



Modern College of Engineering

Shivajinagar, Pune 5.

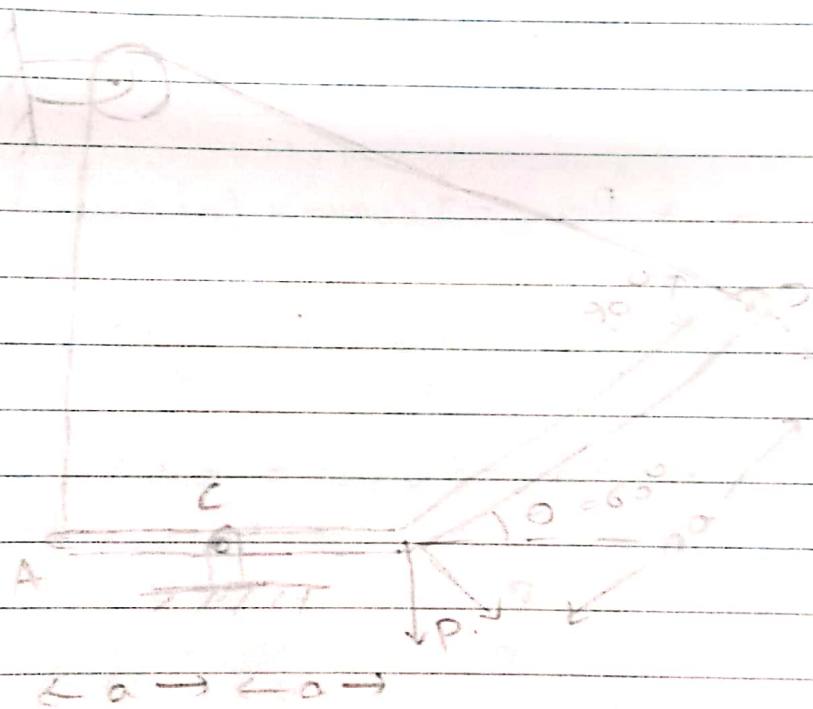
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L3

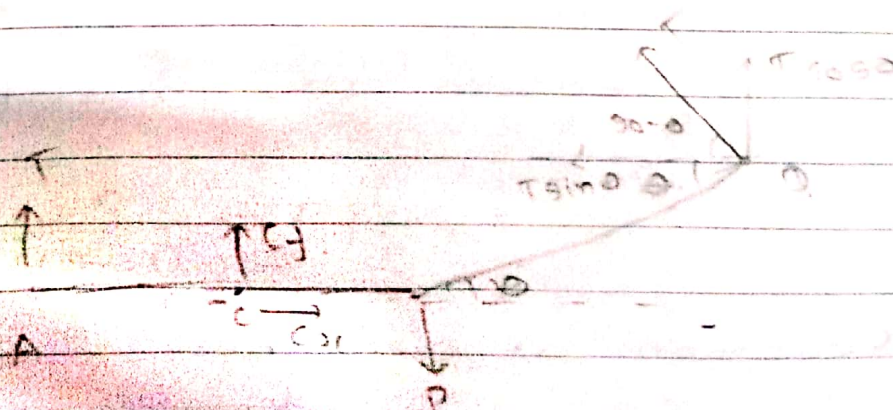
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Assignment - 3

- 1] Neglecting friction, determine the tension in cable ABD and the reaction at C when $\theta = 60^\circ$.



Soln



$$\sum M_c = 0.$$

$$T_a + P_a - T \sin 30^\circ \times 2a - T \cos 30^\circ \times 2a \sin 60^\circ = 0$$

$$T_a + P_a - T_a - T \times \frac{\sqrt{3}}{2} \times 2a \times \frac{\sqrt{3}}{2} = 0$$

$$P_a - \frac{3T_a}{2} = 0$$

$$\therefore P_a = 3T_a$$

$$T_a + P_a - T \cos \theta \times (a + 2a \cos \theta) - T \sin \theta \times 2a \sin \theta = 0$$

$$T_a + P_a - T_a \cos \theta - 2T_a \cos^2 \theta - 2T_a \sin^2 \theta = 0$$

$$T_a + P_a - T_a \cos \theta - 2T_a \times 1 = 0$$

$$P_a = T_a \cos \theta + 2T_a - T_a$$

$$\therefore P = T (\cos \theta + 2 - 1)$$

$$\therefore P = T (\cos \theta + 1)$$

$$\therefore T = \frac{P}{(\cos \theta + 1)}$$

\therefore tension in cable ABD is

$$T = \frac{P}{(\cos \theta + 1)}$$

Reaction at C, when $\theta = 60^\circ$ is

$$T = \frac{2P}{3}$$

$$\sum F_{xc} = 0$$

$$-T \sin 60^\circ + C_x = 0$$

$$\therefore C_x = T \sin 60^\circ$$

$$= T \frac{\sqrt{3}}{2}$$

$$\text{but, } T = \frac{2P}{3}$$

$$\therefore C_x = \frac{2P}{3} \times \frac{\sqrt{3}}{2}$$

$$= \frac{P}{\sqrt{3}}$$

$$\sum F_y = 0$$

$$\therefore T + \frac{T}{2} + C_y - P = 0$$

$$\frac{3T}{2} + C_y = P$$

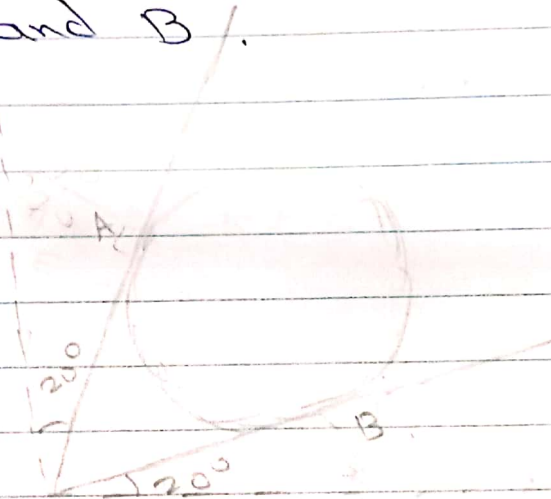
$$\frac{3 \times \frac{2P}{3}}{2} + C_y = P$$

$$\therefore C_y = P - P$$

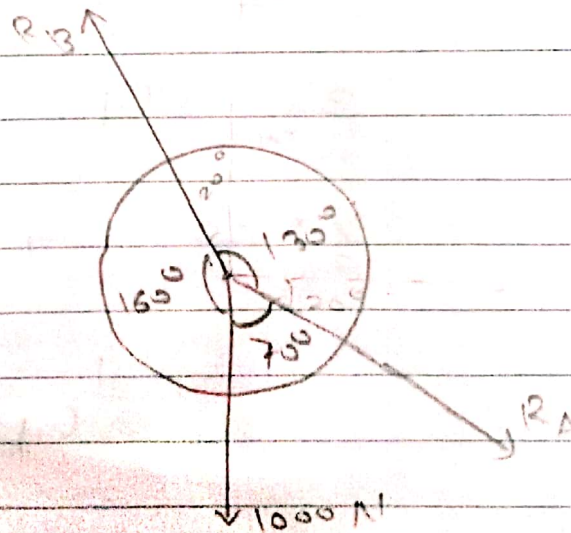
$$\therefore C_y = 0$$

\therefore Reaction at C, when $\theta = 60^\circ$
is, $\frac{P}{\sqrt{3}}$

- 2) A cylinder of mass 100 kg rest between the inclined plane as shown in fig. Determine the normal reaction at A and B.



Soln



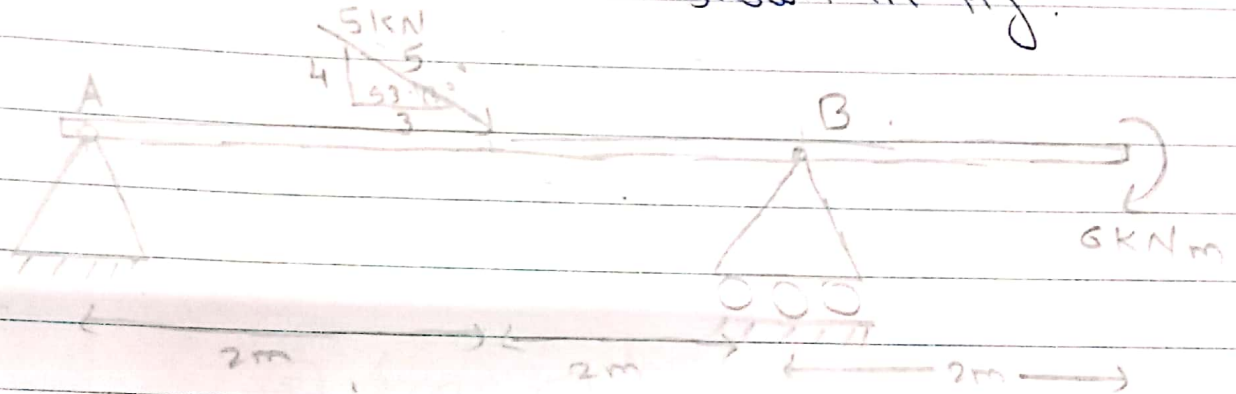
∴ By sine rule.

$$\frac{R_B}{\sin 70^\circ} = \frac{R_A}{\sin(160^\circ)} = \frac{1000}{\sin(130^\circ)} = 1305.407$$

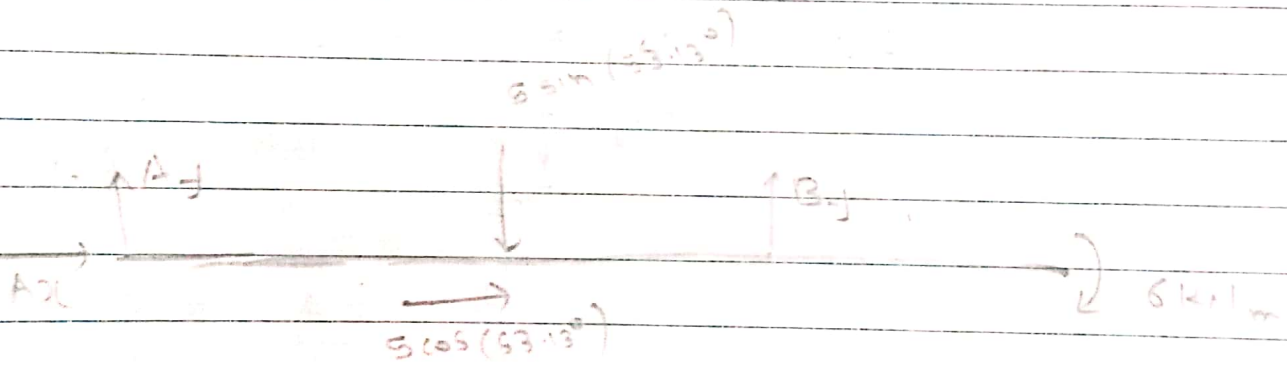
$$\therefore \underline{R_B} = \sin 20^\circ \times 1305.407, \text{ and } \underline{R_A} = \sin 160^\circ \times 1305.407$$

$$\underline{R_B} = 1226.681 \text{ N} \quad \underline{R_A} = 446.475 \text{ N}$$

3] Determine the horizontal and vertical components of reaction at the supports for the beam as shown in fig.



Soln.



$$\sum F_x = 0$$

$$\therefore A_x + 5 \cos(53.13^\circ) = 0$$

$$\therefore \underline{A_x} = -3 \text{ kN}$$

$$\sum F_y = 0$$

$$\therefore A_y - 5 \sin(53.13^\circ) + B_y = 0 \quad \text{--- (1)}$$

$$\sum M_c = 0.$$

$$\therefore A_y \cdot 6 - 5 \sin(53.13^\circ) \times 4 + 2B_y + 6 = 0$$

$$\therefore 6A_y - 16 + 6 + 2B_y = 0$$

$$\therefore 6A_y + 2B_y - 10 = 0$$

$$\therefore 3A_y + B_y - 5 = 0$$

$$\therefore B_y = 5 - 3A_y$$

-(2)

from (1) and (2)

$$A_y - 4 + 5 - 3A_y = 0.$$

$$-2A_y + 1 = 0.$$

$$\frac{1}{2} = A_y.$$

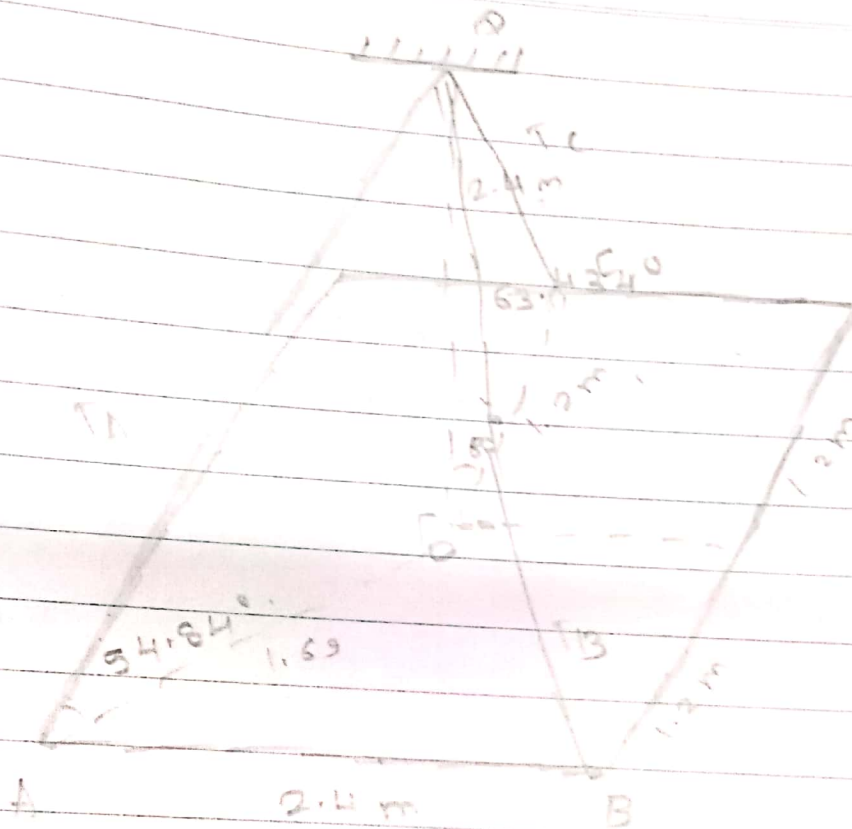
$$\therefore \underline{A_y} = 0.5 \text{ KN}.$$

$$\therefore \underline{B_y} = 5 - 3 \times 0.5$$

$$= 5 - 1.5$$

$$\therefore \underline{B_y} = 3.5 \text{ KN}$$

47]. The square steel plate has a mass of 1800 kg with mass center G as shown in Fig. Determine the tension in each cable so that the plate remains horizontal.



Solⁿ

$$T_A = T_B \quad [\text{by symmetry}]$$

$$\sum F_x = 0.$$

$$2T_A \cos(54.84^\circ) \cos(45^\circ) = T_C \cos(63.434^\circ)$$

$$\Rightarrow T_A = T_C \cdot 0.5491$$

$$\sum F_y = 0.$$

$$2T_A \sin(54.84^\circ) + T_C \sin(63.434^\circ) = 18000$$

$$\therefore 2T_C \cdot 0.5491 + T_C \cdot 0.894 = 18000$$

$$T_C \cdot 1.9922 = 18000$$

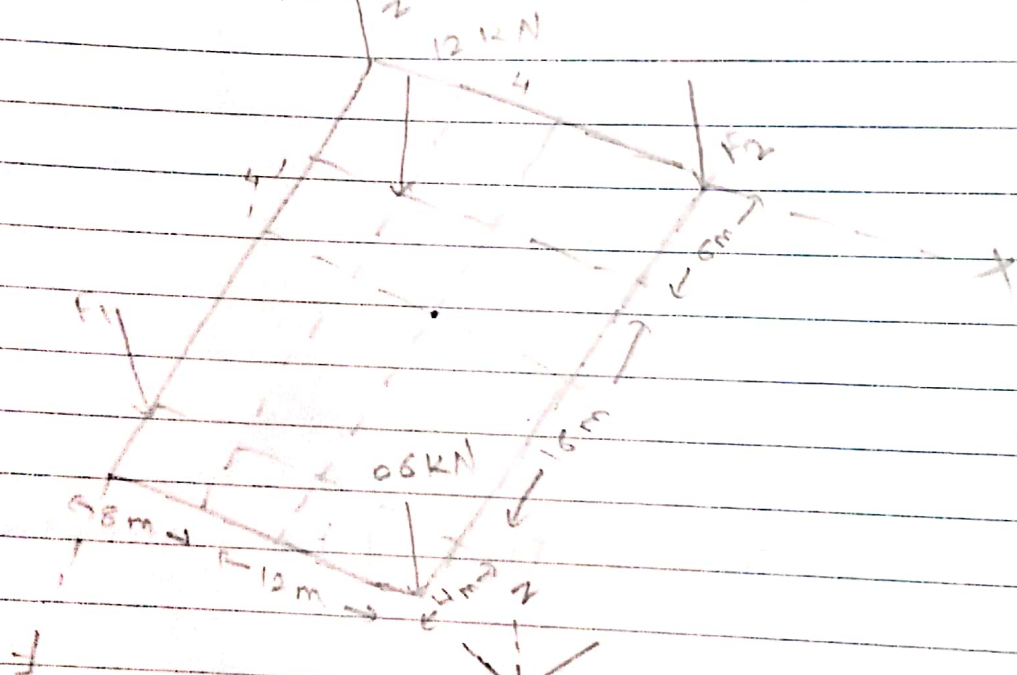
$$\therefore T_C = 9035.2374 \text{ N}$$

$$= 9.035 \text{ kN}$$

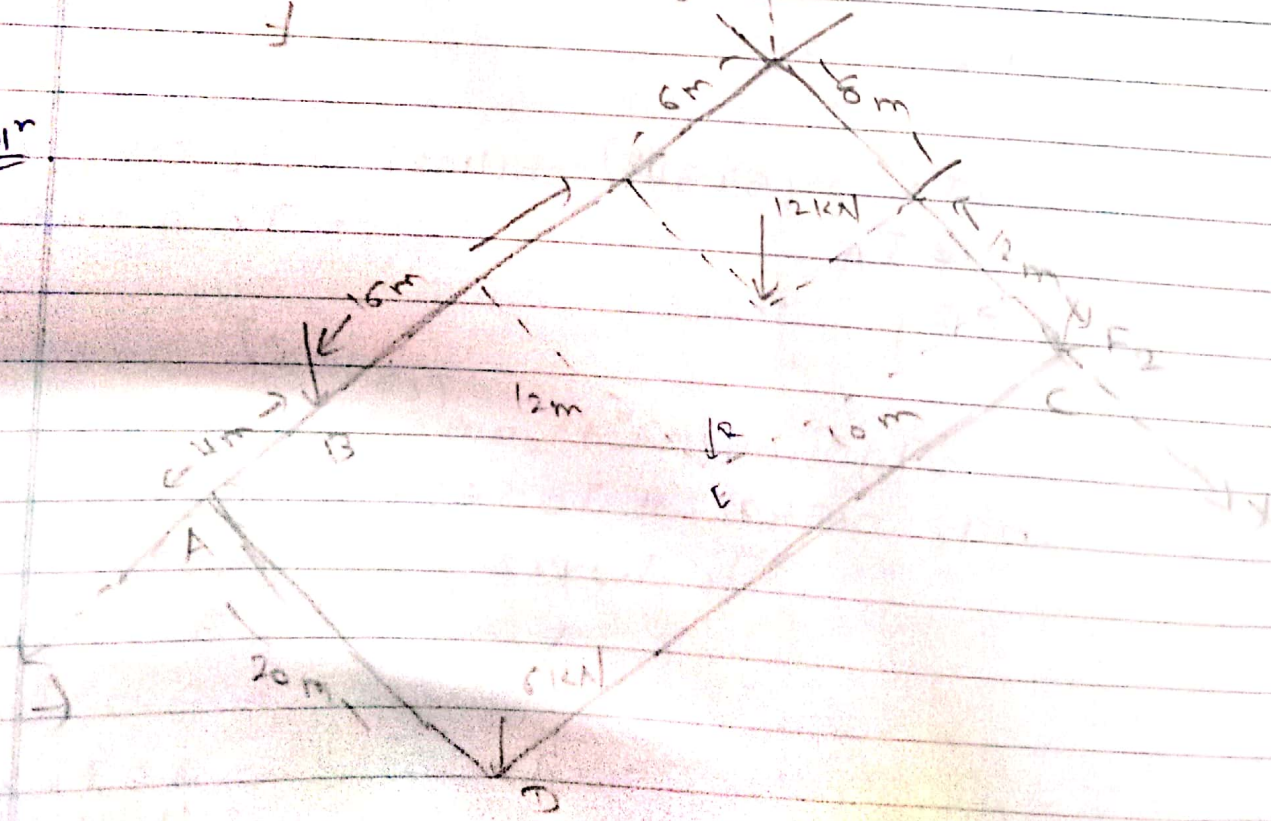
$$\therefore T_A = 9.035 \times 0.5491$$

$$= 4.9611 \text{ kN}$$

5. The building slab is subjected to four parallel column loading shown in Fig. Determine F_1 and F_2 if the result force acts through point $(12\text{m}, 10\text{m})$.



Soln



II)

$$\vec{F}_A = -12\hat{k}$$

$$\vec{F}_B = -F_1\hat{k}$$

$$\vec{F}_C = -F_2\hat{k}$$

$$\vec{F}_D = -6\hat{k}$$

III)

As R is acting at point

$$\therefore \sum M_E = 0$$

$$\begin{vmatrix} \hat{i} & \hat{j} & \hat{k} \\ -4 & -4 & 0 \\ 0 & 0 & -12 \end{vmatrix}_A + \begin{vmatrix} \hat{i} & \hat{j} & \hat{k} \\ -12 & 12 & 0 \\ 0 & 0 & -F_1 \end{vmatrix}_B +$$

$$\begin{vmatrix} \hat{i} & \hat{j} & \hat{k} \\ 8 & -10 & 0 \\ 0 & 0 & -F_2 \end{vmatrix}_C + \begin{vmatrix} \hat{i} & \hat{j} & \hat{k} \\ 8 & 16 & 0 \\ 0 & 0 & -6 \end{vmatrix}_D = 0$$

iv)

(\hat{i} - comp)

$$48 - 12F_1 + 10F_2 - 96 = 0$$

$$-12F_1 + 10F_2 = 48 \quad \text{--- (1)}$$

(\hat{j} - comp)

$$\therefore 48 + 12F_1 - 8F_2 - 48 = 0$$

$$\therefore -12F_1 - 8F_2 = 0 \quad \text{--- (2)}$$

v)

$$\begin{array}{rcl} -12F_1 + 10F_2 & = & 48 \quad \text{--- (1)} \\ -12F_1 - 8F_2 & = & 0 \quad \text{--- (2)} \\ \hline 18F_2 & = & 48 \end{array}$$



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$$\therefore F_2 = \frac{2.66}{24} \text{ kN}$$

$$\therefore F_1 = 16 \text{ kN} - \frac{8F_2}{12} = \frac{-8 \times 2.66}{12}$$

$$\therefore \begin{cases} F_1 = 16 \text{ kN} \\ F_2 = 24 \text{ kN} \end{cases} \quad \begin{matrix} 2.66 \text{ kN} \\ -1.7733 \text{ kN} \end{matrix} \quad = -1.7733 \text{ kN}$$

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