

(i) Given stress tensor $\underline{\underline{\sigma}} = \begin{bmatrix} 1 & 2 & 3 \\ 2 & 4 & 5 \\ 3 & 5 & 0 \end{bmatrix}$ MPa

On planes w/ normal $\hat{n}_1 = (1, 0, 0)$ and $\hat{n}_3 = (0, 0, 1)$
determine (a) normal stress

traction on plane 1 = $\vec{t}_1 = \underline{\underline{\sigma}} \cdot \hat{n}_1 = (1, 2, 3)$ MPa

normal stress = $\hat{n}_1 \cdot \vec{t}_1 = 1$ MPa = σ_{11}

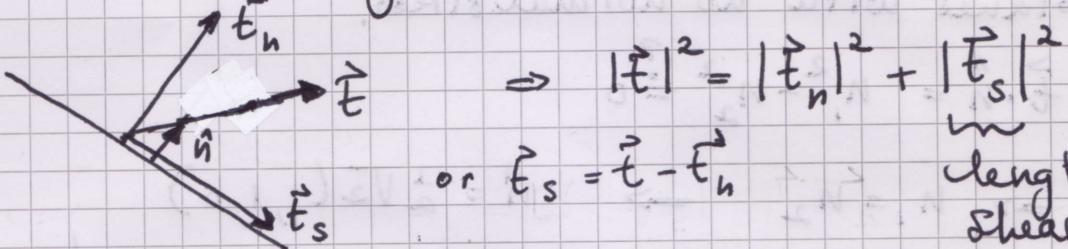
on plane 3

$\vec{t}_3 = \underline{\underline{\sigma}} \cdot \hat{n}_3 = (3, 5, 0)$ MPa

normal stress

$= \hat{n}_3 \cdot \vec{t}_3 = 0$ MPa = σ_{33}

(b) total shearing stress



$$|\vec{T}|^2 = |\vec{T}_n|^2 + |\vec{T}_s|^2$$

or $\vec{T}_s = \vec{T} - \vec{T}_n$

length total
shearing stress

plane 1 $|\vec{T}_s|^2 = (1+4+9) - 1 \Rightarrow |\vec{T}_s| = \sqrt{14} \approx 3.61$ MPa

plane 3 $|\vec{T}_s|^2 = |\vec{T}_3|^2 \Rightarrow |\vec{T}_s| = \sqrt{9+25} \approx 5.83$ MPa.

- (c) traction on a plane w/ a normal in the direction $2\hat{e}_1 + 2\hat{e}_2 + \hat{e}_3$, i.e. $\vec{n} = (2, 2, 1)$
and normalised $\hat{n} = \frac{1}{3}(2, 2, 1)$

$$\vec{T} = \underline{\underline{\sigma}} \cdot \hat{n} = \frac{1}{3} \begin{pmatrix} 9 \\ 17 \\ 16 \end{pmatrix}$$

$$(2) \underline{\sigma} = \begin{bmatrix} 1 & 0 & 0 \\ 0 & -1 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

(a) only plane on which $\vec{t} = 0$?

$$\vec{t} = \underline{\sigma} \cdot \hat{n} = \begin{pmatrix} n_1 \\ -n_2 \\ 0 \end{pmatrix} \stackrel{?}{=} 0$$

This is true if

$$n_1 = n_2 = 0$$

$$n_3 \text{ is free} \quad \left\{ \begin{array}{l} \hat{n} = (0, 0, 1) \\ \text{so indeed} \end{array} \right.$$

plane where
 $\hat{n} = \hat{e}_3$

(b) planes with no normal stress?

$$\vec{t} \cdot \hat{n} = n_1^2 - n_2^2 \stackrel{?}{=} 0$$

$$\Rightarrow n_1 = n_2 \quad \Rightarrow \quad \hat{n} = \frac{1}{2}\sqrt{2}(1, 1, 0)$$

or

$$n_1 = -n_2$$

or

$$n_1 = n_2 = 0$$

(as in a)

$$\hat{n} = \frac{1}{2}\sqrt{2}(1, -1, 0)$$

three
planes

$$\hat{n} = (0, 0, 1)$$

$$(3) \underline{\sigma} = \begin{bmatrix} 0 & 1 & 0 \\ 1 & 2 & 0 \\ 0 & 0 & 1 \end{bmatrix} \text{ MPa}$$

change coordinate system
such that

$$\hat{e}'_1 = \hat{e}_2; \quad \hat{e}'_2 = -\hat{e}_1, \quad \hat{e}'_3 = \hat{e}_3$$

to obtain $\sigma'_{11}, \sigma'_{21}, \sigma'_{33}$

$$\Rightarrow \sigma'_{11} = (0, 1, 0) \cdot \underline{\sigma} \cdot \begin{pmatrix} 0 \\ 1 \\ 0 \end{pmatrix} = 2 \text{ MPa}$$

either use $\sigma'_{ij} = (\hat{e}'_i)^T \cdot \underline{\sigma} \cdot \hat{e}'_j$

↑ single component

or use the transformation matrices A and A^T

$$\underline{\sigma}' = A \cdot \underline{\sigma} \cdot A^T$$

where

$$A^T = (\hat{e}'_1, \hat{e}'_2, \hat{e}'_3)$$

$$= \begin{bmatrix} 0 & -1 & 0 \\ 1 & 0 & 0 \\ 0 & 0 & 1 \end{bmatrix} = \hat{e}_i \cdot \hat{e}'_j$$

new basis vectors
as columns

$$\Rightarrow \underline{\sigma}' = \begin{bmatrix} 2 & -1 & 0 \\ -1 & 0 & 0 \\ 0 & 0 & 1 \end{bmatrix} \text{ MPa}$$

(4) Stress state where only nonvanishing stress components are set of shearing stresses is called simple shear

Take $\sigma_{12} = \sigma_{21} = \tau$, all other $\sigma_{ij} = 0$

$$\Rightarrow \underline{\sigma} = \begin{bmatrix} 0 & \tau & 0 \\ \tau & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

(a) principal values and directions

$$\text{Solve: } -\lambda^3 + \text{tr}(\underline{\sigma})\lambda^2 - \text{minor}(\underline{\sigma})\lambda + \det(\underline{\sigma}) = 0$$

$$\text{tr}(\underline{\sigma}) = 0 = \sigma_{ii}$$

$$\begin{aligned} \text{minor}(\underline{\sigma}) &= \left| \begin{array}{cc} \sigma_{11} & \sigma_{12} \\ \sigma_{21} & \sigma_{22} \end{array} \right| + \left| \begin{array}{cc} \sigma_{11} & \sigma_{13} \\ \sigma_{31} & \sigma_{33} \end{array} \right| + \left| \begin{array}{cc} \sigma_{22} & \sigma_{23} \\ \sigma_{32} & \sigma_{33} \end{array} \right| \\ &= -\tau^2 + 0 + 0 = -\tau^2 \end{aligned}$$

$$\det(\underline{\sigma}) = 0$$

$$\Rightarrow -\lambda^3 + \tau^2\lambda = 0$$

$$-\lambda(\lambda^2 - \tau^2) = 0$$

$$-\lambda(\lambda + \tau)(\lambda - \tau) = 0 \Rightarrow \lambda_1 = -\tau, \lambda_2 = \tau, \lambda_3 = 0$$

\rightarrow diagonalised
stress tensor

$$\begin{bmatrix} -\tau & 0 & 0 \\ 0 & \tau & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

\rightarrow can be arranged in
preferred order as
long as accompanying
eigenvectors similarly
ordered

eigenvectors $x^{(1)}, x^{(2)}, x^{(3)}$

$$\underline{\sigma} \cdot \vec{x}^{(1)} = -\tau \vec{x}^{(1)}$$

$$\begin{pmatrix} \tau x_1^{(1)} \\ \tau x_2^{(1)} \\ 0 \end{pmatrix} = \begin{pmatrix} -\tau x_1^{(1)} \\ -\tau x_2^{(1)} \\ -\tau x_3^{(1)} \end{pmatrix} \Rightarrow$$

$$\begin{aligned} \tau x_2^{(1)} &= -\tau x_1^{(1)} \Rightarrow x_2^{(1)} = -x_1^{(1)} \\ \tau x_1^{(1)} &= -\tau x_2^{(1)} \Rightarrow \\ 0 &= -\tau x_3^{(1)} \Rightarrow x_3^{(1)} = 0 \end{aligned}$$

$\Rightarrow \vec{x}^{(1)}$ can be $\begin{pmatrix} 1 \\ -1 \\ 0 \end{pmatrix}$

$$\text{normalised } \hat{x}^{(1)} = \frac{1}{\sqrt{2}} \begin{pmatrix} 1 \\ -1 \\ 0 \end{pmatrix}$$

$$\underline{\sigma} \cdot \vec{x}^{(2)} = \tau \vec{x}^{(2)}$$

$$\begin{cases} \tau x_2^{(2)} = \tau x_1^{(2)} \\ \tau x_1^{(2)} = \tau x_2^{(2)} \\ 0 = \tau x_3^{(2)} \end{cases}$$

$$x_2^{(2)} = x_1^{(2)}$$

$$x_3^{(2)} = 0$$

$$\begin{cases} \vec{x}^{(2)} \text{ can be } \begin{pmatrix} 1 \\ 1 \\ 0 \end{pmatrix} \\ \hat{x}^{(2)} = \frac{1}{\sqrt{2}} \begin{pmatrix} 1 \\ 1 \\ 0 \end{pmatrix} \end{cases}$$

Last eigenvector

$$\underline{\sigma} \cdot \vec{x}^{(3)} = 0 \cdot \vec{x}^{(3)}$$

$$\Rightarrow \vec{x}_2^{(3)} = 0$$

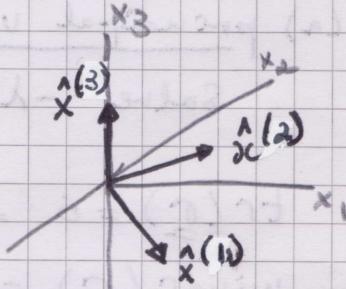
$$\vec{x}_1^{(3)} = 0$$

$$0 = 0 \cdot \vec{x}_3^{(3)} \rightarrow \text{satisfied by any } \vec{x}_3^{(3)}$$

(can check that $\hat{x}^{(1)} \times \hat{x}^{(2)} = \hat{x}^{(3)}$)

\Rightarrow also a righthanded system

$$\left\{ \begin{array}{l} \hat{x}^{(3)} = \begin{pmatrix} 0 \\ 0 \\ 1 \end{pmatrix} \end{array} \right.$$



(b) Max. shearing stress?
Stresses in original coordinate system

Can also show as follows:

Shear stress on a plane

$$\text{In 2D stress state } \begin{bmatrix} -\tau & 0 \\ 0 & \tau \end{bmatrix} = \begin{bmatrix} \sigma_1 & 0 \\ 0 & \sigma_2 \end{bmatrix}$$

$$\begin{aligned} \sigma'_{ns} &= \sigma_{11} \cos^2 \varphi \sin^2 \varphi + \sigma_{22} \sin^2 \varphi - \sigma_{12} \cos^2 \varphi - \sigma_{22} \cos \varphi \sin \varphi \\ &= (\sigma_1 - \sigma_2) \cos \varphi \sin \varphi \end{aligned}$$

$$\max \text{ where } \frac{\partial \sigma'_{ns}}{\partial \varphi} = 0$$

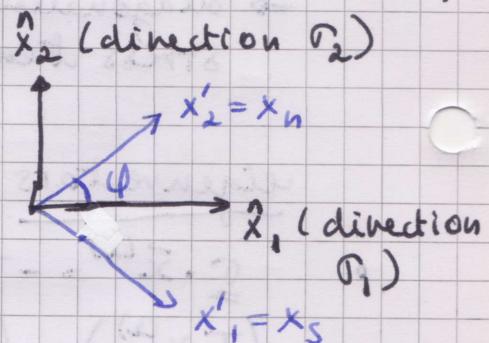
$$\begin{aligned} \frac{\partial \sigma'_{ns}}{\partial \varphi} &= (\sigma_1 - \sigma_2) [\cos^2 \varphi - \sin^2 \varphi] \\ &= (\sigma_1 - \sigma_2) \cos 2\varphi \end{aligned}$$

$$= 0 \text{ if } \cos 2\varphi = 0 \Rightarrow \varphi = 45^\circ \text{ or } \varphi = 135^\circ$$

i.e. indeed original coordinate axes were those in the direction of maximum shear stress

\Rightarrow max. shear stresses on planes w/ normal $(1,0,0)$ and normal $(0,1,0)$

values of these shear stresses = $\bar{\tau}$



$$(5) \text{ Given the stress: } \underline{\underline{\sigma}} = \begin{bmatrix} x_1 + x_2 & \sigma_{12}(x_1, x_2) & 0 \\ \sigma_{12}(x_1, x_2) & x_1 - 2x_2 & 0 \\ 0 & 0 & x_2 \end{bmatrix}$$

Find σ_{12} if

equilibrium w/ zero
body force, $\vec{f} = 0$

Also on plane $x_1=1$, $\vec{T} = (1+x_2)\hat{e}_1 + (5-x_2)\hat{e}_2$

- Plane $x_1=1$ has as normal $\hat{n} = (1, 0, 0)$

Stress on such a plane

$$\vec{\tau} = \underline{\underline{\sigma}} \cdot \hat{n} = \begin{pmatrix} x_1 + x_2 \\ \sigma_{12} \\ 0 \end{pmatrix} \quad \text{at } x_1=1 \text{ this becomes } \begin{pmatrix} 1+x_2 \\ \sigma_{12} \\ 0 \end{pmatrix}$$

$$\vec{f} = \begin{pmatrix} 1+x_2 \\ 5-x_2 \\ 0 \end{pmatrix} = \begin{pmatrix} 1+x_2 \\ \sigma_{12} \\ 0 \end{pmatrix} \Rightarrow \sigma_{12}(x_1=1) = 5-x_2$$

assuming linear in x_1 ,

$$\sigma_{12} = a \cdot x_1 + c - x_2$$

$$\text{where } a+c=5$$

- For equilibrium \rightarrow no acceleration, no body force

$$\frac{\partial \sigma_{ji}}{\partial x_j} = 0$$

$$\text{in } x_1 \text{ direction: } \frac{\partial \sigma_{11}}{\partial x_1} + \frac{\partial \sigma_{21}}{\partial x_2} + \frac{\partial \sigma_{31}}{\partial x_3} \stackrel{?}{=} 0$$

$$1 \quad -1 \quad +0 = 0 \quad \checkmark \text{ satisfied}$$

$$\text{in } x_2 \text{ direction: } \frac{\partial \sigma_{12}}{\partial x_1} + \frac{\partial \sigma_{22}}{\partial x_2} + \frac{\partial \sigma_{32}}{\partial x_3} \stackrel{?}{=} 0$$

$$a \quad -2 \quad +0 \stackrel{?}{=} 0$$

$$\Rightarrow a=2 \Rightarrow c=3$$

in x_3 direction

$$\frac{\partial \sigma_{13}}{\partial x_1} + \frac{\partial \sigma_{23}}{\partial x_2} + \frac{\partial \sigma_{33}}{\partial x_3} = 0$$

$$0 \quad 0 \quad 0 = 0 \quad \checkmark \text{ satisfied}$$

$$\Rightarrow \boxed{\sigma_{12}(x_1, x_2) = 2x_1 - x_2 + 3}$$

$$(6) \quad \underline{\underline{\sigma}}' = \underline{\underline{\sigma}} - \frac{\sigma_{kk}}{3} \underline{\underline{I}} = \begin{bmatrix} \sigma_{11} - \frac{\sigma_{kk}}{3} & \sigma_{12} & \sigma_{13} \\ \sigma_{12} & \sigma_{22} - \frac{\sigma_{kk}}{3} & \sigma_{23} \\ \sigma_{13} & \sigma_{23} & \sigma_{33} - \frac{\sigma_{kk}}{3} \end{bmatrix}$$

$$(a) \text{tr}(\underline{\underline{\sigma}}') = \sigma_{11} + \sigma_{22} + \sigma_{33} - 3\left(\frac{\sigma_{kk}}{3}\right) \\ = \sigma_{kk} - \sigma_{kk} = 0$$

$$(b) \text{tr}(\underline{\underline{\sigma}}) = 6+3+9 = 18 \Rightarrow \frac{\sigma_{kk}}{3} = 6$$

$$\Rightarrow \underline{\underline{\sigma}}' = \begin{bmatrix} 0 & 5 & -2 \\ 5 & -3 & 4 \\ -2 & 4 & 3 \end{bmatrix} \Rightarrow \text{diagonal elements of } \underline{\underline{\sigma}} \text{ modified while} \\ \text{for } i \neq j \quad \sigma'_{ij} = \sigma_{ij}$$

(c) principal stress directions of $\underline{\underline{\sigma}}'$?

remember that tensor transformation is linear and diagonalising of $\underline{\underline{\sigma}}$ comes down to transforming $\underline{\underline{\sigma}}$ into its principal stress direction coordinate frame

$$\Rightarrow \begin{bmatrix} \lambda_1 & & \emptyset \\ & \lambda_2 & \emptyset \\ \emptyset & \emptyset & \lambda_3 \end{bmatrix} = \underline{\underline{Q}}^T \cdot \underline{\underline{\sigma}} \cdot \underline{\underline{Q}} \text{ where } \underline{\underline{Q}} = \begin{bmatrix} \hat{x}_1 & \hat{x}_2 & \hat{x}_3 \end{bmatrix} \\ \text{with } \hat{x}_i \text{ the principal stress directions}$$

$$\underline{\underline{\sigma}} = \underline{\underline{\sigma}}' + \frac{\sigma_{kk}}{3} \underline{\underline{I}} \text{ so}$$

$$\begin{bmatrix} \lambda_1 & & \emptyset \\ & \lambda_2 & \emptyset \\ \emptyset & \emptyset & \lambda_3 \end{bmatrix} = \underline{\underline{Q}}^T \cdot \left[\underline{\underline{\sigma}}' + \frac{\sigma_{kk}}{3} \underline{\underline{I}} \right] \cdot \underline{\underline{Q}}$$

The unit tensor $\underline{\underline{I}}$ is invariant to coordinate transformation, so

$$\begin{bmatrix} \lambda_1 & & \emptyset \\ & \lambda_2 & \emptyset \\ \emptyset & \emptyset & \lambda_3 \end{bmatrix} = \underbrace{\underline{\underline{Q}}^T \cdot \underline{\underline{\sigma}}' \cdot \underline{\underline{Q}}} + \begin{bmatrix} \frac{\sigma_{kk}}{3} & \emptyset & \emptyset \\ \emptyset & \frac{\sigma_{kk}}{3} & \emptyset \\ \emptyset & \emptyset & \frac{\sigma_{kk}}{3} \end{bmatrix}$$

Must also be a diagonal tensor, i.e. same transformation that diagonalises $\underline{\underline{\sigma}}$ also does so for $\underline{\underline{\sigma}}'$