + m_2 \vec{j} + n_2 \vec{k} \\ \vec{N}_3 = l_3 \vec{i} + m_3 \vec{j} + n_3 \vec{k} \end{cases}$$

$$l_1^2 + m_1^2 + n_1^2 = 1 \quad l_2^2 + m_2^2 + n_2^2 = 1 \quad l_3^2 + m_3^2 + n_3^2 = 1$$

$$l_1 l_2 + m_1 m_2 + n_1 n_2 = 0$$

$$l_2 l_3 + m_2 m_3 + n_2 n_3 = 0$$

$$l_1 l_3 + m_1 m_3 + n_1 n_3 = 0$$

$$\begin{aligned} \therefore \sigma_{xx} &= l_1^2 \sigma_{xx} + m_1^2 \sigma_{yy} + n_1^2 \sigma_{zz} \dots \\ &\dots + 2l_1 m_1 \sigma_{xy} + 2m_1 n_1 \sigma_{yz} + 2n_1 l_1 \sigma_{zx} \end{aligned}$$

$$\begin{aligned} \sigma_{yy} &= l_2^2 \sigma_{xx} + m_2^2 \sigma_{yy} + n_2^2 \sigma_{zz} \dots \\ &\dots + 2l_2 m_2 \sigma_{xy} + 2m_2 n_2 \sigma_{yz} + 2n_2 l_2 \sigma_{zx} \end{aligned}$$

$$\begin{aligned} \sigma_{zz} &= l_3^2 \sigma_{xx} + m_3^2 \sigma_{yy} + n_3^2 \sigma_{zz} \dots \\ &\dots + \dots \end{aligned}$$

Stress vector on x-face

$$\vec{\sigma}_x = \sigma_{xx} \vec{N}_1 + \sigma_{xy} \vec{N}_2 + \sigma_{xz} \vec{N}_3$$

$$\Rightarrow \begin{cases} \sigma_{xy} = \vec{\sigma}_x \cdot \vec{N}_2 \\ \sigma_{xz} = \vec{\sigma}_x \cdot \vec{N}_3 \end{cases}$$

$$\begin{aligned} \therefore \sigma_{xy} &= l_1 l_2 \sigma_{xx} + m_1 m_2 \sigma_{yy} + n_1 n_2 \sigma_{zz} \dots \\ &\dots + (l_1 m_2 + l_2 m_1) \sigma_{xy} + (m_2 n_1 + m_1 n_2) \sigma_{yz} \dots \\ &\dots + (n_1 l_2 + n_2 l_1) \sigma_{zx} \end{aligned}$$

$$\begin{aligned} \sigma_{yz} &= l_2 l_3 \sigma_{xx} + m_2 m_3 \sigma_{yy} + n_2 n_3 \sigma_{zz} \dots \\ &\dots + (l_2 m_3 + l_3 m_2) \sigma_{xy} + (m_3 n_2 + m_2 n_3) \sigma_{yz} \dots \\ &\dots + (n_2 l_3 + n_3 l_2) \sigma_{zx} \end{aligned}$$

$$\sigma_{zx} = \dots$$

Principal stresses :

There exist three mutually perpendicular planes at the point on which the shear stresses vanish

The remaining three normal stresses components on these three planes are called principal stresses.

Here the principal plane has a unit normal vector:

$$\vec{N} = l\vec{i} + m\vec{j} + n\vec{k}$$

$$\Rightarrow \boxed{\vec{\sigma}_p = \sigma_{pn} \vec{N}} \quad = \quad \boxed{\sigma \vec{N}}$$

Principal stress

$$\Rightarrow \sigma_{px} \vec{i} + \sigma_{py} \vec{j} + \sigma_{pz} \vec{k}$$

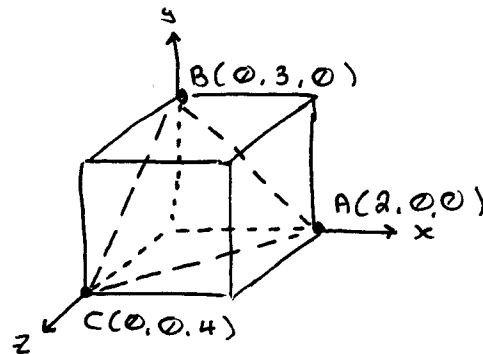
$$= \sigma (l\vec{i} + m\vec{j} + n\vec{k})$$

$$\begin{cases} \sigma_{px} = \sigma l \\ \sigma_{py} = \sigma m \\ \sigma_{pz} = \sigma n \end{cases}$$

$$\Rightarrow \begin{cases} \sigma_{px} = l\sigma_{xx} + m\sigma_{xy} + n\sigma_{xz} = \sigma l \\ \sigma_{py} = l\sigma_{yx} + m\sigma_{yy} + n\sigma_{yz} = \sigma m \\ \sigma_{pz} = l\sigma_{zx} + m\sigma_{zy} + n\sigma_{zz} = \sigma n \end{cases}$$

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Example 1 -



Find the unit normal vector of the plane through A, B, and C.

Solution :
$$\left. \begin{array}{c} ax + by + cz = d \\ \uparrow \quad \uparrow \quad \uparrow \\ \text{The normal vector is } (a, b, c) \end{array} \right\}$$

A (2, 0, 0) $2a + 0 + 0 = d$; $a = d/2$

B (0, 3, 0) $0 + 3b + 0 = d$; $b = d/3$

C (0, 0, 4) $0 + 0 + 4c = d$; $c = d/4$

$\Rightarrow \frac{d}{2}x + \frac{d}{3}y + \frac{d}{4}z = d$

$\Rightarrow \frac{x}{2} + \frac{y}{3} + \frac{z}{4} = 1$

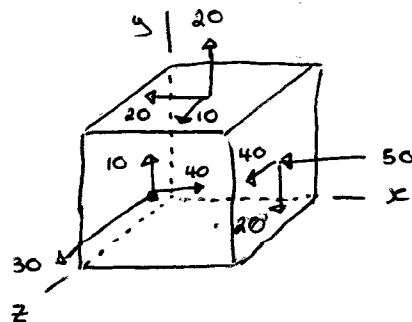
The normal vector $(\frac{1}{2}, \frac{1}{3}, \frac{1}{4})$

Length = $\sqrt{(\frac{1}{2})^2 + (\frac{1}{3})^2 + (\frac{1}{4})^2}$

\therefore the unit normal vector is : $(\frac{1}{2L}, \frac{1}{3L}, \frac{1}{4L})$

Example - The state of stress at a point is given by

unit: kPa



Determine the stress vector on the plane

with unit normal vectors :

$\vec{N} = \frac{1}{\sqrt{2}}\vec{i} + \frac{1}{2}\vec{j} + \frac{1}{2}\vec{k}$

Also find the normal stress and shear stress

$$\begin{aligned} \text{Solution: } \sigma_{xx} &= -50 & \sigma_{xy} &= -20 & \sigma_{xz} &= 40 \\ \sigma_{yx} &= -20 & \sigma_{yy} &= 20 & \sigma_{yz} &= 10 \\ \sigma_{zx} &= 40 & \sigma_{zy} &= 10 & \sigma_{zz} &= 30 \end{aligned}$$

$$\text{and: } l = \frac{1}{\sqrt{2}} \quad m = \frac{1}{2} \quad n = \frac{1}{2}$$

$$\text{Stress vector: } \vec{\sigma}_p = \sigma_{px} \vec{i} + \sigma_{py} \vec{j} + \sigma_{pz} \vec{k}$$

Here

$$\begin{aligned} \sigma_{px} &= l\sigma_{xx} + m\sigma_{xy} + n\sigma_{xz} \\ &= \left(\frac{1}{\sqrt{2}}\right)(-50) + \left(\frac{1}{2}\right)(-20) + \left(\frac{1}{2}\right)(40) \\ &= -25.36 \end{aligned}$$

$$\begin{aligned} \sigma_{py} &= l\sigma_{yx} + m\sigma_{yy} + n\sigma_{yz} \\ &= \left(\frac{1}{\sqrt{2}}\right)(-20) + \left(\frac{1}{2}\right)(20) + \left(\frac{1}{2}\right)(10) \\ &= 0.8579 \end{aligned}$$

$$\begin{aligned} \sigma_{pz} &= l\sigma_{zx} + m\sigma_{zy} + n\sigma_{zz} \\ &= \left(\frac{1}{\sqrt{2}}\right)(40) + \left(\frac{1}{2}\right)(10) + \left(\frac{1}{2}\right)(30) \\ &= 48.78 \end{aligned}$$

$$\therefore \vec{\sigma}_p = -25.36 \vec{i} + 0.8579 \vec{j} + 48.78 \vec{k}$$

Normal stress

$$\begin{aligned} \sigma_{pn} &= l^2 \sigma_{xx} + m^2 \sigma_{yy} + n^2 \sigma_{zz} \dots \\ &\dots + 2lm \sigma_{xy} + 2mn \sigma_{yz} + 2nl \sigma_{zx} \\ &= \left(\frac{1}{\sqrt{2}}\right)^2 (-50) + \left(\frac{1}{2}\right)^2 (20) + \left(\frac{1}{2}\right)^2 (30) \dots \\ &\dots + (2)\left(\frac{1}{\sqrt{2}}\right)\left(\frac{1}{2}\right)(-20) + (2)\left(\frac{1}{2}\right)\left(\frac{1}{2}\right)(10) \dots \\ &\dots (2)\left(\frac{1}{2}\right)\left(\frac{1}{\sqrt{2}}\right)(40) \\ &= \dots \end{aligned}$$

Shear stress :

$$\begin{aligned} \sigma_{ps} &= \sqrt{\sigma_{px}^2 + \sigma_{py}^2 + \sigma_{pz}^2 - \sigma_{pn}^2} \\ &= \dots \end{aligned}$$

$$\vec{\sigma}_p = \sigma_{px} \vec{i} + \sigma_{py} \vec{j} + \sigma_{pz} \vec{k}$$

$$\Leftrightarrow$$

$$\vec{\sigma}_p = \sigma_{pn} \vec{n} + \sigma_{ps} \vec{s}$$

$$\Rightarrow \vec{s} = \frac{1}{\sigma_{ps}} (\vec{\sigma}_p - \sigma_{pn} \vec{n})$$

↪ direction of stress vector in the plane

$$\text{Verify } \left. \begin{array}{l} |\vec{s}| = 1 \\ \vec{s} \cdot \vec{n} = 0 \end{array} \right\}$$