

HWRS 505: Vadose Zone Hydrology

Lecture 2

8/24/2023

Today:

1. Review: Steady-state saturated flow
2. Derive permeability from Hagen-Poiseuille flow

Steady-state saturated flow

Review of Lecture 1

❖ Vadose zone (Overview)

- Conceptual picture
- Societal impacts
- It's role in the global hydrological and carbon cycles, and the global surface energy balances

If not, $\int_{p_0}^{p_1} \frac{1}{\rho(p)} dp$ (instead of $\frac{1}{\rho}(p_1 - p_0)$)

❖ Steady-state saturated flow

- Energy potential; hydraulic head
- Darcy's law; saturated hydraulic conductivity; permeability

Assumptions:

1. Volume of the fluid parcel does not change
2. Kinetic energy is neglected. $\frac{1}{2}(v_1^2 - v_0^2) \approx 0$

$$q = -\tilde{K} \frac{H_2 - H_1}{L}$$

$K [L/T]$

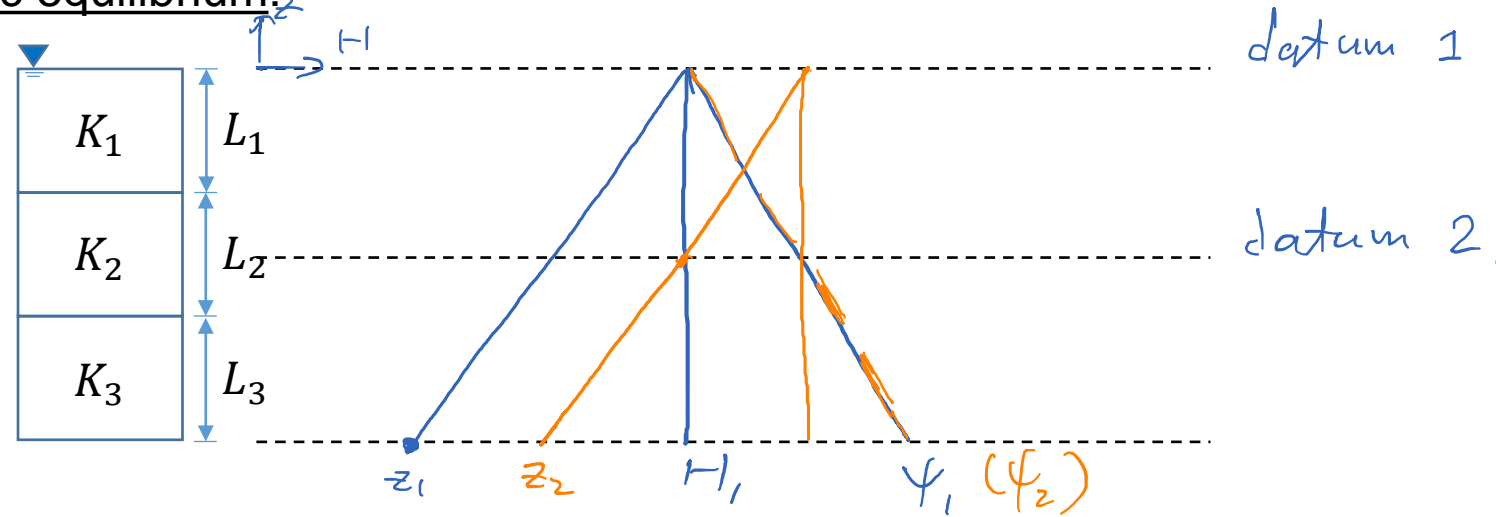
$k [L^2]$

$$q = -\tilde{K} \frac{dH}{dx} \quad (\text{differential form in 1D})$$

$$\tilde{q} = -\tilde{K} \nabla H \quad (\text{differential form in 3D})$$

Steady-state saturated flow

Static equilibrium:



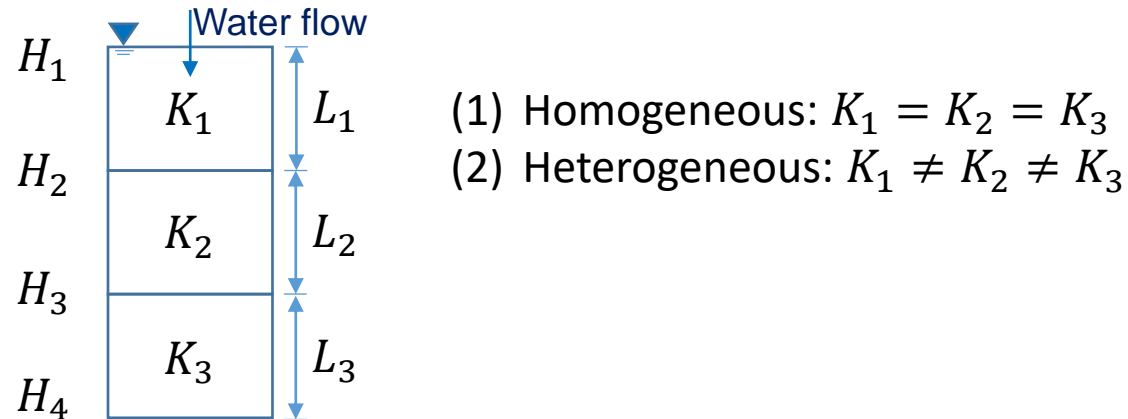
$$H_1 = z_1 + \Psi_1$$

$$H_2 = z_2 + \Psi_2$$

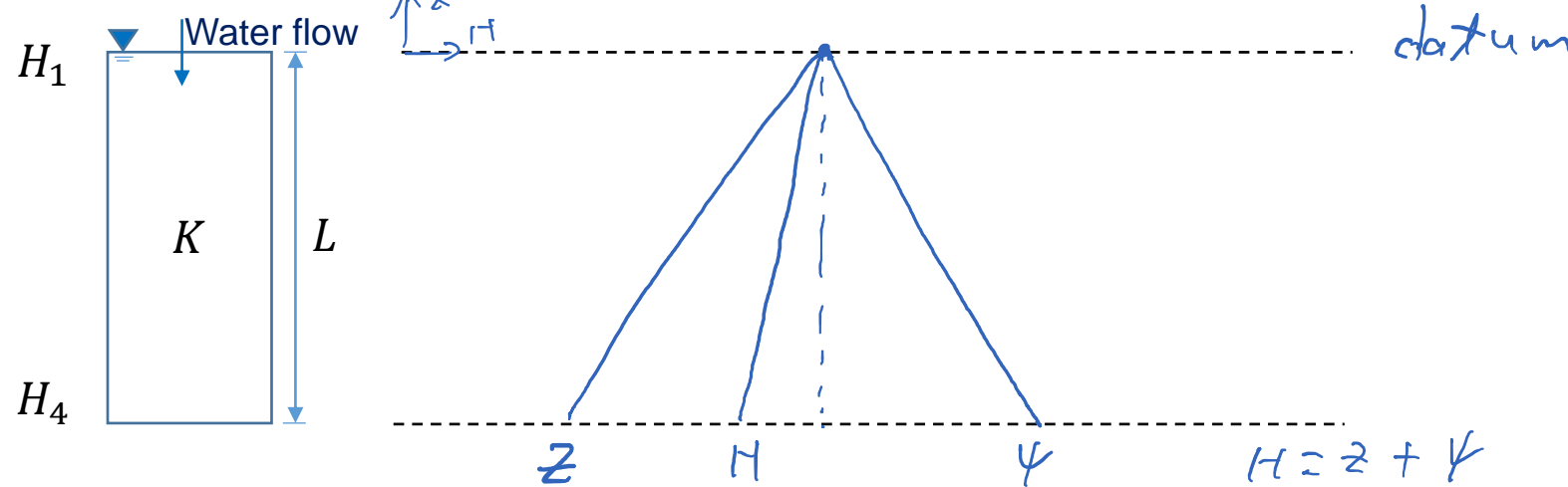
- 1°. Hydraulic head is constant in space.
- 2°. Water pressure head remains the same for different datum.
- 3°. The solution does not change w/ K_1, K_2, K_3 .

Steady-state saturated flow

Steady-state flow:



(1) Homogeneous: $K_1 = K_2 = K_3$

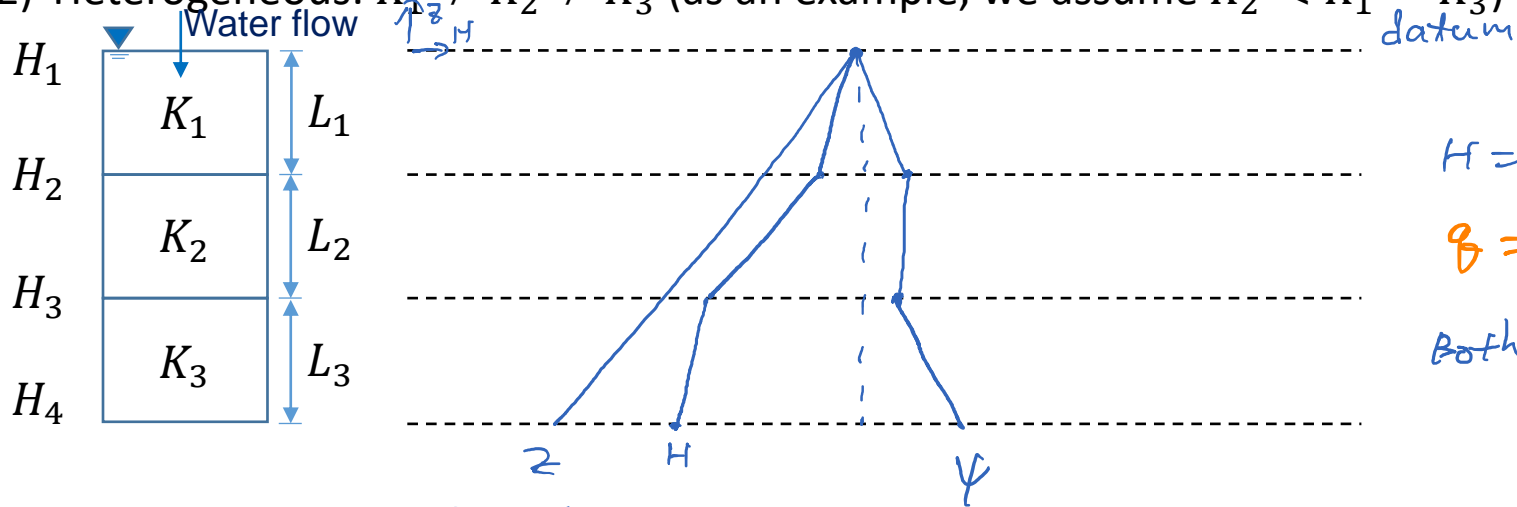


$$q = -K \frac{dH}{dz}$$

H decreases linearly along the flow direction.

Steady-state saturated flow

(2) Heterogeneous: $K_1 \neq K_2 \neq K_3$ (as an example, we assume $K_2 < K_1 = K_3$)



$$H = z + \psi$$

$$q = -K \frac{dH}{dz}$$

Both H and ψ decrease piecewise linearly along the flow direction.

$$\begin{aligned} Q &= -K_{eq} A \frac{H_4 - H_1}{L_1 + L_2 + L_3} \\ Q_1 &= -K_1 A \frac{H_2 - H_1}{L_1} \\ Q_2 &= -K_2 A \frac{H_3 - H_2}{L_2} \\ Q_3 &= -K_3 A \frac{H_4 - H_3}{L_3} \end{aligned} \Rightarrow H_4 - H_1 = -\frac{Q}{A} \left(\frac{L_1}{K_1} + \frac{L_2}{K_2} + \frac{L_3}{K_3} \right)$$

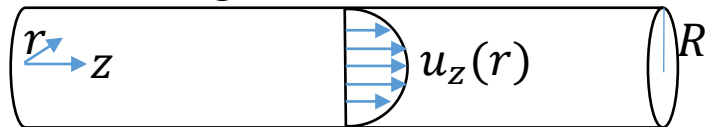
$$\Rightarrow K_{eq} = \frac{L_1 + L_2 + L_3}{\frac{L_1}{K_1} + \frac{L_2}{K_2} + \frac{L_3}{K_3}} = \frac{\sum_{i=1}^n L_i}{\sum_{i=1}^n \frac{L_i}{K_i}}$$

Steady-state saturated flow

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Permeability [L^2]

“Hagen-Poiseuille” flow



Navier-Stokes Equation

$$\rho \frac{Du_z}{Dt} = -\frac{\partial p}{\partial z} + f_z + \mu \nabla^2 u_z$$

(steady-state, low $Re^\#$)

(neglect body force for simplicity)

$$\Rightarrow \mu \frac{1}{r} \frac{\partial}{\partial r} \left(r \frac{\partial u_z}{\partial r} \right) = \frac{\partial p}{\partial z}$$

$$\frac{\partial}{\partial r} \left(r \frac{\partial u_z}{\partial r} \right) = \frac{r}{\mu} \frac{\partial p}{\partial z}$$

$$r \frac{\partial u_z}{\partial r} = \frac{r^2}{2\mu} \frac{\partial p}{\partial z} + C_1$$

$$\frac{\partial u_z}{\partial r} = \frac{r}{2\mu} \frac{\partial p}{\partial z} + \frac{1}{r} C_1$$

$$\Rightarrow u_z = \frac{r^2}{4\mu} \frac{\partial p}{\partial z} + C_1 \ln r + C_2$$

BC:

$$u_z|_{r=R} = 0 \Rightarrow C_2 = -\frac{R^2}{4\mu} \frac{\partial p}{\partial z}$$

$$\frac{\partial u_z}{\partial r}|_{r=0} = 0 \Rightarrow C_1 = 0$$

$$\Rightarrow u_z = \frac{1}{4\mu} \frac{\partial p}{\partial z} (r^2 - R^2)$$

$$\Rightarrow q = \frac{Q}{A} = -\frac{1}{8\mu} R^2 \frac{\partial p}{\partial z}$$

$$\Rightarrow k = R^2/8 \quad [L^2]$$

$$Q = \int_0^R 2\pi r u_z dr = \int_0^R \frac{\pi}{2\mu} \frac{\partial p}{\partial z} (r^3 - R^3) dr$$

=> The permeability of the tube is $k = R^2/8$

$$\frac{D}{Dt} = \frac{\partial}{\partial t} + u_r \frac{\partial}{\partial r} + \frac{u_\theta}{r} \frac{\partial}{\partial \theta} + u_z \frac{\partial}{\partial z}$$

$$\nabla^2 = \frac{\partial^2}{\partial r^2} + \frac{1}{r} \frac{\partial}{\partial r} + \frac{1}{r^2} \frac{\partial^2}{\partial \theta^2} + \frac{\partial^2}{\partial z^2}$$

Note for the Laplacian: $\nabla^2 = \frac{\partial^2}{\partial r^2} + \frac{1}{r} \frac{\partial}{\partial r} = \frac{1}{r} \frac{\partial}{\partial r} \left(r \frac{\partial}{\partial r} \right)$

Notes for Darcy's Law:

$$q = -K \nabla H = -K \nabla \psi$$

$$= -\frac{k \rho g}{\mu} \nabla \psi$$

$$= -\frac{k}{\mu} \nabla p$$

$K = \frac{k \rho g}{\mu}$

$p = \rho g \psi$

Steady-state saturated flow

		Rocks		Unconsolidated deposits		k (darcy)	k (m ²)	κ (cm/s)	κ (m/s)
Karst limestone Permeable basalt Fractured igneous and metamorphic rocks Limestone and dolomite Sandstone Unfractured metamorphic and igneous rocks Shale Unweathered marine clay Glacial fill						10 ⁵	10 ⁻⁷	10 ²	1
						10 ⁴	10 ⁻⁸	10	10 ⁻¹
						10 ³	10 ⁻⁹	1	10 ⁻²
						10 ²	10 ⁻¹⁰	10 ⁻¹	10 ⁻³
						10	10 ⁻¹¹	10 ⁻²	10 ⁻⁴
						1	10 ⁻¹²	10 ⁻³	10 ⁻⁵
						10 ⁻¹	10 ⁻¹³	10 ⁻⁴	10 ⁻⁶
						10 ⁻²	10 ⁻¹⁴	10 ⁻⁵	10 ⁻⁷
						10 ⁻³	10 ⁻¹⁵	10 ⁻⁶	10 ⁻⁸
						10 ⁻⁴	10 ⁻¹⁶	10 ⁻⁷	10 ⁻⁹
Silty sand Clean sand Gravel						10 ⁻⁵	10 ⁻¹⁷	10 ⁻⁸	10 ⁻¹⁰
						10 ⁻⁶	10 ⁻¹⁸	10 ⁻⁹	10 ⁻¹¹
						10 ⁻⁷	10 ⁻¹⁹	10 ⁻¹⁰	10 ⁻¹²
						10 ⁻⁸	10 ⁻²⁰	10 ⁻¹¹	10 ⁻¹³

- Permeability and conductivity values for various soil and rock media
- darcy (d) = $9.869233 \times 10^{-13} \text{ m}^2 \approx 1 \mu\text{m}^2$.
millidarcy (md) = 0.001 darcy.

A porous medium with a permeability of 1 darcy permits a flow of 1 cm³/s of a fluid with viscosity 1 cP (1 mPa·s) under a pressure gradient of 1 atm/cm acting across an area of 1 cm².

$$9.869233 = 1/1.013250. \quad 1 \text{ atm} = 1.013250 \times 10^5 \text{ Pa}$$

- Velocity of groundwater flow $\sim 1 \text{ m/day}$