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@ $O(0,0,0)$, no disp., $u = v = w = 0$

$$\Rightarrow u_0 = v_0 = w_0 = 0$$

$$\Rightarrow \begin{cases} u = c_1 x \\ v = c_2 y \\ w = c_3 z \end{cases}$$

For Assignment #2
Q1

Chapter 3: Linear Stress-strain Temp. Relations

3.1 First law of thermodynamics

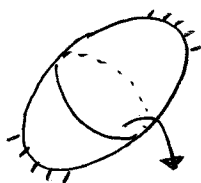
→ Conservation of energy

For adiabatic conditions - no net heat flow into the system, and static equilibrium

$$\delta W = \delta U$$

δW : work done by external forces

δU : increase the internal energy



U_0 = internal energy density

$$\delta U = \iiint_V \delta U_0 dV \quad \text{(volume)}$$

Displacement $\delta u, \delta v, \delta w$ (3 components)

→ strains: $\delta \epsilon_{xx}, \delta \epsilon_{yy}, \delta \epsilon_{zz}, \delta \epsilon_{xy}, \delta \epsilon_{yz}, \delta \epsilon_{xz}$ (6 comp.)

Stress: $\sigma_{xx}, \sigma_{xy}, \sigma_{xz}, \sigma_{yy}, \sigma_{zz}, \sigma_{yz}$

$$\Rightarrow \delta U_0 = \sigma_{xx} \delta \epsilon_{xx} + \sigma_{yy} \delta \epsilon_{yy} + \sigma_{zz} \delta \epsilon_{zz} + 2\sigma_{xy} \delta \epsilon_{xy} + \dots$$

$$\dots + 2\sigma_{yz} \delta \epsilon_{yz} + 2\sigma_{xz} \delta \epsilon_{xz}$$

$$U_0 = U_0(\epsilon_{xx}, \epsilon_{yy}, \epsilon_{zz}, \epsilon_{xy}, \epsilon_{xz}, \epsilon_{yz})$$

$$\sigma_{xx} = \frac{\partial U_0}{\partial \epsilon_{xx}}$$

$$\sigma_{xy} = \left(\frac{1}{2}\right) \left(\frac{\partial U_0}{\partial \epsilon_{xy}}\right)$$

$$\sigma_{yy} = \frac{\partial U_0}{\partial \epsilon_{yy}}$$

$$\sigma_{yz} = \left(\frac{1}{2}\right) \left(\frac{\partial U_0}{\partial \epsilon_{yz}}\right)$$

$$\sigma_{zz} = \frac{\partial U_0}{\partial \epsilon_{zz}}$$

$$\sigma_{xz} = \left(\frac{1}{2}\right) \left(\frac{\partial U_0}{\partial \epsilon_{xz}}\right)$$

3.2 Hookes Law For anisotropic elasticity

A material is said to be elastically isotropic if its elastic coefficients are invariants under any rotation of coordinates

A material is said to be homogeneous if its properties are identical for any point in a member.

$$\begin{array}{ll}
 \sigma_{xx} \leftrightarrow \sigma_{11} \leftrightarrow S_1 & E_{xx} \leftrightarrow E_{11} \leftrightarrow l_1 \\
 \sigma_{yy} \leftrightarrow \sigma_{22} \leftrightarrow S_2 & E_{yy} \leftrightarrow E_{22} \leftrightarrow l_2 \\
 \sigma_{zz} \leftrightarrow \sigma_{33} \leftrightarrow S_3 & E_{zz} \leftrightarrow E_{33} \leftrightarrow l_3 \\
 \sigma_{yz} \leftrightarrow \sigma_{23} \leftrightarrow S_4 & 2E_{yz} \leftrightarrow 2E_{23} \leftrightarrow l_4 \\
 \sigma_{zx} \leftrightarrow \sigma_{31} \leftrightarrow S_5 & 2E_{zx} \leftrightarrow 2E_{31} \leftrightarrow l_5 \\
 \sigma_{xy} \leftrightarrow \sigma_{12} \leftrightarrow S_6 & 2E_{xy} \leftrightarrow 2E_{12} \leftrightarrow l_6
 \end{array}$$

For the linear material :

$$S_1 = C_{11}l_1 + C_{12}l_2 + C_{13}l_3 + C_{14}l_4 + C_{15}l_5 + C_{16}l_6$$

$$S_i = \sum_{j=1}^6 C_{ij}e_j$$

$$S_2 = C_{21}l_1 + C_{22}l_2 + C_{23}l_3 + C_{24}l_4 + C_{25}l_5 + C_{26}l_6$$

$$\vdots$$

$$\vdots$$

$$S_6 = C_{61}l_1 + C_{62}l_2 + C_{63}l_3 + C_{64}l_4 + C_{65}l_5 + C_{66}l_6$$

$$[C_{ij}]_{6 \times 6} = \begin{bmatrix} C_{11} & C_{12} & C_{13} & \cdots & C_{16} \\ \vdots & \ddots & \vdots & \ddots & \vdots \\ C_{61} & \cdots & \cdots & \cdots & C_{66} \end{bmatrix} \begin{array}{l} \text{real symmetric} \\ \text{matrix} \\ 6 \times 6 \end{array}$$

21 material constants

$$\begin{Bmatrix} S_1 \\ S_2 \\ \vdots \\ S_6 \end{Bmatrix} = [C_{ij}] \begin{Bmatrix} l_1 \\ l_2 \\ \vdots \\ l_6 \end{Bmatrix}$$

The internal energy density

$$\begin{aligned}
 U_0 &= \frac{1}{2} (S_1 l_1 + S_2 l_2 + S_3 l_3 + S_4 l_4 + S_5 l_5 + S_6 l_6) \\
 &= \left(\frac{1}{2}\right) \sum_i S_i l_i \\
 &= \left(\frac{1}{2}\right) \sum_{i=1}^6 \sum_{j=1}^6 C_{ij} l_j l_i \\
 &= \left(\frac{1}{2}\right) (l_1, l_2, \dots, l_6) [C_{ij}] \begin{Bmatrix} l_1 \\ l_2 \\ \vdots \\ l_6 \end{Bmatrix}
 \end{aligned}$$

3.3 Hookes Law For isotropic elasticity

21 \rightarrow 2 numbers : λ and G
 λ : Lame's Elastic coefficient
 G : Shear modulus

Define: $e = l_1 + l_2 + l_3 = \epsilon_{xx} + \epsilon_{yy} + \epsilon_{zz}$

$$\sigma_{xx} = \lambda e + 2G\epsilon_{xx}$$

$$\sigma_{xy} = 2G\epsilon_{xy}$$

$$\sigma_{yy} = \lambda e + 2G\epsilon_{yy}$$

$$\sigma_{yz} = 2G\epsilon_{yz}$$

$$\sigma_{zz} = \lambda e + 2G\epsilon_{zz}$$

$$\sigma_{xz} = 2G\epsilon_{xz}$$

The internal energy density

$$U_0 = \left(\frac{1}{2}\right) \lambda (\epsilon_{xx} + \epsilon_{yy} + \epsilon_{zz})^2 + G(\epsilon_{xx}^2 + \epsilon_{yy}^2 + \epsilon_{zz}^2 + 2\epsilon_{xy}^2 + 2\epsilon_{yz}^2 + 2\epsilon_{xz}^2)$$

$$U_0 = \left(\frac{1}{2}\lambda + G\right) \bar{I}_1^2 - 2G\bar{I}_2$$

In the principal strain directions :

$$\epsilon_1, \epsilon_2, \epsilon_3$$

$$\text{Stress : } \sigma_1 = \lambda e + 2G\epsilon_1 = (\lambda + 2G)\epsilon_1 + \lambda\epsilon_2 + \lambda\epsilon_3$$

$$\sigma_2 = \lambda e + 2G\epsilon_2$$

$$\sigma_3 = \lambda e + 2G\epsilon_3$$

no shear stress.

Inverse Hookes Relationship

$$\epsilon_{xx} = \frac{1}{E} [\sigma_{xx} - \nu(\sigma_{yy} + \sigma_{zz})]$$

$$\epsilon_{yy} = \frac{1}{E} (\sigma_{yy} - \nu(\sigma_{zz} + \sigma_{xx}))$$

$$\epsilon_{zz} = \frac{1}{E} (\sigma_{zz} - \nu(\sigma_{xx} + \sigma_{yy}))$$

$$\epsilon_{xy} = \frac{1}{2G} \sigma_{xy} \dots \dots$$

4 constants : λ, G, E, ν

$$E = \frac{G(3\lambda + 2G)}{\lambda + G}$$

$$\nu = \frac{\lambda}{2(\lambda + G)}$$

Plane stress : $\sigma_{xz} = \sigma_{yz} = \sigma_{zz} = 0$

$$\epsilon_{xz} = \epsilon_{yz} = 0$$

$$\epsilon_{zz} = ?$$

$$\sigma_{zz} = \lambda(\epsilon_{xx} + \epsilon_{yy} + \epsilon_{zz}) + 2G\epsilon_{zz} = 0$$

$$\epsilon_{zz} = -\frac{\lambda}{\lambda + 2G}(\epsilon_{xx} + \epsilon_{yy}) \neq 0$$

Plane strain : $\epsilon_{xz} = \epsilon_{yz} = \epsilon_{zz} = 0$

$$\sigma_{xz} = \sigma_{yz} = 0$$

but

$$\sigma_{zz} \neq 0$$

Plane strain \neq Plane stress

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Example

Given the principal strains at a point

$$\epsilon_1; \epsilon_2; \epsilon_3 = 5; 4; 3$$

The largest principal stress $\sigma_1 = 140 \text{ MPa}$. Find σ_2 and σ_3 at the point. Given

$$E = 200 \text{ GPa}, \nu = 0.3$$

Solution :

$$\begin{cases} \sigma_{xx} = \lambda \epsilon + 2G\epsilon_{xx} \\ \sigma_{yy} = \lambda \epsilon + 2G\epsilon_{yy} \\ \sigma_{zz} = \lambda \epsilon + 2G\epsilon_{zz} \end{cases} \quad \begin{matrix} \text{General} \\ \text{Case} \end{matrix}$$

In the principal directions,

$$\begin{cases} \sigma_1 = \lambda \epsilon + 2G\epsilon_1 \\ \sigma_2 = \lambda \epsilon + 2G\epsilon_2 \\ \sigma_3 = \lambda \epsilon + 2G\epsilon_3 \end{cases} \quad \epsilon = \epsilon_1 + \epsilon_2 + \epsilon_3$$

$$\lambda = \frac{\nu E}{(1+\nu)(1+2\nu)} = \frac{0.3(200)}{(1+0.3)(1+0.6)} = 111.11 \text{ GPa}$$

$$G = \frac{E}{2(1+\nu)} = \frac{200}{2(1+0.3)} = 76.923 \text{ GPa}$$

$$\text{Assume } \epsilon_1 = 5C, \epsilon_2 = 4C, \epsilon_3 = 3C$$

$$\Rightarrow \epsilon = \epsilon_1 + \epsilon_2 + \epsilon_3 = 12C$$

$$\text{Using } \sigma_1 = \lambda \epsilon + 2G\epsilon_1$$

$$\Rightarrow (10^3)140 = 111.11(12C) + 2(76.923)5C$$

$$\Rightarrow C = 66.586(10^{-6})$$

$$\Rightarrow \epsilon_1 = 5C \Rightarrow 332.93(10^{-6})$$

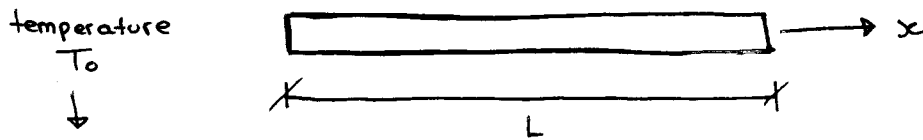
$$\epsilon_2 = 4C \Rightarrow 266.35(10^{-6})$$

$$\epsilon_3 = 3C \Rightarrow 199.76(10^{-6})$$

$$\Rightarrow \sigma_2 = \lambda \epsilon + 2G\epsilon_2 = 129.76 \text{ MPa}$$

$$\Rightarrow \sigma_3 = \lambda \epsilon + 2G\epsilon_3 = 119.53 \text{ MPa}$$

3.4 Thermoelasticity For isotropic materials

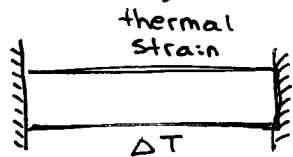


$T_1 = T_0 + \Delta T \rightarrow$ change in length ΔL

The coefficient of thermal expansion

$$\alpha = \frac{\Delta L}{L \cdot \Delta T} \Rightarrow \Delta L = \alpha \cdot L \cdot \Delta T$$

$$\Rightarrow \text{Strains } \epsilon = \frac{\Delta L}{L} = \alpha \Delta T$$



(case where there is thermal stress)

In 3D, Change temperature from $T_0 \rightarrow T_1 = T_0 + \Delta T$

\Rightarrow equal expansion

$$\rightarrow \epsilon'_{xx} = \epsilon'_{yy} = \epsilon'_{zz} = \alpha \Delta T$$

$$\epsilon'_{xy} = \epsilon'_{xz} = \epsilon'_{yz} = 0$$

Strains due to the loadings

$$\epsilon''_{xx}, \epsilon''_{yy}, \epsilon''_{zz}, \epsilon''_{xy}, \epsilon''_{xz}, \epsilon''_{yz}$$

The total deformation is

$$\left\{ \begin{array}{l} \epsilon_{xx} = \epsilon'_{xx} + \epsilon''_{xx} = \frac{1}{E} [\sigma_{xx} - \nu(\sigma_{yy} + \sigma_{zz})] + \alpha \Delta T \\ \epsilon_{yy} = \epsilon'_{yy} + \epsilon''_{yy} = \frac{1}{E} [\sigma_{yy} - \nu(\sigma_{xx} + \sigma_{zz})] + \alpha \Delta T \\ \epsilon_{zz} = \epsilon'_{zz} + \epsilon''_{zz} = \frac{1}{E} [\sigma_{zz} - \nu(\sigma_{xx} + \sigma_{yy})] + \alpha \Delta T \\ \epsilon_{xy} = \epsilon_{xy}'' = \frac{1}{2G} \sigma_{xy} \\ \epsilon_{yz} = \epsilon_{yz}'' = \frac{1}{2G} \sigma_{yz} \\ \epsilon_{xz} = \epsilon_{xz}'' = \frac{1}{2G} \sigma_{xz} \end{array} \right.$$

3.5 Hookes Law : orthotropic materials

3 orthogonal planes of Symmetry and 3 Corresponding orthogonal axes — called orthotropic axes.

9 independent material constants

$$\begin{bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \epsilon_{zz} \\ \epsilon_{yz} \\ \epsilon_{zx} \\ \epsilon_{xy} \end{bmatrix} = \begin{bmatrix} \frac{1}{E_x} & -\frac{\nu_{yx}}{E_y} & -\frac{\nu_{zx}}{E_z} & 0 & 0 & 0 \\ -\frac{\nu_{xy}}{E_x} & \frac{1}{E_y} & -\frac{\nu_{yz}}{E_z} & 0 & 0 & 0 \\ -\frac{\nu_{xz}}{E_x} & -\frac{\nu_{yz}}{E_y} & \frac{1}{E_z} & 0 & 0 & 0 \\ 0 & 0 & 0 & \frac{1}{2G_{yz}} & 0 & 0 \\ 0 & 0 & 0 & 0 & \frac{1}{2G_{zx}} & 0 \\ 0 & 0 & 0 & 0 & 0 & \frac{1}{2G_{xy}} \end{bmatrix} \begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{zz} \\ \sigma_{yz} \\ \sigma_{zx} \\ \sigma_{xy} \end{bmatrix}$$

where $\frac{\nu_{yz}}{E_y} = \frac{\nu_{zy}}{E_z}$, $\frac{\nu_{zx}}{E_z} = \frac{\nu_{xz}}{E_x}$, $\frac{\nu_{yx}}{E_y} = \frac{\nu_{xy}}{E_x}$

Example For Rochelle Salt the stress components at a point is

$$\sigma_{xx} = 7 \text{ MPa} \quad \sigma_{yy} = 2.1 \text{ MPa} \quad \sigma_{zz} = -2.8 \text{ MPa}$$

$$\sigma_{xy} = 1.4 \text{ MPa} \quad \sigma_{xz} = \sigma_{yz} = 0$$

Find 1° strain components

2° the principal stresses and directions

3° the principal strains and directions

Solution 1° :

$$\begin{Bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \epsilon_{zz} \\ \epsilon_{xy} \\ \epsilon_{yz} \\ \epsilon_{xz} \end{Bmatrix} = 10^{-12} \begin{bmatrix} 52 & -16.3 & -11.6 & 0 & 0 & 0 \\ -16.3 & 36.8 & -12.2 & 0 & 0 & 0 \\ -11.6 & -12.2 & 35.9 & 0 & 0 & 0 \\ 0 & 0 & 0 & 150.2 & 0 & 0 \\ 0 & 0 & 0 & 0 & 350.3 & 0 \\ 0 & 0 & 0 & 0 & 0 & 104.2 \end{bmatrix} \begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{zz} \\ \sigma_{yz} \\ \sigma_{zx} \\ \sigma_{xy} \end{Bmatrix}$$

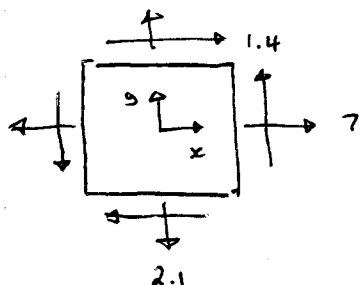
$$\Rightarrow \left\{ \begin{array}{c} 362.25 \mu \\ -2.66 \mu \\ -207.34 \mu \\ \emptyset \\ \emptyset \\ 145.88 \mu \end{array} \right\} \mu = 10^{-6}$$

2°: Stress Analysis

Since $\sigma_{xz} = \sigma_{yz} = \emptyset$, $\sigma_{zz} = -2.8 \text{ MPa}$

\therefore Z-axis is a principal direction, σ_{zz} is a principal stress

The other two principal directions in x-y plane



$$\begin{aligned} \tan \theta_p &= \frac{2\tau_{xy}}{\sigma_{xx} - \sigma_{yy}} \\ &= \frac{2(1.4)}{(7 - 2.1)} = 0.5714 \end{aligned}$$

$$2\theta_p = 29.74^\circ, 180^\circ + 29.74^\circ$$

$$\theta_p = 14.87^\circ, 104.87^\circ$$

$$\sigma_{avg} = \frac{\sigma_{xx} + \sigma_{yy}}{2} = \frac{7 + 2.1}{2} = 4.55$$

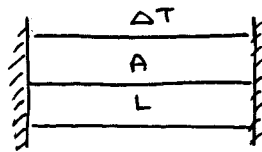
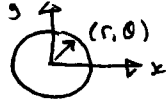
$$R = \sqrt{\left(\frac{\sigma_{xx} - \sigma_{yy}}{2}\right)^2 + \tau_{xy}^2} = 2.82$$

$$\therefore \sigma = \sigma_{avg} \pm R = 7.37, 1.73$$

Check $\theta_p = 14.87^\circ$ corresponds to $\sigma = 7.37 \text{ MPa}$

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Example 3.7, Page 98



radius
circumferential
 $\sigma_{rr}, \sigma_{\theta\theta}, \sigma_{zz}$ ← axial
 $\sigma_{r\theta}, \sigma_{rz}, \sigma_{\theta z} = 0$

Unknowns: $\sigma_{\theta\theta}, \sigma_{zz}$ in two cylinders

Stress in the aluminum (inner) cylinder: $\sigma_{\theta A}, \sigma_{z A}$

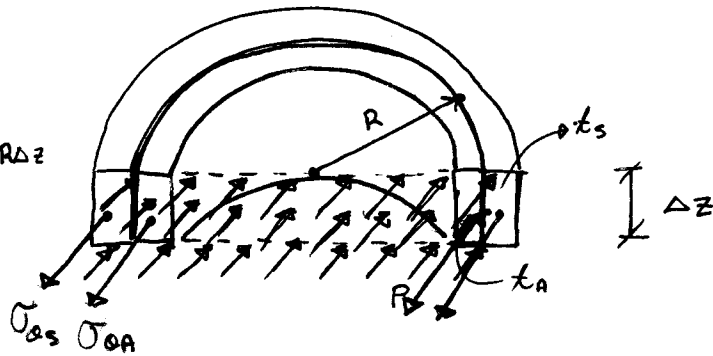
Stress in the steel (outer) cylinder: $\sigma_{\theta S}, \sigma_{z S}$

Equilibrium:

$$\sum F_\theta = 0:$$

$$2\sigma_{\theta S} t_S \Delta z + 2\sigma_{\theta A} t_A \Delta z - p_2 R \Delta z$$

$$\textcircled{1} \Rightarrow \sigma_{\theta S} t_S + \sigma_{\theta A} t_A = pR$$



Deformation:

Inner cylinder: $\epsilon_{\theta A}, \epsilon_{z A}, \epsilon_{r A}$

Outer cylinder: $\epsilon_{\theta S}, \epsilon_{z S}, \epsilon_{r S}$

Constraints:

$$\text{Axial} \begin{cases} \epsilon_{z A} = 0 & \textcircled{2} \\ \epsilon_{z S} = 0 & \textcircled{3} \end{cases}$$

Circumferential:

$$\text{at } r = R: \epsilon_{\theta A} = \epsilon_{\theta S} \quad \textcircled{4}$$

$$\left(\text{since } \epsilon_\theta = \frac{u}{r} + \frac{1}{r} \frac{\partial v}{\partial \theta} \right)$$

$$\epsilon_z = \frac{1}{E} (\sigma_z - \nu \sigma_\theta) + \alpha \Delta T$$

$$\epsilon_\theta = \frac{1}{E} (\sigma_\theta - \nu \sigma_z) + \alpha \Delta T$$

$$\text{Aluminum: } \epsilon_{z A} = \frac{1}{E} (\sigma_{z A} - \nu \sigma_{\theta A}) + \alpha \Delta T$$

$$\epsilon_{\theta A} = \frac{1}{E} (\sigma_{\theta A} - \nu \sigma_{z A}) + \alpha \Delta T$$

$$\text{Steel: } \epsilon_{z S} = \frac{1}{E} (\sigma_{z S} - \nu \sigma_{\theta S}) + \alpha \Delta T$$

$$\epsilon_{\theta S} = \frac{1}{E} (\sigma_{\theta S} - \nu \sigma_{z S}) + \alpha \Delta T$$

Given : $E_s = 207 \text{ GPa}$ $\nu_s = 0.280$
 $E_A = 69 \text{ GPa}$ $\nu_A = 0.333$
 $\alpha_A = 21.6 \times 10^{-6}/^\circ\text{C}$
 $\alpha_s = 10.8 \times 10^{-6}/^\circ\text{C}$

$$t_s = t_A = t = 0.02 \text{ m}$$

$$P = 689.4 \text{ kPa}, \quad \Delta T = 100^\circ\text{C}$$

$$\Rightarrow \begin{cases} \sigma_{\theta A} = 12.840P - 0.86337(10^6) \Delta T \\ \sigma_{\theta s} = 37.160P + 0.86337(10^6) \Delta T \\ \sigma_{zA} = 4.2757P - 1.7779(10^6) \Delta T \\ \sigma_{zs} = 10.405P - 1.9939(10^6) \Delta T \end{cases}$$