6115 -MAHENDRA INSTITUTE OF ENGINEERING AND TECHNOLOGY

FLOOD MONITORING AND EARLY WARNING

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1. UrbanFlood early warning system computational workflow

The Sensor Monitoring module receivesta streams from the sensors installed in the dike. Raw sensor data is filtered by the AI Anomaly Detector that identifies abnormalities in dike behavior or sensor malfunctions. The Reliability Analysis module calculates the probability of dike failure in case of abnormally high water levels or an upcoming storm and extreme rainfalls. If the failure probability is high then the Breach Simulator predicts the dynamics of a possible dike failure, calculates water discharge through the breach and estimates the total time of the flood. After that, the Flood simulator models the inundation dynamics. For expert users, the Virtual Dike component is available. All these EWS modules

are described in more detail in Sections 2-6. Information from all the modules is fed into the Decision Support System (DSS), some el

The primary user-interface to this DSS system runs on the Microsoft Surface interactive graphics device (further called Surface for short). The Surface is a multiĞtouch system, which means that multiple people can collaboratively work with the applications, using their hands and fingers as input devices. The combination of multiple people interacting with the application at the same time makes the use of this device very intuitive and helpful for crisis situations —exactly what Early Warning Systems are designed for. The simulation modules and visualization components are integrated into the Common Information Space. They are accessed from the interactive graphical environment of the multi-touch table or through a web-based application. Examples of decision support information modules accessed via the interactive multi-touch Microsoft Surface environment

2. Artificial Intelligence Anomaly Detector

Structurally stable dikes are considered to be in a normal state. For the real-life flood defense systems, there are no historical records of the abnormal sensor parameters in critical pre-failure conditions. For the abnormality detection, we use a one-side classification concept based on the Neural Clouds (NC) method [5]. NC approach allows detection of anomalies based on the historical measurements related to normal behavior used for training.

After the training phase, Artificial Intelligence (AI) component processes the measurements from sensors in realtime and calculates the anomaly probabilities. One of the main advantages of the AI methods in environmental data analysis is the possibility to apply a model-free data-driven approach, which requires only the measured data. The NC classification algorithm uses pre-processed data and/or a set of features extracted from the data as inputs.First, data is clustered by an advanced k-means clustering algorithm [5]. Second, the clustered data is encapsulated using an extended radial basis functions network approach (Fig. 3). These two operations are applied to the training V.V. Krzhizhanovskaya et al. / Procedia Computer Science 4 (2011) 106—115 109 data set forming an N-dimensional diagnostic parameter space. After training, a dike "normality" confidence value is calculated for each new set of measurements, by projecting it to the constructed data encapsulator The probability of normal behavior close or equal to 1 corresponds to the safe dike behavior (similar to the reference data), while values close to zero indicate a previously unknown and potentially abnormal behavior. The AI component is adaptive: in case the reported anomaly was not a critical issue and should be considered as normal, the AI component is retrained on the extended reference data set. The first version of the AI component has been developed and integrated in the UrbanFlood EWS via the Common Information Space as explained in Section 7. A set of classification agents forming a committee of classifiers is constructed for dike abnormality detection. In present prototype, the committee consists of NCs constructed for every sensor measuring more than 1 parameter. After that, the calculated probabilities are used to estimate the condition of the whole cross-section or the whole dike. Further plans for the AI component development include construction of more sophisticated committees and implementation of additional methods for signal pre-processing and feature extraction. Visualization of the AI component in operation and abnormality detection results are shown below

3. Dike reliability analysis

To trigger warnings and further computational flood simulations, exceedence thresholds are specified, based on the probability of structural failure for the monitored dikes. For flood dike structures, the probability of failure is often defined conditionally upon the hydraulic loading (i.e. flood water levels and wave action), the resulting probability distributions are known as fragility curves. These are typically

developed using standard reliability analysis, see for example [6,7,8]. In traditional structural reliability analysis, failure of a structure arises when the loads acting upon the structure, exceed its bearing capacity, or resistance. The probability of failure is often presented in the following generalized form [9]:Pf P G X f dx G X X [( ) 0] …. ( ) ( ) 0³ ³≤= ≤ =

where G is the Limit State Function (LSF); X is the vector of random variables associated with the loads and resistance; and fX(x) is the multivariate probability density function of X. For the development of fragility curves, the probability of failure is derived, conditional on the loading conditions: P P[G(X) 0 L l] f = ≤ = In some cases, the relationships between loads and the resistance are known explicitly and G (X) can be expressed in closed form or the problem can be suitably simplified such that this is the case. A range of methods can Fig. 3. Neural Cloud encapsulator visualized for two sensor parameters X1, X2. Dike "normality" confidence levels are shown by the colored 3D surface.normality confidence level calculated by the AI component for one of the sensors located in a selected cross-section 110 V.V. Krzhizhanovskaya et al. / Procedia Computer Science 4 (2011) 106—115 then be employed to solve the multi-dimensional integral. The most flexible and commonly applied approach is through Monte-Carlo simulation. Under the EU funded FLOODsite project (FP6) [10] we documented the limit state equations for a wide range of flood defense structures and their potential failure mechanisms. In other cases more complex models are required to describe the failure processes. When these models are com putationally intensive,

vanilla Monte-Carlo become impractical to implement. Response Surface methods can then be employed [9] toreduce the computational burden and implement the analysis. An example of this type of implementation in the context of flood defense is described in [11].Under the EU funded FLOODsite Project, a software tool RELIABLE was developed to analyze the reliability offlood defenses and generate fragility curves. The tool includes a total of 72 failure modes [10] represented as simpleLimit State Equations (LSEs), a flexible fault tree component, and a probabilistic failure analysis component based on Monte Carlo simulation (MCS). It is applicable to foreshores, dunes and banks; dikes and revetments; walls; and point structures, and accounts for hydraulic loading due to water level difference across a structure; wave loading; and lateral flow velocities. In the system described here, this software tool is being deployed in an online environment. Information from the sensor systems is used to determine values of parameters that are input to the LSEs. The reliability analysis is then conducted in real time and the threshold probability of failures monitored. Threshold exceedence then triggers downstream modeling and warning activities.

4. Dike breaching and flood simulations

The Flood Simulator module predicts the flood dynamics once a dike is considered failed. This involves two calculations: estimation of the discharge through the breach and computation of the flood spreading. These two calculations are currently done separately, and it is envisaged that those tools could be swapped for other models in the future without changing the functioning of the Flood Simulator module.

The breach discharge calculation is a simplified model based on the following concept: the breach initiates with a given width, this width is unchanged as the breach invert level decreases linearly with time until reaching the toe level of the dike, then the breach invert level stays unchanged and the breach width increases according to one of the two following empirical equations:0.145 67 logt Wsand = for a sand dike and 0.08 20logt Wsand = for a clay dike where W is the breach width (m) at time t. t is the time (hrs) after breach reached the lowest invert level. For all these stages, the discharge is calculated using the weir equation 1.5 Q = CLh , where C=1.7, L is the breach width (m), h is the head of water on eroding crest (m) and is calculated using the load on the dike (water level) and the breach invert. An example of the breach width dynamics and related water discharge through this breach is shown below

The flood spreading is computed by the rapid flood spreading model (RFSM), a simplified and computationally efficient model yet sufficiently robust for use in flood system risk models. The model was

originally developed as a volume spreading approach with no temporal component [12,13]. This has however been extended to include the time domain (Dynamic RFSM or DRFSM). Time dependence of dike breach width and water discharge. Water levels used in this example are not linked to the possible flood conditions Simulated inundation of Amsterdam Science Park, the University of Amsterdam campus. Marked with the red cross X is the location of a hypothetical breach in the Ring Dike

V.V. Krzhizhanovskaya et al. / Procedia Computer Science 4 (2011) 106—115 111 In a pre-processing stage, the domain is discretised in irregular shaped computational elements. These so called impact zones (IZs) are delineated around depressions in the topography. Input to this pre-process is the floodplain topography in the form of a Digital Terrain Model (DTM). Each IZ captures the underlying topography by the means of a table giving the volume of water stored in the IZ for different flood levels. This mesh allows to speed up the simulation by reducing the number of computational elements compared to the initial number of cells in the input DTM. The use of the level-volume relation means that the computation of the water level in an IZ is more precise than the use of an averaged ground level for situations where the IZ is not entirely flooded. The model receives flood volumes discharged into floodplain areas from breached or overtopped defenses and then spreads the water over the floodplain according to the terrain topography (Fig. 7). Spreading of flood water isachieved by transferring water between IZs at each computational time-step. The discharge between IZs can be calculated by two methods, the Manning relationship (i.e. similar to diffusion wave models) or the weir relation. The computational time-step is constant. Water level, average discharge and average velocity are calculated in each IZ during the computation. This approach can be used in probabilistic flood risk analysis where multiple runs are required, or in real time situations (flood forecasting), where the model run time is critical. Flood Simulator has been validated against available flood data and more advanced models.

An interactive visualization application was designed for the Surface which allows a new simulation to be interactively defined: a breach can be added by simply touching on the desired location on a map, pop-up windows allow the characteristics of the dike to be defined, the hydrograph can be interactively adjusted and the total simulation time as well as simulation step length. Once defined, the simulation is delegated to the CIS by the touch of a button. The output of the flood simulator is visualized on the Surface represented as an animated time series of water level in each Impact Zone. The simulation results are visualized on top of a topological representation of the area which can be selected from a range of sources, including Google Maps, Bing Maps, Yahoo! Maps, and OpenStreetMap. Interaction methods allow the visualization to be navigated (zoom, scale, rotate) and introspected (such as a graph that reflects water level over time in the touched Impact Zone). In Fig. 7 the flooding of the Science Park area of Amsterdam is shown. For this experiment we obtained DTM data from Actueel Hoogtebestand Nederland of the Amsterdam Watergraafsmeer area. The total area used in the simulation measures 4.4 by 2.8 km with a resolution of 25 m2 per pixel.

5. Virtual Dike

Virtual Dike is an advanced multiscale multi-model simulation lab for expert users and model developers [18]. This virtual lab is used for validation of all the models involved in the modeling cascade, and serves as a research field for experiment planning and understanding the underlying physical processes influencing dike stability and features. In the first stage of the project, we have studied the structural stability of the LiveDike, a sea dike in Eemshaven. LiveDike is protecting a seaport in Groningen. This dike has been equipped with sensors, and data stream is available in real-time. Pore pressure and dike inclination sensors are placed in four dike cross-sections.These cross-sections have been simulated in 2D models under tidal water loading. Simulation results have beencompared with the pore pressure sensors data in order to calibrate model parameters.The modeling approach is based on a coupled fluid-structure interaction with non-linear dike material properties.At the land side, water stays at the constant average see level. A more detailed description of the Virtual Dike models and simulation results can be found in [18], and parallel performance results in [19]. The resulting transient fields of pore pressure, water content and structural deformations have been obtained. Structural displacements field in one moment in time is shown in Fig. 8. The displacements are composed of (a) the static soil settlement under gravity load (maximal at the top of the dike) and (b) transient displacements resulting from the tidal pressure at

the seaside and volume pore pressure load. Total displacements are maximal at the top of the dike due to gravity settlement component.picture shown below

A comparison of real and simulated water pore pressures for one sensor location is shown in signal is shown with a bold line; a virtual (simulated) sensor signal is shown with a thin line. The amplitude of simulated pore pressure oscillations is higher than the real amplitude, which indicates that the soil permeability around this sensor must be reduced in the model. That would also make real and simulated pressure oscillations more synchronous in this point. Thus local inhomogeneities shall be included into the model in future simulations, in order to obtain good agreement of pore pressure dynamics for all sensors

 Dynamic resource allocation service

(DyReAlla): a service for dynamic allocation of resources to running Early Warning Systems. Robustness is a key requirement for Early Warning Systems. Consequently, the current implementation of CIS relies on several mature and stable technologies. At the same time the framework does not impose any particular technology for developing Early Warning System components, therefore avoiding the risk of vendor lock-in. This is achieved by adopting the Service-Oriented Architecture (SOA) principles. EWS parts are loosely coupled services with well defined interfaces described in WSDL, which can be accessible through a variety of protocols including SOAP, REST, JMS or FTP. These services do not exchange data directly but through a message bus, therefore there are no direct dependencies between them. As a result, they can be developed independently using different technologies. For example, currently we support both Apache Camel and OpenESB / BPEL for component integration and workflow orchestration. Apache ActiveMQ, a JMS implementation, is used as the message bus.

To increase robustness even further, the same mechanics of the CIS are used to create an EWS that instead of having sensors in an embankment, has sensors in the other EWSs, thereby monitoring the state of the appliances,workflows and data streams in an EWS. This sensor data is part of the four step process mentioned at the start of this section. After it has been fed into the self-monitoring EWS, it is processed by the AI to provide anomaly detection in the various signals. If anything is out of the ordinary, the DyReAlla component is notified and action can be taken.Specific models that take into account the behavior of the appliances are an ongoing research.The EWS components have been deployed in a distributed manner across three partner sites, and some modules have been ported to the SARA Supercomputing Center Clouds [15]. The cloud is hosted on a 128-core cluster with the following characteristics: 16 compute nodes; CPU dual quad-core 2.2 GHz; 24 GB RAM per node; 500 GB local hardisk; 100 TB backup storage, network 1 Gb/s per node; 20 Gb/s aggregated connection from the cluster to storage. SARA uses OpenNebula open source cloud computing management toolkit and KVM virtual machines.

6. Performance results of the interactive flood simulation and visualization system

Despite the complexity of the system architecture, simulations are computed and visualized within a timeframe of less than one minute. This allows the system to be used for interactive testing of different flooding scenarios (whatif experiments). Table 1 illustrates this for two scenarios that used the same simulation parameters but the hydrograph in the second scenario generates more water influx than the first, as illustrated at the bottom of the table.

Fig. 10. Flood EWS implementation: EWS components are deployed in the Cloud as

virtualized appliances. The Common Information Space Integration Platform invokes the components and implements workflows to orchestrate data- and control-flows between them. Additional CIS services:

DyReAlla and UFoReg manage dynamic resource allocation and metadata.

The hydrograph affects the flooded area and consequently the size of the simulation output data that needs to be transmitted every time step. The table shows that the time for each simulation is well below one minute and the time between updates is within seconds, even for scenarios in which the demand for computational resources is significantly higher.

CHAPTER ONE

For the establishment and operation of the proposed FEWS to be successful, the contribution and coordination of diverse range of relevant institutions, agencies, communities and individuals are required. These stakeholders have specific functions, roles and responsibilities for which they are accountable in the successful operation of the proposed FEWS. Following a bottom-up approach, these stakeholders include: vulnerable communities resident in the eight hydrological areas of the country; Community Based Organizations; Local, State and Federal Governments of Nigeria; Government Agencies and Institutions; the Private Sector, the Academic and Scientific Community; Non Governmental Organizations; Regional Institutions and Organizations; and International Organizations.
The concept of Flood Early Warning Systems (FEWS) and its application as a flood disaster reduction strategy is at its inception stage in Nigeria. Hitherto, the key components of FEWS such as data collection and transmission, flood forecasting, warning dissemination and communication, and emergency response were carried out by different agencies and institutions which operated independently in standalone mode. There is no coordinated monitoring of floods or established early warning systems for flood disaster reduction in Nigeria. Majority of the river systems in Nigeria do not have functional water level gages while those rivers that have stage and discharge stations are not integrated into a coordinated system. The status of hydrometeorology data collection and monitoring for flood early

warning is grossly inadequate in majority of the river basins in the country.

CHAPTER TWO

The National Workshop on Flood Early Warning System (FEWS) held at Reiz Continental Hotel in Abuja between Tuesday 5th and Wednesday

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th of August 2008, cover wide ranging topics like: the causes of floods; the management of flood disasters; role of NGOs, CBOs and other stakeholders in the management of floods; socio-economic consequences of floods; capacity building for Flood Early Warning Systems.

The Workshop was divided into four (4) different Technical Sessions with a total of twelve paper presentations. The Workshop deliberated on twelve (12) papers presented by seasoned resource persons. All the twelve papers were reviewed.

CHAPTER THREE

The framework of early warning systems consists of three phases including the monitoring of precursors, forecasting of a probable event, and the notification of a warning or an alert should an event of catastrophic proportions take place. Since the incidence of floods in the country are caused by several factors including hydro-meteorological phenomena, the successful operation of an Early Warning System in Nigeria would require an Integrated Flood Management Approach, whereby all related organizations from all sectors of the country should be involved.

Flood Early Warning System (FEWS) is an integrated package of data collection and transmission equipment, forecasting models, response plans and procedures, and human resources working together with the sole objective of empowering individuals and communities threatened by flood hazard to act in sufficient time and in an appropriated manner to reduce the possibility of personal injury, loss of life and damage to property and the environment. FEWS in Nigeria has been designed to

be people-centred and integrate Knowledge of the risks faced by the society; Technical monitoring and warning services; Dissemination of meaningful warning to people at risk; and Public awareness and preparedness to act.

The design incorporates approaches employed for design and operation of Flood Early Warning Systems in the Haddington FEWS in the UK; Norwegian FEWS; and Japan's FEWS and the Flood detection and early warning systems developed in Malaysia which includes a public alert system for possible imminent flood events. The FEWS design for Nigeria focuses on monitoring the water level remotely using wired sensor network. Data is collected from the sensor using data acquisition device and channelled to the National FEWS Centre which relay the information to the public through Global System for Mobile Communication (GSM) using Short Message Service (SMS). The system ensures that the end users receive appropriate warning within suitable time interval to take effective action to save lives and minimize loss of properties.

Longer lead times and improved accuracy of flood forecasts require an effective use of the latest technology available in the fields of hydrology and meteorology as well as Information, Communication and Technology (ICT). The best design option for Nigeria's FEWS is for an integration of flood forecasting and early warning services that is people-centred in focus.

CHAPTER FOUR

The primary goal of this guideline is to work out modalities for the establishment and operation of Flood Early Warning Systems in Nigeria. To effectively achieve this goal the objectives of this guideline shall be to work out modalities to: Reduce the risks of flood disasters on vulnerable communities of Nigeria and thereby save lives and properties; For the establishment of FEWS as a non structural strategy for flood disaster reduction in Nigeria aimed at minimizing the damage

of flood water in flood prone areas of Nigeria; and Establish an effective, integrated, cost effective and sustainable Flood Early Warning System that is institutionally manageable and technologically sound and flexible.

Among the technical challenges in the establishment of an integrated and coordinated Flood Early Warning Systems in Nigeria includes: Availability of hydrological and meteorological data; Diverse topography and geology; Sediment loading problems in river systems and its effects on long term operation of the FEWS; Changes to channel geometry variability; Adaptation of the FEWS for the diverse types of flooding experienced in Nigeria; and Willingness of the Telecommunications service providers to participate in the establishment and operation of the FEWS without unreasonable financial motivations that will kill the system right from inception.

Some institutional challenges to the establishment and operation of FEWS in Nigeria include: Inadequacy in existing institutional capacities and the bureaucratic tendencies of the institutions to interoperability highly essential for timely data sharing and dissemination which is the heart of the FEWS; Inadequate flood hazard assessments capability; Effective monitoring and evaluation of the Flood Early Warning Systems; Cooperation between Government agencies; Private sector involvement; Community ownership and interest; Financial sustainability; and Economic incentives.

THANK YOU