## **HOLT CONSULTING ENGINEERS (PTY) LTD**

#### **TUMELA 18 TON SKIP DESIGN**

### **H-MAX603**

### H-MAX603-CAL-MM-18SKIP-001-SHT-001

Engineer:	Gerald Holt (Pr.Eng)
Registration:	20020259
Email:	gerald@holtconsulting.co.za
Phone:	+27-791-2549
Adress:	Unit G, Mini Park
	16 Gerhardus Street, Strijdom Park
	Randburg, South Africa
Website:	www.holtconsulting.co.za



# **REVISION HISTORY**

REV	DESCRIPTION	DATE	ISSUED BY	REVIEWED BY	APPROVED
A	ISSUED FOR REVIEW	2022-10-15	G.G. HOLT	H.F. HOLT	

# 1 CUSTOMER DETAILS

Customer:	Max Power Services (Pty) Ltd
<b>Customer Name:</b>	Herman de Koker
Customer Email:	harry@maxpower.co.za

## 2 CALCULATION INPUT DATA

## 2.1 Applicable Design Codes

SANS 10208: 3 - 2017: Design of structures for the mining industry Part 3: Conveyances

SANS 10610: Buildling loading code

SANS 10162: Steel design

## 2.2 General Data

Design Method	Limit States (Rope Break Conditions)	
Material of Construction	Main Body: EN10025 S355JR	
Material of Construction	Liners: VRN 500	
Yield Stress	355	MPa
Skip Weight	9878	kg
Payload	18000	kg
Winding Speed	15	m/s
Winding Rope Diameter	54	mm
Winding Rope Unit Mass	12.45	kg/m
Rope Break Force	2319	kN
Ultimate Tensile Strength	1900	MPa
Winder Acceleration	0.8	$m/s^2$
Winder Trip Acceleration	5	$m/s^2$
Winder Travel Distance	1023	m
Number of Cycles per Month	3000	
Skip Internal Height	5600	mm
Skip Internal Width	1557	mm
Skip Internal Depth	1400	mm
Skip Overall Height	10713	mm
Skip Overall Width	1856	mm
Skip Overall Depth	1743	mm
Ore Bulk Density	1950	$kg/m^3$

# 2.3 Assumption Data

Spacing between rails	1800	mm
Top Hat Guide Specification	340 x 175mm	
Top Hat Guide Material Specification	EN10025 S355JR	
Top Hat Guide Unit Mass	85.95	kg/m
Top Hat Guide Width	175	mm
Bunton Stiffness	1608000	N/m
Guide Stiffnes	1600000	N/m

## 2.4 Sketches and Drawings

#### 2.4.1 General Arrangement

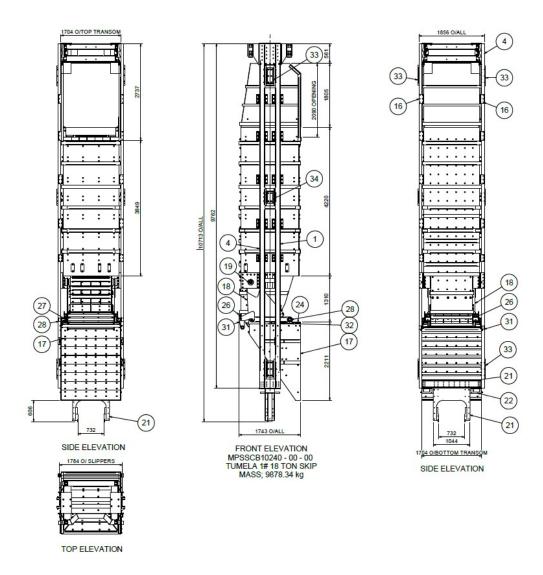
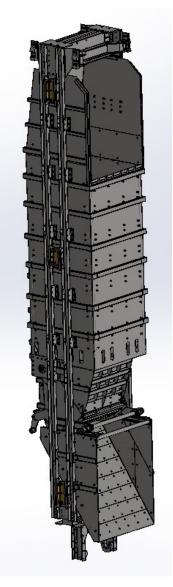
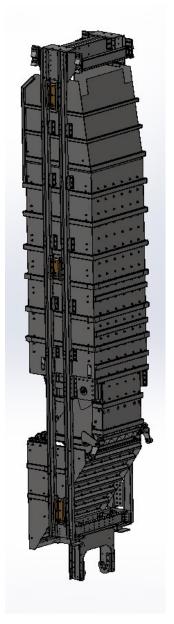


Figure 1: 18 ton Skip Drawing

### 2.4.2 Isometric Views



(a) 18 ton Skip Isometric View 1



(b) 18 ton Skip Isometric View 3

# 3 CALCULATIONS

## 3.1 Skip Loads

## 3.1.1 Basic Skip Parameters

Payload	R	18000	kg
Payload	$R_l$	176580.0	N
Winding Speed	$V_w$	15	m/s
Winder Acceleration	$a_w$	0.8	$m/s^2$
Winder Trip Acceleration	$a_t$	5	$m/s^2$
Winding Rope Diameter	$Rope_d$	54	$m/s^2$
Rope Break Force	RBF	2319	kN
Ultimate Tensile Strength	UTS	1900	MPa
Ore Bulk Density	$ ho_b$	1950	$kg/m^3$
Skip Internal Width	b	1557	mm
Skip Internal Depth	w	1557	mm
Skip Volume Required	$Vol = R/\rho_b$	9.2	$m^3$
Ore Height in Skip	h = Vol/(bw)	4.2	m
Rope Strentch Under Payload	$\Delta L$	765.8	mm

### 3.1.2 Permanent Loads

Skip Bridle Sides	$m_1$	1167	kg
Skip Bridle Top Transom	$m_2$	1522	kg
Skip Bridle Bottom Transom	$m_3$	850	kg
Skip Unit	$m_4$	6336	kg
Permanent Load	$G_c = (m_1 + m_3 + m_4)g$	81943	N

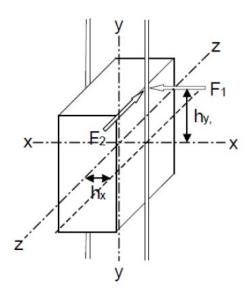


Figure 3: Properties Diagram

## ${\bf 3.1.3}\quad Laterial\ Imposed\ Loads\ (H)\ -\ Fixed\ Guide\ Systems\ in\ Vertical\ Shafts$

Clearance between Roller and Slipper	$\Delta_c$	10	mm
Slipper Plate Impact Factor	$\alpha_n$	2	
Guide Roller Assembly Stiffness	$k_r$	500000	N/m
Bunton Stiffness	$k_{b}$	1608000	N/m
Guide Stiffnes	$k_g$	1600000	N/m
Moment of Inertia about X-axis	$I_X$	80510	$kg.m^2$
Moment of Inertia about Y-axis	$I_{\mathbf{y}}$	6838	$kg.m^2$
Moment of Inertia about Z-axis	$I_{\mathcal{Z}}$	82050	$kg.m^2$
Distance from slipper to center of gravity	$h_X$	892	mm
Distance from slipper to center of gravity	$h_{\mathrm{y}}$	4847	mm
Distance from slipper to center of gravity	$h_z$	28	mm
Guide Roller Lateral Load	$H_f$	5000000	N
Steelwork Stiffness Ratio	$r_k$	1.005	
Weight of Skip System	$m_c$	8353	kg
Effective Mass About y - x Plane	$m_x = (m_c I_z)/(I_z + m_c (h_y)^2)$	2463.0	kg
Effective Mass About y - z Plane	$m_z = (m_c I_x I_y) / (I_x I_y + (m_c I_x (h_y)^2) + (m_c I_y (h_x)^2)$	280.0	kg
Non-Dimensional Laterial Stiffness	$K_x = (k_b L_b^2) / m_x V^2$	9	
Non-Dimensional Laterial Stiffness	$K_z = (k_b L_b^2)/m_z V^2$	83	
Plate Coefficient from graph	$P_b$	0.05	
Maximum Moving Misalighnment	e	0.01	m
Lateral Slipper Pad Load	$H_{S}$	7791	N

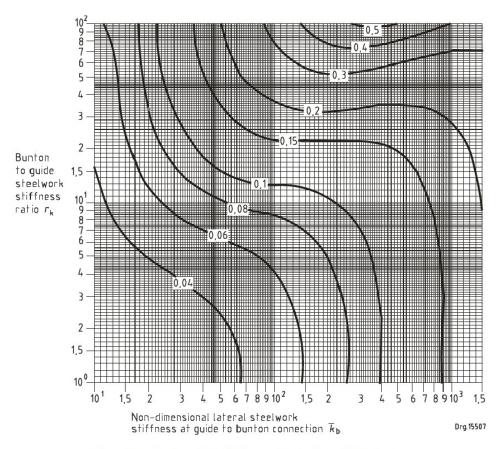


Figure 1 — Contour plot of slipper plate load coefficient  $\overline{P_b}$ 

Figure 4: Slipper Plate Load Coefficient Pb

#### 3.1.4 Winder System Loads

Dynamic Impact Factor	$\alpha_d$	2	
Winder Acceleration and Deceleration	$a_o$	0.8	$m/s^2$
Winder Trip Acceleration	$a_t$	0.8	$m/s^2$
Skip Self Weight	$G_c$	81943	N
Content Load	$C_{\mathrm{y}}$	176580.0	N
Tail Rope Load	T	0	N
Acceleration Load	$A_o = (\alpha_d)a_o(G_c + C_y + T)/g$	42165	N
Acceleration Trip Out Load	$A_t = (\alpha_d)a_t(G_c + C_y + T)/g$	263530	N

#### 3.1.5 Emergency Loads

Emergency Load  $E_r$  2319000 N

#### 3.1.6 Vertical Friction Loads

Lateral Slipper Pad Load  $H_s$  7791 NVertical Friction Load  $F_v = 0.5H_s$  3895.5 N

#### 3.1.7 Rock Loads

Filling Impact Factor in Stationary Position	$\alpha_v$	1.5	
Load on Tipping Rollers Impact Factor	$\alpha_t$	2	
Static Load	R	176580.0	N
Bridle Transom Load While Filling	$R_d = (\alpha_v)(R)$	27000.0	N
Rock Pressure	$p_o = \rho_b g h$	80.3439	$N/m^2$
Pressure on the Door	$p_1 = 1p_o$	80.3	$N/m^2$
Pressure on the Back of Skip	$p_2 = 0.5 p_o$	40.2	$N/m^2$
Pressure on the Lower Portion Skip Back	$p_3 = 0.5 p_o$	120.5	$N/m^2$
Pressure on the Front and Sides of Skip	$p_4 = 0.2 p_o$	16.1	$N/m^2$

## 3.2 Skip Element Design

### 3.2.1 Top Transom

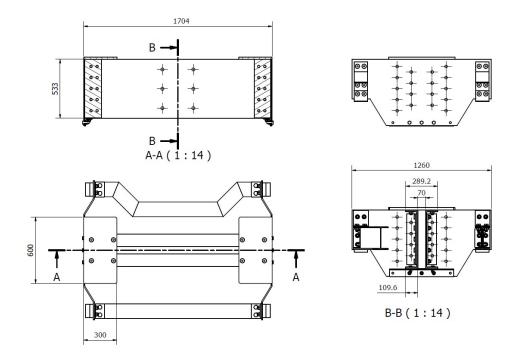


Figure 5: Top Transom Configuration

#### **Material Data**

### $2\,/\,533$ x 110 CHANNEL (MODIFIED 533 X 210 X 93 UB) WITH TOP AND BOTTOM 20 X 100 PLATE

### 4 / 20 X 100 PLATES (TOP AND BOTTOM)

#### 2 / 150 x 150 ANGLE END CONNECTIONS

Length of Transom	L	1700	mm
Channel Width	b	109	mm
Channel Height	h	533	mm
Channel Web Thickness	$t_w$	10.2	mm
Channel Flange Thickness	$t_f$	15.6	mm
Top and Bottom Plate Width	w	110	mm
Top and Bottom Plate Thickness	$t_p$	20	mm
Yield Stress	$f_{y}$	355	MPa
Channel Cross Sectional Area	$A = 2bt_f + h_w * t_w$	8519.0	$mm^2$
Channel Centroid Distance	$x_c = (0.5h_w t_w^2 + t_f b^2)/A$	25.0	mm
Channel Second Moment of Area about x-x	$I_x = bh^3/12 - b_f h_w^3/12$	335071491.0	$mm^4$
Channel Second Moment of Area about y-y	$I_y = h_w t_w^3 / 3 + 2t_f b^3 / 3 - Ax_c^2)$	8397644.0	$mm^4$
Double Plate Polar Moment	$J = I_x + I_y$	340973234.0	$mm^4$
Channel Warping Constant	$C_w = (1/144)(t_f^3 b^3) + (1/36)(h - t_f/2)^3)t_w^3$	4304577645.0	$mm^6$
Channel Plastic Section Modulus	$Z_p = bh^2/4 - b_f h_w^2/4$	1521885.0	$mm^3$
Double Plate Cross Sectional Area	$A = 2(wt_p)$	4400	$mm^2$
Double Plate Second Moment of Area about x-x	$I_x = w/12(H^3 - h^3)$	336536567.0	$mm^4$
Double Plate Second Moment of Area about y-y	$I_y = t_p w^3 / 6$	4436667.0	$mm^4$
Double Plate Plastic Section Modulus	$Z_p = 2(wt_p(h - t_p/2)$	2301200.0	$mm^3$
Combined Second Moment of Area about x-x	$I_X$	671608058.0	$mm^3$
Combined Second Moment of Area about y-y	$I_{y}$	12834311.0	$mm^3$
Combined Plastic Section Modulus	$Z_p$	3823085.0	$mm^3$
Plastic Moment	$M_p = Z_p f_y$	1357.2	kNm
Adjusted Plastic Moment	$0.67M_{p}$	909.3	kNm
Critical Elastic Moment	$M_c = ((\pi^4 E^2 C_w I_y)/L^4 + (\pi^2 E I_y G J)/L^2)^0.5$	20494.3	kNm
Factored Moment Resistance	$M_r = 1.15 \phi M_p (1 - (0.28 M_p/M_c))$	1378.7	kNm
Factored Shear Resistance	$Vr = 0.55\phi A f_y$	2270.0	kNm
D C E I I.			

### **Design for Emergency Loads**

Rope Break Force	RBF	2319	kN
Number of Beams	No.	2	
Combined Shear Resistance	$M_r$	4540.0	kN
Ultimate Bending Moment	$M_u = RBFL/4$	986.0	kN
Ultimate Shear Force	$M_u = RBF/2$	1160.0	kN
Interaction Check	$M_{\mu}/M_r + V_{\mu}/V_r < 1$	0.61	Pass

## **Design for Operation Loads**

Operation Force	$LC = 1.2G_c + 1.6R$	127.0	kN
Number of Beams	No.	2	
Combined Shear Resistance	$M_r$	4540.0	kN
Ultimate Bending Moment	$M_u = (RBF)L/4$	54.0	kN
Ultimate Shear Force	$M_u = (RBF)/2$	64.0	kN
Interaction Check	$M_u/M_r + V_u/V_r < 1$	0.03	Pass

### **Design for Fatigue Loads**

Cycles	3000	
Design	24	months
Trips	72000	
$R_d = (\alpha_v)(R)$	27000.0	N
$G_c = (m_1 + m_3 + m_4)g$	81943	N
$MCL = R_d + 0.25G_c$	47485.75	N
$\sigma_a = 0.5(MCL)L/(4Z_p))$	2.65	MPa
$2A_o$	84330	N
$\sigma_b = 0.5(2A_oL/(4Z_p))$	9.4	MPa
0.2R	3600.0	N
$\sigma_c = 0.5(0.2RL/(4Z_p))$	0.4	MPa
$S_e$	70	MPa
$\sigma_a/S_e + \sigma_b/S_e + \sigma_c/S_e < 1$	0.11	Pass
	Design  Trips $R_d = (\alpha_v)(R)$ $G_c = (m_1 + m_3 + m_4)g$ $MCL = R_d + 0.25G_c$ $\sigma_a = 0.5(MCL)L/(4Z_p)$ ) $2A_o$ $\sigma_b = 0.5(2A_oL/(4Z_p))$ $0.2R$ $\sigma_c = 0.5(0.2RL/(4Z_p))$ $S_e$	Design24 $Trips$ 72000 $R_d = (\alpha_v)(R)$ 27000.0 $G_c = (m_1 + m_3 + m_4)g$ 81943 $MCL = R_d + 0.25G_c$ 47485.75 $\sigma_a = 0.5(MCL)L/(4Z_p))$ 2.65 $2A_o$ 84330 $\sigma_b = 0.5(2A_oL/(4Z_p))$ 9.4 $0.2R$ 3600.0 $\sigma_c = 0.5(0.2RL/(4Z_p))$ 0.4 $S_e$ 70

#### 3.2.2 Bottom Transom

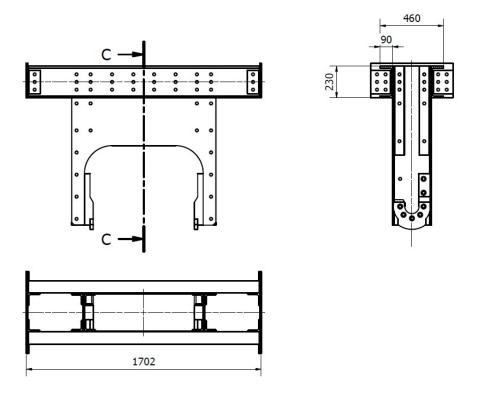


Figure 6: Bottom Transom Configuration

#### **Material Data**

- 2 / 230 x 90 CHANNEL
- 4/20~X~230~PLATES~(TOP~AND~BOTTOM)
- $2\,/\,100$  x 100 ANGLE END CONNECTIONS

Length of Transom	L	1700	mm
Channel Width	b	90	mm
Channel Height	h	230	mm
Channel Web Thickness	$t_w$	7.5	mm
Channel Flange Thickness	$t_f$	14	mm
Top and Bottom Plate Width	w	230	mm
Top and Bottom Plate Thickness	$t_p$	20	mm
Yield Stress	$f_{y}$	355	MPa
Channel Cross Sectional Area	$A = 2bt_f + h_w * t_w$	4035.0	$mm^2$
Channel Centroid Distance	$x_c = (0.5h_w t_w^2 + t_f b^2)/A$	30.0	mm
Channel Second Moment of Area about x-x	$I_x = bh^3/12 - b_f h_w^3/12$	34585945.0	$mm^4$
Channel Second Moment of Area about y-y	$I_y = h_w t_w^3 / 3 + 2t_f b^3 / 3 - Ax_c^2)$	3318071.0	$mm^4$
Double Plate Polar Moment	$J = I_x + I_y$	184613334.0	$mm^4$
Channel Warping Constant	$C_w = (1/144)(t_f^3 b^3) + (1/36)(h - t_f/2)^3)t_w^3$	143847363.0	$mm^6$
Channel Plastic Section Modulus	$Z_p = bh^2/4 - b_f h_w^2/4$	348668.0	$mm^3$
Double Plate Cross Sectional Area	$A = 2(wt_p)$	9200	$mm^2$
Double Plate Second Moment of Area about x-x	$I_x = w/12(H^3 - h^3)$	144056667.0	$mm^4$
Double Plate Second Moment of Area about y-y	$I_y = t_p w^3 / 6$	40556667.0	$mm^4$
Double Plate Plastic Section Modulus	$Z_p = 2(wt_p(h - t_p/2)$	2024000.0	$mm^3$
Combined Second Moment of Area about x-x	$I_X$	178642612.0	$mm^3$
Combined Second Moment of Area about y-y	$I_{y}$	43874738.0	$mm^3$
Combined Plastic Section Modulus	$Z_p$	2372668.0	$mm^3$
Plastic Moment	$M_p = Z_p f_y$	842.3	kNm
Adjusted Plastic Moment	$0.67M_{p}$	564.3	kNm
Critical Elastic Moment	$M_c = ((\pi^4 E^2 C_w I_y)/L^4 + (\pi^2 E I_y G J)/L^2)^0.5$	21605.1	kNm
Factored Moment Resistance	$M_r = 1.15 \phi M_p (1 - (0.28 M_p / M_c))$	862.3	kNm
Factored Shear Resistance	$Vr = 0.55\phi A f_y$	2326.0	kNm

## **Design for Emergency Loads**

Rope Break Force	RBF	2319	kN
Number of Beams	No.	2	
Combined Shear Resistance	$M_r$	4652.0	kN
Ultimate Bending Moment	$M_u = RBFL/4$	986.0	kN
Ultimate Shear Force	$M_u = RBF/2$	1160.0	kN
Interaction Check	$M_u/M_r + V_u/V_r < 1$	0.82	Pass

### 3.2.3 Bridle Hangers

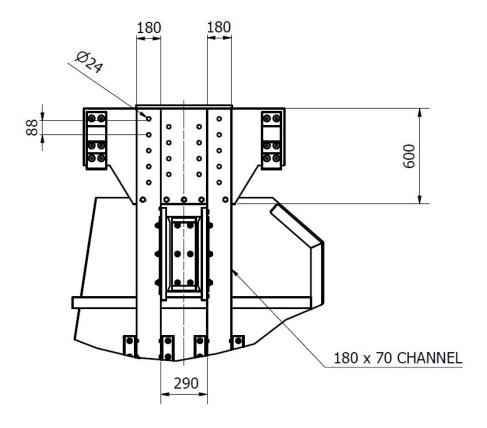


Figure 7: Bridle Configuration

### **Design for Emergency Loads**

Total Cross Sectional Bridle Area	$A_t$	10720	$mm^2$
Total Net Cross Sectional Bridle Area	$A_e$	10048	mm
Rope Break Force	RBF	2319	kN
Tensile Resistance per Channel	$T_r = 0.85 \phi A f_y$	682.0	N
Number of Bridless	No.	4	
Total Tensile Resistance	$T_r$	2728.0	N
Interaction Check	$T_u/T_r < 1$	0.85	Pass

## **Design for Fatigue Loads**

Cycles per Month	Cycles	3000	
Design Life	Design	24	months
Total Number of Trips	Trips	72000	
Number of Bridle Channels	$N_b$	4	
Rock during Filling	$R_d = (\alpha_v)(R)$	27000.0	N
Permanent Load	$G_c = (m_1 + m_3 + m_4)g$	81943	N
Major Cycle Load	$MCL = R_d + 0.25G_c$	47485.75	N
Stress Amplitude	$\sigma_a = MCL/(N_b A_n))$	18.9	MPa
Acceleration Cycles Load	$2A_o$	84330	N
Stress Acceleration Increase	$\sigma_b = (2A_o/(N_b A_n))$	8.4	MPa
Bounding Load	0.2R	3600.0	N
Stress Acceleration Increase	$\sigma_c = (0.2R/(N_b A_n))$	0.4	MPa
Steel Endurance Limit	$S_e$	70	MPa
Interaction Check	$\sigma_a/S_e + \sigma_b/S_e + \sigma_c/S_e < 1$	0.4	Pass

# 4 SUMMARY

ززززززز