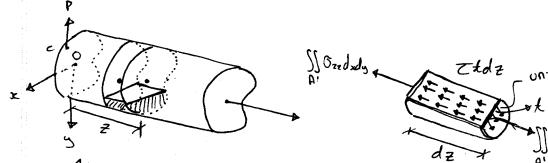


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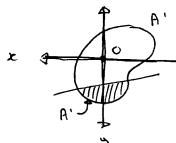
Uniform shear stress

II (Ozz+d Ozz)dxdy



$$\iint_{\mathbf{R}} (\Im z + d\Im z) dx dy - \iint_{\mathbf{R}} \frac{\partial z}{\partial z} dx dy - 7 dz = 0$$

$$= q = 7 dz = \iint_{\mathbf{R}} \frac{\partial z}{\partial z} dx dy$$



Since
$$G_{22} = \frac{M_x I_y + M_y I_{xy}}{\Delta} = \frac{M_y I_x + M_x I_{xy}}{\Delta}$$

$$= \frac{d\sigma_{zz}}{dz} = \frac{dMx}{dz} \frac{dMy}{dz} \frac{Jxy}{dz} \frac{dMy}{dz} \frac{Jxy}{dz} \frac{dMx}{dz} \frac{Jxy}{dz} \frac{dMx}{dz} \frac{Jxy}{dz} \frac{dMx}{dz} \frac{Jxy}{dz} \frac{dMx}{dz} \frac{Jxy}{dz} \frac{dMy}{dz} \frac{Jxy}{dz} \frac{dMx}{dz} \frac{Jx}{dz} \frac{Jx}{dz} \frac{dMx}{dz} \frac{Jx}{dz} \frac{Jx}{$$

because of:
$$\frac{dMx}{dz} = Vy$$
 $\frac{dMy}{dz} = -Vx$

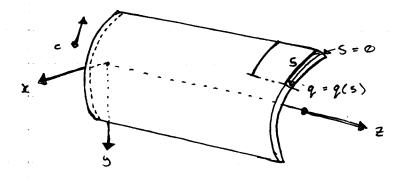
=)
$$\frac{d\sigma_{zz}}{dz} = \frac{V_y I_y - V_x I_{xy}}{\Delta} - \frac{-V_x I_x + V_y I_{xy}}{\Delta}$$

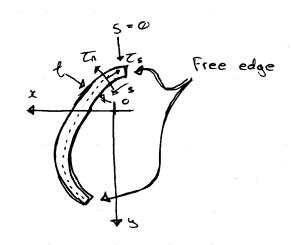
= $\frac{V_y I_y - V_x I_{xy}}{\Delta} + \frac{V_x I_x + V_y I_{xy}}{\Delta}$

=>
$$q = ZL = \iint_{A'} \frac{d\sigma_{zz}}{dz} dzdg$$

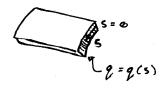
If
$$I_{xy} = \emptyset$$
, $q = (V_3/I_x)A'\bar{y}' + (V_x/I_y)A'\bar{z}'$
If $V_x = \emptyset$, $q = (V_y/I_x)A'\bar{y}'$

2. Thin-wall open section



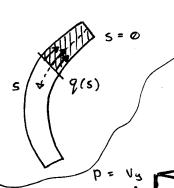


- 1° Tn = 0 (thickness very small)
- 2. Is is uniform through thickness of the wall
- 3° 9 = Zt : Shear Flow
- 4° Shear flow is zero at the free edge

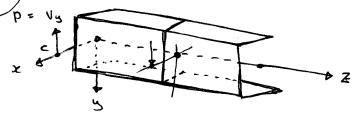


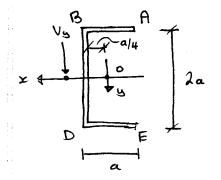
Positive Shear Flow:

the Shear Flow points into the area

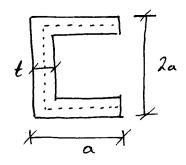


Example: Determine the shear Flow in a C channel section due to a shear Force by through its shear center.





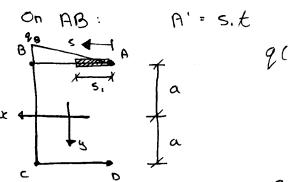
(t << a)



$$Q = Zt = \frac{\sqrt{3}}{I_{x}} A'\bar{y}'$$

$$I_{x} = \left(\frac{1}{12}\right)t(2a)^{3} + \left[\left(\frac{1}{12}\right)(a)(t)^{3} + \alpha t \cdot \alpha^{2}\right] \times 2$$

$$= {8/3} \alpha^{3}t \qquad (\alpha t^{3} << \alpha^{3}t)$$



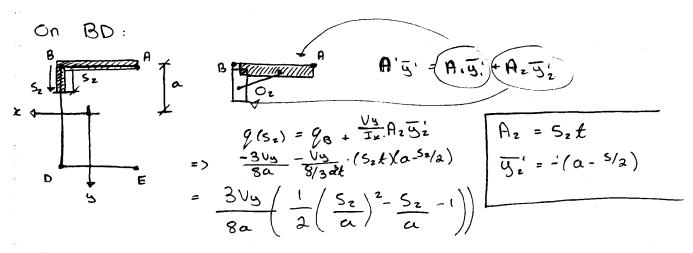
页'=-a

$$Q(5.) = \frac{V_5}{I_{\kappa}} A'\bar{g}'$$

$$= \frac{V_5}{8/3a_{k}^3} (5.k)(-a) = -\frac{3V_5}{8a^2} (5.)$$

(when @ = 5. = a)

$$98 = \frac{3v_y}{8a^2}(a) = \frac{-3v_y}{8a}$$

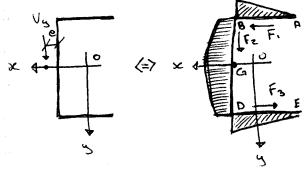


A+ D₁, S₂ =
$$2a$$

 $20 = 9(2a) - -30y = 20$
 $8a$



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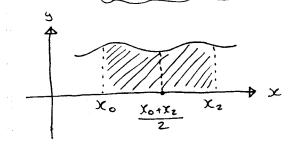
5:nce
$$q_B = \frac{3v_9}{8a}$$

.: $F_1 = \frac{1}{2}q_B \cdot AB$
 $= \frac{1}{2}(\frac{3v_5}{8a}) \cdot a = \frac{3}{16}v_9$

D is the moment center Fi. 2a = Vye

=>
$$e = \frac{F_1 \cdot 2a}{V_3} = \frac{3/6V_3 \cdot 2a}{V_3} = \frac{3a}{8}$$

 $F_2 = -\int_0^{2a} 9(s_2) ds_2$



$$\int_{x_{0}}^{x_{z}} \int (x) dx = \frac{x_{z} - x_{0}}{6} \left(\int_{0}^{x_{0}} + H \int_{x_{0}}^{x_{0}} + \int_{z_{0}}^{x_{0}} \right)$$

$$\int_{0}^{x_{z}} \int (x) dx = \frac{x_{z} - x_{0}}{6} \left(\int_{0}^{x_{0}} + H \int_{x_{0}}^{x_{0}} + \int_{z_{0}}^{x_{0}} \right)$$

$$\int_{z_{0}}^{z_{0}} \int (x) dx = \frac{x_{z} - x_{0}}{6} \left(\int_{0}^{x_{0}} + H \int_{x_{0}}^{x_{0}} + \int_{z_{0}}^{x_{0}} \right)$$

$$\int_{z_{0}}^{z_{0}} \int (x) dx = \frac{x_{z} - x_{0}}{6} \left(\int_{0}^{x_{0}} + H \int_{x_{0}}^{x_{0}} + \int_{z_{0}}^{x_{0}} \right)$$

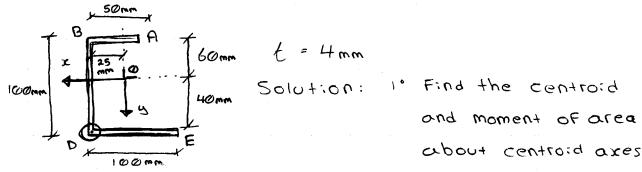
$$\int_{z_{0}}^{z_{0}} \int (x) dx = \frac{x_{z} - x_{0}}{6} \left(\int_{0}^{x_{0}} + H \int_{x_{0}}^{x_{0}} + \int_{z_{0}}^{x_{0}} \left(\int_{0}^{x_{0}} + H \int_{x_{0}}^{x_{0}} + H \int_{x_{0}}^{x_{0}} + \int_{z_{0}}^{x_{0}} \left(\int_{0}^{x_{0}} + H \int_{x_{0}}^{x_{0}} + \int_{x_{0}}^{x_{0}} \left(\int_{0}^{x_{0}} + H \int_{x_{0}}^{x_{0}} + H \int_{x_{0}}^{x_{0}} + \int_{x_{0}}^{x_{0}} \left(\int_{0}^{x_{0}} + H \int_{x_{0$$

$$F_{2} = \frac{2a}{6} \left(9B + 49G + 90 \right)$$

$$= \frac{a}{3} \left(\frac{-3Vy}{8a} - 4 \left(\frac{9Vy}{16a} \right) - \frac{3Vy}{8a} \right)$$

$$F_{2} = -Vy$$

Example: Find the Shear Center of a C-section.

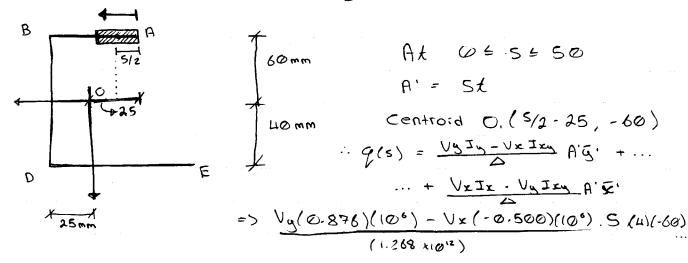


Ix = 1.733 x106 mm4

Is = 0.876 x106 mm4

Ixy = -0.500 x10 mm +

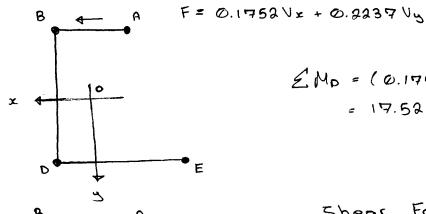
2° Find internal Shear Flow due to the internal Shear Forces Vx and Vy



 $\frac{\sqrt{(1.733)(10^6)} - \sqrt{\sqrt{(-0.500)(10^6)}} 5/4)(5/2-25)}{(1.268 \times 10^{12})}$

=> Vx[2.7335(5-50)-94.635](106) + Vy[0.7885(5-50)-165.795](106)

Resultant on AB:



$$\angle M_D = (0.1752V_X + 0.2237V_Y)(100)$$

= 17.52V_X + 22.37V_Y

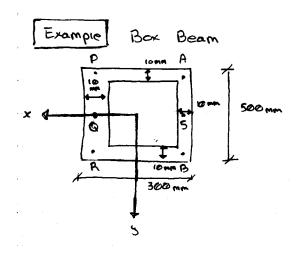
Shear Force Vx and Vy

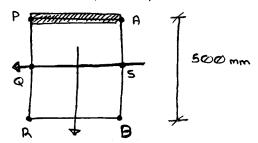
EMp = Vxey + Vyex

=> Sey = 17.52 mm

ex = 22.37 mm

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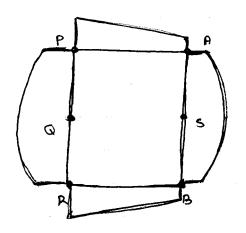
$$f_{p} = f_{a} + \frac{V_{y}}{I_{x}} A'_{y}'$$

$$= f_{a} + (300)(10)(-250)$$

$$= f_{a} - 750000$$

$$\begin{aligned}
\varphi_{Q} &= \varphi_{P} + \frac{V_{9}}{I_{x}} \dot{A} \dot{5} \\
&= (\varphi_{A} - 750000) + (250)(20)(125) \\
&= \varphi_{A} - 1375000
\end{aligned}$$

$$f_{R} = g_{P}$$
; $g_{B} = g_{A}$
 $\rightarrow g_{S} = g_{B} + \frac{V_{S}}{I_{X}}A'_{S}'$
 $= g_{A} + (250)(10)(125)$
 $= g_{A} + 313500$

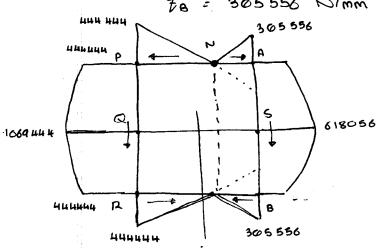


Angle of twist (per unit length):
$$\int = \frac{1}{2GA} \int \frac{9}{4} \frac{9}{4} dl = \emptyset$$

$$= \int \frac{9}{4} dl + \int \frac{9}{4} \frac{9}{4} dl + \int \frac{9}{4} dl + \int \frac{9}{4} dl + \int \frac{9}{4} \frac{9}{4} dl + \int \frac{9}{4}$$

$$\int_{AP} q dl = \frac{1}{2} (q_{P} + q_{P})(AP) = \frac{1}{2} (q_{P} + q_{A} - 750000)(300)$$

$$= 3000 q_{A} - 1125000000$$



1° Symmetrical?

2° Edge (Parallel) Shear Force => quadratic

3° Edge (perpend:cular) Shear Force

> linear

Finding Shear Center (using Point A)

A: moment Center

C = 203.0 mm

Here Ix = 687500000 mm "