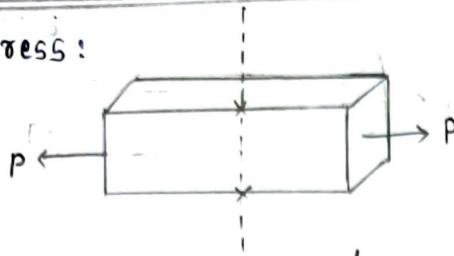


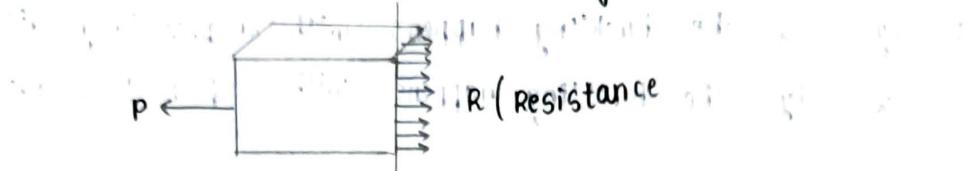
31/10/2019

# Simple stress and strains\*\*\*

## \* Stress :



Stress : The resistance/force acting per unit area



All bodies will have some amount of deformation.

## \* Definition : - Independent

The force of resistance offered by a body against deformation is called stress.

$$\text{Stress } (\sigma) = \frac{R}{A} = \frac{P}{A}$$

$A$  = cross-sectional area ;  $P$  = force or load

⇒ External force is called load.

## \* Strain : The ratio of change in length to the original length is called strain.

$$\text{Strain } (\epsilon) = \frac{\text{Change in length}}{\text{Original Length}}$$

$$\epsilon = \frac{\Delta l}{l} \text{ mm/mm}$$

⇒ No unit for strain.

⇒ There is no stress without strain. Stress exists from strain which can be measured.

## \* Types of stress :

1) Tensile stress

2) Compressive stress

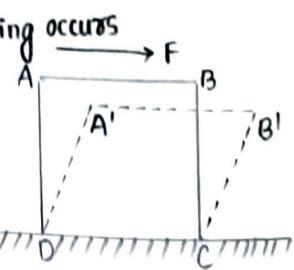
3) Shear stress or tangential stress

Because of twisting shearing occurs

Bending stress : Bending causes tensile and compression stress

Torsion stress : It causes by shearing stress

Tensile, compression and shearing are the primary stress.



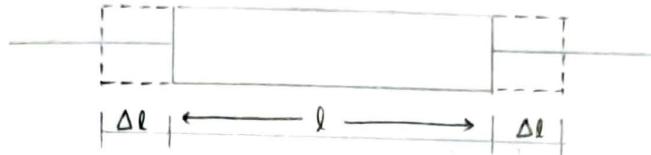
i) Tensile stress: Elongated body gets deformed.



$$\sigma = \frac{R}{A} = \frac{P}{A}$$



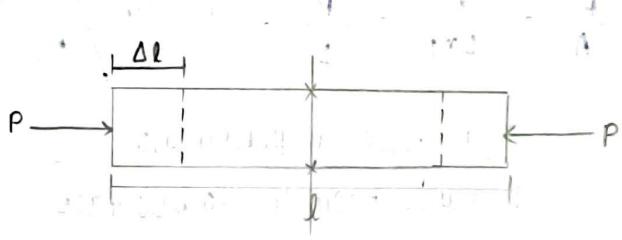
\* Tensile strain:



$$\text{Tensile strain} = \frac{\text{Increased length}}{\text{Original length}}$$

$$\epsilon = \frac{\Delta l}{l}$$

a) Compressive stress:



$$\text{compressive stress } (\sigma) = \frac{R}{A} = \frac{P}{A}$$

$$\text{compressive strain} = \frac{\text{decreased length}}{\text{original length}}$$

$$\epsilon = -\frac{\Delta l}{l}$$

Units:

Stress  $\rightarrow N/m^2$  or Pascal

Strain  $\rightarrow$  No units or  $mm/mm$

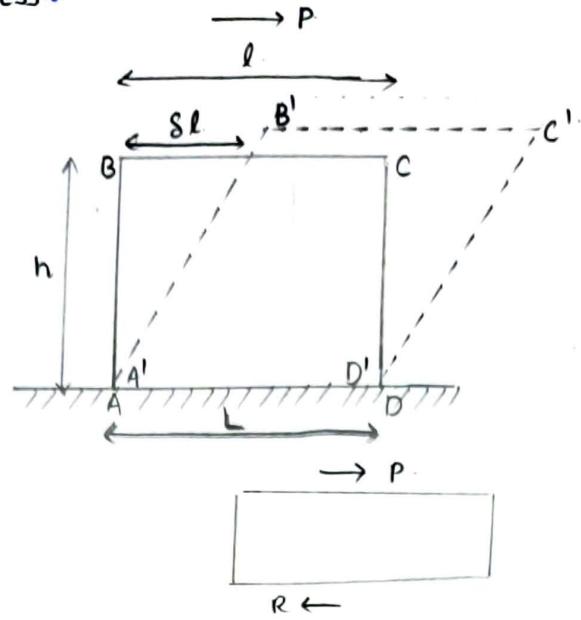
$mm/m$  or  $m/m \Rightarrow \frac{\text{Length}}{\text{Length}}$

Stress - dependent quantity

Strain - independent quantity

$$\text{Stress } \propto \frac{1}{\text{cross-section \& area}}$$

### 3) Shear stress:



$$\text{Shear stress } (\gamma) = \frac{\text{Shear resistance}}{\text{Shear area}}$$

Thou.

Shear area =  $L \times 1$  — unit width inside the surface

$$\text{Shear stress} = \frac{R}{A} = \frac{P}{L \times 1} = \frac{P}{L} = \frac{\text{N/mm}^2}{\text{m}}$$

$L \rightarrow$  in the direction of shear force.

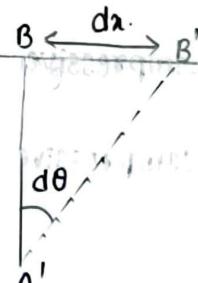
$$\text{Shear strain } (\gamma) = \frac{\text{Transverse displacement}}{\text{Distance from the lower face}}$$

$$= \frac{S l}{h}$$

$$\gamma = \frac{dx}{x}$$

$$\tan \phi / \tan \theta = \frac{dx}{x}$$

$$\theta = \frac{dx}{x} = \gamma$$



### \* Hooke's law:

P - point of proportionality

e - point of yielding

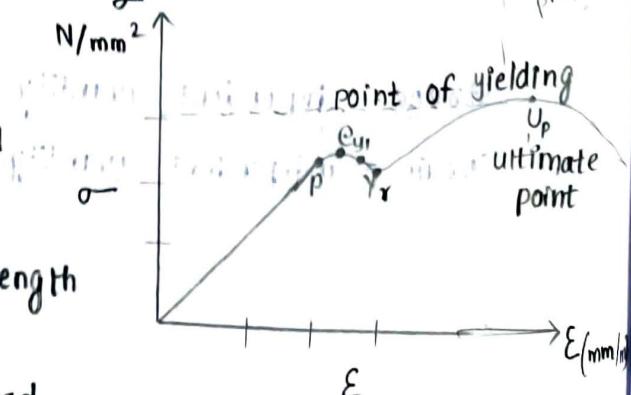
Hooke's law is not obeyed

$y_1$  - upper yielding point

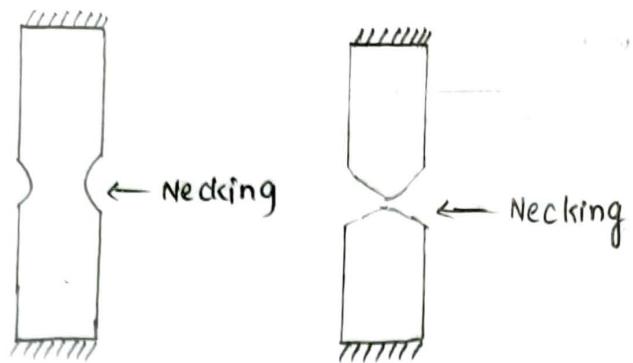
we can't get back original length

$y_2$  - lower yielding point

material can't take any load



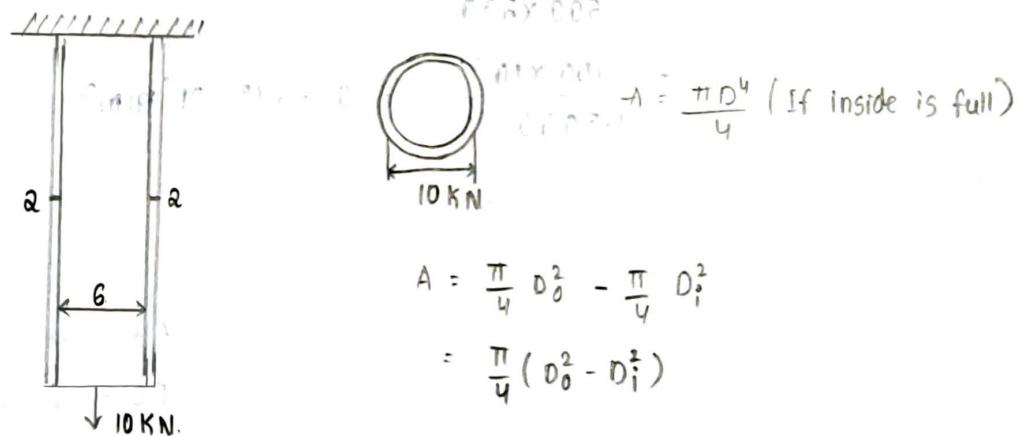
Beyond up continue the elongation the material would break



Break point (where upper part and lower part are separated)

- 04/11/2019  
1) A pipe of 10 mm outer diameter and 6 mm thickness subjected to axial-tension of 10 kN. Find the stress induced in the pipe

2)



$$\text{Stress} = \frac{\text{Load}}{\text{Area}}$$

$$= \frac{10 \times 10^3 \text{ N}}{\frac{\pi}{4} (10^2 - 6^2)} = \frac{10^4}{\frac{\pi}{4} (100 - 36)}$$

$$= \frac{10^4}{\frac{\pi}{4} \times 64}$$

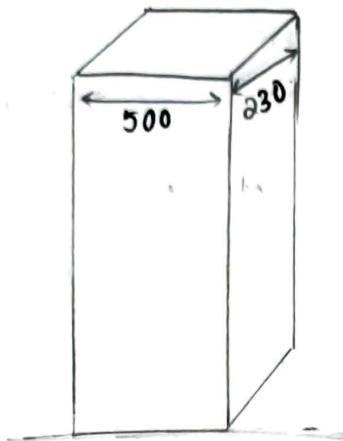
$$= \frac{10^4}{16\pi}$$

$$= \frac{10000}{16 \times 3.14}$$

$$= \frac{10000}{50.24}$$

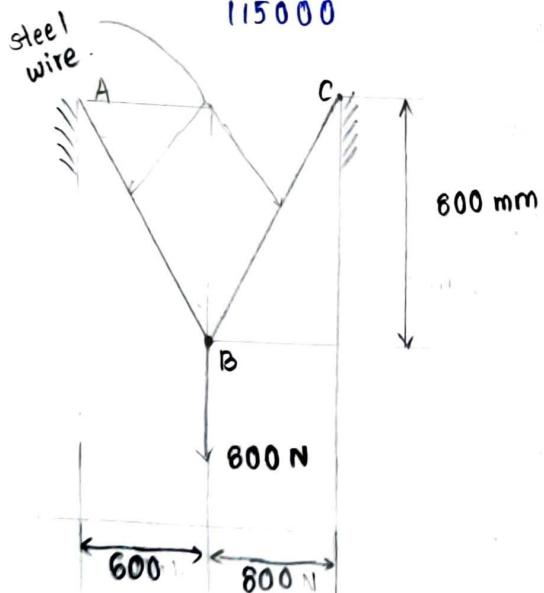
$$= 199.04 \text{ N/mm}^2$$

- 2) A column of  $230 \times 500$  mm, cross-section is subjected to a compression force of 100 KN. Find the compressive stress induced in the column.



$$\begin{aligned} \text{Stress} &= \frac{\text{Load}}{\text{Area}} \\ &= \frac{100 \text{ kN}}{500 \times 230} \\ &= \frac{100 \times 10^3}{115000} = 0.8695 \text{ N/mm}^2 \end{aligned}$$

3)



$$AB = 2.68 \text{ mm } \phi$$

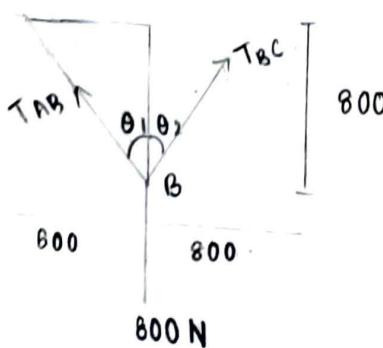
$$BC = 2.52 \text{ mm } \phi$$

Find force and stress.

$$\tan \theta_2 = \frac{800}{800}$$

$$\tan \theta_2 = 1$$

$$\theta_2 = 45^\circ$$



$$\tan \theta_1 = \frac{600}{800}$$

$$\tan \theta_1 = \frac{3}{4}$$

$$\theta_1 = \tan^{-1}\left(\frac{3}{4}\right)$$

$$\theta_1 = 36.86^\circ$$

$$\sum F_x = 0 \Rightarrow -T_A \sin 36.86^\circ + T_c \sin 45^\circ = 0$$

$$-T_A (0.5998) + T_c (0.7071) = 0$$

$$T_A (0.5998) = T_C (0.7071)$$

$$T_A = T_C \frac{(0.7071)}{(0.5998)}$$

$$T_A = 1.1788 T_C$$

$$\sum F_y = 0 \Rightarrow T_A \cos 36.86 - 800 + T_C \cos 45^\circ = 0$$

$$(0.8001) T_C \times 0.8001 - 800 + T_C (0.7071) = 0$$

$$(1.1788) T_C \times 0.8001 + T_C (0.7071) = 800$$

$$T_C (0.9431 + 0.7071) = 800$$

$$T_C = \frac{800}{1.6502}$$

$$T_C = 484.789 \text{ N}$$

$$\therefore T_A = 1.1788 \times 484.789$$

$$= 1178.260 \times 484.789 \text{ N}$$

~~stress at A~~ = ~~Load at A~~  
~~Area~~

$$\text{Area} = \frac{\pi}{4} (D)^2$$

$$= \frac{571.469}{\pi (0.68)^2}$$

$$= \frac{571.469}{5.638} = 101.36 \text{ N/mm}^2$$

~~stress at B~~ = ~~Load at B~~  
~~Area~~

$$= \frac{484.789}{\pi (0.52)^2}$$

$$= \frac{484.789}{4.985} = 97.249 \text{ N/mm}^2$$

5/11/2019 Stress = Young's Modulus  $\rightarrow$  D  $\rightarrow$  Rubber band Increase in length only

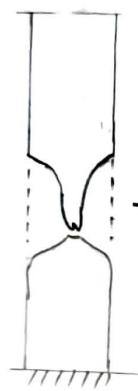
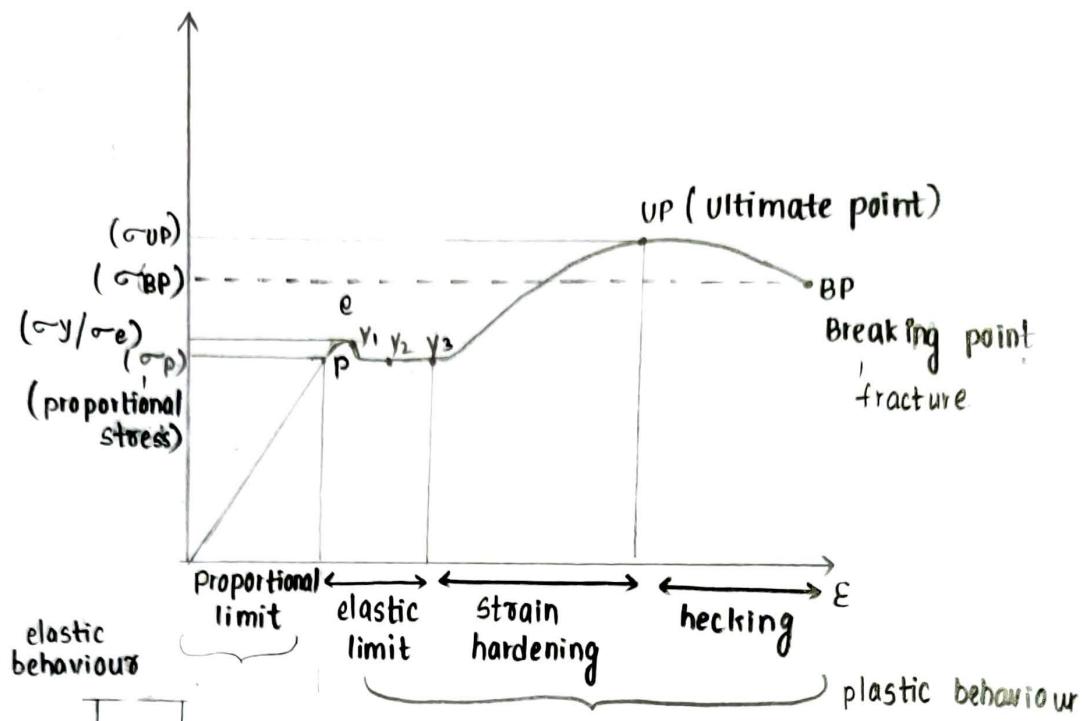
Strain = Modulus  $\rightarrow$  D  $\rightarrow$  Thick Rubber band increase in length and decrease in thickness

$$\boxed{\frac{\sigma}{E} = \epsilon}$$

$$\boxed{\frac{\sigma}{\epsilon} = E}$$

3D  $\rightarrow$  Sponge  $\rightarrow$  increase in x direction  
decrease in y, z direction

Poisson's ratio 1 direction increase, other 2 decreases or Vice versa.



### ★ Materials:

i) Ductile material:

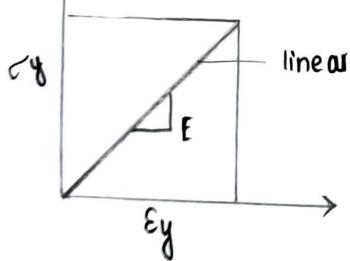
— cup cone  
ductile material

will have reducing cross-section

ii) Fragile material:

will not have reducing cross-section.  
Perfectly horizontal.

\* Hooke's law: Strain is directly proportional to the stress up to the elastic limit.



$$\sigma \propto E$$

$$\sigma = E \epsilon$$

$$E = \frac{\sigma}{\epsilon}$$

⇒ E - Young's modulus / modulus of elasticity

⇒ It is the material property

⇒ Units: N/mm²

E = 210 Gpa for steel

Gpa = Giga pascal

E = 70 Gpa for aluminium

=  $10^9 \text{ N/m}^2$

E = 0.1 Gpa for wood

Mpa = Mega pascal

E = 20-40 Gpa for concrete

=  $10^6 \text{ N/m}^2$

1 Mpa = 1 N/mm²

$$\frac{\sigma}{E} = \epsilon$$

$$E = \frac{\sigma}{\epsilon}$$

$$\frac{\delta l}{l} = \frac{P}{AE}$$

$$\delta l = \frac{Pl}{AE}$$

$E$  = Young's modulus

$$\delta l = \text{Elongation}$$

- Q) A rod of diameter 5 mm is subjected to a tensile force of 200 KN. Assuming  $E$  (Young's modulus) = 200 Gpa, find the stress in the material, elongation, strain. length of rod is 1m

$$d = 5 \text{ mm} ; F = 200 \text{ KN} ; E = 200 \text{ Gpa} ; l = 1 \text{ m}$$

$$A = \frac{\pi}{4} (D)^2 = 200 \times 10^3 \text{ Mpa} = 10^3 \text{ mm}^2$$

$$(or)$$

$$= 200 \times 10^3 \text{ N/mm}^2$$

$$\text{Stress } (\sigma) = \frac{P}{A}$$

$$= \frac{200 \times 10^3}{\frac{\pi}{4} \times (5)^2} = \frac{200000}{\frac{\pi}{4} \times 25}$$

$$= \frac{200000}{19.625} = 10191.08 \text{ N/mm}^2$$

$$\cong 10191 \text{ N/mm}^2$$

$$\text{Elongation } (\delta l) = \frac{Pl}{AE}$$

$$= \frac{10191 \times 1000}{200 \times 10^3}$$

$$= 50.955 \text{ mm}$$

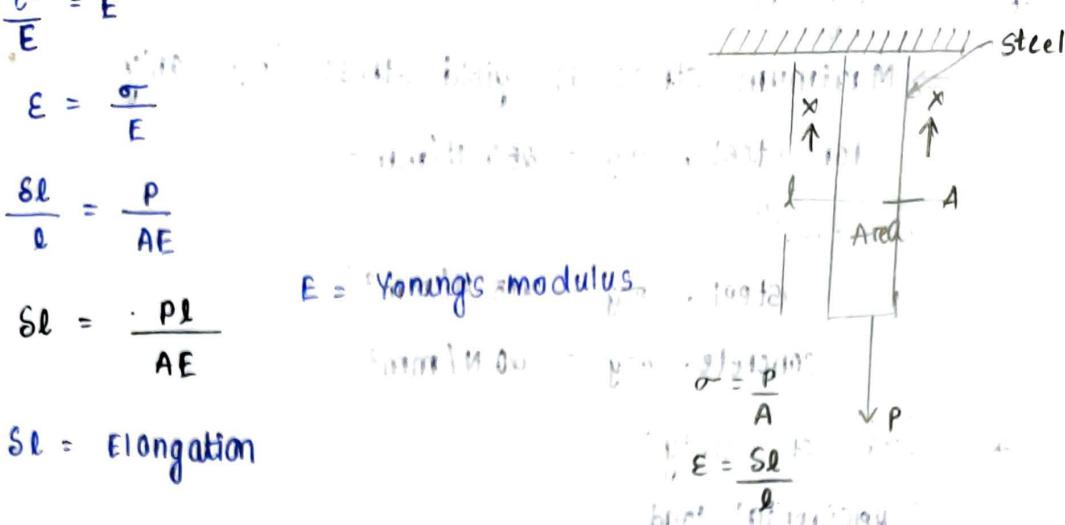
$$\text{Strain } (\epsilon) = \frac{\delta l}{l}$$

$$= \frac{50.955 \text{ mm}}{1 \text{ m}}$$

$$= \frac{50.955 \text{ mm}}{1000 \text{ mm}}$$

$$= 0.050955 \text{ mm/mm}$$

$$\cong 0.05 \text{ mm/mm}$$



## \* Working stress

⇒ Maximum stress is yield stress ( $\sigma_y$ ) only.

for steel,  $\sigma_y = 250 \text{ N/mm}^2$

↳ structural steel (Max. stress)

↳ Torque steel (Max. stress)

steel,  $\sigma_y = 415 \text{ N/mm}^2$

concrete,  $\sigma_y = 80 \text{ N/mm}^2$

## \* Factor of safety:

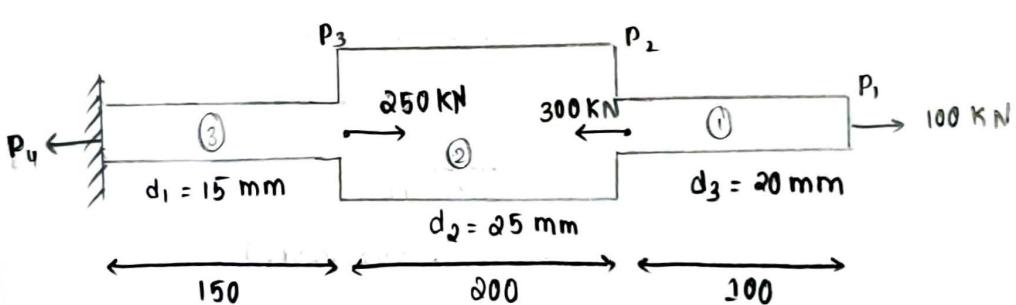
i) Incidental load

ii) Material quality

$$\text{Working stress} = \frac{\sigma_y}{\text{FOS}} \text{ N/mm}^2 - \text{Factor of safety}$$

FOS → 1.0 to 2.0

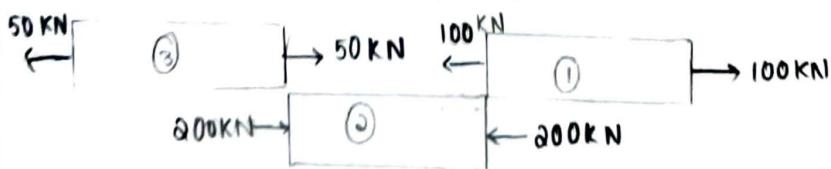
$$W.S = \frac{250}{2} = 125 \text{ N/mm}^2$$



Determine the stresses in various segments of circular part.  
Compute total elongation assuming Young's modulus,  $E = 195 \text{ GPa}$

$$\sum F_x = 0 \Rightarrow -P_4 + 250 - 300 + 100 = 0$$

$$P_4 = 50 \text{ kN}$$



$$\text{Stress at } ①, = \frac{\text{Load } (P_1)}{\text{Area } (A_1)}$$

$$= \frac{100 \text{ kN}}{\frac{\pi}{4} \times (20)^2} = \frac{100 \times 10^3}{314}$$

$$= 318.47 \text{ N/mm}^2$$

$$\begin{aligned}
 \text{Stress at } ② &= \frac{P_2}{A_2} \\
 &= \frac{200 \text{ kN}}{\frac{\pi}{4} \times (25)^2} \\
 &= \frac{200 \times 10^3}{490.625} \\
 &= 407.643 \text{ N/mm}^2
 \end{aligned}$$

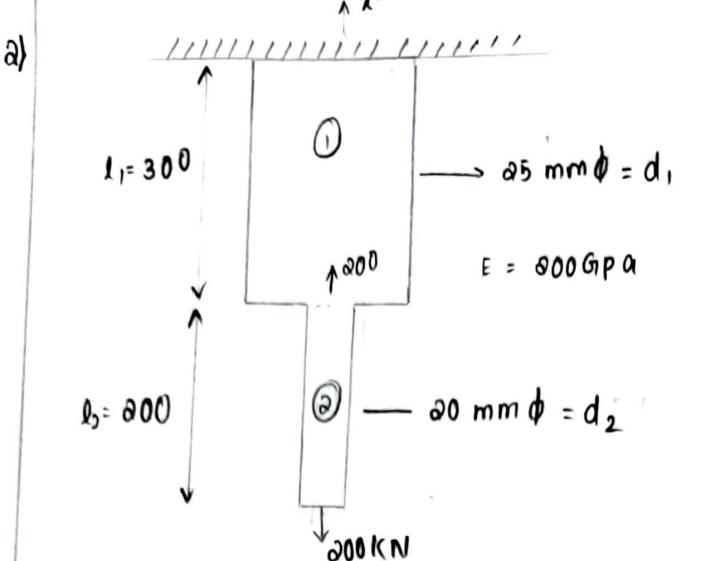
$$\begin{aligned}
 \text{Stress at } ③ &= \frac{P_3}{A_3} \\
 &= \frac{50 \text{ kN}}{\frac{\pi}{4} \times (15)^2} \\
 &= \frac{50 \times 10^3}{176.625} \\
 &= 283.085 \text{ N/mm}^2
 \end{aligned}$$

$$\begin{aligned}
 \text{at } ① \Rightarrow \delta l_1 &= \frac{P_1 l_1}{A_1 E} \quad [l_1 = 100 \text{ mm}] \\
 &= 318.47 \times \frac{100}{195 \times 10^3} \\
 &= \frac{31847}{195000} \\
 &= 0.1633 \text{ mm (elongation)}
 \end{aligned}$$

$$\begin{aligned}
 \text{at } ② \Rightarrow \delta l_2 &= \frac{P_2 l_2}{A_2 E} \\
 &= 407.643 \times \frac{200}{195 \times 10^3} \\
 &= \frac{81528.6}{195000} \\
 &= -0.4180 \text{ mm (compression)}
 \end{aligned}$$

$$\begin{aligned}
 \text{at } ③ \Rightarrow \delta l_3 &= \frac{P_3 l_3}{A_3 E} \\
 &= 283.085 \times \frac{150}{195 \times 10^3} \\
 &= \frac{42462.75}{195000} = 0.2177 \text{ mm (elongation)}
 \end{aligned}$$

$$\begin{aligned}
 \text{Total elongation} &= \delta_1 + \delta_2 + \delta_3 \\
 &= 0.1633 - 0.4180 + 0.2177 \\
 &= -0.037 \text{ mm}
 \end{aligned}$$



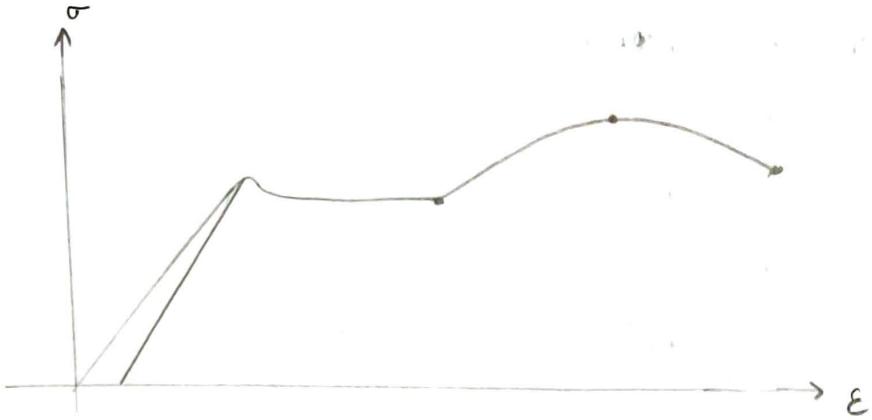
$$\begin{aligned}
 \text{Stress at ①} &= \frac{\text{Load } (P_1)}{\text{Area } (A_1)} \\
 &= \frac{200 \text{ kN}}{\frac{\pi}{4} \times (25)^2} \\
 &= \frac{200 \times 10^3}{490.625} \\
 &= 407.643 \text{ N/mm}^2
 \end{aligned}$$

$$\begin{aligned}
 \text{Stress at ②} &= \frac{\text{Load } (P_2)}{\text{Area } (A_2)} \\
 &= \frac{200 \text{ kN}}{\frac{\pi}{4} \times (20)^2} \\
 &= \frac{200 \times 10^3}{314} = 636.942 \text{ N/mm}^2
 \end{aligned}$$

$$\begin{aligned}
 \text{At ①} \Rightarrow \delta l_1 &= \frac{P_1 l_1}{A_1 E} \\
 &= \frac{407.643 \times 300}{200 \times 10^3} \\
 &= \frac{1222.929}{200000} = 0.6114645 \\
 &\approx -0.611 \text{ mm (compression)}
 \end{aligned}$$

$$\begin{aligned}
 \text{At ②} \Rightarrow \delta l_2 &= \frac{P_2 l_2}{A_2 E} \\
 &= \frac{636.942 \times 200}{200 \times 10^3} = 0.636942 \\
 &\approx 0.636 \text{ mm}
 \end{aligned}$$

$$\begin{aligned}
 \text{Total elongation} &= \delta l_1 + \delta l_2 \\
 &= -0.611 + 0.636 = 0.025 \text{ mm}
 \end{aligned}$$

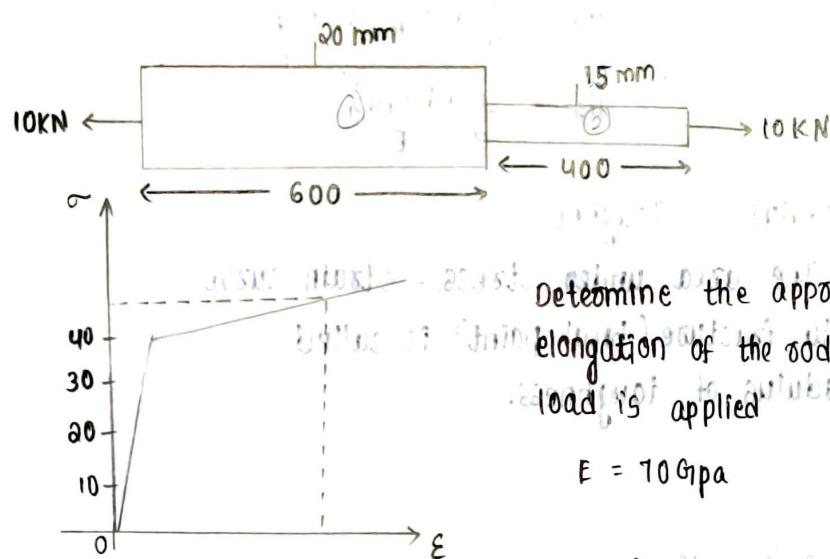


$0.2\% = \epsilon = \text{proof strain}$

$$0.002 = 0.2\% = \epsilon$$

$\Rightarrow$  A strain equal to  $0.2\%$  is called proof strain at which material gets yielding. Then draw a parallel line to the proportional limit, which cuts  $\sigma$  because of elongation.

$\Rightarrow$  At a position it cuts  $\sigma$ -stress and  $\epsilon$ -strain curve, the corresponding reading on  $\sigma$ -axis is called yield stress.



$$\Delta l_1 + \Delta l_2 = \frac{P_1 l_1}{A_1 E_1} + \frac{P_2 l_2}{A_2 E_2}$$

$$= \frac{P}{E} \left[ \frac{l_1}{A_1} + \frac{l_2}{A_2} \right]$$

$$\epsilon = \frac{\Delta l}{l} \Rightarrow \Delta l_1 = \epsilon l_1$$

$$= 400 \times 0.0450$$

$$= 18 \text{ mm}$$

$$\Delta l_2 = \frac{P_2 l_2}{A_2 E} = \frac{31.8 \times 600}{70 \times 10^3}$$

$$= 0.072 \text{ mm}$$

$$\text{Stress } \sigma_1 = \frac{\text{Load } P_1}{\text{Area } A_1}$$

$$= \frac{10 \times 10^3}{\frac{\pi}{4} \times (20)^2}$$

$$= \frac{10000}{314}$$

$$= 31.8 \text{ N/mm}^2$$

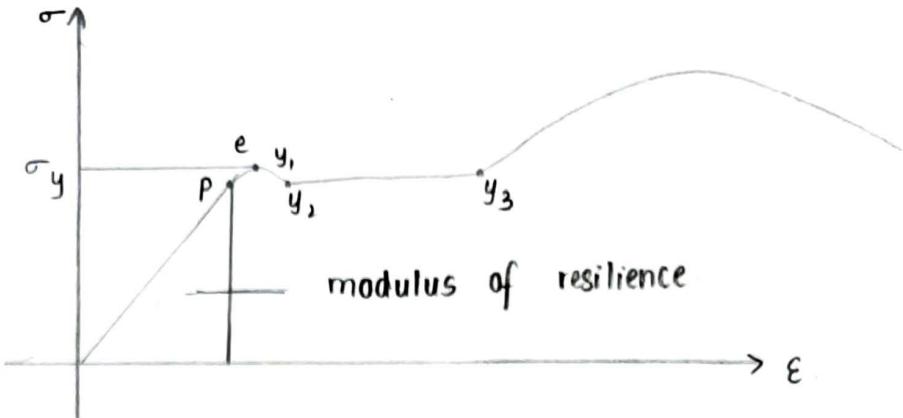
$$\sigma_2 = \frac{P_2}{A_2}$$

$$= \frac{10 \times 10^3}{\frac{\pi}{4} \times (15)^2}$$

$$= \frac{10^4}{176}$$

$$= 56 \text{ N/mm}^2$$

## \* Modulus of resilience:



⇒ The area under stress-strain diagram upto proportional limit is called modulus of resilience.

$$U_R = \frac{1}{2} \sigma_{PL} \times \epsilon_{PL}$$

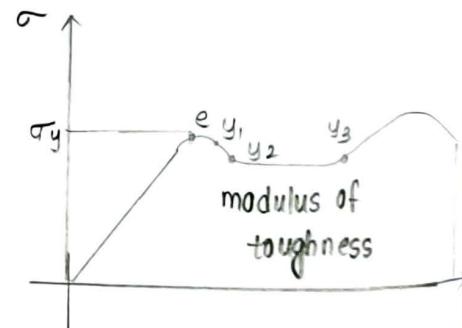
⇒ This property represents the ability of the material to absorb energy without any permanent damage to the material.

$$U_R = \frac{1}{2} \times \sigma_{PL} \times \frac{\sigma_{PL}}{E}$$

$$U_R = \frac{1}{2} \frac{(\sigma_{PL})^2}{E}$$

## \* Modulus of Toughness

⇒ The area under stress-strain curve upto fracture (break point) is called modulus of toughness.



## \* Poisson's ratio :

$$\nu = \frac{\text{Lateral strain}}{\text{Longitudinal strain}}$$

(new)

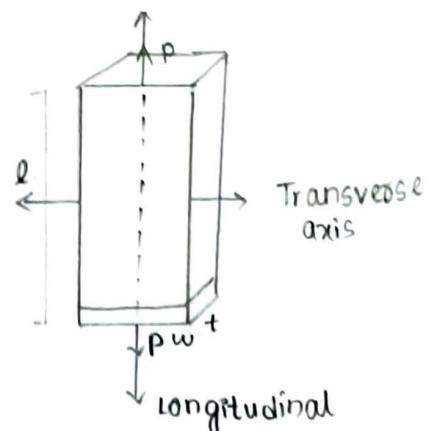
$$= \frac{s_w/w}{s_l/l}$$

$$= +\frac{E_t}{E_l} \quad (\text{compressive force})$$

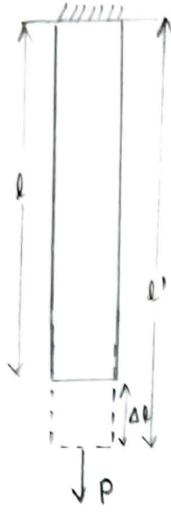
- indicates the dimension is getting reduced

$$= -\frac{E_t}{E_l} \quad (+\text{tensile force})$$

It is a symmetric property.



For Rubber,  $\nu = 0.5$   
 Steel,  $\nu = 0.3$   
 Al,  $\nu = 0.22$   
 concrete,  $\nu = 0.15$



changed in length of the bar,

$$l' = l + \Delta l$$

$$= \frac{l}{l} [l + \Delta l]$$

$$= l \left[ 1 + \frac{\Delta l}{l} \right]$$

$$= l(1 + \varepsilon)$$

$$l' = l(1 + \varepsilon)$$

changed length =  $l(1 + \varepsilon)$   $\varepsilon_t$  - transverse strain

change in diameter  $\Delta d = \varepsilon_t d = \nu \varepsilon d$

changed in diameter of the bar is,

$$\varepsilon_t = \frac{\Delta d}{d}$$

$$d' = d - \Delta d$$

$$= d - \nu \varepsilon d$$

$$= d(1 - \nu \varepsilon)$$

The volume of bar after loading is,

$$\begin{aligned} \text{Changed in volume, } V' &= \frac{\pi}{4} d'^2 \times l' \\ &= \frac{\pi}{4} [d^2(1 - \nu \varepsilon)^2] \times l(1 + \varepsilon) \\ &= \frac{\pi}{4} d^2 l (1 + \nu^2 \varepsilon^2 - 2\nu \varepsilon)(1 + \varepsilon) \\ &= \frac{\pi}{4} d^2 l (1 + \cancel{\nu^2 \varepsilon^3} - 2\nu \varepsilon + \varepsilon + \cancel{\nu^2 \varepsilon^3} - \cancel{2\nu \varepsilon^2}) \\ &= \frac{\pi}{4} d^2 l (1 - 2\nu \varepsilon + \varepsilon) \end{aligned}$$

$$\text{Change in volume } \Delta V = V' - V$$

$$= \frac{\pi}{4} d^2 l (1 - 2\nu \varepsilon + \varepsilon) - \frac{\pi}{4} d^2 l$$

$$= \frac{\pi}{4} d^2 l [1 - 2\nu \varepsilon + \varepsilon - 1]$$

$$= \frac{\pi}{4} d^2 l \varepsilon [1 - 2\nu]$$

$$\text{Volumetric strain, } \varepsilon_v = \frac{\Delta V}{V} = \varepsilon(1 - 2\nu)$$

i) Determine the change in the volume of steel bar of 25 mm diameter and 500 mm long when subjected to a tensile stress  $\sigma_t = 200 \text{ MPa}$ . The young's modulus of steel  $E_s = 200 \text{ GPa}$ . The poisson's ratio of steel  $\nu = 0.3$

$$d = 25 \text{ mm} \quad \sigma_t = 200 \text{ MPa} \quad \nu = 0.3$$

$$L = 500 \text{ mm} \quad E_s = 200 \text{ GPa}$$

$$= 200 \times 10^3$$

$$\text{Axial stress} = 200 \text{ N/mm}^2$$

$$\frac{\sigma_t}{E} = \nu$$

$$E = \frac{\sigma_t}{\nu} = \frac{200}{0.3} = 200 \times 10^3 = 10^{-3}$$

$$\Delta V = V \cdot \epsilon (1 - \nu)$$

$$= \frac{\pi}{4} \times 25^2 \times 500 \times 10^{-3} (1 - 2 \times 0.3) \times 10^{-3}$$

$$= \frac{3.14}{4} \times 625 \times 500 \times 10^{-3} (1 - 0.6) \times 10^{-3}$$

$$= 0.785 \times 312.5 \times 0.4 \times 10^{-3}$$

$$= 98.125 \text{ mm}^3$$

*8/11/2019*  
Volumetric strain,  $\frac{dv}{v} = \epsilon_v$

$\epsilon = \epsilon_t = \text{axial strain}$

$\epsilon_t = \text{transverse strain}$

$\epsilon_v = \text{volumetric strain}$

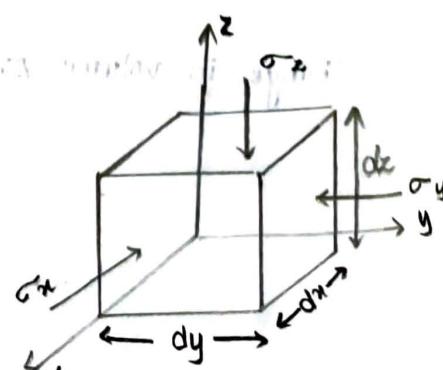
$$\frac{\epsilon_t}{E} = \nu$$

$$\epsilon_v = \epsilon (1 - 2\nu)$$

#### \* Hydrostatic pressure

$$\sigma_x = \sigma_y = \sigma_z = \sigma = \rho P$$

After the pressure on, the cube  
Changed dimension =  $(1 + \epsilon_x) dx$ ,  
 $(1 + \epsilon_y) dy$ ,  $(1 + \epsilon_z) dz$



Change of volume ( $\Delta V$ ) =  $(1 + \epsilon_x) dx \cdot (1 + \epsilon_y) dy \cdot (1 + \epsilon_z) dz$

$$\Delta V = V' - V = (\epsilon_x + \epsilon_y + \epsilon_z) dx dy dz$$
$$\Delta V = (\epsilon_x + \epsilon_y + \epsilon_z) V$$

$$\frac{\Delta V}{V} = \epsilon_v = \underbrace{\epsilon_x + \epsilon_y + \epsilon_z}_{\text{force/pressure in all directions}} / \frac{\Delta V}{V} = \epsilon (1 - \alpha v)$$

force/pressure in all directions

valid for when body is subjected in one direction

### \* Bulk Modulus ( $K$ ):

It is defined as the ratio between volumetric stress by volumetric strain upto proportional limit only.

$$K = \frac{\text{Volumetric stress}}{\text{volumetric strain}} = r$$

When the body is subjected to uniform stress from all directions it exists upto proportional limit. then,

$$K = \frac{\sigma_x + \sigma_y + \sigma_z}{dV/V}$$

$$\text{volumetric stress} = \sigma_x + \sigma_y + \sigma_z = 3\sigma$$

$$\text{volumetric strain} = \frac{dV}{V} = \epsilon (1 - \alpha v)$$

$$\frac{dV}{V} = \frac{\sigma_x + \sigma_y + \sigma_z}{E} (1 - \alpha v)$$

$$\epsilon_v = \frac{3\sigma}{E} (1 - \alpha v)$$

$$\frac{E}{3(1 - \alpha v)} = \frac{\sigma}{\epsilon_v}$$

$$K = \frac{E}{3(1 - \alpha v)}$$

$(K, E, \alpha v)$   
↓ material property

⇒ The Bulk Modulus is caused only by normal strain not by shear strain.

⇒ It is a measure of stiffness of a volume of a material.

⇒ This material property provides an upper limit to Poisson's ratio of  $\nu = 0.5$

\* Shear Modulus or Rigidity Modulus

The constant of the proportionality of shear stress and shear strain is known as rigidity modulus of the material.

→ It is denoted by  $G_1$ .

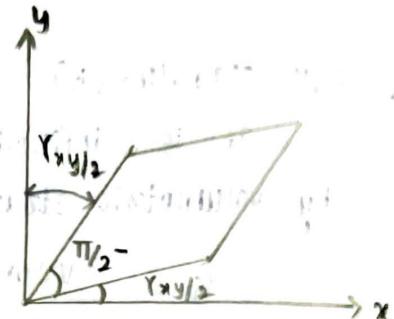
$$G_1 = \frac{T_{pc}}{\gamma_{pc}} = \frac{E}{\alpha(1+\nu)}$$

⇒ This rigidity modulus changes its shape but not its volume

$$E = \frac{\alpha G K}{G_1 + 3K}$$

$$\nu = \frac{E}{2G}$$

$$\nu = \frac{1 - E/3K}{2}$$



- i) An aluminium specimen has diameter of 25 mm and gauge length of 250 mm. If a force of 165 kN elongates the gauge length 1.20 mm, determine the modulus of elasticity ( $E$ ) and also determine by how much the force causes the diameter of specimen to contract. Taking  $G_1 = 26 \text{ GPa}$  (rigidity modulus) and yield stress  $\sigma_y = 440 \text{ MPa}$ .

$$d = 25 \text{ mm}$$

$$\Delta L = 1.20 \text{ mm}$$

$$\sigma_y = 440 \text{ MPa}$$

$$l_i = 250 \text{ mm}$$

$$G_1 = 26 \text{ GPa}$$

$$E = ?$$

$$P = 165 \text{ kN}$$

$$= 165 \times 10^3 \text{ MPa}$$

$$\text{Stress} = \sigma = \frac{P}{A} \quad \text{if } \sigma < \sigma_y \text{ Proportional limit}$$

$$= \frac{165 \times 10^3}{\frac{\pi}{4} (25)^2}$$

$$= \frac{165000}{0.785 \times 625} = \frac{165000}{490.625}$$

$$= 336.1 \text{ N/mm}^2$$

$$\text{strain} = \epsilon = \frac{\Delta l}{l}$$

$$\epsilon = \frac{1.20}{250} = 4.8 \times 10^{-3} \text{ mm/mm}$$

$$\epsilon = 0.0048 \text{ mm/mm}$$

$$E = \frac{\sigma}{\epsilon} = \frac{336.1}{0.0048}$$

$$= 70020.8 \text{ N/mm}^2$$

$$= 70.0208 \text{ Gpa}$$

$$= 70.02 \text{ Gpa}$$

$$G = \frac{E}{2(1+\nu)}$$

$$\nu = \frac{E}{2G} - 1$$

$$= \frac{70.02}{2 \times 26} - 1$$

$$= \frac{70.02}{52} - 1 = 1.34615 - 1$$

$$= 0.34615$$

$$= 0.347$$

$$\nu = \frac{E_t}{E}$$

$$E_t = 0.347 \times 10 \times 25$$

$$= 0.347 \times 10 \times 10^3$$

$$= 1.6656 \times 10^3$$

$$= 1.6656 \text{ mm/mm.}$$

$$\frac{\delta d}{d} = E_t$$

$$\delta d = E_t \times d$$

$$= 0.00166 \times 25$$

$$= 0.0415 \text{ mm}$$

$\sigma - \epsilon$  relation - different points are identified.

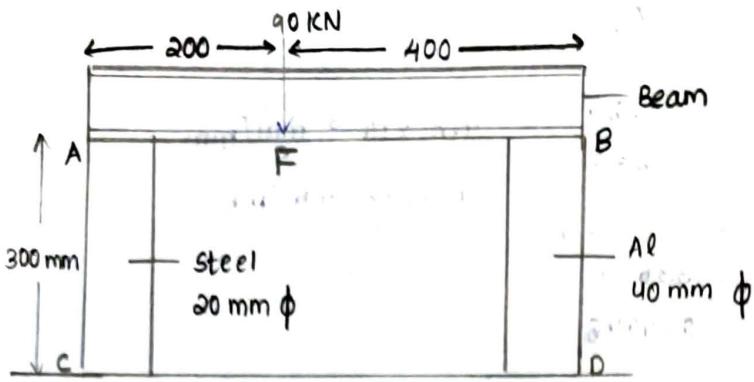
$$\sigma \propto P/A \Rightarrow \sigma$$

$$\epsilon \propto \Delta l/l \Rightarrow \epsilon$$

Elongation

Poisson's ratio.

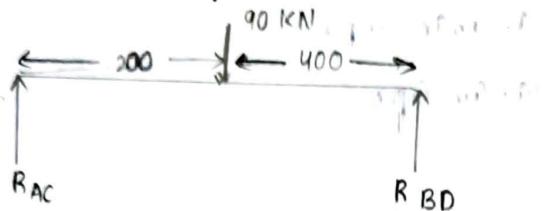
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$$E_{st} = 200 \text{ GPa}$$

$$E_{Al} = 70 \text{ GPa}$$

Determine the displacement of point F.



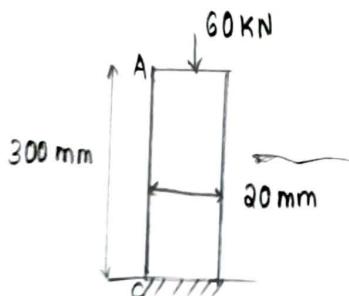
$$\sum F_y = 0 \Rightarrow R_{AC} + R_{BD} = 90$$

$$\sum M_A = 0 \Rightarrow R_{BD} \times 600 - 90 \times 200 = 0$$

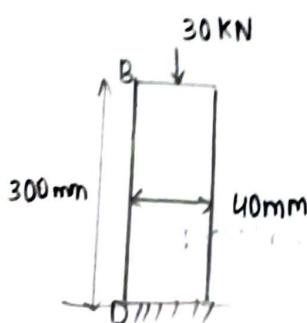
$$R_{BD} = \frac{90 \times 200}{600} \\ = 30 \text{ kN}$$

$$R_{AC} = 90 - 30$$

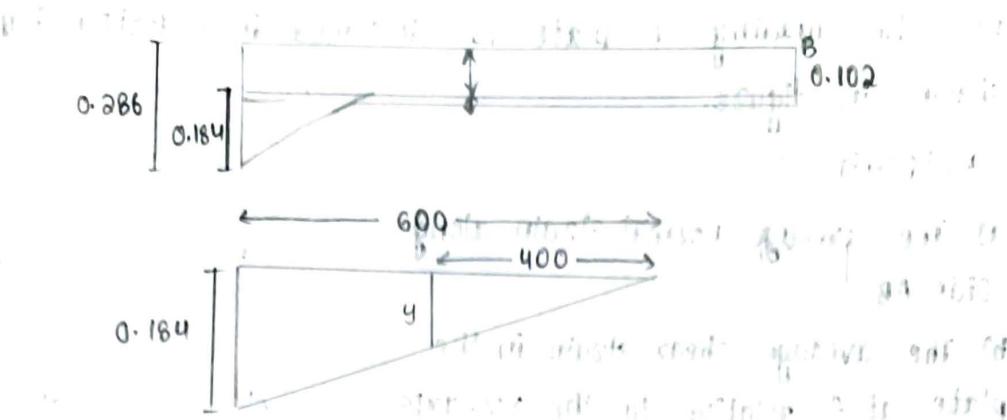
$$= 60 \text{ kN}$$



$$\delta L_{st} = \frac{PL}{AE_{st}} \\ = \frac{60 \times 10^3 \times 300}{\frac{\pi}{4} \times (20)^2 \times 200 \times 10^3} \\ = \frac{18 \times 10^6}{\frac{\pi}{4} \times 80000000} = \frac{18 \times 10^6}{\frac{\pi}{4} \times 2 \times 10^4} \\ = \frac{18 \times 10^6}{62.8} \\ = 0.286 \text{ mm}$$



$$\delta L_{Al} = \frac{PL}{AE_{Al}} \\ = \frac{30 \times 10^3 \times 300}{\frac{\pi}{4} \times (40)^2 \times 70 \times 10^3} \\ = \frac{9 \times 10^6}{\frac{\pi}{4} \times 112 \times 10^6} = \frac{9}{\frac{\pi}{4} \times 28} = \frac{9}{87.92} \\ = 0.1023 \text{ mm}$$



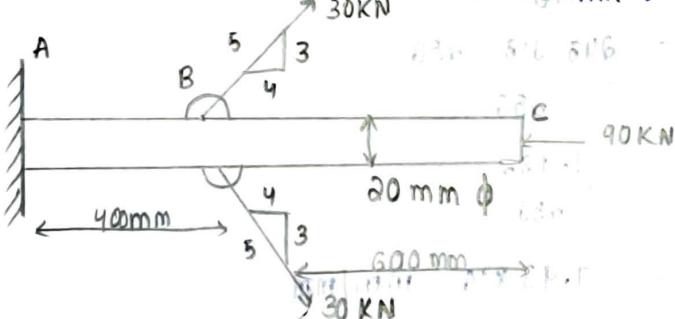
$$\frac{y}{0.184} = \frac{400}{600}$$

$$y = \frac{4}{6} \times 0.184$$

$$= 0.122 \text{ mm}$$

Deflection of point F =  $0.102 + 0.122$

$$= 0.224 \text{ mm } \downarrow$$



Determine the displacement of end 'C' w.r.t end 'A'.

$$\sum F_x = 30 \times \frac{4}{5} + 30 \times \frac{4}{5} - 90$$

$$= 24 + 24 - 90 = -42 \text{ KN} \leftarrow \text{(compression)}$$

$$Sl = \frac{Pl}{AE}$$

$$= \frac{42 \times 10^3 \times 1000}{\frac{\pi}{4} \times (20)^2 \times 200 \times 10^3}$$

$$= \frac{42 \times 10^3}{0.705 \times 8 \times 10^4}$$

$$= \frac{42}{62.8}$$

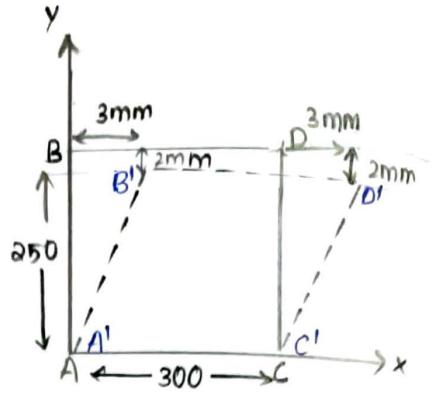
$$= -0.6687 \text{ mm}$$

- 3) Due to loading a plate is deformed into dotted shape as shown in figure.

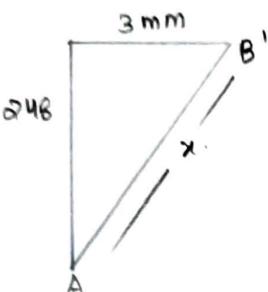
Determine :

(a) The average normal strain along side AB

(b) The average shear strain in the plate at A relative to the xy-axis.



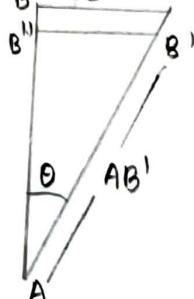
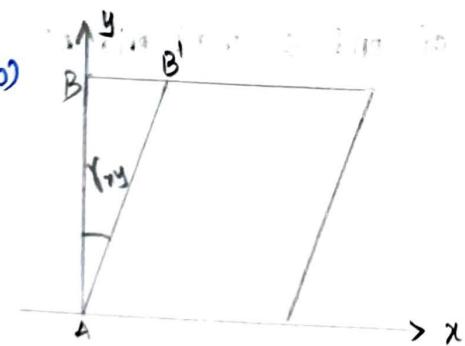
(a)



$$AB' = \sqrt{3^2 + 248^2} \\ = 248.018 \text{ mm}$$

$$\gamma = \frac{AB' - AB}{AB} \\ = \frac{248.018 - 250}{250} \\ = \frac{-1.982}{250} \\ = -7.93 \times 10^{-3} \text{ mm/mm}$$

(b)



$$c = \gamma \theta \\ \theta = \frac{c}{\gamma}$$

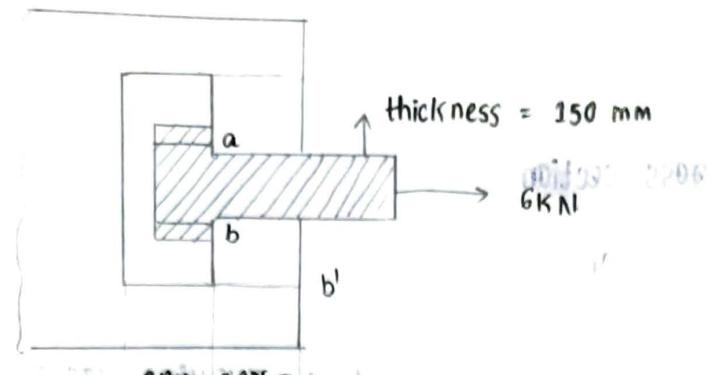
$$\tan \theta = \frac{3}{250-2}$$

$$\gamma_{xy} = \frac{3}{248} = 0.0121$$

$$\theta = \tan^{-1} \left( \frac{3}{248} \right) \\ = 0.6875$$

In radians,

$$\gamma_{xy} (\theta) = \frac{0.6875}{57.25} \\ = 0.0121 \text{ radians}$$



$$\sigma_{aa} = \frac{P}{A_1}$$

$$= \frac{3 \times 10^3}{100 \times 150}$$

$$= 0.2 \text{ N/mm}^2$$

$$\sigma_{bb} = \frac{P}{A_2}$$

$$= \frac{3 \times 10^3}{125 \times 150}$$

$$= \frac{3 \times 10^2}{1875} \approx 0.16 \text{ N/mm}^2$$