# CE 710 Advanced Composites

Neal Gordon Homework #2

## Anti-symmetric Composite modeled with CLPT

- [0 45 -45 90 0] composite
- Plate a = 20", b=10"
- Ply thickness = 0.0025"
- MATLAB was used
- A FSDT solution was not found due to errors in the programming

#### **CLPT**

#### Equations used from Reddy

$$\begin{split} \varepsilon_{xx} &= \frac{\partial u_0}{\partial x} + \frac{1}{2} \left( \frac{\partial w_0}{\partial x} \right)^2 - z \frac{\partial^2 w_0}{\partial x^2} \\ \varepsilon_{xy} &= \frac{1}{2} \left( \frac{\partial u_0}{\partial y} + \frac{\partial v_0}{\partial x} + \frac{\partial w_0}{\partial x} \frac{\partial w_0}{\partial y} \right) - z \frac{\partial^2 w_0}{\partial x \partial y} \\ \varepsilon_{yy} &= \frac{\partial v_0}{\partial y} + \frac{1}{2} \left( \frac{\partial w_0}{\partial y} \right)^2 - z \frac{\partial^2 w_0}{\partial y^2} \\ \varepsilon_{xz} &= \frac{1}{2} \left( -\frac{\partial w_0}{\partial x} + \frac{\partial w_0}{\partial x} \right) = 0 \\ \varepsilon_{yz} &= \frac{1}{2} \left( -\frac{\partial w_0}{\partial y} + \frac{\partial w_0}{\partial y} \right) = 0 \\ \varepsilon_{zz} &= 0 \end{split}$$

$$(3.3.8)$$

$$\begin{cases}
\varepsilon_{xx} \\
\varepsilon_{yy} \\
\gamma_{xy}
\end{cases} = \begin{cases}
\varepsilon_{xx}^{(0)} \\
\varepsilon_{yy}^{(0)} \\
\gamma_{xy}^{(0)}
\end{cases} + z \begin{cases}
\varepsilon_{xx}^{(1)} \\
\varepsilon_{yy}^{(1)} \\
\gamma_{xy}^{(1)}
\end{cases}$$

$$\begin{cases}
\varepsilon_{xx} \\
\varepsilon_{yy} \\
\gamma_{xy}^{(1)}
\end{cases} = \begin{cases}
\varepsilon_{xx}^{(0)} \\
\varepsilon_{yy}^{(0)} \\
\gamma_{xy}^{(1)}
\end{cases} = \begin{cases}
\varepsilon_{xx}^{(1)} \\
\varepsilon_{yy}^{(1)}
\end{cases} = \begin{cases}
\varepsilon_{xx}^{(1)} \\
\varepsilon_{xy}^{(1)}
\end{cases} = \begin{cases}
\varepsilon_{xy}^{(1)} \\
\varepsilon_{xy$$

 $\{\varepsilon^{0}\} = \left\{ \begin{array}{l} \varepsilon_{xx}^{(0)} \\ \varepsilon_{yy}^{(0)} \\ \gamma_{xy}^{(0)} \end{array} \right\} = \left\{ \begin{array}{l} \frac{\partial u_{0}}{\partial x} + \frac{1}{2} \left( \frac{\partial w_{0}}{\partial x} \right)^{2} \\ \frac{\partial v_{0}}{\partial y} + \frac{1}{2} \left( \frac{\partial w_{0}}{\partial y} \right)^{2} \\ \frac{\partial u_{0}}{\partial y} + \frac{\partial v_{0}}{\partial x} + \frac{\partial w_{0}}{\partial x} \frac{\partial w_{0}}{\partial y} \end{array} \right\} , \quad \{\varepsilon^{1}\} = \left\{ \begin{array}{l} \varepsilon_{xx}^{(1)} \\ \varepsilon_{xy}^{(1)} \\ \varepsilon_{yy}^{(1)} \\ \gamma_{xy}^{(1)} \end{array} \right\} = \left\{ \begin{array}{l} -\frac{\partial^{2} w_{0}}{\partial x^{2}} \\ -\frac{\partial^{2} w_{0}}{\partial y^{2}} \\ -2\frac{\partial^{2} w_{0}}{\partial x \partial y} \end{array} \right\}$  (3.3.10)

#### **CLPT**

$$\begin{cases}
N_{xx} \\
N_{yy} \\
N_{xy}
\end{cases} = \begin{bmatrix}
A_{11} & A_{12} & A_{16} \\
A_{12} & A_{22} & A_{26} \\
A_{16} & A_{26} & A_{66}
\end{bmatrix} \begin{cases}
\varepsilon_{yy}^{(0)} \\
\varepsilon_{yy}^{(0)} \\
\gamma_{xy}^{(0)}
\end{cases} + \begin{bmatrix}
B_{11} & B_{12} & B_{16} \\
B_{12} & B_{22} & B_{26} \\
B_{16} & B_{26} & B_{66}
\end{bmatrix} \begin{cases}
\varepsilon_{xx}^{(1)} \\
\varepsilon_{yy}^{(1)} \\
\gamma_{xy}^{(1)}
\end{cases} (3.3.36)$$

$$\begin{cases}
M_{xx} \\
M_{yy} \\
M_{xy}
\end{cases} = \sum_{k=1}^{N} \int_{z_{k}+1}^{z_{k+1}} \begin{cases}
\sigma_{xx} \\
\sigma_{yy} \\
\sigma_{xy}
\end{cases} z dz$$

$$= \sum_{k=1}^{N} \int_{z_{k}+1}^{z_{k+1}} \begin{bmatrix}
\bar{Q}_{11} & \bar{Q}_{12} & \bar{Q}_{16} \\
\bar{Q}_{12} & \bar{Q}_{22} & \bar{Q}_{26} \\
\bar{Q}_{26} & \bar{Q}_{26}
\end{bmatrix} \begin{cases}
\varepsilon_{yy}^{(0)} + z\varepsilon_{xx}^{(1)} \\
\varepsilon_{yy}^{(0)} + z\varepsilon_{xy}^{(1)}
\end{cases} z dz$$

$$\begin{cases}
M_{xx} \\
M_{yy} \\
M_{xy}
\end{cases} = \begin{bmatrix}
B_{11} & B_{12} & B_{16} \\
B_{12} & B_{22} & B_{26} \\
B_{16} & B_{26} & B_{66}
\end{bmatrix} \begin{cases}
\varepsilon_{xx}^{(0)} \\
\varepsilon_{yy}^{(0)} \\
\gamma_{xy}^{(0)}
\end{cases} + \begin{bmatrix}
D_{11} & D_{12} & D_{16} \\
D_{12} & D_{22} & D_{26} \\
D_{16} & D_{26} & D_{66}
\end{bmatrix} \begin{cases}
\varepsilon_{xx}^{(1)} \\
\varepsilon_{yy}^{(1)} \\
\gamma_{xy}^{(1)}
\end{cases} (3.3.37)$$

$$\begin{cases}
\{N\} \\
\{M\} \} = \begin{bmatrix}
A_{1} & B_{1} \\
B_{1} & D_{1}
\end{bmatrix} \begin{cases}
\varepsilon^{0} \\
\xi^{1} \end{cases} \} - \begin{cases}
\{N^{T} \\
M^{T} \} - \begin{cases}
N^{P} \\
M^{P} \end{cases} \end{cases}$$

$$\begin{cases}
N_{xx} \\
N_{yy} \\
N_{xy}
\end{cases} = \begin{bmatrix}
A_{11} & A_{12} & A_{16} \\
A_{16} & A_{26} & A_{66}
\end{bmatrix} \begin{cases}
\frac{\partial w_{0}}{\partial x} + \frac{1}{2}(\frac{\partial w_{0}}{\partial x})^{2} \\
\frac{\partial w_{0}}{\partial x} + \frac{1}{2}(\frac{\partial w_{0}}{\partial x})^{2} \\
\frac{\partial w_{0}}{\partial y} + \frac{\partial w_{0}}{\partial x} + \frac{\partial w_{0}}{\partial x} \frac{\partial w_{0}}{\partial y}
\end{cases}$$

$$- \begin{bmatrix}
B_{11} & B_{12} & B_{16} \\
B_{12} & B_{22} & B_{26} \\
B_{16} & B_{26} & B_{66}
\end{bmatrix} \begin{cases}
\frac{\partial^{2} w_{0}}{\partial x^{2}} \\
\frac{\partial^{2} w_{0}}{\partial y^{2}}
\end{cases}$$

$$2\frac{\partial^{2} w_{0}}{\partial y^{2}}
\end{cases}$$

$$(3.3.43)$$

$$\begin{cases}
M_{xx} \\
M_{yy} \\
M_{xy}
\end{cases} = 
\begin{bmatrix}
B_{11} & B_{12} & B_{16} \\
B_{12} & B_{22} & B_{26} \\
B_{16} & B_{26} & B_{66}
\end{bmatrix} 
\begin{cases}
\frac{\partial u_0}{\partial x} + \frac{1}{2} (\frac{\partial w_0}{\partial x})^2 \\
\frac{\partial v_0}{\partial y} + \frac{1}{2} (\frac{\partial w_0}{\partial y})^2 \\
\frac{\partial u_0}{\partial y} + \frac{\partial v_0}{\partial x} + \frac{\partial w_0}{\partial x} \frac{\partial w_0}{\partial y}
\end{cases}$$

$$- 
\begin{bmatrix}
D_{11} & D_{12} & D_{16} \\
D_{12} & D_{22} & D_{26} \\
D_{16} & D_{26} & D_{66}
\end{bmatrix} 
\begin{cases}
\frac{\partial^2 w_0}{\partial x^2} \\
\frac{\partial^2 w_0}{\partial y^2} \\
2 \frac{\partial^2 w_0}{\partial x \partial y}
\end{cases}$$
(3.3.44)

#### **CLPT**

$$0 = \frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \sigma_{xy}}{\partial y} + \frac{\partial \sigma_{xz}}{\partial z}$$

$$0 = \frac{\partial \sigma_{xy}}{\partial x} + \frac{\partial \sigma_{yy}}{\partial y} + \frac{\partial \sigma_{yz}}{\partial z}$$

$$0 = \frac{\partial \sigma_{xz}}{\partial x} + \frac{\partial \sigma_{yz}}{\partial y} + \frac{\partial \sigma_{zz}}{\partial z}$$

$$(4.2.13)$$

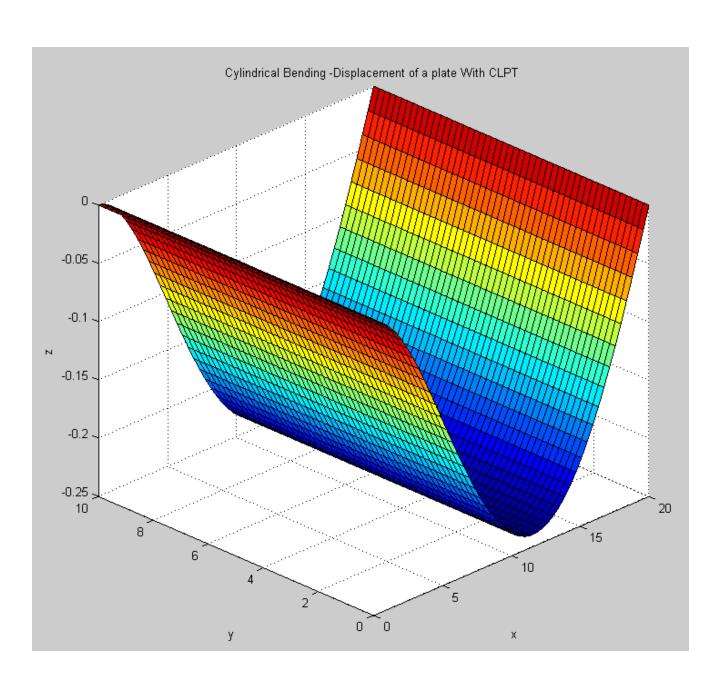
$$A_{11}\frac{\partial^2 u_0}{\partial x^2} + A_{16}\frac{\partial^2 v_0}{\partial x^2} - B_{11}\frac{\partial^3 w_0}{\partial x^3} - \frac{\partial N_{xx}^T}{\partial x} = I_0\frac{\partial^2 u_0}{\partial t^2} - I_1\frac{\partial^3 w_0}{\partial x \partial t^2}$$
(4.4.1a)

$$A_{16} \frac{\partial^2 u_0}{\partial x^2} + A_{66} \frac{\partial^2 v_0}{\partial x^2} - B_{16} \frac{\partial^3 w_0}{\partial x^3} - \frac{\partial N_{xy}^T}{\partial x} = I_0 \frac{\partial^2 v_0}{\partial t^2}$$
(4.4.1b)

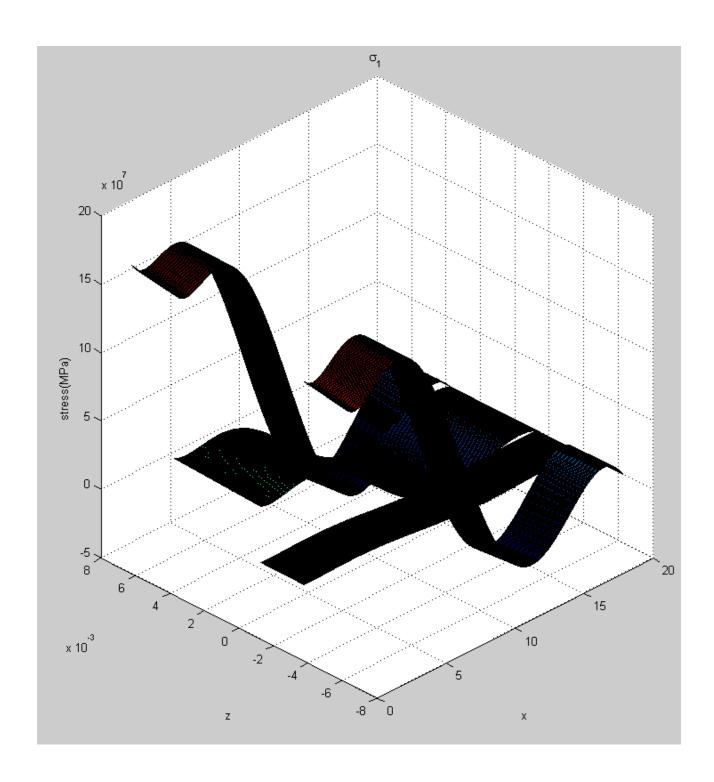
$$\begin{split} B_{11} \frac{\partial^3 u_0}{\partial x^3} + B_{16} \frac{\partial^3 v_0}{\partial x^3} - D_{11} \frac{\partial^4 w_0}{\partial x^4} + \frac{\partial}{\partial x} \left( \hat{N}_{xx} \frac{\partial w_0}{\partial x} \right) - \frac{\partial^2 M_{xx}^T}{\partial x^2} + q \\ &= I_0 \frac{\partial^2 w_0}{\partial t^2} - I_2 \frac{\partial^4 w_0}{\partial x^2 \partial t^2} + I_1 \frac{\partial^3 u_0}{\partial x \partial t^2} \end{split} \tag{4.4.1c}$$

Edge Condition	CLPT	FSDT
z free	$N_{xx} = 0$ $N_{xy} = 0$	$N_{xx}=0$ $N_{xy}=0$
x	$M_{xx} = 0  \frac{dM_{xx}}{dx} = 0$	$M_{xx}=0$ $Q_x=0$
z roller	$w_0 = 0 \qquad \frac{dv_0}{dx} = 0$	$w_0 = 0 \qquad \frac{dv_0}{dx} = 0$
	$N_{xx}=0$ $M_{xx}=0$	$N_{xx}=0$ $M_{xx}=0$
z ▲ simple support	$u_0 = 0$ $w_0 = 0$	$u_0 = 0$ $w_0 = 0$
-x	$\frac{dv_0}{dx} = 0 \qquad M_{xx} = 0$	$\frac{dv_0}{dx} = 0 \qquad M_{xx} = 0$
clamped	$u_0 = 0$ $v_0 = 0$	$u_0 = 0$ $v_0 = 0$
<b>→</b> x	$w_0 = 0 \qquad \frac{dw_0}{dx} = 0$	$w_0 = 0$ $\phi_x = 0$

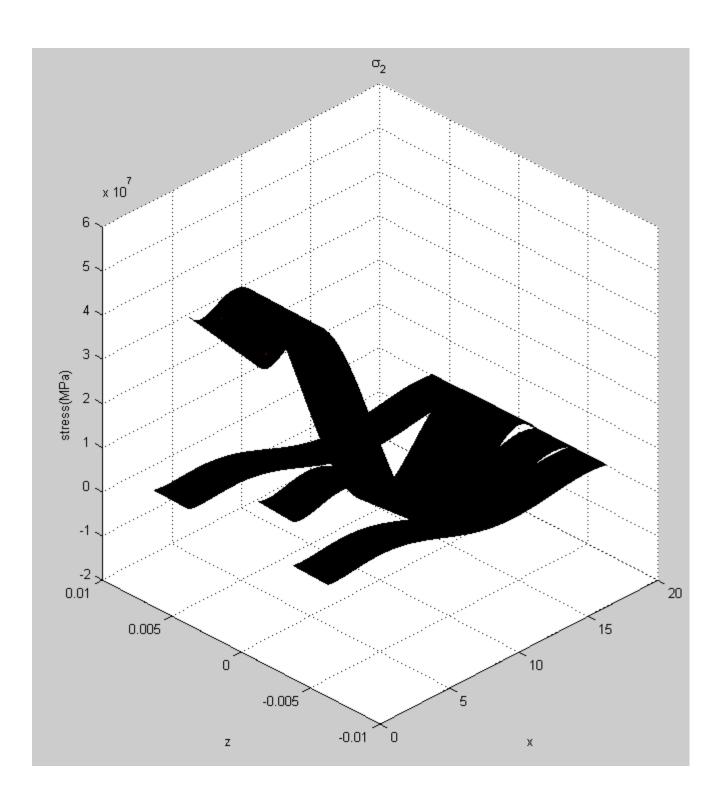
### Displacement of the plate



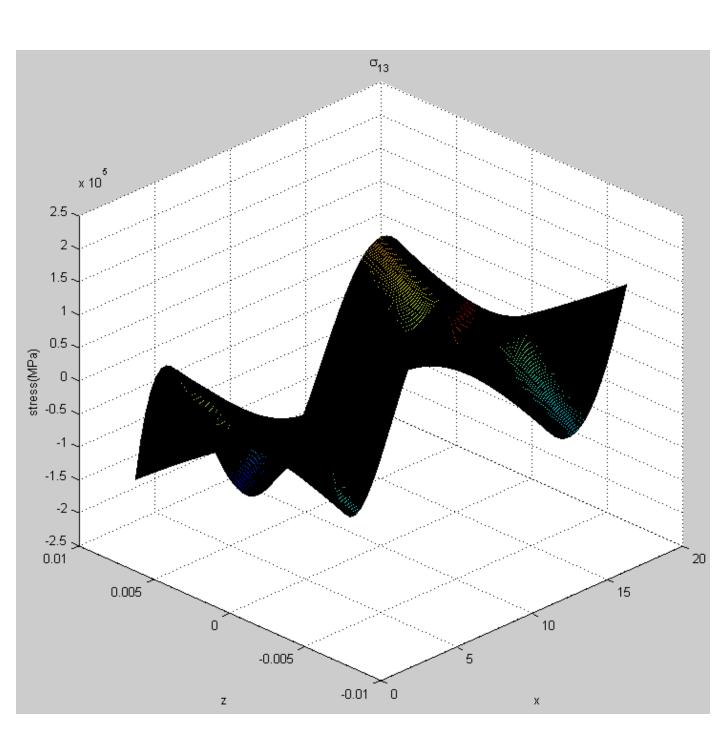
## Stress $\sigma_1$ in each ply



## Stress $\sigma_2$ in each ply



## Stress $\sigma_{13}$ in each ply



```
% ce 710 hmk2
clear all
clc
close all
%% Variables
               1 (top) ... nl (to bottom)
% layer
theta = fliplr([0 45 - 45 90 0].* pi/180);
thk = zeros(1, length(theta)) + 0.0025;
nl = length(thk);
    20; % plate width;
b = 10; % plate height
q0 = 5.7; % plate load;
% Transversly isotropic material properties
Ell = 150e9;
Ett = 12.1e9;
vlt = 0.248;
Glt = 4.4e9;
vtt = 0.458;
Gtt = Ett / (2*(1+vtt));
% Failure Strengths
SLLt = 1500e6;
SLLc = -1250e6;
STTt = 50e6;
STTc = -200e6;
SLTs = 100e6;
Sxzs = 100e6;
Strength = [SLLt SLLc;
           STTt STTc;
           SLTs Sxzs];
%% Stiffness Matrix
syms th
% tranformation
Tij6 = [\cos(th)^2 \sin(th)^2 0 0 - \sin(2*th);
       sin(th)^2 cos(th)^2 0 0 0 sin(2*th);
       0 0 1 0 0 0;
       0 0 0 cos(th) sin(th) 0;
       0 0 0 -sin(th) cos(th) 0;
       Tij = [\cos(th)^2 \sin(th)^2 2*\sin(th)*\cos(th);
       \sin(th)^2 \cos(th)^2 -2*\sin(th)*\cos(th);
       -\cos(th) *\sin(th) \sin(th) *\cos(th) (\cos(th)^2-\sin(th)^2);
% compliance matrix
Sij6 = [1/Ell - vlt/Ell - vlt/Ell 0 0 0;
      -vlt/Ell 1/Ett -vtt/Ett 0 0 0;
      -vlt/Ell -vtt/Ett 1/Ett 0 0 0;
      0 0 0 1/Gtt 0 0;
      0 0 0 0 1/Glt 0;
      0 0 0 0 0 1/Glt];
```

```
% Stiffnes matrix in material coordinates
Cijm6 = inv(Sij6);
% Stiffness matrix in Structural coordinates
Cij6 = Tij6*Cijm6*Tij6.';
% reduced stiffness in structural
Cij = [Cij6(1,1) \ Cij6(1,2) \ 0; \ Cij6(1,2) \ Cij6(2,2) \ 0; \ 0 \ 0 \ Cij6(6,6)];
hlam = sum(thk);
% Create z dimensions of laminate
z(1) = -hlam/2;
for i = 1:n1
   z(i+1) = z(1) + sum(thk(1:i));
end
% extensional stiffness
Aij = zeros(6,6);
for i = 1:n1
    Aij = Aij + subs(Cij6, th, theta(i)) * (z (i+1)-z (i));
end
% coupling stiffness
Bij = zeros(6,6);
for i = 1:n1
    Bij = Bij + 0.5* subs(Cij6,th,theta(i)) * (z (i+1)^2-z (i)^2);
end
% bending or flexural laminate stiffness relating moments to curvatures
Dij = zeros(6,6);
for i = 1:nl
    Dij = Dij + (1/3)* subs(Cij6, th, theta(i)) * (z (i+1)^3-z (i)^3);
end
%% Cylindrical Bending of a laminated plate
% displacement in w (z direction)
syms x y z q0 C1 C2 C3 C4 C5 C6 C7 A11 B11 D11 A16 B16
syms wfun ufun
% EQ 4.4.1a
    = A11*diff(ufun,x,2) - B11*diff(wfun,x,3); % C5 C1
% EQ 4.4.1b
eq2
    = A16*diff(ufun,x,2) - B16*diff(wfun,x,3); % C5 C1
% EQ 4.4.1c
eq3 = B11*diff(ufun,x,3) - D11*diff(wfun,x,4) + q0;
% solve eq1 eq2 and eq3 to get the w and u functions
% displacement in w (z direction) from eq1,eq2,eq3
wfun = A11*q0*x^4 / (4*(6*B11^2-6*A11*D11)) + C1 + C2*x + C3*x^2 + C4*x^3; % C1 C2 C3 C4
% displacement in u (x direction) from eq1,eq2,eq3
ufun = B11*q0*x^3 / (6*(B11^2-A11*D11)) + C7 + x*C6 + 3*B11*x^2*C5/A11;% C5 C6 C7
```

```
% cond1 \rightarrow w(0)=0 at x(0), roller
C1sol = solve(subs(wfun, x, 0) == 0, C1); % = 0
% cond2 -> angle at dw/dx at x(0) is 0, cantilever
C2sol = solve(subs(diff(wfun,x),x,0),C2); % = 0
% cond3 -> w(z) = 0 at x(a), roller
C4sol1 = solve(subs(wfun,[x C1 C2],[a C1sol C2sol ]),C4); % C3
% cond4 u = 0 at x = 0
C7sol = solve(subs(ufun,x,0),C7); %=0
u=0 at x = a
C5sol1 = solve(subs(ufun,[x C7],[a C7sol]),C5); %C6
% cond 5 EQ 4.4.14a Myy = 0 0 x(a) (Mxx , B11 D11) (Myy, B12 D12) roller no moment
C6sol1 = solve(subs( [B11*(diff(ufun,x)+0.5*diff(wfun,x)^2) - D11*diff(wfun,x,2)],...
               [x C1
                     C2
                            C4
                                   C5
                                          C7],...
               [a C1sol C2sol C4sol1 C5sol1 C7sol]),C6); % C6 C3
% EQ 4.4.13a, Nxx = 0 @ x(0) roller has no Nxx
C6sol2 = solve(subs([A11* (diff(ufun,x) +0.5*diff(wfun,x)^2)-B11*diff(wfun,x,2)],...
    [x C1 C2 C4 C5 C7], [a C1sol C2sol C4sol1 C5sol1 C7sol]), C6); % C6 C3
C3sol = solve(C6sol1 == C6sol2,C3);
C4sol = subs(C4sol1, C3, C3sol);
C6sol = simplify(subs(C6sol2,C3,C3sol));
C5sol = simplify(subs(C5sol1,C6,C6sol));
% substitute integration constants with actual values ( is actual number)
C1 = C1sol;
C2_ = C2sol;
C7 = C7sol;
C3_{=} subs(C3sol,[q0 A11 B11 D11],[q0_ Aij(1,1) Bij(1,1) Dij(1,1)]);
C4_ = subs(C4sol, [q0 A11 B11 D11], [q0_ Aij(1,1) Bij(1,1) Dij(1,1)]);
C5 = subs(C5sol,[q0 A11 B11 D11],[q0 Aij(1,1) Bij(1,1) Dij(1,1)]);
C6 = subs(C6sol,[q0 A11 B11 D11],[q0 Aij(1,1) Bij(1,1) Dij(1,1)]);
wsol = subs(wfun,[q0 C1 C2 C3 C4 A11 B11 D11],...
                [q0_ C1_ C2_ C3_ C4_ Aij(1,1) Bij(1,1) Dij(1,1)]);
% function u(x) horizontal displacement u along x with actual vaules
usol = subs(ufun,[q0 C5 C6 C7 A11 B11
                                                   D11],...
                [q0_ C5_ C6_ C7_ Aij(1,1) Bij(1,1) Dij(1,1)]);
ezsurf(x,y,wsol,[0,a,0,b])
view(-45,30)
xlabel('x')
ylabel('y')
zlabel('z')
title('Cylindrical Bending -Displacement of a plate With CLPT')
wsol opt = matlabFunction(wsol);
[xmax, wmax] = fminsearch(wsol opt, 0);
%% Strain calculation
% \text{ eq } 3.3.8 \text{ (pg } 116 \text{ reddy (pdf = } 138))}
epstotal = [diff(usol,x) + 0.5* diff(wsol,x)^2 - z*diff(wsol,x,2),0,0].';
epsx = epstotal(1);
%% Calculating and plotting Stress in each layer
res = 8; % accuracy of finding max and min stress
xplot = linspace(0,a,res);
```

```
yplot = linspace(0,b,res);
for kstress = 1:3 % stress state s_x, s_y, s_xz
    figure(kstress+1)
   hold on
    for klay = 1:nl % loop through all layers
        thplot = theta(klay);
        zplot = linspace(z (klay), z (klay+1), res);
        %% Calc Stresses
        if kstress == 3
            % Shear stresses
            syms G0
            G0 = -int(diff(s stress(1),x),z)+G0.';
            % solve for shear stresses from s 1
            s xz = solve(G0,G0);
            % out of plane shear S xz does not need to be transformed ??
            ezsurf(s xz, [0, a, z (klay), z (klay+1)])
        else
            % normal stresses
            % Cij = reduced structural stiffness in strictural coordinates 3x3
            % stress in structural coordinates
            s stress = subs(Cij,th,thplot)*epstotal;
            % stressin material coordinates
            m stress = subs(Tij,th,thplot)*s stress ;
            ezsurf(m stress(kstress),[0,a,z (klay),z (klay+1)])
        end
        %% find max stress in each layer
        ii=1;
        for i = xplot
            jj=1;
            for j = zplot
                if kstress == 3
                    stressplot(ii,jj) = subs(s_xz,[x z],[i j]);
                else
                    stressplot(ii,jj) = subs(m stress(kstress),[x z],[i j]);
                end
                jj=jj+1;
            end
            ii=ii+1;
        Globalminstress(kstress, klay) = min(min(stressplot));
        Globalmaxstress(kstress, klay) = max(max(stressplot));
   end
    hold off
    axis auto
    title(strcat('\sigma_',num2str(kstress)))
    zlabel('stress(MPa)')
   view(-45,30)
%% Plot max stress and failure strength
figure
```

```
for i = 1:3
    subplot(1,3,i)
    bar(Globalmaxstress(i,:))
    hold on
    bar(Globalminstress(i,:))
    scatter(1:nl,ones(nl,1).*Strength(i,1),'filled')
    scatter(1:nl,ones(nl,1).*Strength(i,2),'filled')
    hold off
    xlabel('layer')
    title(strcat('\sigma',num2str(i)))
end
```